	Index of Sheets					
No.	Description					
Sheets	Bridge Plan					
A.1	Title Sheet					
A.2	Location Map Sheet					
V.1	Estimated Quantities - Design ####					
V.2 - XX	Design -					
SPS Sheets	Bridge Plan Soils Sheet					
SPS.1	Bridge Plan Soils Sheet					
Road Sheets	Road Plan					
A.??-?.??	Road Plans					
C.1	Estimated Quantities - Road					
C.2	Standard Plans - Road					
RC.1 - 5	Estimated Quantities - Erosion Control					



PLANS OF PROPOSED IMPROVEMENT ON THE

PRIMARY ROAD SYSTEM **HARRISON COUNTY**

Bridge Replacement

On US 30 over Youngs Ditch 0.8 mi. E of SR K-45 Location

Refer to the Plan Sheets for list of applicable specifications.

Value Engineering Saves. Refer to Article 1105.14 of the Specifications.



Field Exam Notes:

Design Number and File Number provided during D2 Field Exam:

Design No. 129 Harrison County File No. 32871

Designer to verify Lat./Long. to request FHWA number and send in so correct file naming, etc. can be included in draft B01 submittal.

Revisions

Revisions to this Design Plan and/or Project Specifications should be submitted by



Standard Road Plans

Standard Road Plans are listed on tied Grading Project No.

Design	Data R	ural
20?? AADT		V.P.D.
20?? AADT		V.P.D.
20?? DHV		V.P.H.
TRUCKS		%
Total Design ESALs		-

Index of Seals										
Sheet No.	Name	Туре								
A.1	Mark C. Currie	Structural Design								

Harrison COUNTY

S	Structural Design
ONALCH GINERA	I hereby certify that this engineering dod by me or under my direct personal super a duly licensed Professional Engineer un State of Iowa.
15 🐔 🧂	Signature

tify that this engineering document was prepared oder my direct personal supervision and that I am sed Professional Engineer under the laws of the

XX-XX-XXXX

TOTAL

XX

PROJECT IDENTIFICATION NUMBER

24-43-030-040

PROJECT NUMBER BRF-030-1(211)--38-43 R.O.W. PROJECT NUMBER

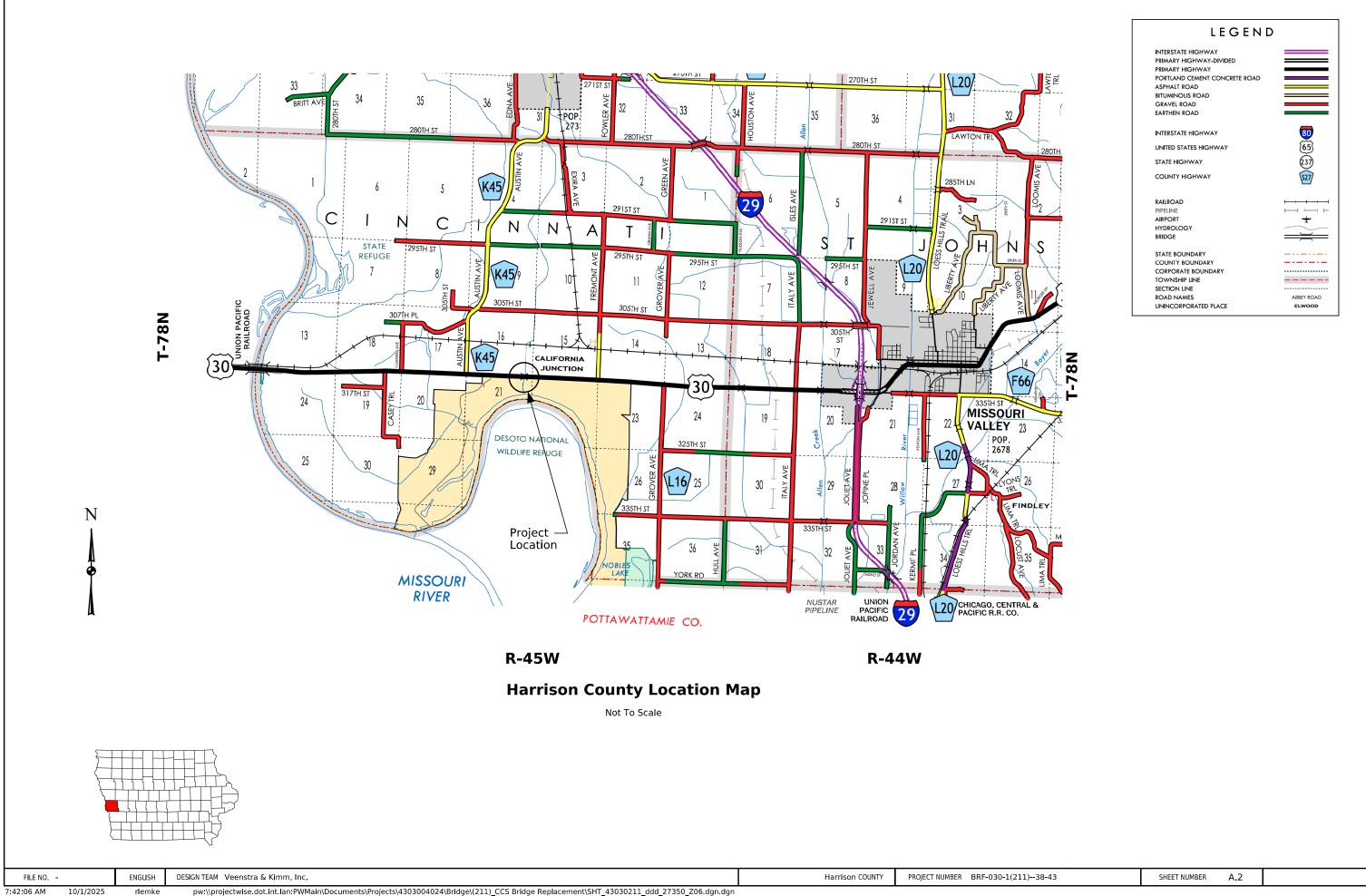
PROIECT DIRECTORY NUMBER

4303004024

Mark C. Currie Printed or Typed Name

My license renewal date is December 31, 2025

PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER



		INDEX OF SHEETS
	No.	DESCRIPTION
A	A.3 * A.4 - A.16 A.17 - A.20	Title Sheets Index of Sheets Project Concept Field Exam Notes
В	Sheets	Typical Cross Sections and Details
	B.1 - B.2	Typical Cross Sections and Details
C	Sheets	Quantities and General Information
	C.1	Project Description
G	Sheets	Survey Sheets
	G.1	Survey Information
	* G.2	Control Point Vicinity Map
	G.3	Horizontal and Vertical Project Control Coordinate Listing
J	Sheets * J.1	Traffic Control and Staging Sheets Bridge Staging Sheet
U	Sheets	500 Series, Mod.Stds. and Detail Sheets
	* U.1 * U.2 - U.4	Modified BR-211 Modified BR-203
.,		111111111111111111111111111111111111111
V	Sheets	Bridge and Culvert Situation Plans
	* V.1	Situation Plan
	* V.2	Site Plan
		* Color Plan Sheets

VEENSTRA & KIMM INC.



6775 Vista Drive West Des Moines, Iowa 50266

515.225.8000 // 800.241.8000 www.v-k.net

IOWA DEPARTMENT OF TRANSPORTATION

TO OFFICE: District 4 DATE: April 8, 2025

ATTENTION: Scott Schram PROJECT: Harrison County

BRF-030-1(211)--38-43

PIN: 24-43-030-040

FROM: Mark Currie

OFFICE: Veenstra & Kimm, Inc.

SUBJECT: Project Concept Statement; (Final, D0)

This project involves the replacement of the US 30 bridge (Maint. No. 4303.8S030) over Youngs Ditch, 0.8 mi E of SR K-45.

The Draft Concept Statement was sent out for review on March 11, 2025, and a concept review was held on March 31, 2025. Those present included Luka Arroyo, Jimmy Ellis, Tom Lovan, John Bartholomew, Jeremy Harris, Nicole Cuva, Patricia Schwarz, Christine Schwienebart, and Claire Asberry from the Iowa DOT; and Mark Currie and Edward Gapatan from Veenstra & Kimm, Inc.

Comments received from the Draft Concept Statement, as well as from the concept review meeting, have been considered and resolved.

It is recommended to remove the bridge and replace it with 130' x 44' CCS at 0 degree skew and reconstruct the roadway with a typical 44' section, with traffic maintained using staged construction, at an estimated cost of \$2,653,563. No additional right of way/right of entry will be required.

This project is recommended for construction in FY 2029. Veenstra & Kimm will coordinate the plan preparation with the assistance of the Project Management Bureau, Bridges and Structures Bureau, and Design Bureau.

Cc: K. Nicholson C. Brakke J. Nelson J. Ellis M. Nop S. Majors M. Swenson D. Stokes B. Hofer W. Sorenson E. Wright K. Brink C. Poole J. Laaser-Webb S. Anderson D. Sprengeler N. Cuva J. Bartholomew M. Dell R. Harris N. Pohlen B. Bradley B. Smith D. Ta M. Van Dyke D. Heeren G. Cagle D. Blue S. Cook B. Hucker B. Worrel R. Meyer A. Buss Y. Jia J. Holst P. Flattery W. Mayberry S. Schram D. Dorsett D. Redmond J. Sallach N. Epperson O. Lechnowsky J. Kohl J. Woodcock C. Duncan C. Asberry D. Newell

C. Schwienebart

BUILDING RELATIONSHIPS ENGINEERING SOLUTIONS

FINAL PROJECT CONCEPT STATEMENT

US 30 – Bridge over Youngs Ditch, 0.8 mi E of SR K-45

Harrison County

BRF-030-1(211)--38-43

PIN: 24-43-030-040

Maint. No. 4303.8S030

FHWA No. 27350

Mark C. Currie, P.E., S.E. 515-225-8000

April 8, 2025

I. STUDY AREA

A. <u>Project Description</u>

This project involves the replacement of the US 30 bridge (Maint. No. 4303.8S030) over Youngs Ditch, 0.8 mi E of SR K-45.

One alternative was considered:

1. Remove bridge and replace with 130' x 44' CCS at 0 degree skew.

Alternative 1 is the preferred alternative due to proximity of the project with the Missouri river and consideration of the 10-Yr Missouri river flood stage. The preferred alternative satisfies both backwater and freeboard requirements of the DOT and DNR.

Traffic will be maintained via staged construction with traffic reduced to one lane via the use of temporary traffic signals.

The preliminary project cost is \$2,653,563.

B. Need for Project

The existing structure is a 194' x 28' continuous concrete slab bridge built in 1953 and overlaid in 2012 and is near the end of its useful life. The bridge was designed for H20 design load.





Looking East along US 30

Looking North at Existing Bridge

C. Present Facility

US 30 is a two-lane roadway. The existing structure is a five span, 194' long x 28' wide continuous concrete slab (CCS) bridge constructed in 1953.

US 30 in the project area was originally constructed in 1937 as an 18' PCC road. US 30 was then widened to 24' and resurfaced with HMA in 1966, then resurfaced again in 1983, then widened to 28' of pavement and resurfaced in 2001 and in 2021. US 30 has 6' wide granular shoulders with 3:1 foreslopes.

D. Traffic Estimates

The 2029 construction year and 2049 design year average daily traffic estimates are 4,900 ADT with 24% trucks and 5,400 ADT with 24% trucks, respectively.

E. <u>Sufficiency Ratings</u>

US 30 is classified as a Rural Principal Arterial route and is a maintenance service level "B" road. The federal bridge sufficiency rating is 66.1.

F. <u>Access Control</u>

Access rights will not be acquired for this project.

G. <u>Crash History</u>

During the five-year study period from 2020 through 2025, there was one single-car crash with one occupant that had no reported injuries. Cause of the accident was listed as driving too fast for conditions. Property damage totaling \$8,173 was reported.

3

4

FILE NO. - ENGLISH DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER A.5

II. PROJECT CONCEPT

A. Project Description

Alternative #1 - Replace with a 130' CCS

Remove the existing $194' \times 28'$ CCS bridge and replace with a standard three-span $130' \times 44'$ Continuous Concrete Slab (CCS) bridge with 0-degree skew. The typical cross section will consist of a 24' roadway with 10' effective shoulders (4' paved and 6' granular) and 3:1 foreslopes.

The existing CCS bridge replaced a much smaller 24′ x 24′ I-beam bridge that was wiped out during historic 1952 Missouri river flooding. Since then, the Missouri river has been rerouted by the Army Corps of Engineers, resulting in the creation of DeSoto Lake and Desoto National Wildlife Refuge. Several levees were also constructed around the project area. The current bridge was not inundated during the major 2011 and 2019 Missouri River flood events.

Preliminary sizing of the replacement structure indicated a 120' CCS bridge could accommodate the design discharge; however, the interior span length of 47' for a standard 120' CCS poses a foundation conflict with the existing pier battered piles. Therefore, a 130' CCS bridge was selected, as it provided a clearance of approximately 2' between the existing pier and proposed abutment piles.

The 130' CCS bridge was then evaluated using 1D HEC RAS to determine if the proposed 130' bridge meets the desired freeboard and backwater criteria of DOT and DNR. Two cases were considered, including 1) Youngs Ditch alone, and 2) Youngs Ditch with 10-year Missouri River water surface elevation as a boundary condition. While the replacement structure has a smaller opening than the existing bridge, it has sufficient opening to carry the design discharge for Youngs Ditch, and the thinner superstructure of the CCS allows it to achieve the desired freeboard of 3'.

The roadway will be reconstructed on the existing vertical and horizontal alignment. Since the proposed bridge will be shorter, fill will be required to extend the typical approach section and to build the new abutments and berms. The proposed channel width will meet or exceed the existing channel width, and a maximum abutment berm slope of 2.5:1 will be used.

Proposed berm revetment limits will extend to within 5' of the Right-of-Way on both sides of the bridge and will have key-in detail on upstream and downstream ends. The toe of the revetment will be keyed-in assuming a 10' width, down on

a 1:1 slope to a minimum depth of 3'. Class E will be used for the revetment and erosion stone will be used for flat 3' at top of berm.

The removal of the existing bridge, full bridge approach pavement and guardrail will require the construction of 70' of standard BR-203 bridge approach pavement on each end, 30' of new 10" PCC pavement beyond each new approach, and installation of new guardrail, guardrail grading, and pavement shoulder next to the guardrail. Erosion control and rural seeding and fertilizing will be performed on all disturbed areas.

It appears that no right of way will be required for this project.

A former field entrance is located on the north side of US 30 just west of the bridge that is blocked by the guardrail. An existing 36" CMP, parallel with US 30, is located under the entrance that drains the ditch into Youngs Ditch just upstream of the bridge. Removal of this field entrance and CMP will be reviewed during preliminary design to ensure it removal does not impact the Drainage District negatively.

One lane of traffic in each direction will be maintained via staged construction utilizing temporary traffic signals. The traffic control staging will be similar to the recently completed US 30 over Wilsons Ditch bridge replacement, located 2 miles east of the Youngs Ditch bridge.

During stage one, a 12 ft. wide traffic lane on the north side will be provided with a horizontal clearance width of 13'-6" between barriers, while 17'-7" (including 1'-7" barrier) of the CCS is being constructed on the south side. Similarly, in stage two, a 12 ft. wide traffic lane will be provided with a horizontal clearance width of 13'-6" between barriers on the completed south side while the remainder of the CCS bridge is constructed on the north side. A 4' gap will be maintained between the existing and proposed bridge to accommodate staged construction. Temporary longitudinal sheet piling will be required to construct the abutments and abutment berms.

During Stage 1 construction, it is recommended that at least 4 piles support each new abutment for stability/redundancy purposes. This requirement should be evaluated during Preliminary Design.

Due to scour concerns, unsupported pile lengths should be evaluated during Preliminary Design. An alternative pier foundation may be necessary.

5

6

FILE NO. - ENGLISH

DESIGN TEAM Veenstra & Kimm, Inc.

Harrison COUNTY

PROJECT NUMBER BRF-030-1(211)--38-43

SHEET NUMBER A.6

BRIDGE ESTIMATE:					
Item	Quantity	Unit	Rate	Amount	
Bridge Removal	6810	SF	\$20	\$136,200	
130' x 44' CCS Bridge	6171	SF	\$145	\$894,795	
Engineering Fabric	453	SY	\$4	\$1,812	
Revetment	1800	TON	\$50	\$90,000	
Erosion Stone	36	SY	\$50	\$1,800	
Soil Remediation - Rammed Agg. Piers	1	LS	\$450,000	\$450,000	
Staged Construction	1	LS	15%	\$236,191	
Mobilization	1	LS	10%	\$181,080	
	Base Cost:			\$1,991,878	
	Contingency:		20%	\$398,376	
	0 Years Inflation	on:	4.5%		
	BRIDGE TOTA	L:	\$2,390,254		
ROADWAY ESTIMATE:					
Item	Quantity	Unit	Rate	Amount	
Removal of Pavement	880	SY	\$12	\$10,560	
Embankment in Place, Contractor Furnished	2456	CY	\$10	\$24,560	
Modified Subbase	400	CY	\$50	\$20,000	
PCC Pavement	1200	SY	\$60	\$72,000	
- · · · ·	1	LS	\$10,000	\$10,000	
Erosion Control		T -			
Traffic Control	1	LS	10%	\$13,712	
	1 1	LS LS	10% 15%		
Traffic Control	_			\$13,712 \$22,625 \$26,019	

Base Cost:

Contingency:

O Years Inflation:

PROJECT TOTAL:

ROADWAY TOTAL:

Other Alternatives Considered

An RCB culvert alternative was dismissed due to excessive settlement experienced at a nearby RCB culvert project. The recently completed RCB culvert replacement project along US 30 over Wilsons Ditch (NHSX-030-1(190)—3H-43) replaced a similar 194' long x 28' wide CCS in 2022, just two miles east of the Youngs Ditch bridge. The roadway at this culvert has been overlayed due to excessive settlement issues. District staff noted multiple other culvert projects in this region have also experienced excessive settlement, likely due to poor soils that are expensive to mitigate, and they prefer to replace the bridge with a bridge and not a culvert.

A 105′ x 44′ PPCB bridge with BTC beams, constructed using staged construction, was also evaluated. However, the low beam elevation of the PPCB option does not meet the desired freeboard of 3′ in consideration of the 10-year Missouri River water surface elevation. While the shorter length of the PPCB superstructure would reduce the cost of the bridge, the beams would need to be vented and the abutment designed for hydrostatic and buoyant forces associated with the Missouri River. In addition, staged construction of the bridge would likely require a 7-beam configuration instead of a standard 6-beam configuration, thereby not fully utilizing the beams' capacity. If a 6-beam configuration is used, an alignment shift would likely be necessary to provide the minimum gap for staged construction, which would extend the length of approach roadway and pavement replacement significantly.

A bridge (CCS or PPCB) with an on-site run-around option north of the bridge was discussed with the DOT. Significant fill and an additional channel crossing (temporary bridge or multiple pipes) would be required to the north of the bridge to fill the 15-20' difference between stream bed and road profile without having a significant sag curve in the vertical profile of the roadway. The alignment shift and ditch grades along the run-around also pose a safety concern due to the high traffic volumes on US 30. An additional easement would also be required north of the bridge to accommodate the run-around. This option was not advanced due to cost, safety, environmental impacts to the nearby Wildlife Refuge, and maintenance required.

A 130' x 44' CCS with 7.5 degree skew was considered to align the substructure units with Youngs Ditch, but the pile layout at the proposed pier conflicts with the existing pier battered piles. Since the proposed channel width using the 0 degree skew is wide enough to accommodate the slight skew of the ditch, the 7.5 degree skew alternative was not advanced.

7

8

FILE NO. - ENGLISH DESIGN TEAM Veenstra & Kimm, Inc.

Harrison COUNTY

PROJECT NUMBER BRF-030-1(211)--38-43

SHEET NUMBER A.7

Mobilization

LS

10%

20%

4.5%

\$19,948

\$219,424

\$43,885

\$263,309

\$2,653,563

B. <u>Detour Analysis</u>

An off-site detour was considered but dismissed due to lack of an acceptable route that would support the traffic volumes and loads. Traffic will be maintained via staged construction with traffic reduced to one lane via the use of temporary traffic signals.

Coordination with the Missouri Valley Bypass Project, tentatively scheduled for construction in 2027, will be required if project schedules overlap.

C. Recommendations

It is recommended that the present structure be removed and replaced as described in Alternative No. 1.

D. <u>Construction Sequence</u>

It is anticipated all work on this project will be awarded to one prime contractor. Veenstra & Kimm, Inc. will coordinate the plan preparation with the assistance of the Project Management Bureau, the Design Bureau, and the Bridges and Structures Bureau.

E. ADA Accommodations

There are no bike paths or sidewalks adjacent to US 30; therefore, no ADA accommodations are planned in conjunction with this project.

F. Special Considerations

This will not be a traffic critical project.

The Accelerated Bridge Construction (ABC) Rating Score of 9 using staged construction and 33 for an off-site detour with 7 miles out-of-distance travel, both of which are less than the first stage filter threshold of 50, therefore no further evaluation is considered.

Due to settlement concerns, areas of proposed abutments and new approach fill areas will likely require soil remediation. Terracon anticipates IFIs (rammed aggregate piers) at 5' spacing to depths of 15' will be required in these areas, but they will evaluate the site further once soils information is obtained.

The existing drainage ditch geometry was not able to be located by the County Drainage Clerk or the Drainage District Review Engineer (Troy Growth – Sundquist Engineering). The Review Engineer noted the ditch was likely constructed with 1:1 side slopes on an assumed elevation since no benchmarks were convenient at the time of construction (1910s-1920s). Coordination with

the drainage district is necessary during Preliminary Design to confirm the original channel geometry and to review the proposed channel under the bridge.

No bike path or sidewalk will be required as part of this project.

Standard survey coverage will be required.

A listing of existing utilities present within the project limits are shown in Attachment A.

The District cultural resources manager has not yet completed a cultural resources review on this project.

G. <u>Program Status</u>

Site data has been developed by Veenstra & Kimm, Inc. This project is listed in the 2025-2029 Iowa Transportation Improvement Program with \$3,250,000 for replacement in FY 2029. A schedule of events will be developed following approval of the Project Concept.

A map of Harrison County and the proposed Detour Route is attached.

Attachment A - utilities

9

10

SHEET NUMBER A.8

ATTACHMENT A

HARRISON COUNTY

(CTLIA01) CENTURYLINK Company name : CENTURYLINK Design contact: SADIE HULL Phone: 9185470147

Email: sadie.hull@lumen.com

(INS) AUREON NETWORK SERVICES

Company name: AUREON NETWORK SERVICES

Design contact: Jeff Klocko Phone: 5158300445

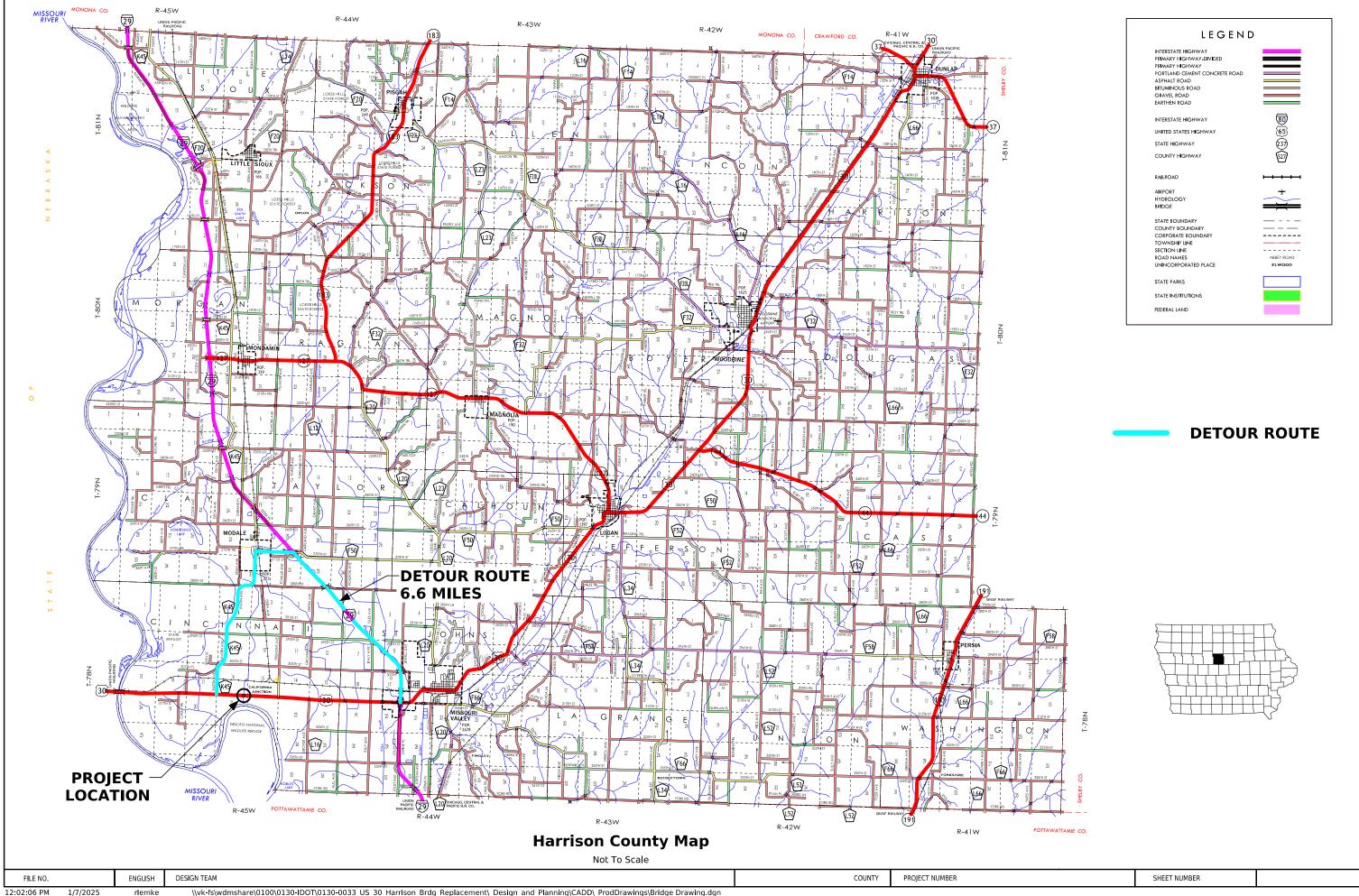
Email: jeff.klocko@aureon.com

(M39E) MIDAMER-ELEC

Company name : MIDAMER-ELEC Design contact: David Fitch

Phone: 7123665669

Email: dlfitch@midamerican.com





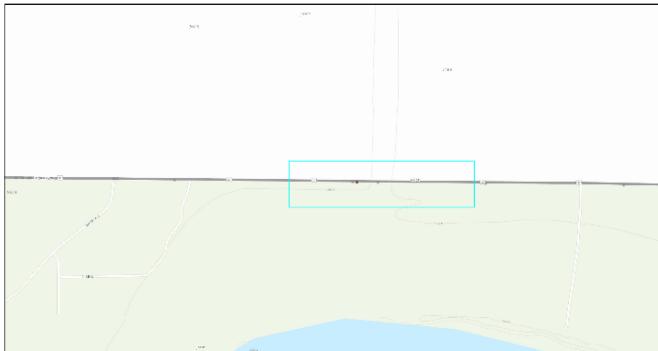
lowa Crash Analysis Tool Quick Report 2020-2025

Crash Severity	1
Fatal Crash	0
Suspected Serious Injury Crash	0
Suspected Minor Injury Crash	0
Possible/Unknown Injury Crash	0
Property Damage Only	1

Injury Status Summary	0
Fatalities	0
Suspected serious/incapacitating	0
Suspected minor/non-incapacitating	0
Possible (complaint of pain/injury)	0
Uninjured	0
Unknown	0
Not Reported	0

Property/Vehicles/Occupants									
Property Damage Total (dollars):	8,173.00								
Average (per crash dollars):	8,173.00								
Total Vehicles:	1.00								
Average (per crash):	1.00								
Total Occupants:	1.00								
Average (per crash):	1.00								

Average Severity		
	Fatalities/Fatal Crash:	0.00
	Fatalities/Crash:	0.00
	Injuries/Crash:	0.00
	Major Injuries/Crash:	0.00
	Minor Injuries/Crash:	0.00
Possib	le/Unknown Injuries/Crash:	0.00





lowa Crash Analysis Tool Quick Report 2020-2025

Major Cause			•
Animal	0	Ran traffic signal	(
Ran stop sign	0	Failed to yield to emergency vehicle	C
FTYROW: At uncontrolled intersection	0	FTYROW: Making right turn on red signal	(
FTYROW: From stop sign	0	FTYROW: From yield sign	(
FTYROW: Making left turn	0	FTYROW: From driveway	(
FTYROW: From parked position	0	FTYROW: To pedestrian	C
FTYROW: Other	0	Drove around RR grade crossing gates	C
Disregarded RR Signal	0	Crossed centerline (undivided)	C
Crossed median (divided)	0	Traveling wrong way or on wrong side of road	C
Aggressive driving/road rage	0	Driving too fast for conditions	1
Exceeded authorized speed	0	Improper or erratic lane changing	C
Operating vehicle in an reckless/erratic/care	0	Followed too close	C
Passing: On wrong side	0	Passing: Where prohibited by signs/markings	(
Passing: With insufficient distance/inadequa	0	Passing: Through/around barrier	(
Passing: Other passing	0	Made improper turn	(
Driver Distraction: Manual operation of an e	0	Driver Distraction: Talking on a hand-held d	(
Driver Distraction: Talking on a hands free	0	Driver Distraction: Adjusting devices (radio	(
Driver Distraction: Other electronic device	0	Driver Distraction: Passenger	(
Driver Distraction: Unrestrained animal	0	Driver Distraction: Reaching for object(s)/f	(
Driver Distraction: Inattentive/lost in thou	0	Driver Distraction: Other interior distracti	(
Driver Distraction: Exterior distraction	0	Ran off road - right	(
Ran off road - straight	0	Ran off road - left	(
Lost control	0	Swerving/Evasive Action	(
Over correcting/over steering	0	Failed to keep in proper lane	(
Failure to signal intentions	0	Traveling on prohibited traffic way	(
Vehicle stopped on railroad tracks	0	Other: Vision obstructed	(
Other: Improper operation	0	Other: Disregarded warning sign	(
Other: Disregarded signs/road markings	0	Other: Illegal off-road driving	(
Downhill runaway	0	Separation of units	(
Towing improperly	0	Cargo/equipment loss or shift	(
Equipment failure	0	Oversized load/vehicle	(
Other: Getting off/out of vehicle	0	Failure to dim lights/have lights on	(
Improper backing	0	Improper starting	C
Illegally parked/unattended	0	Driving less than the posted speed limit	(
Operator inexperience	0	Other	(
Unknown	0	Not reported	(
Other: No improper action	0		

01/13/2025 1 of 7 01/13/2025 2 of 7

DESIGN TEAM Veenstra & Kimm, Inc. FILE NO. -



lowa Crash Analysis Tool Quick Report 2020-2025

Time of Day/Day	of Wee	k												
Day of Week	12 AM to 2 AM	2 AM to 4 AM	4 AM to 6 AM	6 AM to 8 AM	8 AM to 10 AM	10 AM to Noon	Noon to 2 PM	2 PM to 4 PM	4 PM to 6 PM	6 PM to 8 PM	8 PM to 10 PM	10 PM to 12 AM	Not reporte d	Total
Sunday	0	0	1	0	0	0	0	0	0	0	0	0	0	1
Monday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Tuesday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Wednesday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Thursday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Friday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Saturday	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	1	0	0	0	0	0	0	0	0	0	0	1

Manner of Crash Collision	1	Surface Conditions	1
Non-collision (single vehicle)	1	Dry	0
Head-on (front to front)	0	Wet	0
Rear-end (front to rear)	0	Ice/frost	0
Angle (oncoming left turn)	0	Snow	1
Broadside (front to side)	0	Slush	0
Sideswipe (same direction)	0	Mud/dirt	0
Sideswipe (opposite direction)	0	Water (standing or moving)	0
Rear to rear	0	Sand	0
Rear to side	0	Oil	0
Not reported	0	Gravel	0
Other	0	Not reported	0
Unknown	0	Other	0
		Unknown	0

Fixed Object Struck			1
Bridge overhead structure	0	Bridge pier or support	0
Bridge/bridge rail parapet	0	Curb/island/raised median	0
Ditch	0	Embankment	0
Ground	0	Culvert/pipe opening	0
Guardrail - face	1	Guardrail - end	0
Concrete traffic barrier (median or right sid	0	Other traffic barrier	0
Cable barrier	0	Impact attenuator/crash cushion	0
Utility pole/light support	0	Traffic sign support	0
Traffic signal support	0	Other post/pole/support	0
Fire hydrant	0	Mailbox	0
Tree	0	Landscape/shrubbery	0
Snow bank	0	Fence	0
Wall	0	Building	0
Other fixed object	0	None (no fixed object struck)	0



lowa Crash Analysis Tool Quick Report 2020-2025

Driver Age/Drive	r Gender					Alcohol Test Given
Driver Age - 5 year Bins	Female	Male	Not reported	Unknown	Total	None Blood Urine
< 14	0	0	0	0	0	Breath
= 14	0	0	0	0	0	Vitreous
= 15	0	0	0	0	0	Refused
= 16	0	0	0	0	0	Not reported
= 17	0	0	0	0	0	Not reported
= 18	0	0	0	0	0	Drug Test Given
= 19	0	0	0	0	0	None
= 20	0	0	0	0	0	Blood
>= 21 and <= 24	0	0	0	0	0	Urine
>= 25 and <= 29	0	0	0	0	0	Breath
>= 30 and <= 34	1	0	0	0	1	Vitreous
>= 35 and <= 39	0	0	0	0	0	Refused
>= 40 and <= 44	0	0	0	0	0	Not reported
>= 45 and <= 49	0	0	0	0	0	
>= 50 and <= 54	0	0	0	0	0	Drug Test Result
>= 55 and <= 59	0	0	0	0	0	Negative
>= 60 and <= 64	0	0	0	0	0	Cannabis
>= 65 and <= 69	0	0	0	0	0	Central Nervous Syste
>= 70 and <= 74	0	0	0	0	0	Central Nervous Syste
>= 75 and <= 79	0	0	0	0	0	Hallucinogens
>= 80 and <= 84	0	0	0	0	0	Inhalants
>= 85 and <= 89	0	0	0	0	0	Narcotic Analgesics
>= 90 and <= 94	0	0	0	0	0	Dissociative Anesthet
>= 95	0	0	0	0	0	Prescription Drug
Not reported	0	0	0	0	0	Not reported
Unknown	0	0	0	0	0	Other
Total	1	0	0	0	1	

None	
Blood	(
Jrine	(
Breath	(
/itreous	(
Refused	(
Not reported	(
Orug Test Given	1
None	

Not reported	0
Dura Took Doorld	
Drug Test Result	1
Negative	0
Cannabis	0
Central Nervous System depressants	0
Central Nervous System stimulants	0
Hallucinogens	0
Inhalants	0
Narcotic Analgesics	0
Dissociative Anesthetic (PCP)	0
Prescription Drug	0
Not reported	1
Other	0

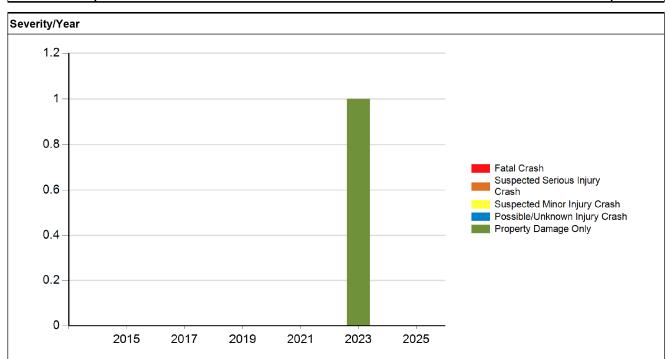
Drug/Alcohol Related	1
Drug	0
Alcohol (< Statutory)	0
Alcohol (Statutory)	C
Drug and Alcohol (< Statutory)	C
Drug and Alcohol (Statutory)	0
Refused	0
Under Influence of Alcohol/Drugs/Medications	0
None Indicated	1

01/13/2025 3 of 7 01/13/2025 4 of 7



lowa Crash Analysis Tool Quick Report 2020-2025

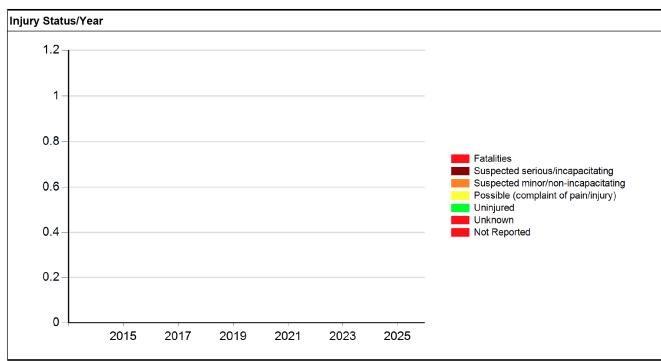
Crash Severity -	Annual					
Crash Year	Fatal Crash	Suspected Serious Injury Crash	Suspected Minor Injury Crash	Possible/Unknown Injury Crash	Property Damage Only	Total
2014	0	0	0	0	0	0
2015	0	0	0	0	0	0
2016	0	0	0	0	0	0
2017	0	0	0	0	0	0
2018	0	0	0	0	0	0
2019	0	0	0	0	0	0
2020	0	0	0	0	0	0
2021	0	0	0	0	0	0
2022	0	0	0	0	0	0
2023	0	0	0	0	1	1
2024	0	0	0	0	0	0
2025	0	0	0	0	0	0
Total	0	0	0	0	1	1





lowa Crash Analysis Tool Quick Report 2020-2025

Injury Status - Ar	nnual							
Crash Year	Fatalities	Suspected serious/incapac itating	Suspected minor/non-incapacitating	Possible (complaint of pain/injury)	Uninjured	Unknown	Not Reported	Total
2014	0	0	0	0	0	0	0	0
2015	0	0	0	0	0	0	0	0
2016	0	0	0	0	0	0	0	0
2017	0	0	0	0	0	0	0	0
2018	0	0	0	0	0	0	0	0
2019	0	0	0	0	0	0	0	0
2020	0	0	0	0	0	0	0	0
2021	0	0	0	0	0	0	0	0
2022	0	0	0	0	0	0	0	0
2023	0	0	0	0	0	0	0	0
2024	0	0	0	0	0	0	0	0
2025	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0



6 of 7

01/13/2025 5 of 7 01/13/2025

FILE NO. - ENGLISH DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER A.12



Iowa Crash Analysis Tool

DOT	Quick Report 2020-2025	
Meeting the following criteria		
Jurisdiction: Counties (Harrison) Year: 2020, 2021, 2022, 2023, 2024, 2025 Map Selection: Yes Filter: None		
Analyst Information		
US 30 over Youngs Ditch		

01/13/2025 7 of 7

PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER A.13 DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY

General input FHWA or Structure Number 27350 PIN Number Project Number BRF-030-1(211)--38-43 Design Number County Name Youngs Ditch Feature Crossed Location Description 0.8 mi. E of SR K-45 Required SI&A Input for Calculation of ABC Rating Score SI&A Item Value SI&A Units Note: If the ABC Rating Score is less than 50 and the Route Signing Prefix structure is an interstate bridge or the detour is greater Bypass, Detour Length than or equal to 30 miles then the score is set to 50. Average Daily Traffic (On) Average Daily Traffic (Under) Number of Spans in Main Unit Number of Approach Spans Average Daily Truck Traffic **Concept Measure Scores** Concept Measure Score Average Annual Daily Traffic No traffic impacts Combined value of 100% on and 25% under = Less than 5000 5000 to less than 10,000 10,000 to less than 15,000 15,000 to less than 20,000 20,000 or more Out of Distance Travel No detour Less than 5 5 to less than 10 10 to less than 15 15 to less than 20 20 or more No user costs Less than \$10,000 \$10,000 to less than \$50,000 Value in \$ = \$50,000 to less than \$75,000 \$75,000 to less than \$100,000 \$100,000 or more Economy of Scale 1 span 2 or 3 spans Value is total number of spans = 4 or 5 spans 3 6 spans or more **ABC Rating Score Factors and Weights Concept Measure** Score Average Annual Daily Traffic Out of Distance Travel User Costs Economy of Scale Total Score 15 Max. Score 165 Calculated ABC Rating Score 9 ABC Rating Score 9 PROJECT NUMBER BRF-030-1(211)--38-43 DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY SHEET NUMBER A.14 FILE NO. -**ENGLISH** $pw: \projectwise. dot. int. lan: PWMain \projects \pro$ 8:02:22 AM 10/1/2025

Roadway	US 30									
PIN Number	24-43-030-040		Submittal Date	04/08/25						
Project Number	BRF-030-1(211)38-43			Approval Date						
District	District 4	Assistant District Engineer	Wes Mayberry	••						
County	HARRISON	Of								
Route	US 30	Office Director								
Location	Bridge over Youngs Ditch, 0.8 mi. E	over Youngs Ditch, 0.8 mi. E of SR K-45								
Work Type	Bridge Replacement									
Segment Manager	Luka Arroyo									
Designer	Veenstra & Kimm, Inc.									
Design Manual Section 1C-1 Last Updated: 04-29-19		Rural Two-Lane Highwa	ys (Rural Arterials)							
De	sign Element	Preferred	Acceptable	Project Values						
Design speed (mph)		60	50	60						
Maximum superelevation rate (Ref	er to Section 2A-2)	6%	8%	6%						
Design lane width (ft)		12	12	12						
-ull depth paved width (ft)		12	12	12						
Right turn lane (ft)		12	10	N/A						
Climbing Lane (ft)		12	12	N/A						
_eft turn lane (ft)		12	10	N/A						
Pavement cross-slope	Through lanes	2%	1.5% minimum, 2% maximum	2%						
on tangent sections)	Auxiliary and turn lanes	3%	3% maximum	N/A						
Crown break at centerline		4%	4% maximum	4%						
Shoulder cross-slope (on tangent s	sections)	4%	Shoulder cross-slope cannot be less than the adjacent lane, 6% max for paved or granular shoulders, 8% max for earth shoulders	4%						
Curb type	Design speed = 50 or 55 mph	6-inch sloped	6-inch standard	N/A						
(Refer to Section 3C-2)	Design speed ≥ 60 mph	4-inch sloped	6-inch sloped	N/A						
oreslope	Adjacent to shoulder	10:1 for 4' then 6:1	3:1	3:1						
(For fill areas greater than 40 ft, contact the Soils Design Section	Beyond standard ditch depth and design clear zone	3.5:1	3:1	3:1						
for assistance)	Curbed roadways	2%	not steeper than 3:1	N/A						
Backslope (For cut areas greater the Section for assistance with backslo	han 25 feet, contact the Soils Design	3:1	2.5:1	3:1						
	w/ drainage structures	8:1	6:1	N/A						
Fransverse Slopes	w/o drainage structures	10:1	6:1	N/A						
Ditches (Refer to Section 3G-1)	Outside ditch (depth x width) (ft)	5 x 10								
	Bridge length ≤ 200 ft	design lane widths + effective shoulder widths	design lane widths + effective shoulder widths	44'						
Bridge width—new*	Bridge length > 200 ft	design lane widths + effective shoulder widths	design lane width + 4' right and left of the design lane widths	44'						
Bridge width—existing*		design lane widths + no less than 2 ft left and right	design lane widths + 2 ft. offset left and right	28'						
/ertical clearance (ft)	Over primary	16.5	16	N/A						
above lanes, shoulders and 25	Over non-primary	16.5 at interchange locations, 15 at all other locations	14	N/A						
eet left and right of the center of	Over railroad	23.3	23.3	N/A						
ailroad tracks)	Sign trusses and pedestrian bridges	17.5	17	N/A						
Structural Capacity		Contact Office of Bridges and Structures	Contact Office of Bridges and Structures							
evel of Service		В	В	В						
HWA notification via email is red	uired if acceptable critera is not met on the N	HS system (No formal design exeption is required)	·							

Design year ADT =	5	187				
Design Manual Section 1C-1 Last Updated: 04-29-19		Effective S	Shoulder Width and Type fo	r Two-Lane	Highways	
Preferred (values shown in feet)		Acceptable (values	shown in feet)		Project Values
	Rural Roadways	Urban Roadways		Rural Roadways	Urban Roadways	Project values
Turn lanes with shoulders	6	6	Turn lanes with shoulders	6	0	N/A
Turn lanes with curbs	6	See Section 3C-2	Turn lanes with curbs	6	0	N/A
	Effective Shoulder Width	Paved Width		Effective Shoulder Width	Paved Width	
Climbing Lanes	6	4	Climbing Lanes	4	0	N/A
Two-Lane Highways	Effective Shoulder Width	Paved Width	Two-Lane Highways	Effective Shoulder Width	Paved Width	
Routes where bicycles are to be accommodated	10	10			0*	
On roadways approaching urban areas (due to increased bike traffic)	10	10	Design year ADT > 2000 vpd	8		Effective = 10' Paved = 6'
On all curves with a superelevation rate of 7.0% or greater	10	10				
On roadways with design year ADT > 5000	10	6	Design year ADT between 400 - 2000 vpd	6	0*	
On all other NHS	10	6	Design year AD1 between 400 - 2000 vpd	Ü	U	T avca = 0
On non-NHS routes with design year ADT > 3000	10	6	Design year ADT < 400 vpd	4	0*	
On non-NHS routes with design year ADT < 3000	8	0*	Design year ADT < 400 Vpd	7	U	
*Requires safety edge-Refer to Section 3C-6						
Curbs should be located beyond the outer edge of the effective should	er width in rural are	eas				
Refer to Section 3C-2 for curb offsets in urban areas						
Notes:						
3:1 foreslopes since the land adjacent to the roadway is flat farmgroun	d with plenty of rec	overy area and grad	e difference between road and recovery area i	s small.		

Roadway Design Speed (mph) = 60															
Design Manual Section 1C-1 Last Updated: 04-29-19							Design (Criteria f	or High S	Speed Ro	adways				
					Preferre	d Criteria					Acceptab	le Criteria			
D	esign Element				Design S	peed, mph					Design S	peed, mph			Project Values
			50	55	60	65	70	75	50	55	60	65	70	75	values
Stopping sight distance (ft) (F	tefer to Section 6D	<u>-1</u>)	425	495	570	645	730	820	425	495	570	645	730	820	570
Minimum horizontal curve radius (ft)	Method 5 superelevation	e _{max} = 6%	833	1060	1330	1660	2040	2500	833	1060	1330	1660	2040	2500	1330
(Refer to Sections <u>2A-2</u> and and side friction <u>2A-3</u>)	and side friction distribution	e _{max} = 8%	-						758	960	1200	1480	1810	2210	N/A
Minimum vertical curve length	(ft) (Refer to Seci	tion <u>2B-1</u>)	150	165	180	195	210	225	150	165	180	195	210	225	180
Minimum rate of vertical	crest vertical curv	/es	84	114	151	193	247	312	84	114	151	193	247	312	151
curvature (K)	sag vertical	roadways without fixed-source lighting	96	115	136	157	181	206	96	115	136	157	181	206	136
(Refer to Section 2B-1)	curves	roadways with fixed- source lighting	96	115	136	157	181	206	54	66	78	91	106	121	
Minimum gradient (%)	(Refer to Section	2B-1)			0	.5				0.39	6 with a curb,	0.0% without a	curb		0.5
	(Refer to Section	Urban roadways							7	6	6	-	-	_	
Maximum gradient (%)	2B-1)	Rural roadways	4			;	3		5	5	4	4	4	4	3
	20-1)	Interstates							5	5	4	4	4	4	
Clear zone				See "Pref	erred Clear Zo	ne" table in Se	ction8A-2		See "Acceptable Clear Zone" table in Section 8A-2				30'		

Bridge Bureau Attachment for Concept Statement

Date: April 8, 2025

By: Veenstra & Kimm, Inc.
Location: US 30 over Youngs Ditch

County: Harrison County

Phase No.: BRFN-030-1(211)--38-43 Project Code: 24-43-030-040

- 1. Regulatory/Coordination
 - a. Iowa DNR Flood Plain permit = Yes, likely for misc. fill
 - b. Iowa DNR Sovereign Lands permit = No
 - c. Local Record of Coordination = No
 - d. Flood Insurance Study = Yes, Zone AE, Panel 19085C0295C, Jan 29, 2021
 - e. Drainage District = Yes, Young D.D. #58
 - f. Corps of Engineers Section 408 = No
 - g. State Water Trail or Paddling Route = No
 - h. Historic Structure = No
 - i. Federally owned land in vicinity = Yes, DeSoto National Wildlife Refuge
 - j. USGS or Iowa Flood Center (IFC) gage or sensor impacted? No
 - k. Obstruction Evaluation/Airport Airspace Analysis per FAA website = No
- 2. Hydrologic/Hydraulic Analysis/RIDB Dataset
 - a. Design discharge methodology = TR-55
 - b. Hydraulic analysis done = Yes, preliminary 1D HEC RAS using LIDAR, Missouri River Backwater effects elevations.
 - c. If DA > 10 sq. mi. Riverine Infrastructure Database (RIDB) dataset is required with B1 submittal = No
 - d. Coordinate flowlines and berm slopes with drainage district during the B01 phase of the project.
 - NOAA Atlas 15 may be available during the B01 phase of the project. If so, the design discharge should be reviewed. It is not anticipated that an increase in discharge would impact the design of the replacement structure, as the bridge has excess hydraulic capacity.
- 3. Structure/Roadway Layout Considerations
 - a. A roadway profile grade raise is not anticipated.
 - The existing drainage ditch geometry was not able to be located by the County Drainage Clerk or the Drainage District Review Engineer (Troy Growth Sundquist Engineering). The Review Engineer noted the ditch was likely constructed with 1:1 side slopes on an assumed elevation since no benchmarks were convenient at the time of construction (1910s-1920s).
 - c. Proposed berm revetment limits will extend to within 5' of the Right-of-Way on both sides of the bridge and will have key-in detail on upstream and downstream ends. The toe of the revetment will be keyed-in assuming a 10' width, down on a 1:1 slope to a minimum depth of 3'. Class E will be used for the revetment and erosion stone will be used for flat 3' at top of berm.

~ 1 ~

Concept Statement - Bridge Bureau Attachment

- 4. Special construction issues
 - a. It is desirable for new structure foundations to avoid existing foundations.
 - b. Due to settlement concerns, areas of proposed abutments and new approach fill areas will likely require soil remediation. Terracon anticipates IFIs (rammed aggregate piers) at 5' spacing to depths of 15' will be required in these areas, but they will evaluate the site further once soils information is obtained.
 - c. Unsupported pile lengths should be evaluated during Preliminary Design. An alternative pier foundation could be considered.
 - d. A field entrance is located on the north side of US 30 approximately 250' east of the bridge.
 - e. A former field entrance is located on the north side of US 30 just west of the bridge that is blocked by the guardrail. An existing 36" CMP, parallel with US 30, is located under the entrance that drains the ditch into Youngs Ditch just upstream of the bridge. Removal of this field entrance and CMP will be reviewed during preliminary design to ensure it removal does not impact the Drainage District negatively.
- 5. Special survey = No
- 6. Aesthetic enhancements = No
- 7. Other
 - a. Maintenance of Traffic Staged Construction
 - b. Coordination with the Missouri Valley Bypass Project, tentatively scheduled for construction in 2027, will be required if project schedules overlap.

Special Survey:

None.

Field Exam Notes

- -Concept and B01 should be transmitted to Drainage District reviewer for coordination.
- -Pipe and driveway/dike removal should be brought to the attention of the Drainage District Review Engineer.
- -LEB commented on DeSoto Wildlife Refuge impact, and they resonded they have Federal concurrence on No-Use.
- -DNR Floodplain/Miscellaneous Fill permitting coordination is required to confirm approach that is to be taken. Patricia Schwarz provided guidance for alternative approaches after the Field Exam.

~ 2 ~

10-22-2025

Field Exam Notes

D2 virtual field exam meeting was held on October 22, 2025. Those present included Luka Arroyo, Jimmy Ellis, Tom Lovan, John Bartholomew, Christian Kennel, Nicole Cuva, Patricia Schwarz, Mohammad Dokmak, Trisha Miller, Austin Yates from DOT, Deeann Newell, Christine Schwienebart from NEPA; Jacob Woodcock, Chelsea Duncan and Claire Asberry from the LEB; Wes Mayberry and Orest Lechnowsky from Iowa DOT District 4, and Mark Currie, Edward Gapatan and Russ Lemke from Veenstra & Kimm, Inc.

Mark discussed the concept, detour considered, and proposed staging concept. John B noted in regards of the staging that special signal timing is required given that 2029 AADT is 4900 vpd and soft cap for standard signal timing is 3000 vpd.

- John B noted and Wes concurred to pave the shoulder 300' beyond end of approach for staging/shifting traffic and future maintenance. Full depth 10' wide. Jim E. added that 300' is dictated by maintenance requirement and not by traffic control.
- Jim E. asked Wes regarding the condition of the existing roadway in consideration of paving the shoulder 300' from the ends. Will it take wear and tear of the mainline pavement? Wes said we don't need to replace the pavement and that he will do a work order if needed.

Mark continued with proposed plans for the field entrances. Field Entrance northeast of the bridge to remain (it was confirmed that there is an existing pipe under it), field entrance immediately northwest of the bridge is intended to be removed including the existing pipe culvert. Additional field entrance further to the west to remain. Any ditching or grading should consider the two pipes under the field entrances that are to remain. Patricia noted access pipes are not part of B02. Jim to remove B02 event after meeting.

In line with the proposal to remove the field entrance and the pipe culvert immediately northwest of the bridge, it was clarified by Patricia that coordination with Drainage District is required but not local record of coordination as required by DOT since we plan to apply for DNR permit for Miscellaneous fill.

- Concept and B01 should go to Drainage District for coordination. Patricia noted hopefully they got a copy of the Concept. Mark will make another effort to distribute concept and B01.
- Patricia noted we do need to coordinate with Drainage District representative regarding dike system. Use the As-Built Plan sheet that shows both the dikes but the one on the east doesn't appear to be there. Our intent is to remove the pipe, grade the ditch, armor, and possibly put a rock let down. On the bottom of the as-built sheet it does have commentary on the dike. We could put a date on there, e.g., "If you have concerns please respond within 30-60 days".

Discussion continued with the revetment and revetment limits. To the north we are proposing limits extending to 5' from the ROW. To the south we will limit the extent of revetment to minimize disturbance to the nearby DeSoto Wildlife refuge.

- Per DeSoto Wildlife Refuge impact, LEB was asked to comment on this we are close but as long as we stick within ROW are we OK? LEB responded Yes and that they actually already have Federal concurrence on No-Use.
- Jim and Patricia noted that heavy revetments proposed in the concept concur with overflow structures.
- Patricia commented on possible transitioning challenges of the revetments. Patricia suggested to consider benching to aid with keeping stream in the center. We could also put A points a little higher and have it sloping down.

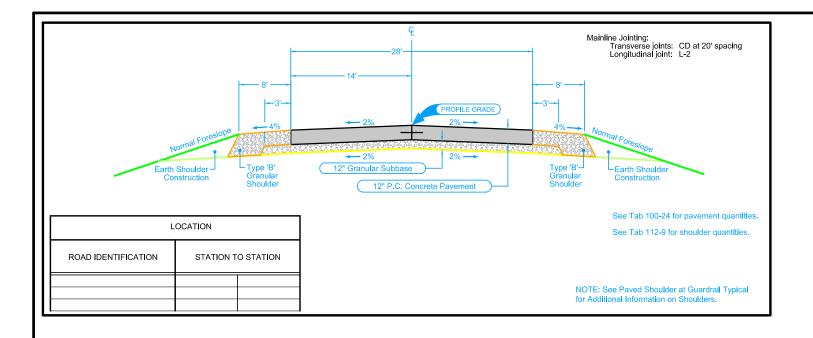
V&K is doing B01. Patricia confirmed that V&K would include submittal of DNR Permit as part of prelim bridge design. Patricia will be the reviewer. Patricia has a few minor comments she will send after meeting. Patricia clarified her comments will be for B01.

She is working to include Design number, file number, etc. and requested the lat and long for FHWA #. (When they put in the lat/long shown on the Draft Plans, Google Earth shows it on the west end of the bridge). After the meeting V&K checked and revised the lat long as follows: Latitude 41.551202°, Longitude -96.016691°

Regarding constructability, Jim noted the following:

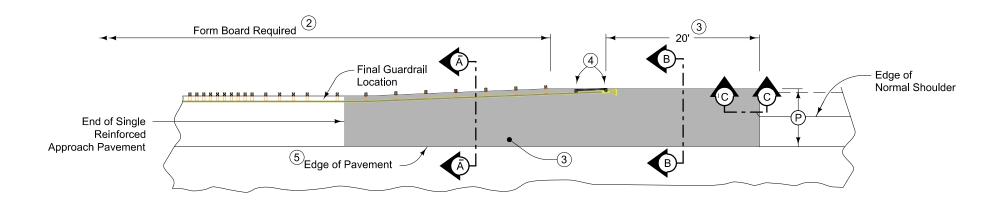
- We need to pin/anchor the TBR since we don't have the width. Jim says it is not ideal but unavoidable.
- Make sure there is a note to make sure the forms remain on for Stage 1 when building pouring Stage 2.
- Jim said his preference is for fully-encased piles at the piers. Fully encasing Stage 1 or Stage 2 piles or alternative encasement layout should be considered this during final design.

FIELD EXAM NOTES Harrison COUNTY PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER A.18 DESIGN TEAM Veenstra & Kimm, Inc.



TYPICAL SECTION





Plan View

Field Exam Notes

-Full-width (10'), full-depth paved shoulders should extend 300' beyond approach pavement for traffic staging and future maintenance.

face of guardrail posts for the length shown. Refer to note 4 for final 2 posts.

depth paved shoulders
eeyond approach pavement

face of guardrail posts for the length shown. Refer to note 4 for final 2 posts.

3 Continue paved shoulder to existing paved shoulder or 20 feet beyond the center of the first post.

installed through pavement. Do not drive posts through pavement.

9" HMA Paved Shoulder at guardrail. 8" PCC may be substituted with the

Match mainline pavement joint spacing. When mainline pavement is 8" or greater in thickness, place additional transverse 'C' joints in shoulder at

mid-panel of the mainline pavement. Place longitudinal 'C' joint at P/2

Terminate longitudinal joint at transverse joint less than 10' in length.

Compaction of HMA is required to face of guardrail post. Hand compaction will

from edge of mainline pavement when P is greater than 10' wide.

be allowed under guardrail. Removal and reinstallation of guardrail will be

(5) 'KT-1 joint for PCC shoulder.
'B' joint for HMA shoulder.

following jointing layout:

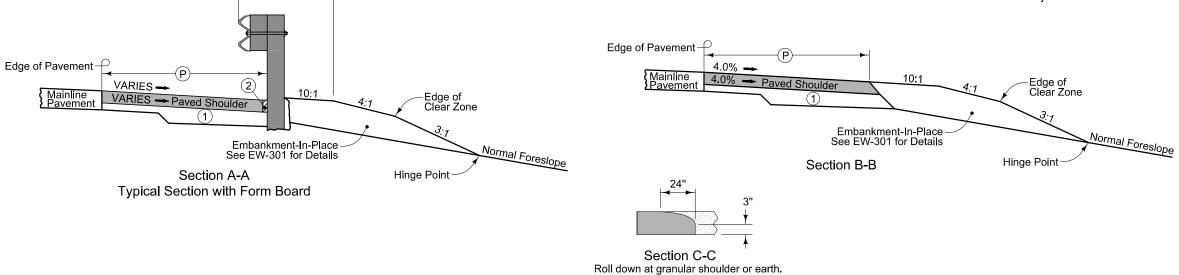
allowed with no additional payment.

Refer to Tabulation 112-9 for shoulder quantities.

1) For subgrade treatment, refer to other details in the plan.

2 PCC option only: When guardrail posts are installed prior to construction of PCC paved shoulder, fasten form board to the

(4) Shoulder may be notched for final 2 posts or post sleeves may be

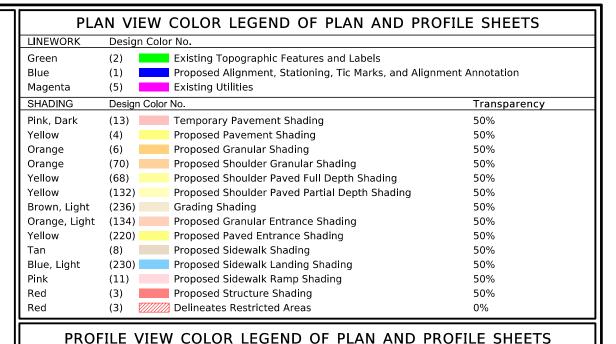


PAVED SHOULDER AT GUARDRAIL

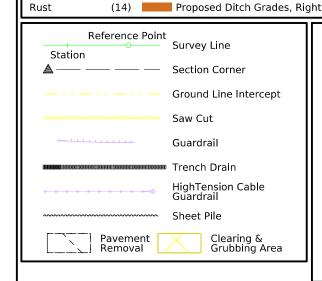
FILE NO. - ENGLISH DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER B.2

SURVEY SYMBOLS Septic Tank Interstate Highway Symbol U.S. Highway Symbol Cistern (LP) Iowa Highway Symbol L.P. Gas Tank (No Footing) County Road Highway Symbol Underground Storage Tank Evergreen Tree Latrine Deciduous Tree Satellite TV Dish Fruit Tree Water Hook Up Shrub (Bushes) Radio Tower □ RT Timber Tower Anchor Hedge Guardrail (Beam or Cable) 2 Stump Guard Post (one or two) Guard Post (over two) Ш≣ Rock Outcrop Filler Pipe 0000 **Broken Concrete** Gas Valve Revetment (Rip Rap) Water Valve † Cemetery Speed Limit Sign ⊙ SL Grave MM Mile Marker Post (CV) Cave ☐ SIGN Sign (SH) Sink Hole □ TCB Traffic Signal Control Box Board Fence ☐ RRB Rail Road Signal Control Box - # Chain Link or Security Fence □ TSB Telephone Switch Box Wire Fence Electric Box Terrace Earth Dam or Dike (Existing) Tile Outlet Edge of Water **Existing Drainage** Right of Way Rail or Lot Corner Concrete Monument X Well Windmill \otimes Beehive Intake \bowtie Existing Intake Existing Utility Access (Manhole) Fire Hydrant WH Water Hydrant (Rural)

UTILITY LEGEND



LINEWORK Design Color No. Green (10) Existing Ground Line Profile Blue (1) Proposed Profile and Annotation Magenta (5) Existing Utilities Blue, Light (230) Proposed Ditch Grades, Left Black (0) Proposed Ditch Grades, Median



Proposed Right-of-Way Symbol Proposed Right-of-Way Line △ Existing Right of Way Existing and Proposed Right-of-Way Easement and Existing Right-of-Way □ Easement (Temporary) Symbol Easement (Temporary) Line □ Easement C/A Access Control Property Line Symbol

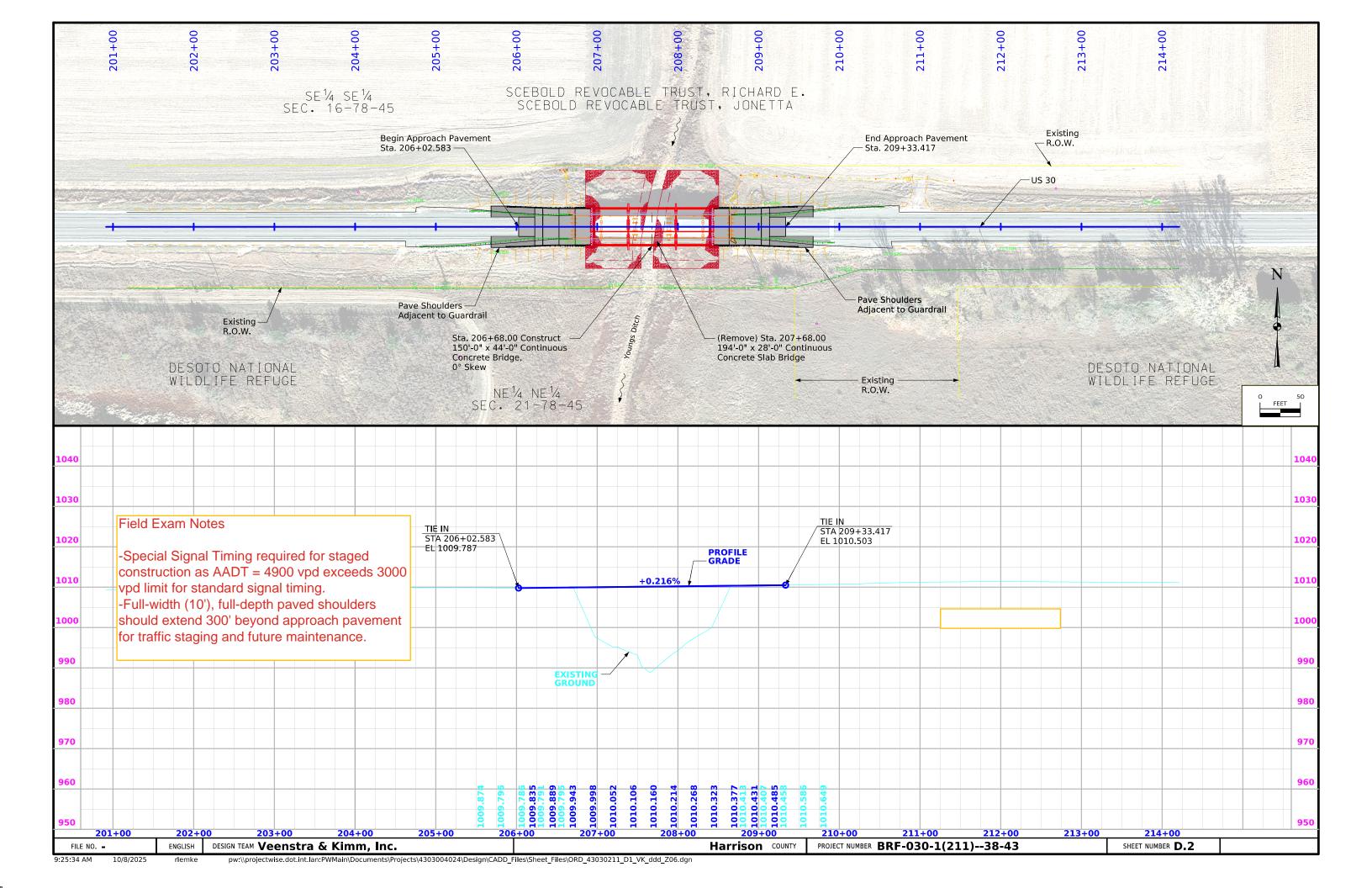
RIGHT-OF-WAY LEGEND

PLAN AND PROFILE LEGEND AND SYMBOL INFORMATION SHEET

Property Line

(COVERS SHEET SERIES D, E, F, & K)

FILE NO. - ENGLISH DESIGN TEAM Veenstr



Survey Information

SURVEY INDEX

County: Harrison PIN: 24-43-030-040

Project Number: BRF-03-1(211)—38-43

Location: Harrison County Bridge - US 30 over Youngs Ditch, 0.8 mi E

of SR K-45

Type of Work: Bridge Replacement Project Directory: 4303004024

Survey Personnel

Jerett Still-Survey Party Chief TJ Coyle – Assistant Survey Party Chief

Date(s) of Survey

Begin Date 12/05/2024 **End Date** 05/07/2025

General Information

This survey is for Harrison County Bridge – US 30 over Youngs Ditch, 0.8 mi E of SR K-45. This survey request was for the lowa DOT. This project is a Full Field DTM survey. This project is a Partial Field DTM with Photo control.

Utility Information

For logging data and other utility details see Utility Survey and Ownership Report in the Utility folder of the PrelimSurvey project directory.

Project Control

Nearby Iowa Real Time Network reference stations were utilized to obtain horizontal and vertical control on primary project control points. Three five-minute observations were taken with a minimum two-hour time span between and used in a weighted average to obtain final coordinate values. Vertical for control was determined by leveling from BM1. For additional details of the control survey, contact the Preliminary Survey department.

VERTICAL DATUM: NAVD88

GEOID MODEL: 2018

Vertical Control

Vertical control was established by verifying two NGS monuments. Vertical datum for this survey is relative to NAVD88. Geoid 2018 was used in processing. The height was computed at GNS 43 58 & GNS RV 111. Vertical control was checked with IARTN checks.

This survey observed GNS 43 58 & GNS RV 111:

GNS 43 58 – survey disk set in prefabricated concrete post imbedded in ground flush Elevation = 1004.40

GNS RV 111 – standard monel metal rivet set in top of the south ball wall of east abutment

Elevation = 1036.88

Horizontal Control

The project coordinate system for this survey is Iowa RCS zone 06 (U.S. Survey Feet). This survey control is relative to IARTN reference stations IARTN Reference Station coordinates are relative to the National Reference Station network datum: NAD83 (2011). Coordinates were determined conducting a 5-minute observation in the morning, afternoon, and evening. Coordinates were then averaged between the three to determine final coordinate.

Alignment Information

The horizontal alignment for Harrison Co. US 30 is a retrace of As-built Plans No. ERF-30(2). Survey stationing was equated to the plan PI at Sta. 207+68.00 and carried back and ahead with/without equation throughout the survey.

CONTROL POINT VICINITY MAP

This map is a guide to the vicinity of the primary project control points. Primary control is for use with RTK base stations and for RTN validation. Future surveys will use primary project control to establish temporary control as needed for construction or other surveying applications.



HORIZ. DATUM: NAD83(2011) EPOCH 2010.00 VERT. DATUM: NAVD88 - GeoID Model: 2018

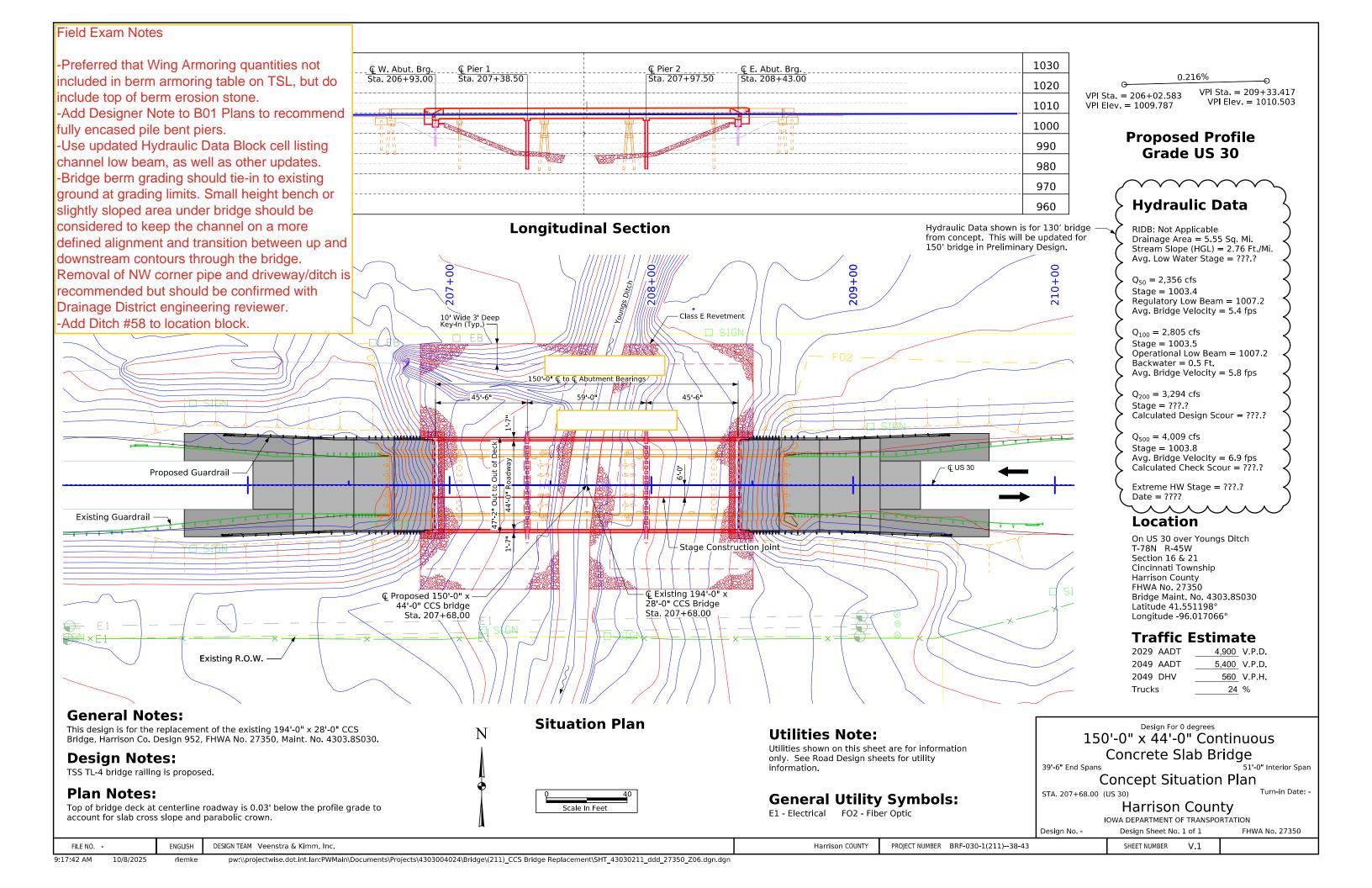
Coordinate listing from next sheet will be used with IaRTN for monument recovery. No other reference ties are given.

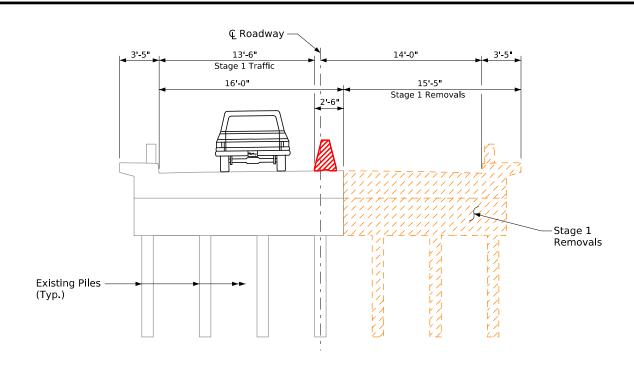
HORIZONTAL AND VERTICAL PROJECT CONTROL COORDINATE LISTING

HORIZ. DATUM: NAD83(2011) EPOCH 2010.00 IA. Regional Coordinate System Zone 06

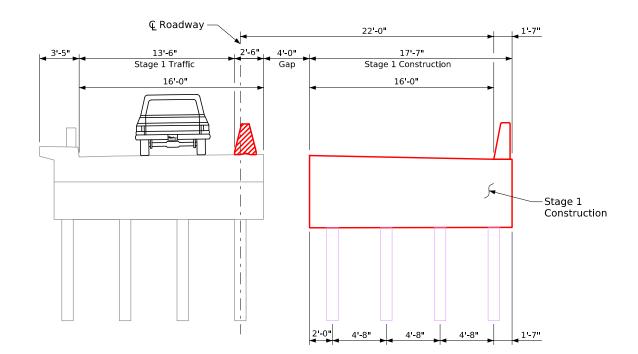
VERT. DATUM: NAVD88 GEOID MODEL: 2018

Point Name	Northing	Easting	Elevation	Code - Description
CP-103	7074107.153	16422638.4	1003.63	CP103 3/4" rebar, .8' deep, 120.05' E of BRG, Sta. 209+71.99, 119.86' Rt. of CL
CP-104	7074270.826	16422937.16	1004.78	CP104 3/4" rebar, .8' deep, 380.56' E of BRG, Sta. 212+68.65, 47.59' Lt. of CL
CP-105	7074276.929	16422072.75	1001.54	CP105 3/4" rebar, .8' deep, 260.30' W of BRG, Sta. 204+04.23, 42.72' Lt. of CL
CP-106	7074071.129	16422194.63	999.08	CP106 3/4" rebar, .8' deep, 134.04' W of BRG, Sta. 205+28.71, 161.52' Rt. of CL

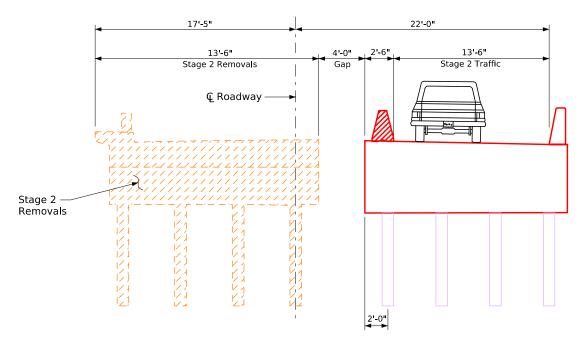




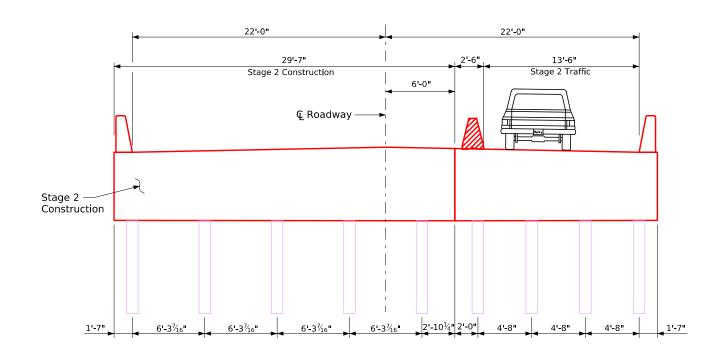
Stage 1 Removals



Stage 1 Construction



Stage 2 Removals



Stage 2 Construction

Field Exam Notes

-TBR should be pinned/anchored to the deck due to the limited width available.

-Fully-encased pier piles should be considered during final design.

-Add Designer Note to TSL to make sure forms from Stage 1 remain on for Stage 2 to limit deflection.

Bridge Staging Sheet

FILE NO. - ENGLISH DESIGN TEAM Veenstra & Kimm, Inc. Harrison COUNTY PROJECT NUMBER BRF-030-1(211)--38-43 SHEET NUMBER J.1