

Roadway	U.S. 34		
PIN Number	22-02-034-020	Submittal Date	
Project Number	BRF-034-3(041)--38-02		Approval Date
District	District 4	Assistant District Engineer	Wes Mayberry
County	ADAMS	or	
Route	U.S. 34	Office Director	
Location	Over BNSF and Nodaway River, just SW of Corning		
Work Type	Bridge Replacement		
Segment Manager			
Designer	Snyder & Associates, Inc.		

[Design Manual Section 1C-1](#)
[Last Updated: 04-29-19](#)

Rural Two-Lane Highways (Rural Arterials)

Design Element		Preferred	Acceptable	Project Values	
Design speed (mph)		60	50	60	GB Section 7.2.2
Maximum superelevation rate (Refer to Section 2A-2)		6%	8%	6%	GB page 7-6
Design lane width (ft)		12	12	12	GB table 7-3
Full depth paved width (ft)		12	12	12	--
Right turn lane (ft)		12	10	12	GB section 9.7.1
Climbing Lane (ft)		12	12	12	GB page 3-141
Left turn lane (ft)		12	10	12	GB section 9.7.1
Pavement cross-slope (on tangent sections)	Through lanes	2%	1.5% minimum, 2% maximum	2%	GB page 7-6
	Auxiliary and turn lanes	3%	3% maximum	3%	GB page 7-15
	Crown break at centerline	4%	4% maximum	4%	GB page 4-6
Shoulder cross-slope (on tangent sections)		4%	Shoulder cross-slope cannot be less than the adjacent lane, 6% max for paved or granular shoulders, 8% max for earth shoulders	4%	GB Section 4.4.3
Curb type (Refer to Section 3C-2)	Design speed = 50 or 55 mph	6-inch sloped	6-inch standard	N/A	
	Design speed ≥ 60 mph	4-inch sloped	6-inch sloped	4" sloped	
Foreslope (For fill areas greater than 40 ft, contact the Soils Design Section for assistance)	Adjacent to shoulder	10:1 for 4' then 6:1	3:1	10:1 / 6:1	
	Beyond standard ditch depth and design clear zone	3.5:1	3:1	3.5:1	RDG section 3.3.2
	Curbed roadways	2%	not steeper than 3:1		
Backslope (For cut areas greater than 25 feet, contact the Soils Design Section for assistance with backslope benches.)		3:1	2.5:1	3:1	GB Section 4.8.4
Transverse Slopes	w/ drainage structures	8:1	6:1	8:1	RDG Section 3.2.3
	w/o drainage structures	10:1	6:1	10:1	
Ditches (Refer to Section 3G-1)	Outside ditch (depth x width) (ft)	5 x 10	--	5 x 10	--
Bridge width—new*	Bridge length ≤ 200 ft	design lane widths + effective shoulder widths	design lane widths + effective shoulder widths	N/A	
	Bridge length > 200 ft	design lane widths + effective shoulder widths	design lane width + 4' right and left of the design lane widths	44'	GB Section 7.2.5
Bridge width—existing*		design lane widths + no less than 2 ft left and right	design lane widths + 2 ft. offset left and right	N/A	
Vertical clearance (ft) (above lanes, shoulders and 25 feet left and right of the center of railroad tracks)	Over primary	16.5	16	N/A	GB Section 7.2.5
	Over non-primary	16.5 at interchange locations, 15 at all other locations	14	N/A	GB pages 5-9 and 6-8
	Over railroad	23.3	23.3	23.5'	--
	Sign trusses and pedestrian bridges	17.5	17	N/A	GB section 7.2.5
Structural Capacity		Contact Office of Bridges and Structures	Contact Office of Bridges and Structures		--
Level of Service		B	B	B	GB section 7.2.2

*FHWA notification via email is required if acceptable criteria is not met on the NHS system (No formal design exception is required)

Design year ADT =						
Design Manual Section 1C-1 Last Updated: 04-29-19		Effective Shoulder Width and Type for Two-Lane Highways				
Preferred (values shown in feet)			Acceptable (values shown in feet)			Project Values
	Rural Roadways	Urban Roadways		Rural Roadways	Urban Roadways	
Turn lanes with shoulders	6	6	Turn lanes with shoulders	6	0	6 GB section 9.7.1
Turn lanes with curbs	6	See Section 3C-2	Turn lanes with curbs	6	0	6 GB section 9.7.1 and 4.7.3
	Effective Shoulder Width	Paved Width		Effective Shoulder Width	Paved Width	
Climbing Lanes	6	4	Climbing Lanes	4	0	GB page 3-141
Two-Lane Highways	Effective Shoulder Width	Paved Width	Two-Lane Highways	Effective Shoulder Width	Paved Width	
Routes where bicycles are to be accommodated	10	10	Design year ADT > 2000 vpd	8	0*	10' effective, 10' paved GB table 7-3
On roadways approaching urban areas (due to increased bike traffic)	10	10				
On all curves with a superelevation rate of 7.0% or greater	10	10	Design year ADT between 400 - 2000 vpd	6	0*	
On roadways with design year ADT > 5000	10	6				
On all other NHS	10	6	Design year ADT < 400 vpd	4	0*	
On non-NHS routes with design year ADT > 3000	10	6				
On non-NHS routes with design year ADT < 3000	8	0*				
*Requires safety edge-Refer to Section 3C-6						
Curbs should be located beyond the outer edge of the effective shoulder width in rural areas						
Refer to Section 3C-2 for curb offsets in urban areas						

Roadway Design Speed (mph) =		60													
Design Manual Section 1C-1 Last Updated: 04-29-19		Design Criteria for High Speed Roadways													
Design Element			Preferred Criteria						Acceptable Criteria						Project Values
			Design Speed, mph						Design Speed, mph						
			50	55	60	65	70	75	50	55	60	65	70	75	
Stopping sight distance (ft) (Refer to Section 6D-1)			425	495	570	645	730	820	425	495	570	645	730	820	570 GB Table 3-1
Minimum horizontal curve radius (ft) (Refer to Sections 2A-2 and 2A-3)	Method 5 superelevation and side friction distribution	e _{max} = 6%	833	1060	1330	1660	2040	2500	833	1060	1330	1660	2040	2500	1330 GB Table 3-7
		e _{max} = 8%	--	--	--	--	--	--	758	960	1200	1480	1810	2210	GB Table 3-7
Minimum vertical curve length (ft) (Refer to Section 2B-1)			150	165	180	195	210	225	150	165	180	195	210	225	180 GB page 3-167
Minimum rate of vertical curvature (K) (Refer to Section 2B-1)	crest vertical curves		84	114	151	193	247	312	84	114	151	193	247	312	151 GB Table 3-35
	sag vertical curves	roadways without fixed-source lighting	96	115	136	157	181	206	96	115	136	157	181	206	136 GB Table 3-37
		roadways with fixed-source lighting	96	115	136	157	181	206	54	66	78	91	106	121	136 GB page 3-176
Minimum gradient (%) (Refer to Section 2B-1)			0.5						0.3% with a curb, 0.0% without a curb						.5 GB page 3-130
Maximum gradient (%) (Refer to Section 2B-1)	Urban roadways								7	6	6	--	--	--	GB Table 7-4a
	Rural roadways		4			3			5	5	4	4	4	4	3 GB Table 7-2
	Interstates								5	5	4	4	4	4	IDG page 3
Clear zone			See "Preferred Clear Zone" table in Section 8A-2						See "Acceptable Clear Zone" table in Section 8A-2						30'

PROJECT CONCEPT STATEMENT

Bridge on U.S. 34 over BNSF Railway and East Nodaway River

Adams County
Project # BRF-034-3(41)--38-02
PIN: 22-02-034-020
Maintenance No. 0261.5S034
FHWA No. 013320

Prepared for:
Iowa Department of Transportation
District 4
Wes Mayberry, P.E.

Prepared by: Snyder & Associates, Inc. / Shuck-Britson Inc.

November 17, 2023

I. STUDY AREA

A. Project Description

This project involves replacement of the U.S. 34 bridge over the BNSF Railway and East Nodaway River (Maintenance No. 0261.5S034) on the south side of Corning, in Adams County.

B. Present Facility--Need for Project

The existing bridge is an 824' x 30' continuous welded girder bridge constructed in 1964. The bridge has 6 spans, with 116-foot-long end spans and 148-foot-long center spans, and was constructed at a 45 degree skew. U.S. 34 is skewed at 59 degrees to the railroad, meaning bridge piers are not parallel to the railroad. Past repairs have consisted of a bridge deck overlay in 1995 and bridge painting in 2012.

The bridge was last inspected in April 2023 and has deck, superstructure, and substructure condition ratings of 5, 7 and 5, respectively, on a scale of 0 to 9. A rating of 4 or less on any of the condition ratings would make the bridge "Structurally Deficient". An asbestos inspection noted the presence of asbestos in both samples taken on the existing bridge.

Per the programming recommendations, dated December 23, 2019, contained with the Asset Details on SIIMS:

1. The bridge deck overlay that was added in 1995 is now reaching the end of its service life.
2. The bridge deck has leaching cracks, delamination, and spalls with exposed

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- reinforcement.
- 3. The abutments have narrow to wide cracking, delamination in the bridge seats, and spalling.
- 4. Due to the condition of the bridge, this structure should be replaced.

For these reasons, the bridge is not a candidate for rehabilitation or widening but should be held as a replacement candidate for a future letting. The bridge's expected replacement type and total project cost will be determined with this Project Concept phase.

The roadway east and west of the bridge, located on the south side of Corning, is a 24-foot-wide paved rural section with 10-foot-wide composite shoulders. Roadway foreslopes vary from 2.5:1 to 3:1, and the roadway was built without clear zone considerations. The posted speed limit through the project area is 55 mph.

U.S. 34 intersects with Loomis Avenue approximately 1,771 feet east of the bridge. Approximately 800 feet west of the bridge is a former quarry site on each side of the road that has been flooded to create ponds. Steel beam guardrail is installed through the former quarry site to protect traffic. The BNSF railway beneath the bridge currently has a single track, although the as-built plans for the bridge indicate that three tracks were present at that time. We would anticipate that the BNSF railway would request that the new bridge accommodate three tracks.

The FRA crossing number for this location is 095320M. Per the FRA Crossing Inventory Form, the BNSF Railway through the project area carries 13 trains per day, which includes 2 daily AMTRAK passenger trains, at a maximum timetable speed of 79 miles per hour.

C. Hydrology

StreamStats discharges for the East Nodaway River at the project site are 18,000 cfs (50-year), 21,200 cfs (100-year) and 24,400 cfs (500-year) for the 168 square mile drainage area.

D. Traffic Estimates

The 2016 traffic count was 2,960 vehicles per day (VPD). The 2020 truck traffic was 560 trucks per day, which is 19% of the 2016 ADT. Historic traffic counts dating back to 2000 vary between 1,710 VPD and 3,310 VPD.

Iowa DOT Office of Systems Planning forecasts an AADT volume of 3,238 VPD in Year 2027 and 3,800 VPD in Year 2047 with 19% truck percentage for both years. Year

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2047 design hour forecast volumes are 390 vehicles per hour for the AM peak hour PM peak hour.

E. Crash History

One property damage only crash was reported on U.S. 34 in the project limits in the last 5 years. This was an “animal” crash located away from the bridge. It appears that the existing alignment has adequate sight distance available.

F. Sufficiency Rating – Bridge Condition Index

The official Federal bridge sufficiency rating is 48.3. With a sufficiency rating below 50 and the near-deficiency condition ratings of two of the major bridge components (deck and substructure), replacement is the clear choice.

The bridge condition rating is considered fair. The Bridge Condition Index (BCI) is 46.5.

The bridge has a priority ranking of 37.4.

G. Accelerated Bridge Construction Score

Traffic Carried on Existing Bridge:

The Accelerated Bridge Construction (ABC) normalized score using state roads is 15, which is far below the threshold (50) that would qualify the project for further evaluation of ABC techniques. The contributing factors to the raw score are the out of distance travel (OODT) weighted score of 0, average annual daily traffic (AADT) weighted score of 10, a daily road user costs (DRUC) weighted score of 0, and an economy of scale (EOS) weighted score of 15.

Traffic Carried on Detour:

The Accelerated Bridge Construction (ABC) normalized score using state roads is 27, which is below the threshold (50) that would qualify the project for further evaluation of ABC techniques. The out of distance travel detour is 3 miles (4.8 km). The contributing factors to the raw score are the out of distance travel (OODT) weighted score of 10, average annual daily traffic (AADT) weighted score of 10, a daily road user costs (DRUC) weighted score of 10, and an economy of scale (EOS) weighted score of 27.

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H. Access Control

Access rights will not be acquired on this project.

I. Utilities

An electrical transmission line owned by CIPCO runs along the north side of the existing roadway. Preliminary estimates indicate that moving the electrical lines will cost \$400,000 - \$500,000, plus the cost of right-of-way if necessary.

II. PROJECT CONCEPT

A. Proposed Improvements

Bridge alternatives proposed as part of this concept are all on relocated alignments and are not expected to impact the existing bridge during construction. Extensive roadway reconstruction will be necessary to connect the new roadway and bridge alignment to existing U.S. 34. Four alternative alignments were reviewed. Refer to attached sheets for additional information.

For all alignments, the vertical profile is controlled by clearance over the BNSF Railway. Per the Union Pacific Railroad and BNSF Railway Guidelines for Railroad Grade Separation Projects, 23’-6” clearance is required from the top of rail to the lowest part of the bridge, within an envelope that extends to 9 feet outside of the track centerline. If the BNSF Railway wishes for the bridge to be able to accommodate three tracks, then that envelope extends to 9 feet from the center of the outermost tracks. The railroad also requires that bridge piers and / or abutments be located outside railroad right-of-way. For this project, railroad right-of-way width varies from 250 feet to 390 feet and includes the East Nodaway River. These are perpendicular widths, and the actual length of roadway within right-of-way will be much longer due to the skew of the roadway with respect to the tracks. Keeping all structural piers out of the railroad right-of-way is impractical, and further coordination will be necessary with the BNSF Railway during project design.

All four alternative alignments use a 60-mph design. The U.S. 34 roadway section will consist of two – 12-foot lanes with 10-foot paved shoulders. A barnroof section will be used, with a 6:1 slope to a 30-foot clear zone, then 3.5:1. Ditches will be 10 feet wide with a 3:1 backslope.

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1. Alternative Number 1: Far Northern Realignment – Replace Existing Bridge with a 4-span horizontally curved 820'-0 x 44'-0 CWPG Bridge

The aim of this realignment is to reduce the skew of the roadway relative to the BNSF Railway tracks, which requires a greater deviation from the existing alignment. At its furthest point, this alignment is 123 feet north of existing U.S. 34. The alignment uses three horizontal curves, including 5,000-foot radius curves at the east and west tie-ins and a 3,500-foot radius curve just west of the railroad. Note that the 3,500-foot radius curve will continue onto the west end of the proposed bridge.

The proposed realignment impacts three drainage structures: a 4' x 5' RCB, a 36" RCP with a flume, and an 8' x 8' RCB. The 5' x 4' RCB would be extended with a 60" RCP. The flume off the 36" RCP does not appear to be very tall, and will likely be able to be removed with the pipe extension at a sufficient slope. The 8' x 8' RCB will be extended to account for grading to barnroof section.

The overall length of reconstruction with this option is 3,504.93 feet. The skew of the roadway over the railroad is reduced to 54 degrees with this option.

The existing 824'-0" x 30'-0 Continuous Welded Plate Girder (CWPG) bridge will be replaced with an 820'-0" x 44'-0" CWPG Bridge with a skew of 45 degrees L.A. The bridge has four spans: (250'-0") – (250'-0") – (180'-0") – (140'-0"). A horizontal curve extends onto the west half of the bridge, which will necessitate the use of horizontally curved steel continuous welded plate girders. The 44'-0" wide roadway width consists of two 12'-0" lanes and 10'-0" shoulders on each side. The bridge will have stub abutments and T-Piers. Since the bridge is located over the BNSF Railway it will have standard single slope TL-5 barriers for its full length.

Estimated Construction Cost

<u>Bridge Item</u>	<u>Estimated Cost</u>
Bridge	\$ 7,412,000
Remove Existing Bridge	\$ 308,000
Cofferdams (3 required)	\$ 150,000
Revetment, Class E (East Abutment Only)	\$ 25,000
Mobilization (10%)	\$ 790,000
Contingency (20%)	\$ 1,737,000
Bridge Total	\$10,422,000

<u>Roadway Item</u>	<u>Estimated Cost</u>
Embankment-in-Place	\$ 2,298,384
Excavation, Class 10 Roadway and Borrow	\$ 173,720
Modified Subbase	\$ 124,511
Paved Shoulders, 10" PCC	\$ 345,360
Bridge Approach Section	\$ 211,845
PCC Pavement, Class C, Class 3, 10"	\$ 498,045
Removal of Existing Structures	\$ 25,000
RCB Extension	\$ 56,595
Pipe Extensions	\$ 54,802
Guardrail	\$ 31,738
Removal of Pavement	\$ 94,904
Traffic Control (5%)	\$ 195,745
Mobilization (5%)	\$ 195,745
Contingency (30%)	\$ 1,291,918
Roadway Total	\$ 5,598,312
Utility Relocation	\$ 500,000

Project Total: \$ 16,520,312

2. Alternative Number 2: Parallel Alignment, Replace Existing Bridge with a 4-span 880'-0 x 44'-0 CWPG Bridge

The as-built plans for existing U.S. 34 show a survey alignment parallel to the existing roadway at the bridge, located 40 feet to the north. Alternative 2 approximates the survey alignment, with some small changes. To accommodate a new 44' wide bridge without impacting the existing bridge, the offset between existing and proposed alignments is increased to 42 feet. A 6,000-foot radius curve ties the relocated alignment back to existing on the west end, and a 3,770-foot radius is used at the east end, to tie the alignment with adequate distance to runout the superelevation prior to the spiral immediately to the east. Total length of roadway reconstruction with this alternative is 3,786 feet. The portion of the alignment through railroad ROW is 750.6 feet long. The skew of the roadway to the railroad is 59 degrees, which matches the existing roadway skew to the tracks.

The proposed realignment impacts three drainage structures, a 4' x 5' RCB that would be extended with a 60" RCP, a 36" RCB with a flume, and an 8' x 8' RCB. The 5' x 4' RCB would be extended with a 60" RCP. It is assumed that the flume on the 36" RCP can be eliminated with the pipe extension.

Replace the existing 824'-0" x 30'-0 Continuous Welded Plate Girder (CWPG) bridge with an 880'-0" x 44'-0" CWPG Bridge with a skew of 45 degrees L.A.

The bridge has four spans: (200'-0") – (265'-0") – (235'-0") – (180'-0"). The 44'-0" wide roadway width consists of two 12'-0" lanes and 10'-0" shoulders on each side. The bridge will have stub abutments and T-Piers. Since the bridge is located over the BNSF Railway the bridge will have standard single slope TL-5 barriers for the full length of the bridge.

Estimated Construction Cost

<u>Bridge Item</u>	<u>Estimated Cost</u>
Bridge	\$ 7,531,000
Remove Existing Bridge	\$ 308,000
Cofferdams (3 required)	\$ 150,000
Revetment, Class E (East Abutment Only)	\$ 25,000
Mobilization (10%)	\$ 801,000
Contingency (20%)	\$ 1,763,000
Bridge Total	\$10,578,000

<u>Roadway Item</u>	<u>Estimated Cost</u>
Embankment-in-Place	\$ 645,858
Excavation, Class 10 Roadway and Borrow	\$ 323,728
Modified Subbase	\$ 136,125
Paved Shoulders, 10" PCC	\$ 376,338
Bridge Approach Section	\$ 189,660
PCC Pavement, Class C, Class 3, 10"	\$ 544,508
Removal of Existing Structures	\$ 25,000
RCB Extension	\$ 70,101
Pipe Extensions	\$ 49,037
Guardrail	\$ 31,738
Removal of Pavement	\$ 105,055
Traffic Control (5%)	\$ 124,857
Mobilization (5%)	\$ 124,857
Contingency (30%)	\$ 824,059
Roadway Total	\$ 3,570,921
Utility Relocation	\$ 500,000

Project Total: \$ 14,648,921

- Alternative Number 3: Southern Realignment. Replace Existing Bridge with a 4-span horizontally curved 915'-0 x 44'-0 CWPG Bridge

Similar to Alternative 1, this alternative aims to reduce the skew of the roadway

over the BNSF railroad. The railroad right-of-way is relatively narrower south of the existing roadway, so this would also reduce overall impact to railroad property. However, the railroad right-of-way is shifted to the west just north of the existing bridge and therefore the west pier is located further west of the railroad tracks to be placed outside the right-of-way. The existing tangent from the west is extended over the tracks, with a horizontal curve added east of the tracks to redirect the alignment back toward existing. At the east end of the alignment, the existing horizontal curve (PI Sta. 522+28.74) will be lengthened to the west, with a tangent off the lengthened curve extending to the tangent from the west. Note that the existing curve that is being extended has spirals; we do not propose adding the spiral back in after the curve is extended. This will result in a curve with a spiral at one end and not at the other. Quarry Road, south of U.S. 34, will need to be relocated, since the proposed U.S. 34 alignment is approximately 18 feet closer to Quarry Road than the existing highway. The proposed alignment is also closer to the pond south of U.S. 34 and east of the River, though we do not believe that construction will impact the pond.

Note that this project will have excess cut to waste off-site. Alternatives 1 and 2 will both require fill material to be imported to the project.

The proposed realignment impacts three drainage structures, a 4' x 5' RCB that would be extended with a 60" RCP, a 36" RCB with a flume which will be completely replaced with a 36" pipe (pipe may be able to be downsized after hydraulic review), and an 8' x 8' RCB that will be extended.

The overall length of reconstruction with this option would be 3,832.76 feet. The skew of U.S. 34 to the BNSF railroad would be reduced to 54 degrees.

Replace the existing 824'-0" x 30'-0 Continuous Welded Plate Girder (CWPG) bridge with a 915'-0" x 44'-0" horizontally curved CWPG Bridge with a skew of 45 degrees L.A. The bridge has four spans: (245'-0") – (310'-0") – (200'-0") – (160'-0"). The 310-foot center span is necessary to keep the west pier out of the railroad right-of-way and to position the east pier at a distance greater than 25 feet from the east future railroad track. The bridge is partially located in a horizontal curve (approximately 56 feet of bridge at the east end is in a horizontal curve with a radius of 3,000 feet). The 44'-0" wide roadway width consists of two 12'-0" lanes and 10'-0" shoulders on each side. The bridge will have stub abutments and T-Piers. Since the bridge is located over the BNSF Railway the bridge will have standard single slope TL-5 barriers for the full length of the bridge.

Estimated Construction Cost

<u>Bridge Item</u>	<u>Estimated Cost</u>
Bridge	\$ 8,698,000
Remove Existing Bridge	\$ 308,000
Cofferdams (3 required)	\$ 150,000
Revetment, Class E (East Abutment Only)	\$ 25,000
Mobilization (10%)	\$ 918,000
Contingency (20%)	\$ 2,020,000
Bridge Total	\$12,119,000

<u>Roadway Item</u>	<u>Estimated Cost</u>
Excavation, Class 10 Roadway and Borrow	\$ 1,488,360
Modified Subbase	\$ 136,832
Paved Shoulders, 10" PCC	\$ 378,216
Bridge Approach Section	\$ 189,660
PCC Pavement, Class C, Class 3, 10"	\$ 547,320
Removal of Existing Structures	\$ 25,000
RCB Extension	\$ 85,522
Culvert Pipe	\$ 57,732
Guardrail	\$ 31,738
Removal of Pavement	\$ 106,203
Traffic Control (5%)	\$ 152,329
Mobilization (5%)	\$ 152,329
Contingency (30%)	\$ 1,005,372
Roadway Total	\$ 4,356,613

Project Total: \$ 16,475,613

4. Alternative Number 4: Southern Parallel Realignment, Replace Existing Bridge with a 4-span horizontally curved 1070'-0" x 44'-0" CWPG Bridge

Alternative number 3 cuts through a fairly large bluff on the east side of the Nodaway River, resulting in earthwork waste. This alternative aims to reduce waste dirt and overall project impacts by moving the proposed bridge closer to the existing. Similar to Alternative Number 2, the bridge is located parallel to and 42 feet from the centerline of the existing roadway, this time to the south. The tangent from the west is extended to the tangent over the railroad and river. East of the river, the horizontal curve east of the project (PI Sta. 522+28.74) will be lengthened to the west, with a tangent off the lengthened curve extending to the tangent over the river and railroad. Similar to Alternative 3, this removed the west spiral from the curve, which will not be replaced. The resulting alignment does not

meet preferred design practice in that it results in two horizontal curves in the same direction with a relatively short tangent between them. In this case, the curves on either end of the proposed bridge have a 1,193-foot tangent between.

Similar to Alternative 3, this alternative will have more cut than fill.

Total length of roadway reconstruction with this alternative is 3,889.3 feet. The skew of the roadway to the railroad is 59 degrees, which matches the existing roadway skew to the tracks.

The proposed realignment impacts three drainage structures, a 4' x 5' RCB that would be extended with a 60" RCP, a 36" RCB with a flume, which will be extended with a 36" RCP to the south, and an 8' x 8' RCB that will be extended.

Replace the existing 824'-0" x 30'-0" Continuous Welded Plate Girder (CWPG) bridge with a 1070'-0" x 44'-0" horizontally curved CWPG Bridge with a skew of 45 degrees L.A. The bridge has four spans: (265'-0") – (355'-0") – (250'-0") – (200'-0"). The 355-foot center span is necessary to keep the west pier out of the railroad right-of-way and to position the east pier at a distance greater than 25 feet from the east future railroad track. The shift to the west of the railroad ROW results in this long span. One option to shorten this span would be to coordinate with the BNSF Railway and determine if they would allow Pier 1 to be located outside of a projected ROW line from the ROW location north of the existing bridge. The end span to interior span ratios are set close to the ideal span ratio for structural efficiency and to limit or eliminate uplift at the abutment. The west end span is a ratio of approximately 0.75 of span 1, this ratio is close to the ideal end span to interior span ratio of 0.8. The east interior span was set at 250'-0" to place Pier 3 in the east riverbank. The east end span is a ratio of 0.8. The bridge is partially located in a horizontal curve (approximately 186 feet of bridge at the west end is in a horizontal curve with a radius of 3,000 feet). The 44'-0" wide roadway width consists of two 12'-0" lanes and 10'-0" shoulders on each side. The bridge will have stub abutments and T-Piers. Since the bridge is located over the BNSF Railway the bridge will have standard single slope TL-5 barriers for the full length of the bridge.

Estimated Construction Cost

<u>Bridge Item</u>	<u>Estimated Cost</u>
Bridge	\$10,160,000
Remove Existing Bridge	\$ 308,000
Cofferdams (3 required)	\$ 150,000
Revetment, Class E (East Abutment Only)	\$ 25,000

Mobilization (10%)	\$ 1,064,000
Contingency (20%)	\$ 2,341,000
Bridge Total	\$14,048,000

<u>Roadway Item</u>	<u>Estimated Cost</u>
Excavation, Class 10 Roadway and Borrow	\$ 533,520
Modified Subbase	\$ 132,066
Paved Shoulders, 10" PCC	\$ 365,508
Bridge Approach Section	\$ 185,310
PCC Pavement, Class C, Class 3, 10"	\$ 528,263
Removal of Existing Structures	\$ 25,000
RCB Extension	\$ 67,823
Culvert Pipe	\$ 42,249
Guardrail	\$ 31,738
Removal of Pavement	\$ 108,729
Traffic Control (5%)	\$ 101,010
Mobilization (5%)	\$ 101,010
Contingency (30%)	\$ 666,668
Roadway Total	\$ 2,888,894

Project Total: \$ 16,936,894

B. Recommendations

We recommend Alternative 2 since the overall construction cost is the lowest for this option. It also minimizes impacts to surrounding properties.

C. Traffic Control

U.S. 34 will remain open to traffic during construction. Short duration lane closures will be necessary to construct the tie-ins at each end of the roadway reconstruction. Shoulder closures with temporary barrier rail will be necessary for bridge construction and construction of embankment and culvert extensions within the clear zone of the existing highway.

Note that construction coordination will be necessary with the BNSF Railway. Railroad flaggers will be needed for all work within 50 feet of the railroad. The time for setting of beams may be controlled by railroad operations.

D. Special Considerations

Concept_2023-11-17_AdamsUS34BNSF.docx

Extensive coordination will be necessary with BNSF Railway during preliminary design to ensure that they approve of the design.

Additional right-of-way may be necessary north of the existing north right-of-way line to accommodate the new bridge location and fill.

The need for permits is unknown. The need for permits will be evaluated as part of the environmental review for the project.

E. Construction Sequence

It is anticipated that all work will be awarded to one prime contractor. The Bridges and Structures Bureau will coordinate the plan preparation with the District 4 Office.

F. Program Status

This project is listed in the 2024-2028 Iowa Transportation Improvement Program with \$16,421,000 programmed for construction in FY 2028. \$13,000 is programmed for right-of-way. The project is currently scheduled for an October 20, 2026 letting.

Concept_2023-11-17_AdamsUS34BNSF.docx

LT and RT, and at 505+84 LT. The Iowa DOT will review records to ensure that these are permitted. All three additional entrances are located at or near ditch high points and should not require entrance pipes. A safety dike will be constructed opposite Loomis Avenue. Reflectors / delineators have recently been installed at this location so that traffic doesn't attempt to access Quarry Road here – these will need to be removed and reinstalled with this project.

Staging was discussed. Project staging is intended as follows:

- Stage 1: Roadway grading, paving, and bridge construction. Two-way traffic will be maintained on U.S. 34 during this stage. TBR will be installed on the north shoulder of U.S. 34 to allow construction within clear zone. The Iowa DOT would like to review a typical section showing the proposed TBR placement. Snyder & Associates will also need to review staged drainage to ensure water doesn't get trapped between new and existing roadway embankment.
- Stage 2: Close WB lanes using temporary signals and construct WB tie-ins between new and existing pavement. Separate signal installations will be necessary at the east and west tie-ins due to the length between them.
- Stage 3: Move traffic only newly constructed WB lanes and new bridge. Construct EB tie-ins. Will require temporary signals (2 set-ups).
- Stage 4: Two-way traffic utilizes new pavement and bridge. Remove existing bridge and pavement and grade through embankment.

The Iowa DOT anticipates that this will be a multi-year contract. An analysis will need to be done to determine what state is acceptable to leave the project in over the winter, and notes added to the J sheets to that effect.

Bridge design was discussed in greater detail. Finger plate expansion joints will be used on the bridge due to length. Steel girders will utilize disc bearings at all substructure units, with fixed disc bearings at Pier 2. Structural steel will be weathering steel, not painted. Proposed piers are T-piers on pile. Barrier rail will be TL-5 single slope rail for entire bridge length. It is assumed that aesthetics will not be included in bridge design. Hydraulics are not a controlling factor in bridge design. Freeboard and backwater requirements are met. It is assumed that any future access road required for BNSF Railway will be constructed by railroad forces at a later time and will not be included in this project. Northeast wingwall of bridge may need to be extended (similar to a retaining wall) to limit fill into river. It may be necessary to armor that west river bank at Pier 2 due to bank erosion throughout the river bend. The District also discussed the erosion associated with the discharge of the bridge deck drains with such a large fall distance. The discharge from the bridge deck drains blows around and misses the 8' x 8' splash basins that are installed with bridge construction. The District indicated a desire to provide protection under the bridge deck drains to eliminate the erosion issue. SBI indicated that the design would attempt to limit the number of deck drains to reduce the number of areas where erosion can occur. SBI also mentioned that perhaps a rippapped flume type channel be centered under the drains to protect the drip line area from the drain discharge erosion. SBI mentioned that they would discuss potential details with the BSB.

Culverts impacted by construction include the following:

- Culvert Sta. 475+40. Existing 48" RCP with D section and elbow at inlet (LT) and flume at outlet. Field review indicated FES at inlet is separated from pipe, and flume at outlet is very short and the waterway silted. Proposed extension LT will interfere with existing 36" CMP that is noted to be in "terrible shape." Field visit indicated severe erosion around inlet headwall, with berm over top eroded away and pipe exposed. It is believed that the CMP originally was a pond outlet, but the pond is no longer present. CMP is entirely within U.S. 34 ROW. DOT is OK with removing the CMP. Flume at

outlet will be removed and pipe extended to new foreslope.



36" CMP (failed)

- Sta. 476+40. Existing 4' x 5.5' arched RCB, believed to be a cattle pass. Site visit confirmed. Iowa DOT will coordinate with landowner to determine if cattle pass still required. If so, replacement with a new, larger diameter pipe or structure might be more economical than a special structure extension.



Cattle Pass. Opening within existing clear zone.

- Culvert Sta. 483+79. Existing 4' x 5' RCB. Remove headwall LT (existing headwall bulging / failing) and extend with 60" RCP.
- Culvert Sta. 504+89. Existing 36" RCP with flume LT. Flume is shallow and can be removed. Extend with 36" RCP left.
- Culvert Sta. 510+89. Existing 8' x 8' RCB. Remove headwalls LT and extend with 8' x 8' RCB. Note heavy erosion at box inlet, particularly at Quarry Road foreslope.

Where pipes are required under entrances, RCP will need to be used. The condition of all structures / pipes will need to be confirmed prior to extension.

During field review, a slope failure was observed at Sta. 490+00 RT. Field review also noted a shallow pond extending within DOT ROW to the roadway foreslope near Sta. 482+00 – 483+00 LT. This pond will need to be addressed during environmental review. It is likely that a rock berm will be constructed near the ROW line to prevent runoff from new roadway embankment from entering the portion of the pond north of DOT ROW.



Pond within roadway ROW LT near Sta. 483+00

Also noted during field review, the existing roadway has centerline and shoulder rumble strips. The Iowa DOT requested double sized end drains / rock flumes be installed off the end of the bridge due to the amount of time it takes for grass to establish itself and general erosion problems in the project corridor. Wood excelsior matting may also be necessary on bridge abutment foreslopes. Large erosion holes were noted underneath the outlets of the existing bridge deck drains – the new bridge will need to incorporate measures to control erosion from runoff / bridge drains.

Project earthwork was calculated using straight cut and fill and not based on staged construction. Quantities include 49,939 CY cut and 141,124 CY fill + 30% shrink (91,184 CY need). Note that most of the cut consists of existing embankment and will not be available for use in construction of new embankment / will need to be wasted.

Project cost estimates are \$4.7 million for roadway work and \$10.2 million for the bridge, for a total project cost of \$14.9 million.

ROW will be required to construct the project. Right of entry permitting from railroad property will also be necessary.

Preliminary plan submittal dates are as follows:

- D3: April 19, 2024. This date will be delayed to accommodate roadway realignment and potential foreslope changes.
- B1: July 19, 2024
- S2: August 30, 2024
- D5: September 6, 2024
- S3: December 4, 2024

This waterway is not on a state water trail or paddling route.

This project is currently scheduled for a January 20, 2027 letting. The estimated cost of construction shown in the final concept was \$14,648,921. The current cost estimate is \$14.9 million.

Machine Guidance Electronic Files Checklist

Add information to address any incomplete items below:

Yes	N/A	No	
<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Horizontal and Vertical Alignments Complete
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Typical Templates showing proposed Pavement, Shoulder, Foreslope design
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Correct Feature Naming for Roadway Breaklines and Components

- | | | | |
|-----|----------------------|----------------|-------------------|
| cc: | B. Hofer | S. J. Gent | M. J. Kennerly |
| | W. A. Sorenson | E. C. Wright | K. D. Nicholson |
| | B. Bradley | K. K. Patel | K. Brink |
| | J. E. Laaser-Webb | C. Poole | N. Pohlen |
| | N. L. Cuva | M. A. Swenson | C. B. Brakke |
| | D. E. Sprengeler | J. S. Nelson | M. V. Dyke |
| | M. Nop | S. P. Anderson | D. Stokes |
| | B. Smith | M. Dell | R. Harris |
| | D. Heeren | D. T. Ta | J. E. Bartholomew |
| | G. Cagle | D. Blue | S. Cook |
| | J. Hart | S. Majors | J. Ellis |
| | B. Worrel | R. Meyer | |
| | T. Quam (RR) | | |
| | S. Schram | W. Mayberry | B. Smith |
| | N. Epperson | D. Redmond | Local FHWA |
| | Others on Field Exam | | |

From: Scott Prouty <sprouty@schildberg.com>
Sent: Tuesday, May 14, 2024 10:16 AM
To: Asberry, Blake <Blake.Asberry@iowadot.us>
Subject: Sorry for the delay

CAUTION:

This email originated from outside the Iowa Department of Transportation.
Do not click links or open attachments unless you recognize the sender and know the content is safe.

Blake,

I am fine with the proposed closing of the cattle crossing under HWY 34 west of Corning in Adams County, Iowa. Please note the owner of the affected properties is Midwest Rock Products, Inc., of which I also serve as the CFO. Should you have any questions or further comments, please contact me at your convenience. Have a great week!

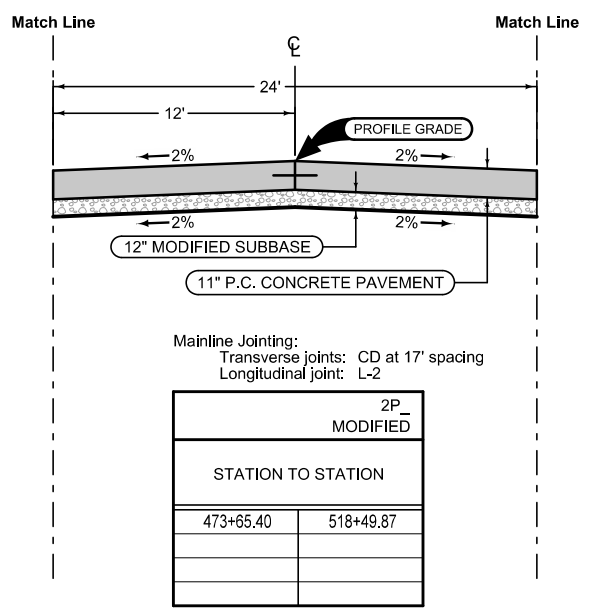
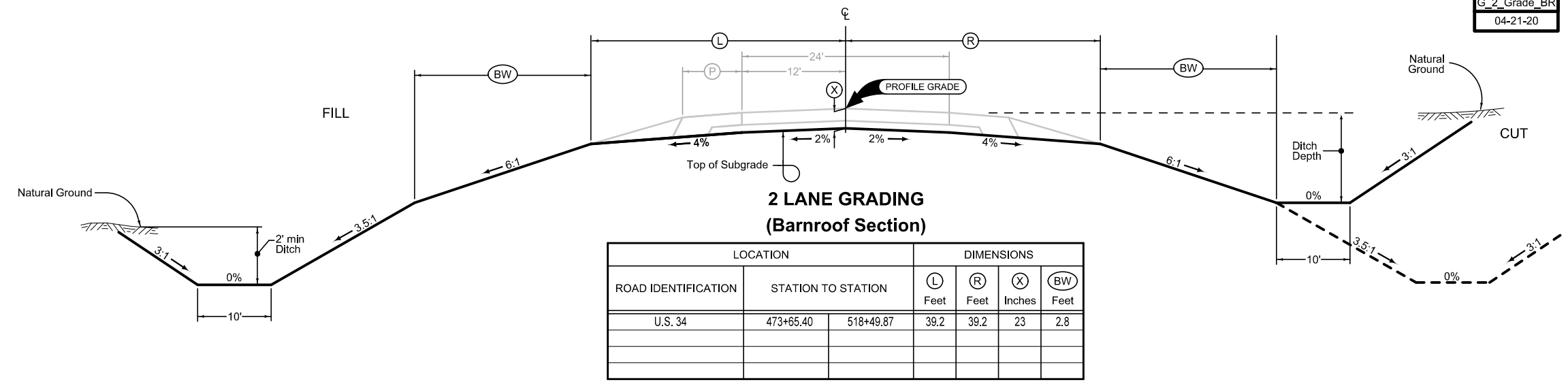
Scott Prouty, CPA
Chief Financial Officer
Schildberg Construction Co., Inc.



G_2_Grade_BR
04-21-20

Normal section shown may be modified appropriately in areas of superelevated curves or other locations specifically designated by the Engineer.

See Plan & Profile sheets and cross sections for additional details of ditches and backstops.



Paved Shoulder Alternates

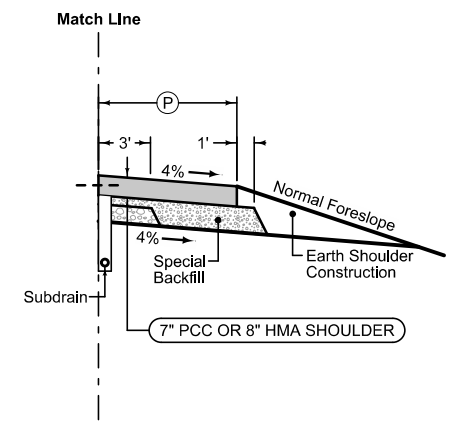
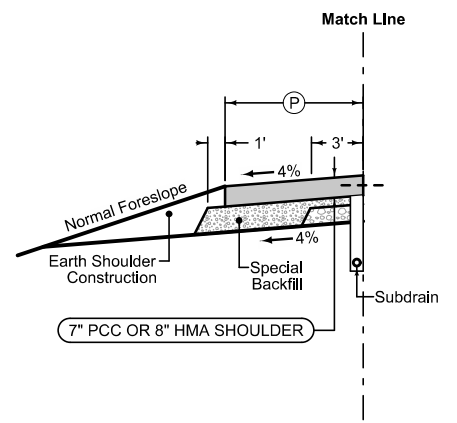
PCC Shoulder Jointing:
Longitudinal joint: BT-1 or BT-5
Transverse joints: C at 17' spacing
HMA Shoulder Jointing:
Longitudinal joint: B

2_P_ALT_ 04-21-20		
STATION TO STATION		(P) Feet
473+65.40	518+49.87	10

Paved Shoulder Alternates

PCC Shoulder Jointing:
Longitudinal joint: BT-1 or BT-5
Transverse joints: C at 17' spacing
HMA Shoulder Jointing:
Longitudinal joint: B

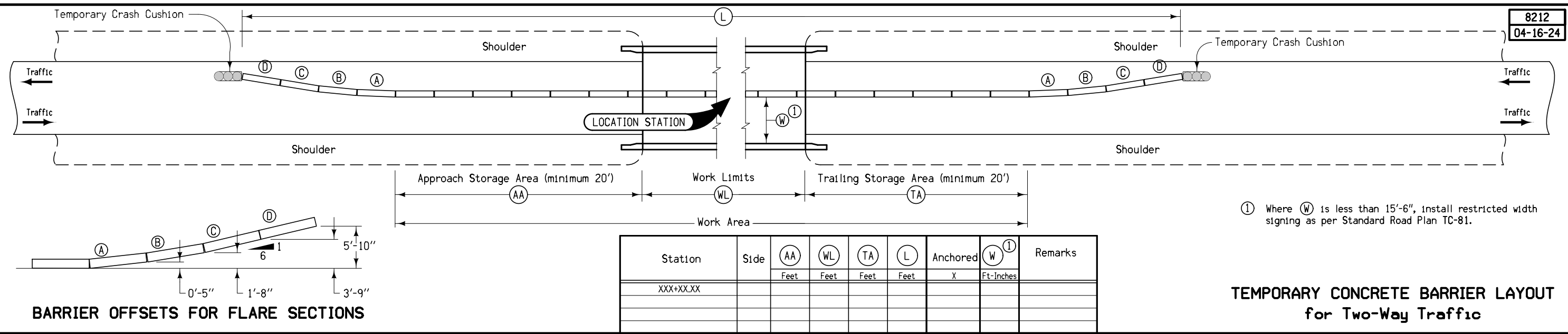
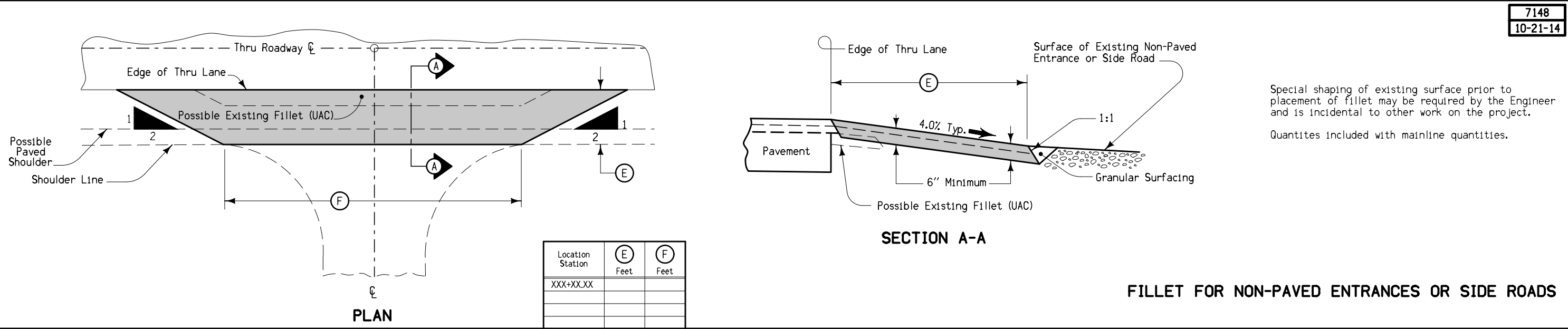
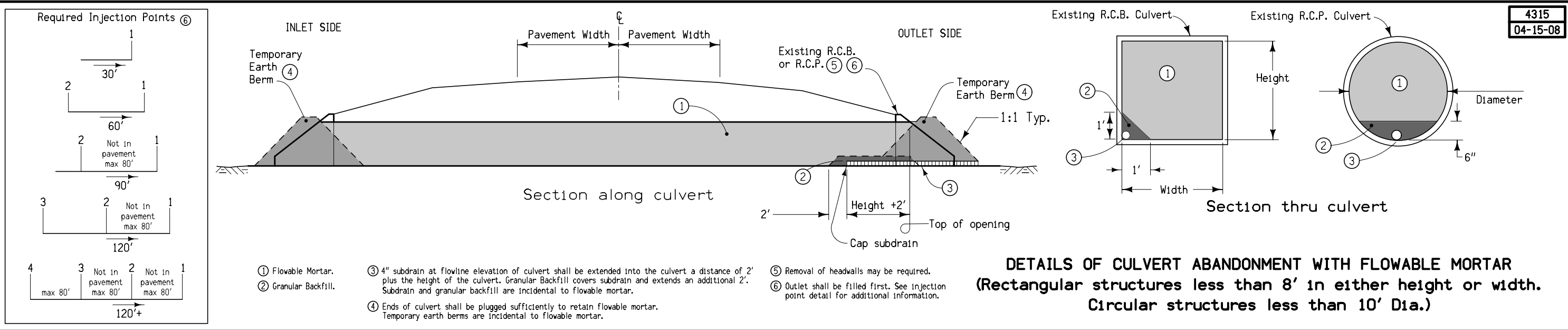
2_P_ALT_ 04-21-20		
STATION TO STATION		(P) Feet
473+65.40	518+49.87	10

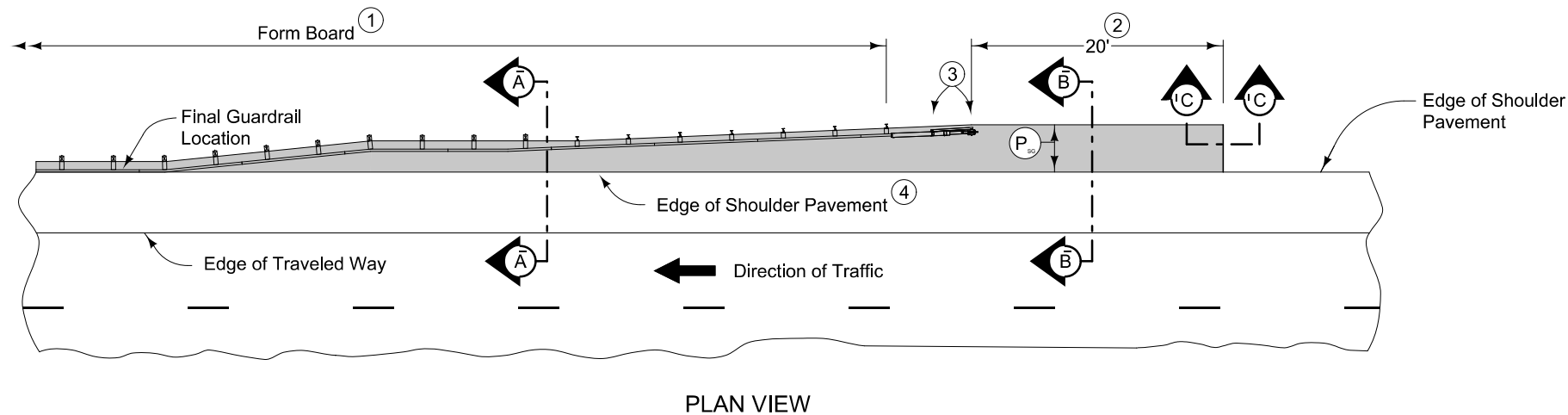


I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

Cindy A. Spencer, P.E. Date _____
License Number P17561
My License Renewal Date is December 31, 2022
Pages or sheets covered by this seal:
A.1-A.12, B.1-B.3, D.1-D.5, E.1, G.1-G.4, J.1-J.18, U.1-U.3, W.1-W.27

PRELIMINARY





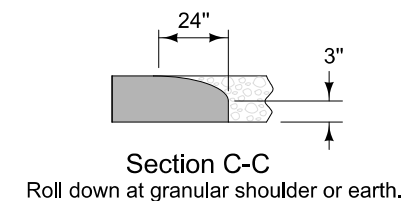
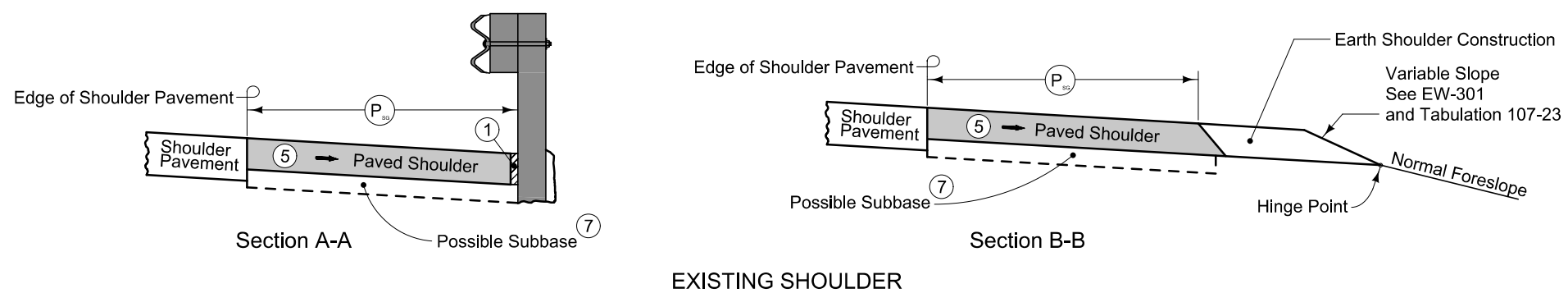
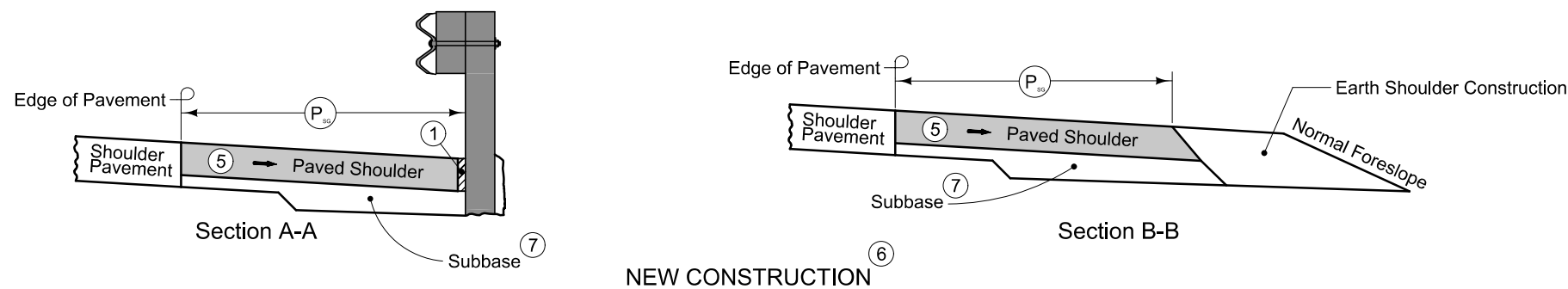
9" HMA Paved Shoulder at guardrail. 8" PCC may be substituted with the following jointing layout:

Match mainline pavement joint spacing. When mainline pavement is 8" or greater in thickness, place additional transverse 'C' joints in shoulder at mid-panel of the mainline pavement. Place longitudinal 'C' joint at P/2 from edge of mainline pavement when P is greater than 10' wide. Terminate longitudinal joint at transverse joint less than 10' in length.

Compaction of HMA is required to face of guardrail post. Hand compaction will be allowed under guardrail. Removal and reinstallation of guardrail will be allowed with no additional payment.

Refer to Tabulation 112-9 for shoulder quantities.

- ① PCC option only: When guardrail posts are installed prior to construction of PCC paved shoulder, fasten form board to the face of guardrail posts for the length shown.
- ② Continue paved shoulder 20 feet beyond the center of the first post.
- ③ Shoulder may be notched for first 2 posts or post sleeves may be installed through pavement. Do not drive posts through pavement.
- ④ 'KT' (per PV-101) joint for PCC shoulder. 'B' (per PV-101) joint for HMA shoulder.
- ⑤ Match shoulder slope.
- ⑥ The Contractor has the option to pave the paved shoulder at guardrail and the full width paved shoulder as one operation.
- ⑦ Refer to other details in the plan.



PAVED SHOULDER AT GUARDRAIL
(ADJACENT TO FULL WIDTH PAVED SHOULDER)

SURVEY SYMBOLS

- Interstate Highway Symbol
- U.S. Highway Symbol
- Iowa Highway Symbol
- County Road Highway Symbol
- Evergreen Tree
- Deciduous Tree
- Fruit Tree
- Shrub (Bushes)
- Timber
- Hedge
- Stump
- Swamp
- Rock Outcrop
- Broken Concrete
- Revetment (Rip Rap)
- Cemetery
- Grave
- Cave
- Sink Hole
- Board Fence
- Chain Link or Security Fence
- Wire Fence
- Terrace
- Earth Dam or Dike (Existing)
- Tile Outlet
- Edge of Water
- Existing Drainage
- Right of Way Rail or Lot Corner
- Concrete Monument
- Well
- Windmill
- Beehive Intake
- Existing Intake
- Existing Utility Access (Manhole)
- Fire Hydrant
- Water Hydrant (Rural)
- Septic Tank
- Cistern
- L.P. Gas Tank (No Footing)
- Underground Storage Tank
- Latrine
- Satellite TV Dish
- Water Hook Up
- Radio Tower
- Tower Anchor
- Guardrail (Beam or Cable)
- Guard Post (one or two)
- Guard Post (over two)
- Filler Pipe
- Gas Valve
- Water Valve
- Speed Limit Sign
- Mile Marker Post
- Sign
- Traffic Signal Control Box
- Rail Road Signal Control Box
- Telephone Switch Box
- Electric Box

UTILITY LEGEND

Sub-Surface Utility Mapping Quality Level is in accordance with C/ASCE 38-02 Standard Guidelines for the Collection and Depiction of Existing Subsurface Utility Data.

Remark Abbreviations

- QLA Quality Level A Highest guideline quality level
- QLD Quality Level D Lowest guideline quality level
- E1(B)** *EL1B, MIDAM-Mid-American Energy - Quality B&D*
- FO1(B)** *FO1B, ICN-Iowa Communications Network - Quality B&D*
- FO2(B)** *FO2B, Windstream Communications - Quality B*
- FO3(B)** *FO3B, Aureon - Quality B&D*
- T1(B)** *TL1B, Frontier Communications - Quality B*
- TV(B)** *TV1B, Mediacom - Quality B*
- W(B)** *WL1B, Southern Iowa Rural Water Association - Quality B*
- W2(B)** *WL2B, Corning Municipal Utilities - Quality B*
- SAN.** *SA1D, City of Corning - Quality D*

PLAN VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

LINEWORK		Design Color No.	
Green	(2)		Existing Topographic Features and Labels
Blue	(1)		Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation
Magenta	(5)		Existing Utilities
SHADING		Design Color No.	
Lavender	(9)		Temporary Pavement Shading
Yellow	(4)		Proposed Pavement Shading
Orange	(6)		Proposed Granular Shading
Orange	(70)		Proposed Shoulder Granular Shading
Yellow	(68)		Proposed Shoulder Paved Full Depth Shading
Yellow	(132)		Proposed Shoulder Paved Partial Depth Shading
Gray, Dark	(112)		Proposed Grade and Pave Shading "In conjunction with a paving project"
Brown, Light	(236)		Grading Shading
Orange, Light	(134)		Proposed Granular Entrance Shading
Yellow	(220)		Proposed Paved Entrance Shading
Tan	(8)		Proposed Sidewalk Shading
Blue, Light	(230)		Proposed Sidewalk Landing Shading
Pink	(11)		Proposed Sidewalk Ramp Shading
Green, Light	(225)		Existing Pavement Shading
Red	(3)		Proposed Structure Shading
Red	(3)		Delineates Restricted Areas

PROFILE VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

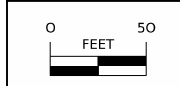
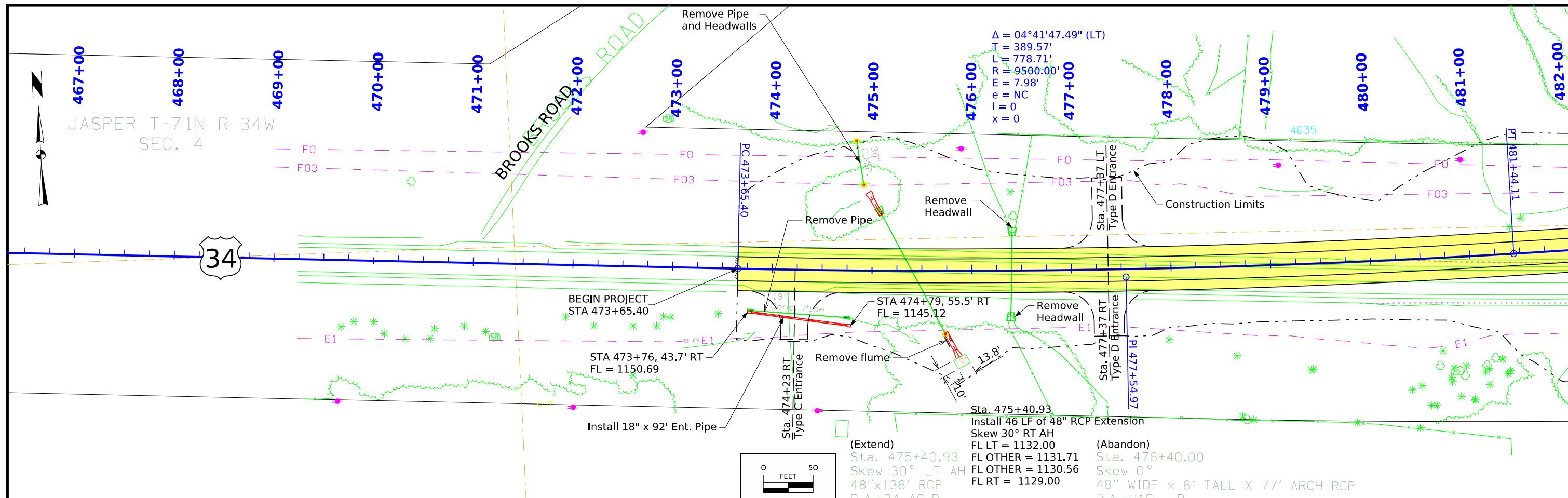
LINEWORK		Design Color No.	
Green	(10)		Existing Ground Line Profile
Blue	(1)		Proposed Profile and Annotation
Magenta	(5)		Existing Utilities
Blue, Light	(230)		Proposed Ditch Grades, Left
Black	(0)		Proposed Ditch Grades, Median
Rust	(14)		Proposed Ditch Grades, Right

- #### REFERENCE POINT
- Station
 - Section Corner
 - Ground Line Intercept
 - Saw Cut
 - Guardrail
 - Trench Drain
 - High Tension Cable Guardrail
 - Sheet Pile
 - Pavement Removal
 - Clearing & Grubbing Area

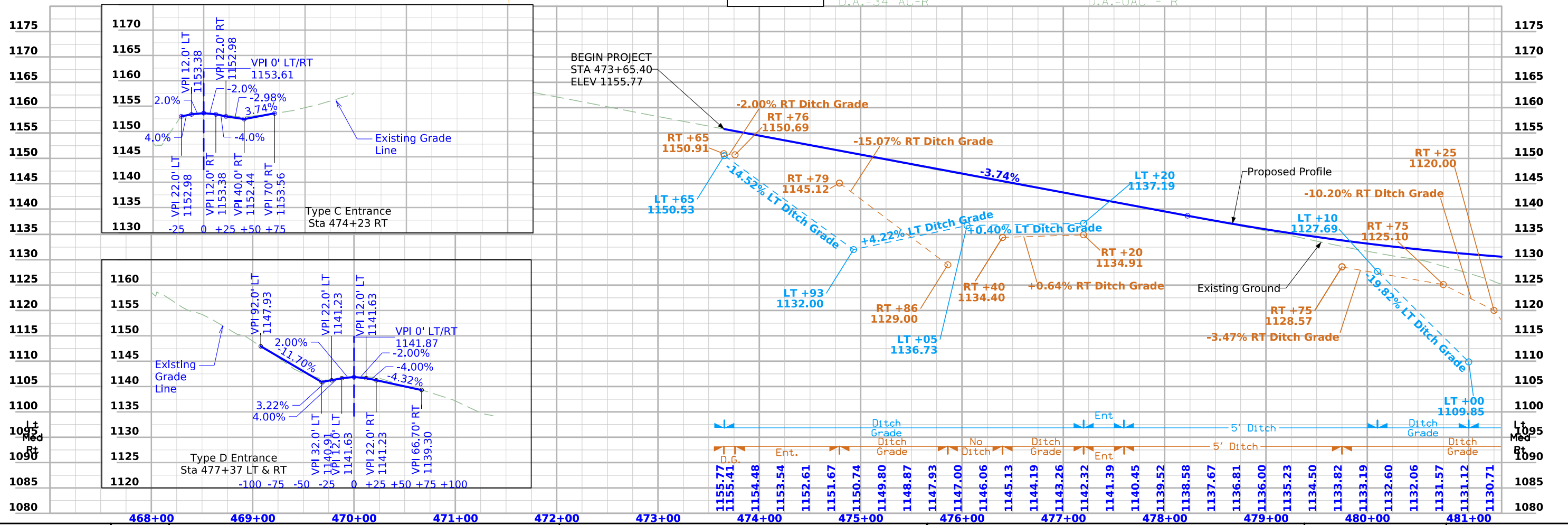
- #### RIGHT-OF-WAY LEGEND
- Proposed Right-of-Way
 - Existing Right of Way
 - Existing and Proposed Right-of-Way
 - Easement and Existing Right-of-Way
 - Easement (Temporary)
 - Easement
 - Access Control
 - Property Line

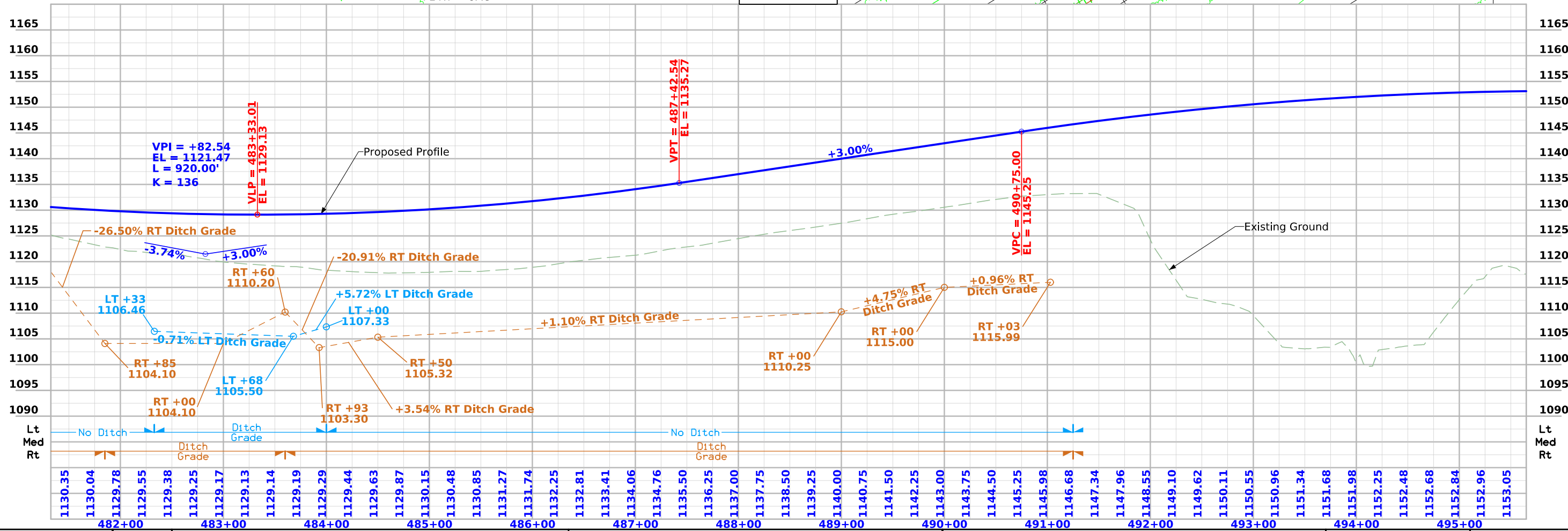
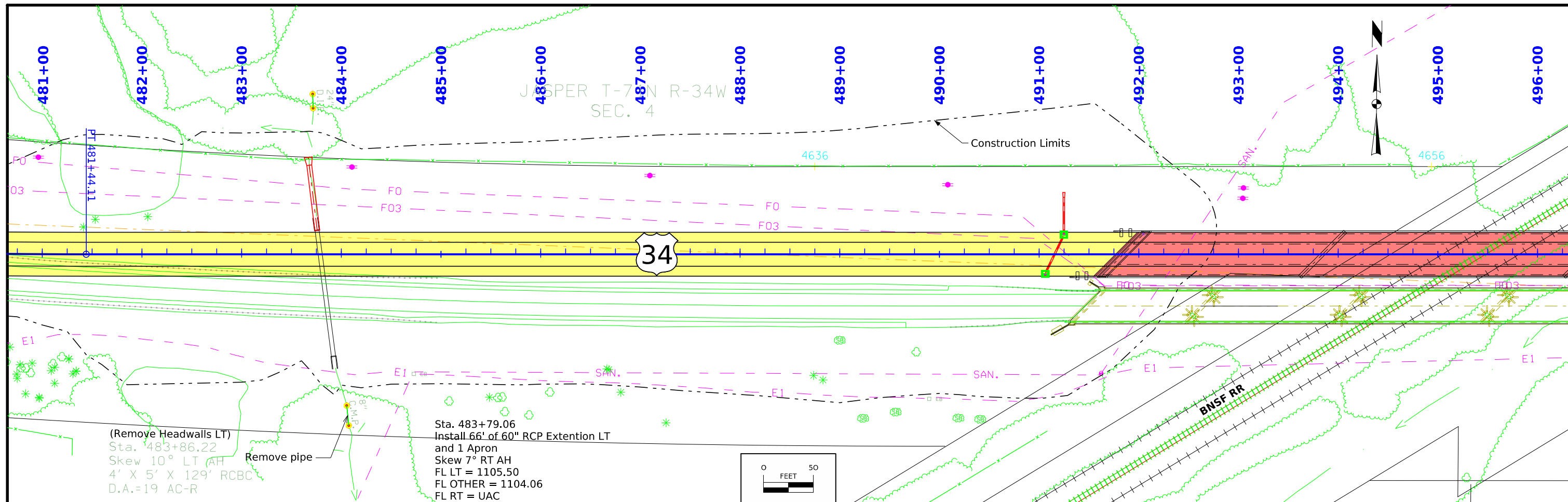
PLAN AND PROFILE LEGEND AND SYMBOL INFORMATION SHEET

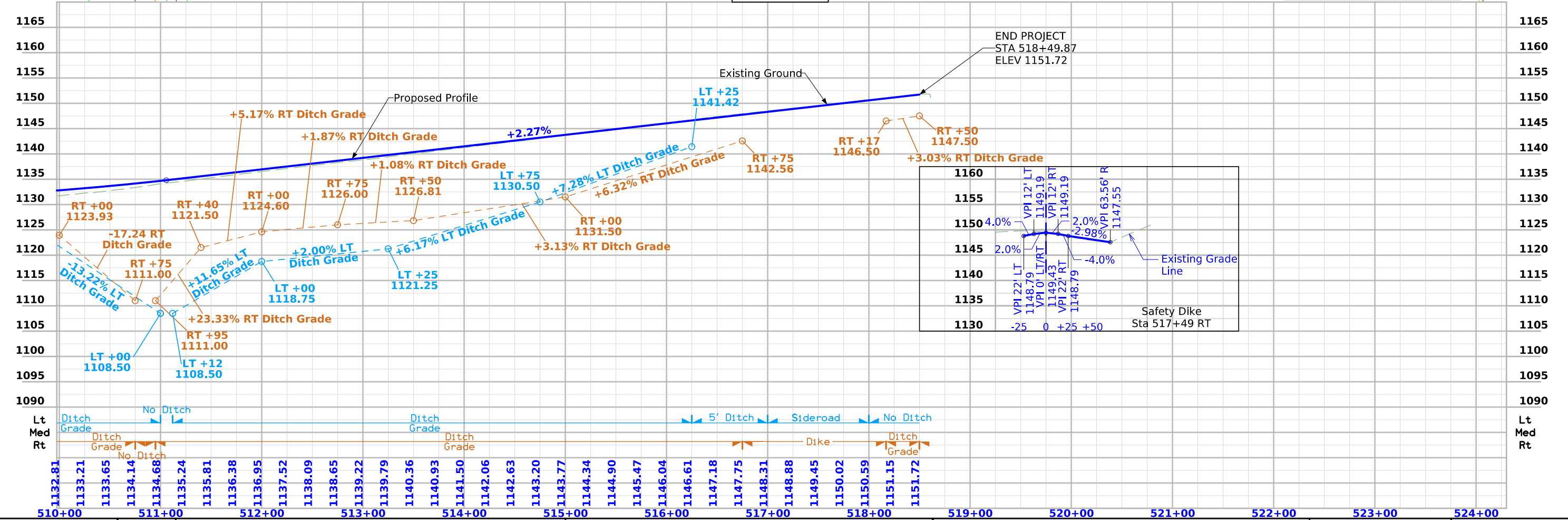
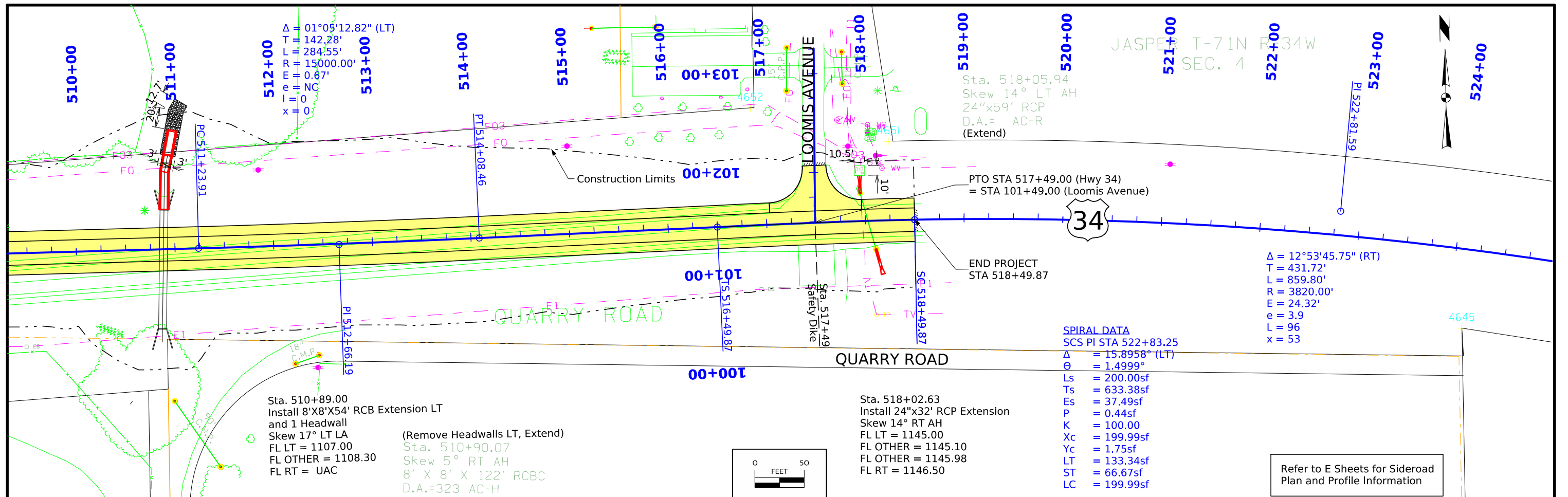
(COVERS SHEET SERIES D, E, F, & K)



(Extend)
 Sta. 475+40.93
 Skew 30° LT AH
 48"x136' RCP
 D.A.=34 AC-R
 (Abandon)
 Sta. 476+40.00
 Skew 0°
 48" WIDE x 6' TALL x 77' ARCH RCP
 D.A.=UAC - R







JASPER T-71N R-34W
SEC. 3



PTO STA 101+49.00 (Loomis Avenue)
STA 517+49.00 (Hwy 34)

Sta. 518+05.94
Skew 14° LT AH
24"x59' RCP
D.A.= AC-R

100+00

101+00

102+00

516+00

103+00

517+00

518+00

519+00

520+00

QUARRY ROAD

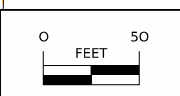
BEGIN PAVING
101+61.01

Sta. 517+49
Safety Dike

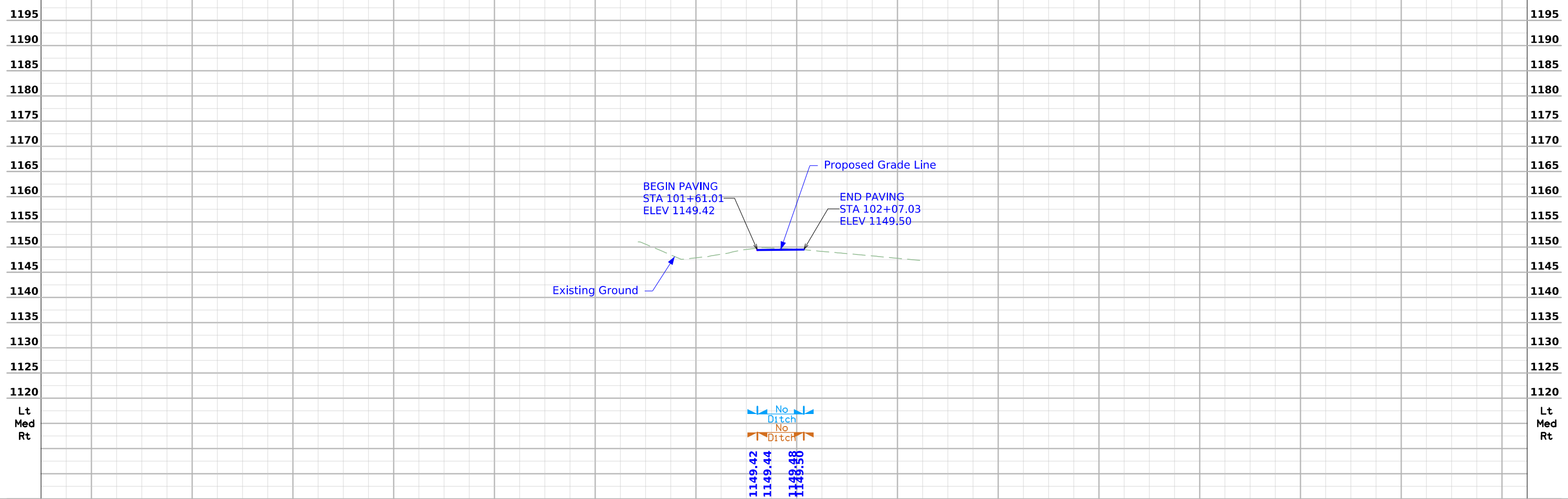
LOOMIS AVENUE

END PAVING
102+07.03

Construction Limits



Refer to D Sheets for Mainline
Plan and Profile Information



Survey Information

Adams County
BRF-034-3(041)—38-02
US34 E Nodaway River and RR 0.8 mi W of IA 148
Project Directory: 0203402022
SAP-671.2

Party Personnel

Eric Miller - Survey Project Manager/PLS
Brian Leonard - Survey Party Chief
Jeff Pavelka - Survey Party Chief
Sam Blaisdell - Office Technician

Date(s) of Survey

Begin Date 09/11/2023
End Date 01/24/2024

General Information

Measurement units for this survey are US survey feet. This project is for preliminary survey and bridge/roadway concept design for the replacement of U.S. 34 Bridge over the E. Nodaway River and the BNSF Railroad in Adams County, Iowa just southwest of Corning.

Vertical Control

Vertical datum for this survey is relative to NAVD88, Geoid 12a. Orthometric elevations were computed by averaging a minimum of five observations with appropriate time spans between from nearby Iowa RTN reference stations at control point FENO1. The vertical standard deviation of these observations was less than 0.05'. Additional benchmarks were placed throughout the project using a GNSS Base-Rover setup relative to control point FENO1. A minimum of three observations with appropriate time spans between were collected. The vertical standard deviation of these observations was less than 0.03'.

Horizontal Control

The project coordinate system is the Iowa Regional Coordinate System, Zone 12. Horizontal datum is NAD83 (2011) for Epoch 2010.00. The projection parameters for Zone 12 of the IaRCS is defined below:

Traverse Mercator Projection North American Datum of 1983
Origin Lat: 40°55'00"N
Origin Central Meridian: 093°45'00"W
Central Meridian Scale: 1.000037
False Northing: 6,200,000
False Easting: 22,500,000

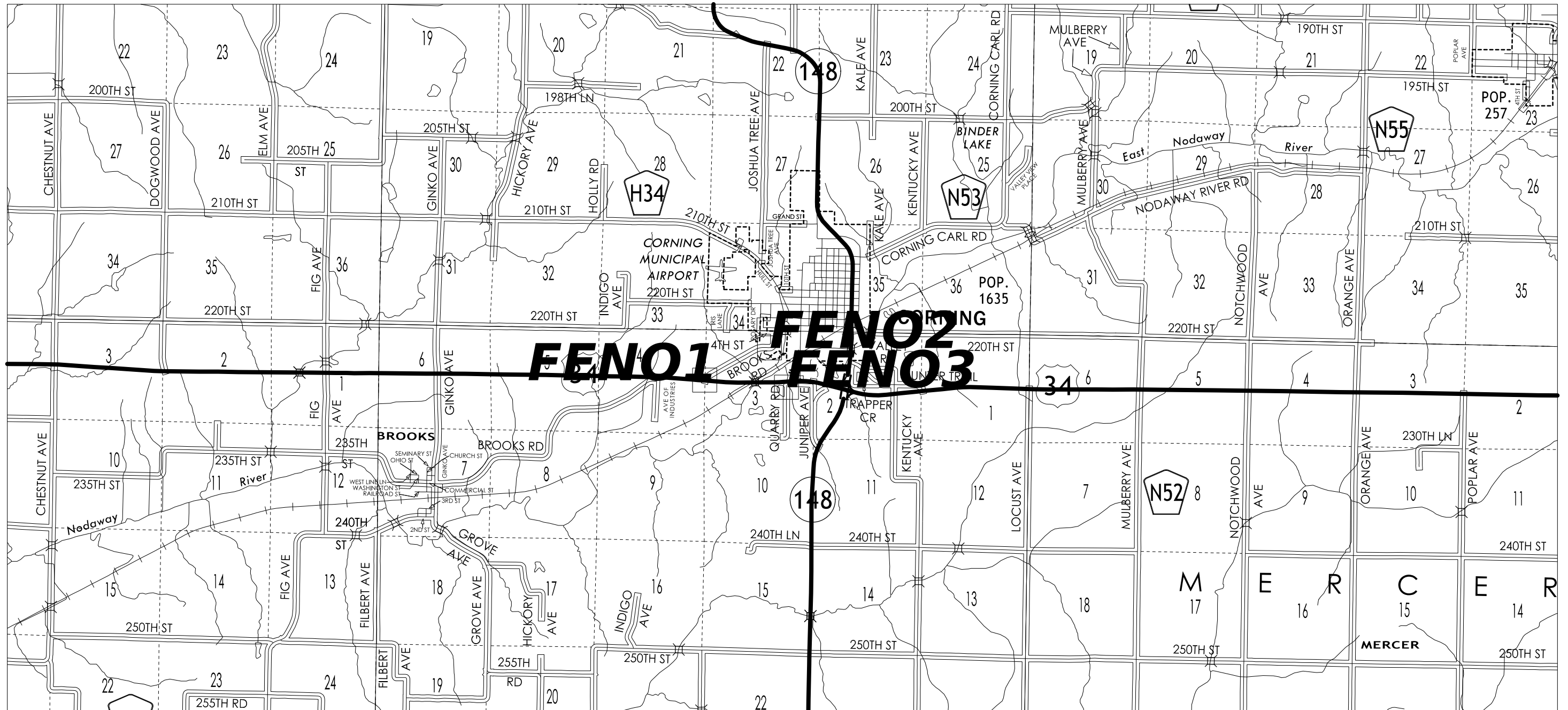
Coordinate values were determined by averaging a minimum of five IaRTN observations with appropriate time spans between on FENO1. The horizontal standard deviation of these observations was less than 0.05'. Additional control points were placed throughout the project using a GNSS Base-Rover setup relative to control point FENO1. A minimum of three observations with appropriate time spans between were collected. The horizontal standard deviation of these observations was less than 0.03'.

Alignment Information

The U.S. 34 alignment for this survey was provided by the Iowa Department of Transportation.

CONTROL POINT VICINITY MAP

This map is a guide to the vicinity of the primary project control points. Primary control is for use with RTK base stations and for RTN validation. Future surveys will use primary project control to establish temporary control as needed for construction or other surveying applications.



HORIZ. DATUM: NAD83(2011) for EPOCH 2010.00 (IaRTN 2019 Adjustment) - Iowa RCS Zone 12 (U.S. Survey Foot)

VERT. DATUM: NAVD88 - Geoid Model: 2018

Coordinate listing from next sheet will be used with IaRTN for monument recovery. No other reference ties are given.

HORIZONTAL AND VERTICAL PROJECT CONTROL COORDINATE LISTING
 HORIZ. DATUM: NAD83(2011) for EPOCH 2010.00 (IaRTN 2019 Adjustment)
 Ia. Regional Coordinate System Zone 12 (U.S. Survey Foot)
 VERT. DATUM: NAVD88
 Geoid Model: 2018

Point Name	Northing	Easting	Elevation	Feature Definition	Description
FENO1	6224606.47	22221777.89	1162.17	FENO	SET FENO MONUMENT, 60'+/- NORTH OF CENTERLINE OF HIGHWAY 34, 40'+/- WEST OF CENTERLINE OF BROOKS ROAD, 12'+/- EAST OF STOP SIGN, 12'+/- WEST OF STREET SIGNS
FENO2	6226176.21	22224934.95	1125.40	FENO	SET FENO MONUMENT, 20'+/- NORTH OF CENTERLINE OF BROOKS ROAD, 25'+/- SOUTH OF FENCE LINE, 60'+/- EAST OF GRAVEL DRIVE AT END OF CURVE
FENO3	6224262.67	22225760.64	1127.91	FENO	SET FENO MONUMENT 15'+/- WEST OF CENTERLINE QUARRY ROAD, 5'+/- NORTH OF SIGN WITH ARROW, 10'+/- EAST OF FENCE LINE AT END OF CURVE

SUPERELEVATION DATA

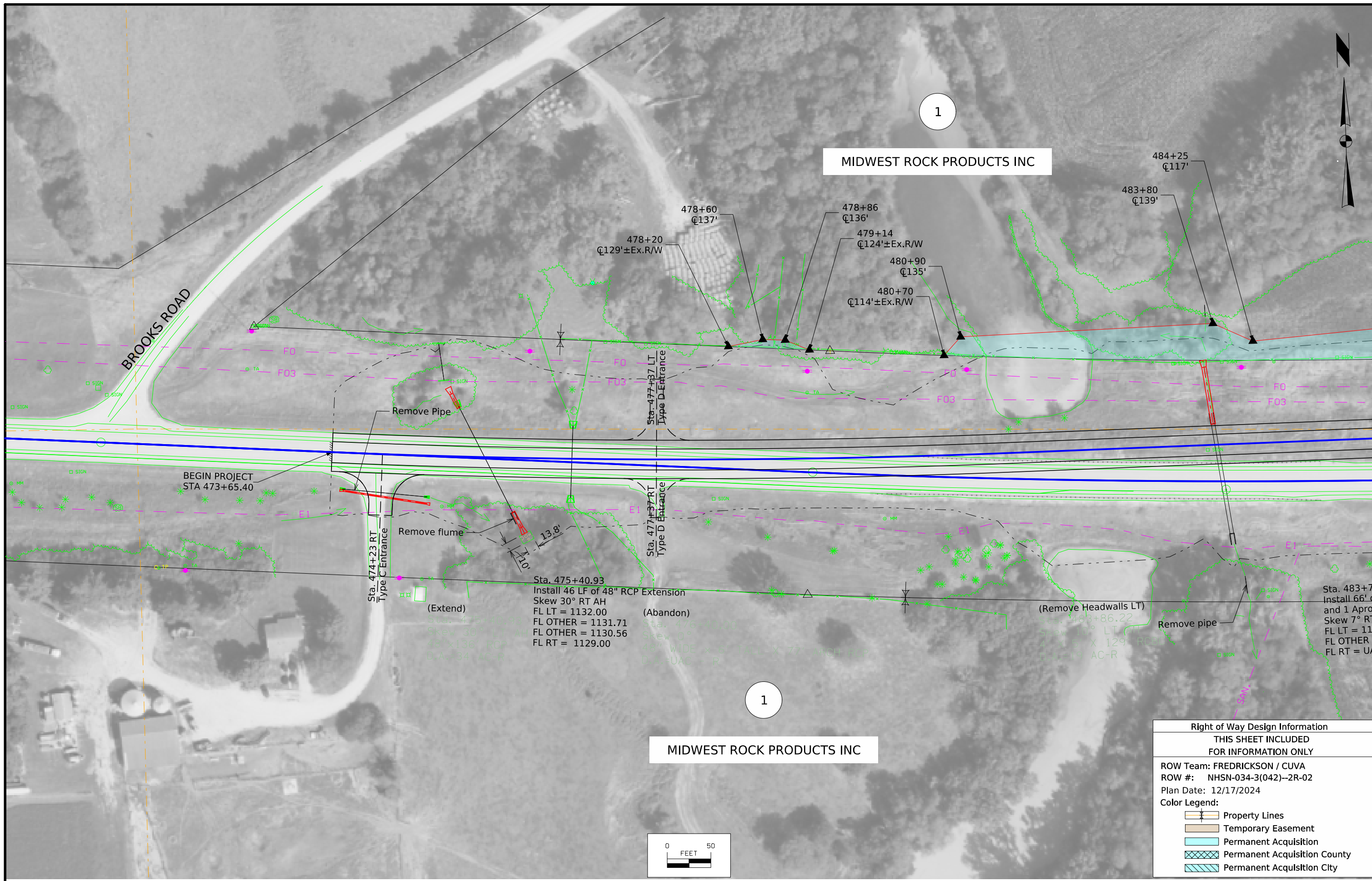
See PV-300 Series

Road Identification	Circular Curve or Spiral Curve Name	Radius FT	Superelevation Data			Standard Road Plan	Section A-A	Section B-B	Section C-C	Section D-D	Section E-E	Section F-F	Case A	Case B	Case C	Case S	Case T	Case U	Remarks
			e %	L FT	x FT														
U.S. 34	Curve 1	9550	NC	0	0	PV-301													
U.S. 34	Curve 2	15000	NC	0	0	PV-301													
U.S. 34	Curve 3	3820	3.9	96	53	PV-301	517+00.87	517+53.87	518+06.87	518+49.87									

SPIRAL OR CIRCULAR CURVE DATA

Horizontal Alignment Data

Name	Location	ΔSCS	Spiral Data							Curve Data					Remarks		
			θS	Ls	Ts	Es	Xc	Yc	L.T.	S.T.	ΔC	T	L	R		E	
Curve 1	477+54.97											04°41'47.49"	389.57	778.71	9500	7.980	
Curve 2	512+66.19											01°05'12.82"	142.28	284.55	15000	0.670	
Curve 3	522+83.25	15.8958	1.4999	200	633.38	37.49	199.99	1.75	133.34	66.67		12°53'45.75"	431.72	859.8	3820	24.320	



BEGIN PROJECT
STA 473+65.40

Sta. 474+23 RT
Type C Entrance

Remove flume

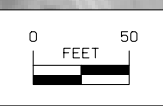
Sta. 475+40.93
Install 46 LF of 48" RCP Extension
Skew 30° RT AH
FL LT = 1132.00
FL OTHER = 1131.71
FL RT = 1129.00

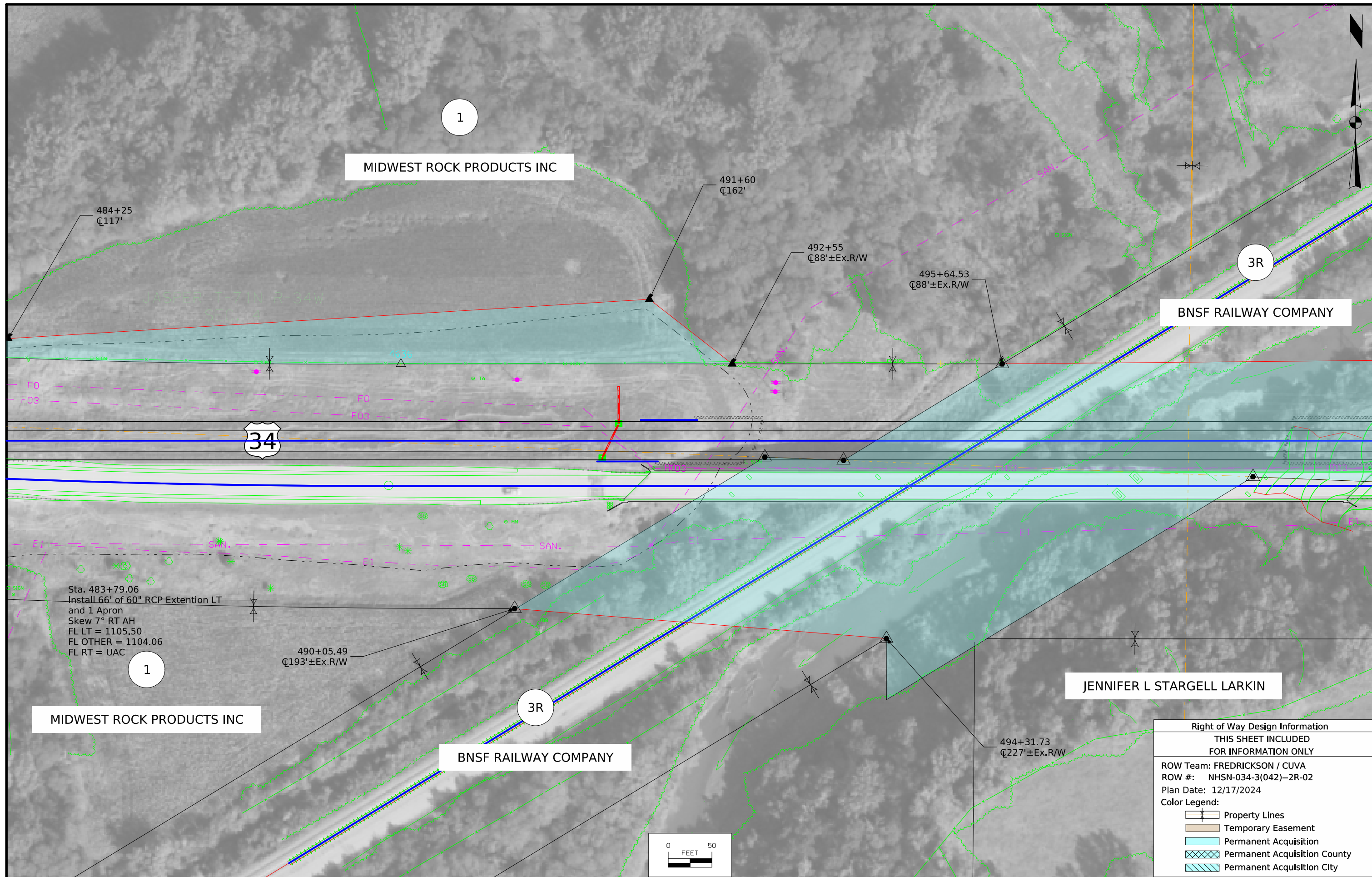
(Abandon)
Sta. 476+40.00
Skew 0°
48" WIDE x 6' TALL x 77' ARCH RCP
D.A. = UAC + R

(Remove Headwalls LT)
Sta. 483+86.22
Skew 17° LT AH
4' x 5' x 129' RCP
D.A. = 19' AC-R

Sta. 483+70
Install 66' and 1 Apr
Skew 7° RT
FL LT = 1131.00
FL OTHER = 1130.00
FL RT = UAC

Right of Way Design Information	
THIS SHEET INCLUDED FOR INFORMATION ONLY	
ROW Team: FREDRICKSON / CUVA	
ROW #: NHSN-034-3(042)--2R-02	
Plan Date: 12/17/2024	
Color Legend:	
	Property Lines
	Temporary Easement
	Permanent Acquisition
	Permanent Acquisition County
	Permanent Acquisition City





Sta. 483+79.06
 Install 66' of 60" RCP Extention LT
 and 1 Apron
 Skew 7° RT AH
 FL LT = 1105.50
 FL OTHER = 1104.06
 FL RT = UAC

Right of Way Design Information	
THIS SHEET INCLUDED FOR INFORMATION ONLY	
ROW Team: FREDRICKSON / CUVA	
ROW #: NHSN-034-3(042)-2R-02	
Plan Date: 12/17/2024	
Color Legend:	
	Property Lines
	Temporary Easement
	Permanent Acquisition
	Permanent Acquisition County
	Permanent Acquisition City

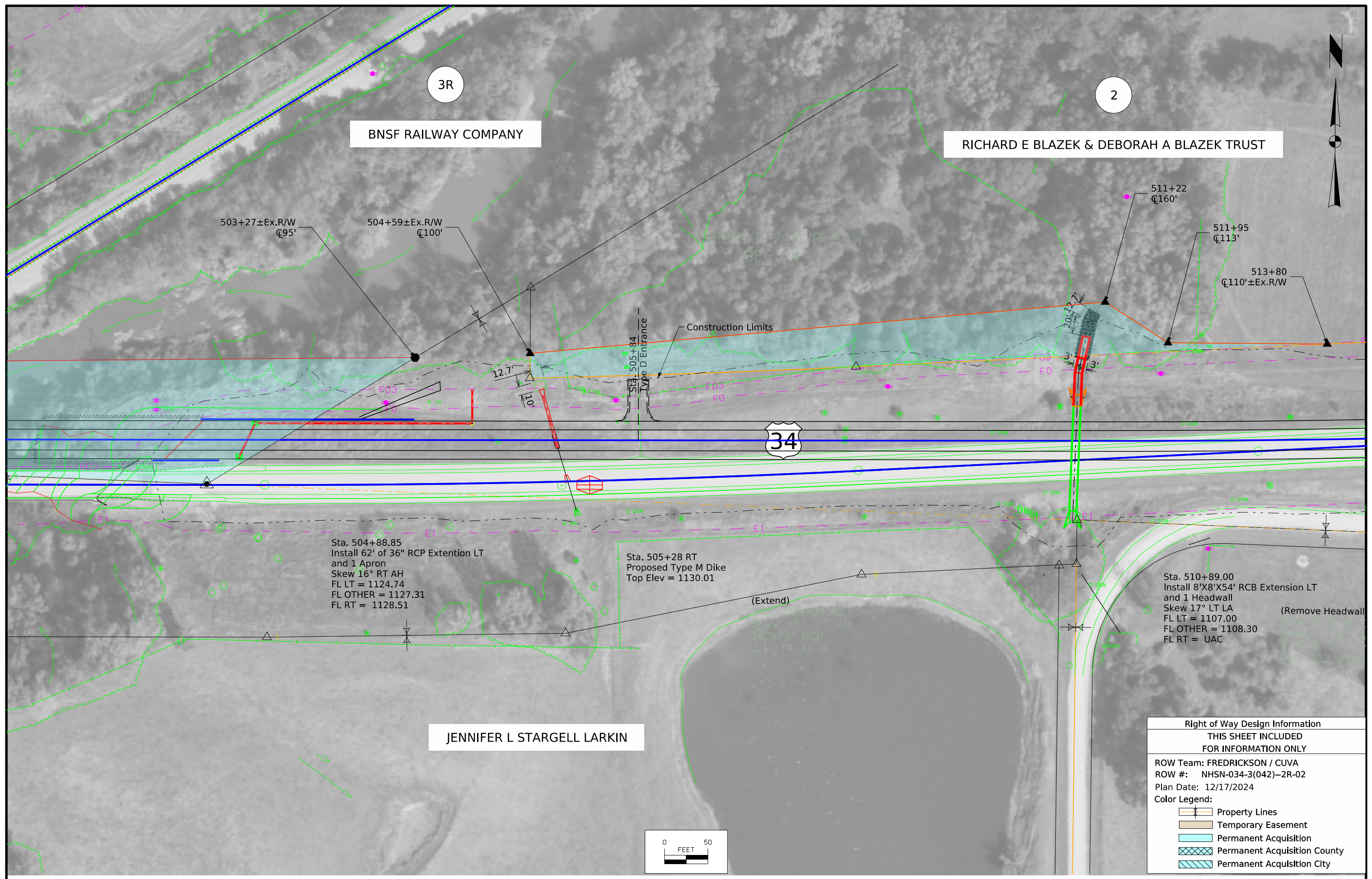
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3R

2

BNSF RAILWAY COMPANY

RICHARD E BLAZEK & DEBORAH A BLAZEK TRUST



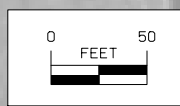
Sta. 504+88.85
 Install 62' of 36" RCP Extension LT
 and 1 Apron
 Skew 16° RT AH
 FL LT = 1124.74
 FL OTHER = 1127.31
 FL RT = 1128.51

Sta. 505+28 RT
 Proposed Type M Dike
 Top Elev = 1130.01

Sta. 510+89.00
 Install 8'X8'X54' RCB Extension LT
 and 1 Headwall
 Skew 17° LT LA
 FL LT = 1107.00
 FL OTHER = 1108.30
 FL RT = UAC

JENNIFER L STARGELL LARKIN

Right of Way Design Information	
THIS SHEET INCLUDED FOR INFORMATION ONLY	
ROW Team: FREDRICKSON / CUVA	
ROW #: NHSN-034-3(042)-2R-02	
Plan Date: 12/17/2024	
Color Legend:	
	Property Lines
	Temporary Easement
	Permanent Acquisition
	Permanent Acquisition County
	Permanent Acquisition City



TRAFFIC CONTROL PLAN

1. U.S. 34 will remain open to traffic at all times.
2. Lane and shoulder closures shall be per the TC series of Standard Road Plans referenced elsewhere in these plans.
3. Access to individual properties shall be maintained at all times.
4. At least one lane of Loomis Avenue shall be maintained at all times.

STAGING NOTES

- 1A. Maintain traffic on existing U.S. 34. Lane closures may be necessary for construction; limit lane closures to overnight hours only.
 - Construct pavement widening on existing U.S. 34, Sta. 469+24.35 - Sta. 482+15.89 and Sta. 503+40.93 - Sta. 527+00.00
- 1B. Maintain 2-lane traffic on existing U.S. 34 and pavement widening as constructed during Stage 1A.
 - Construct bridge, roadway embankment, and paving adjacent to existing U.S. 34
2. Close existing westbound lane using temporary signals per TC-216
 - Construct WB pavement tie-ins on the east and west ends of the project
3. Transition traffic to new pavement / bridge, using new WB lane pavement. Use lane closure with temporary signals per TC-216
 - Construct EB pavement tie-ins on the east and west ends of the project
4. Maintain 2-lane traffic on new pavement / bridge
 - Remove existing bridge
 - Remove existing roadway pavement
 - Grade through existing roadway embankment

**CROSS SECTION VIEW COLOR LEGEND
OF TRAFFIC CONTROL AND STAGING SHEETS**

SHADING	Design Color No.	
Green, Light	(225)	Existing Pavement Shading
Gray, Light	(48)	Previously Constructed Pavement Shading
Gray, Med	(80)	Previously Constructed Granular Surface Shading
Blue, Light	(230)	Proposed Pavement Shading
Lavender	(9)	Temporary Pavement Shading
Brown, Med	(237)	Future Proposed Pavement Shading

**CROSS SECTION VIEW PATTERN AND SYMBOL LEGEND
OF TRAFFIC CONTROL AND STAGING SHEETS**

	Pavement Removal		Proposed Granular Shoulder
	Proposed Granular Subbase		Temporary Shoulder
	Proposed Special Backfill		Existing Shoulder Strengthening
	Temporary Barrier Rail		Permanent Barrier Rail
			Channelizing Device

PLAN VIEW COLOR LEGEND OF TRAFFIC CONTROL AND STAGING SHEETS

LINEWORK	Design Color No.	
Green	(2)	Existing Topographic Features and Labels
Magenta	(5)	Pavement Marking Call Outs
Blue	(1)	Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation
Yellow	(4)	Pavement Markings, Yellow
Off White	(254)	Pavement Markings, White
Violet	(15)	Temporary barrier rail, Unpinned
Flush Orange	(228)	Temporary barrier rail, Pinned

SHADING	Design Color No.	
Green, Light	(225)	Existing Pavement Shading
Gray, Light	(48)	Previously Constructed Pavement Shading
Gray, Med	(80)	Proposed Granular Surface Shading
Gray, Med	(80)	Previously Constructed Granular Surface Shading
Blue, Light	(230)	Proposed Pavement Shading
Lavender	(9)	Temporary Pavement Shading
Brown, Light	(236)	Proposed Grading Limits Shading
Pink, Dark	(13)	Proposed MSE or CIP Wall Shading
Red	(3)	Proposed Bridge Shading and Sign Trusses
Black w/Gray, Light Fill	(0,48)	Previously Constructed Structure

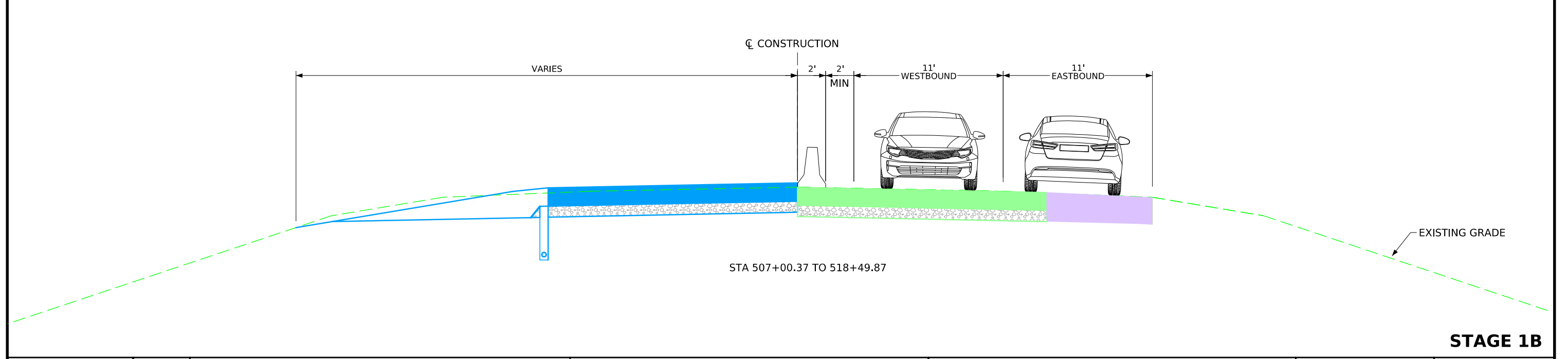
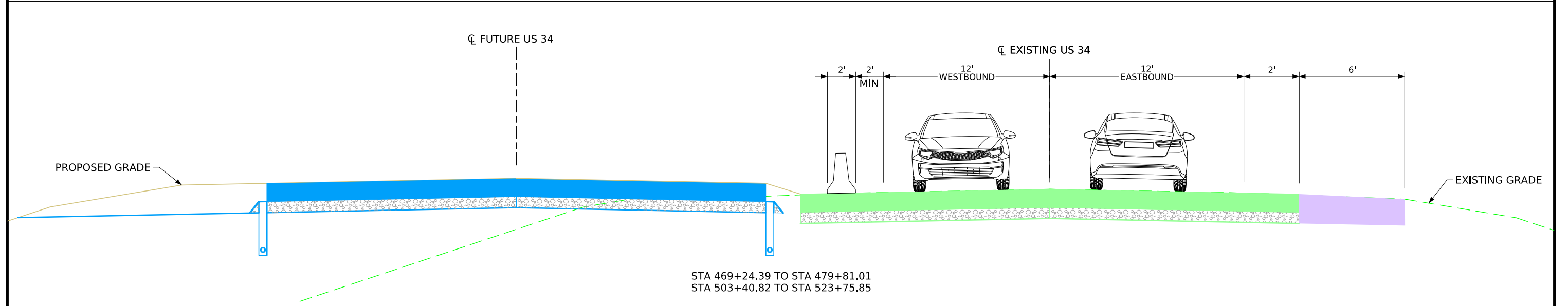
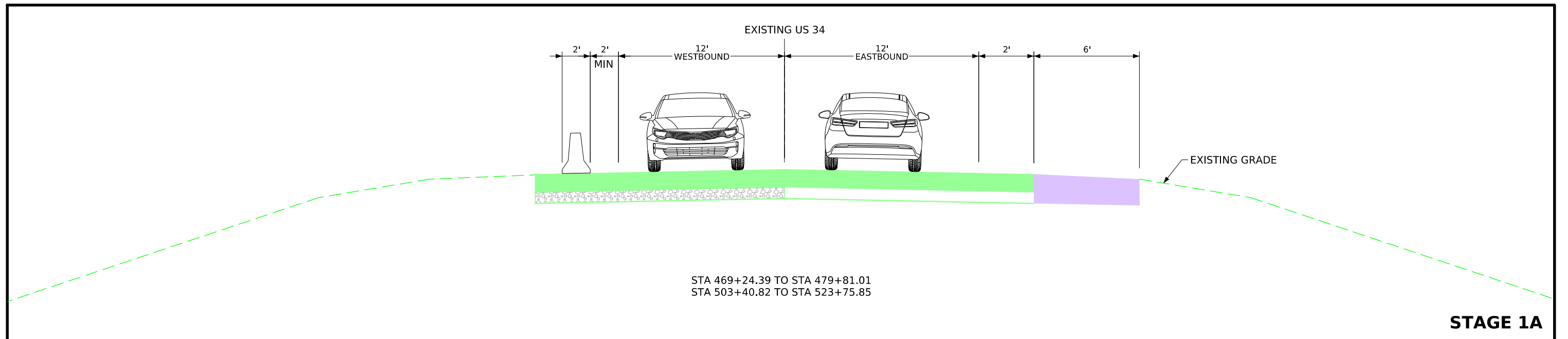
**PLAN VIEW PATTERN AND SYMBOL LEGEND
OF TRAFFIC CONTROL AND STAGING SHEETS**

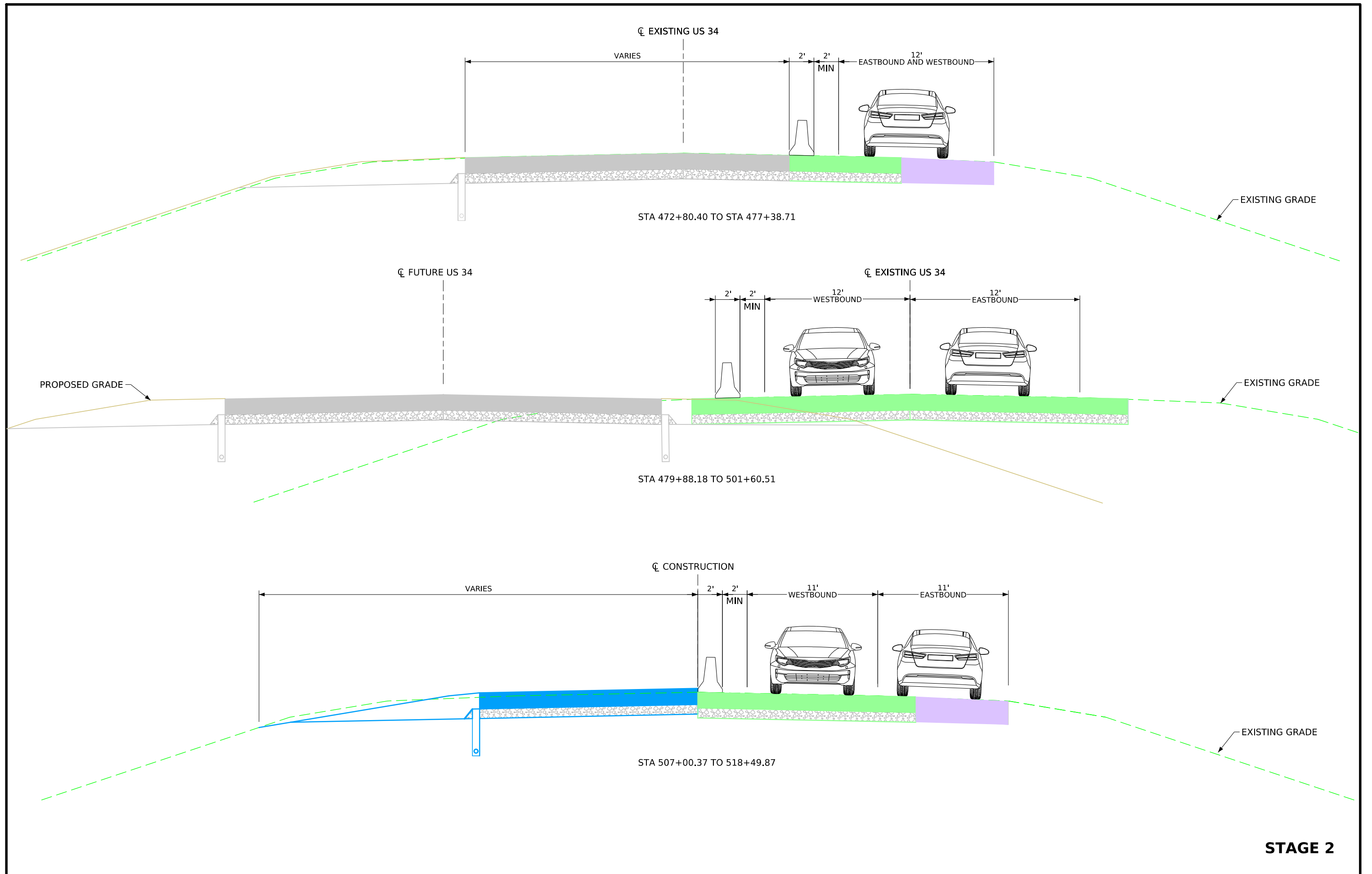
	Channelizing Device		Crash Cushion (Temp or Perm)
	Drum		Traffic Signal
	Temporary Lane Separator		Flagger
	Tubular Marker		Temporary Floodlighting
	Channelizer Marker		Traffic Sign
	Concrete Barrier Marker		Type III Barricade
	Delineator		Type A Warning Light
	Temporary Barrier Rail		Direction of Traffic
	Pavement Removal		Safety Closure
	Sand Barrel Layout		Lane Identification

NOTE: Device spacing according to Standard Road Plans unless specifically dimensioned.

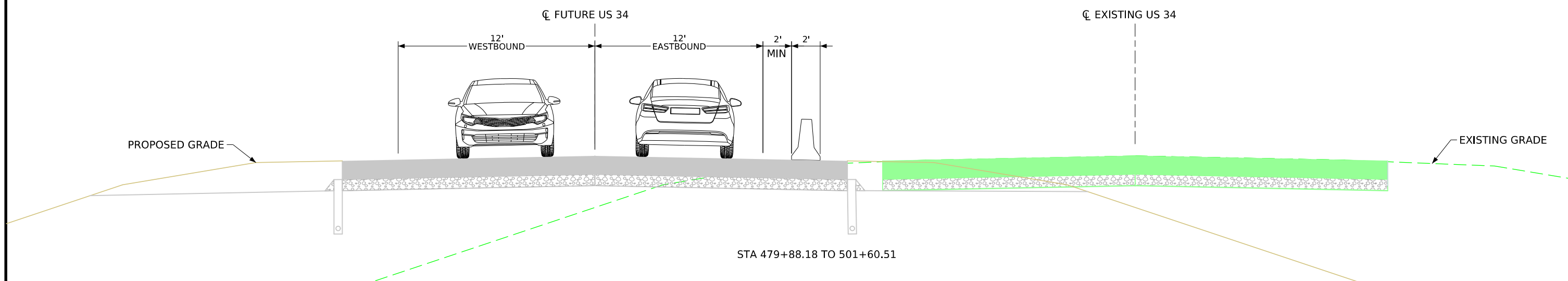
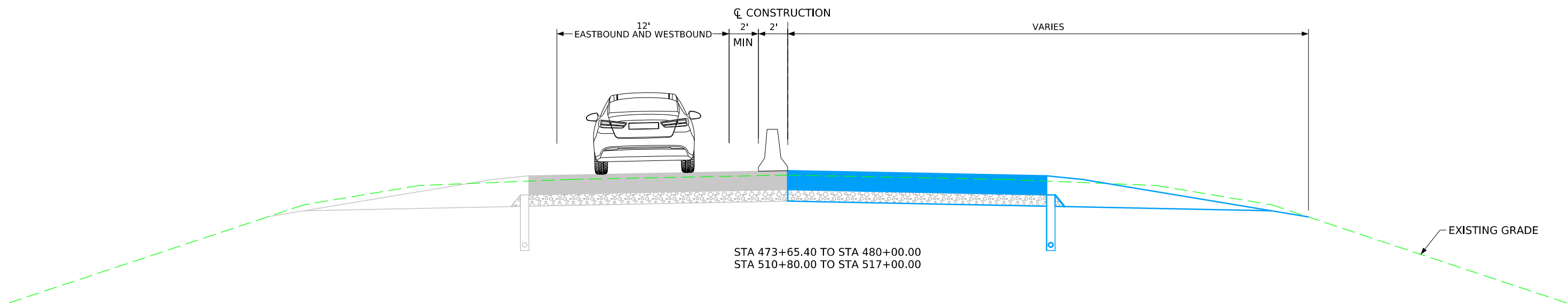
**TRAFFIC CONTROL
AND
STAGING
LEGEND AND SYMBOL
INFORMATION SHEET**

(COVERS SHEET SERIES J)

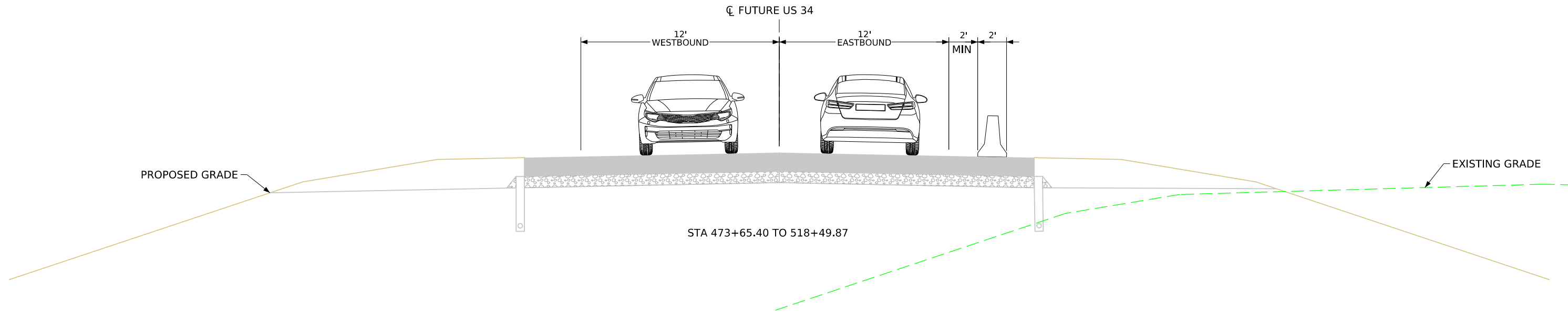




STAGE 2

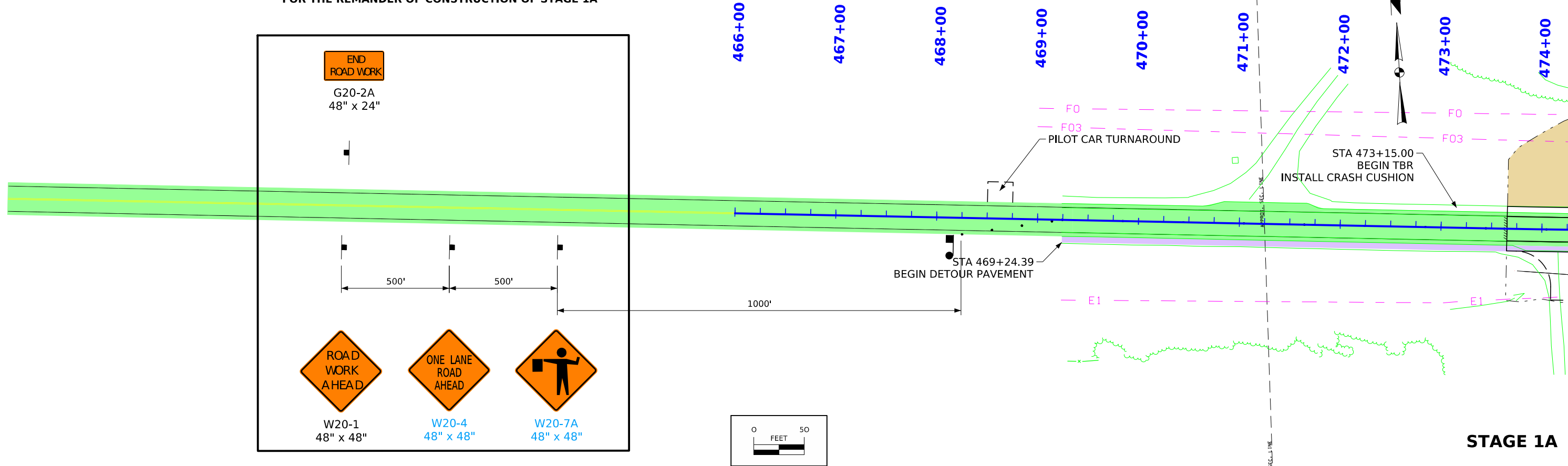


STAGE 3

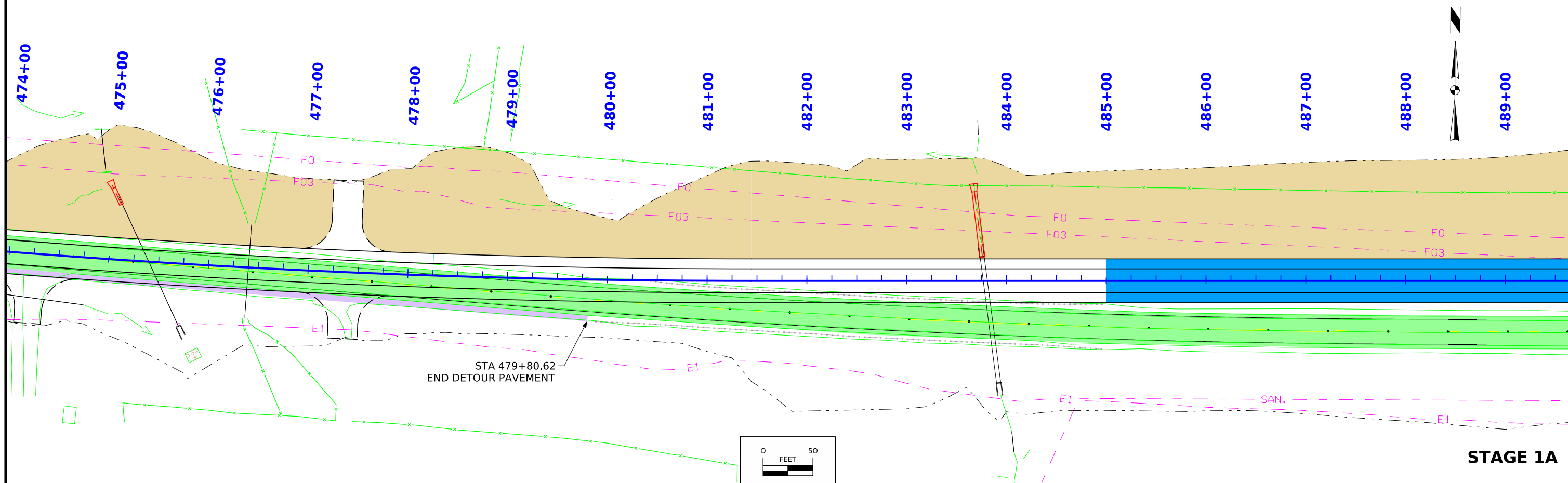


STAGE 4

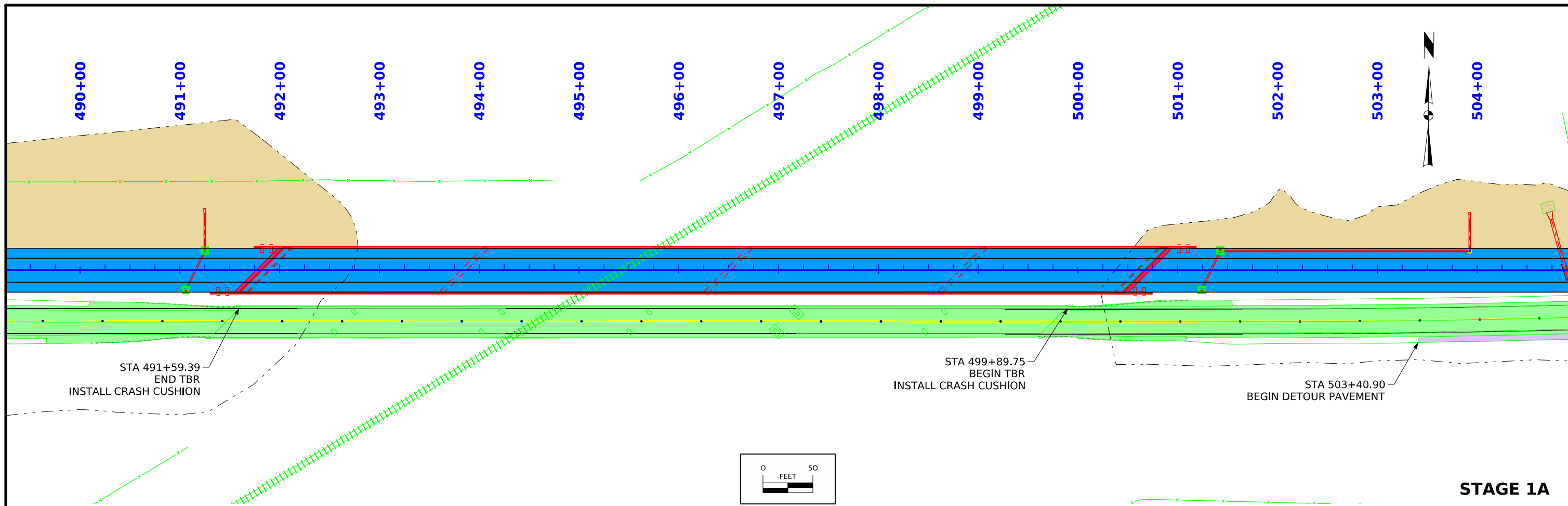
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 WHEN TEMPORARY PAVEMENT IS INSTALLED OR CONSTRUCTION OFF HOURS. REFER TO IOWA DOT STANDARD ROAD PLANS TC-202
 FOR THE REMAINDER OF CONSTRUCTION OF STAGE 1A



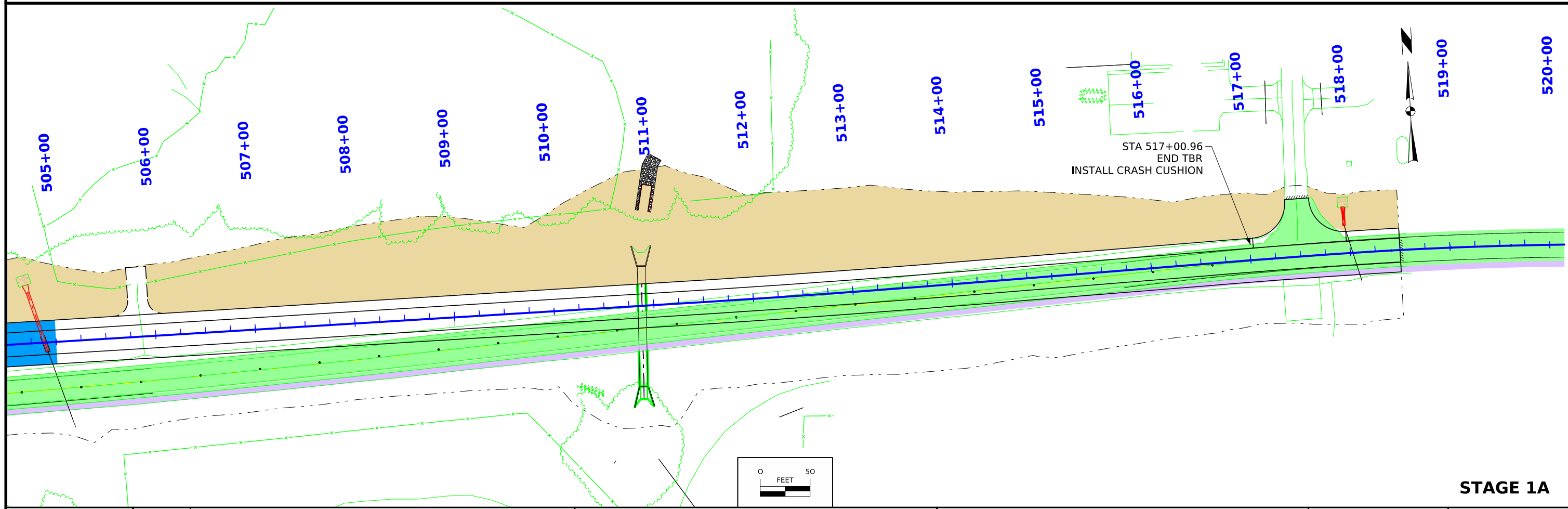
STAGE 1A



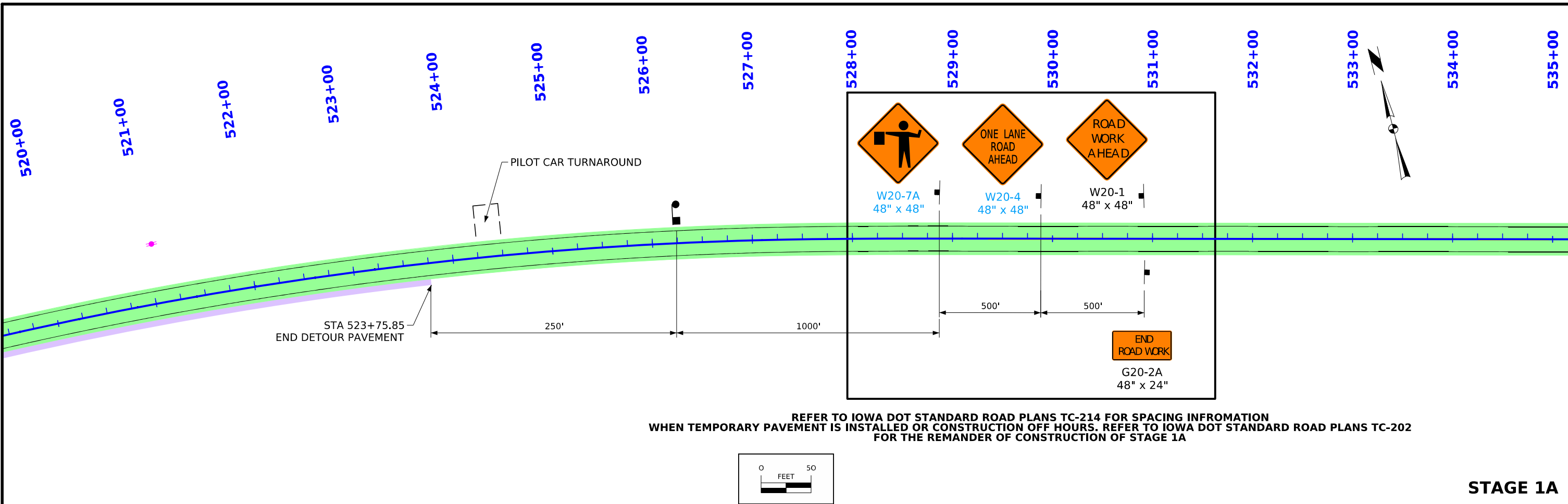
STAGE 1A



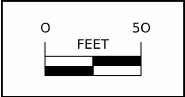
STAGE 1A



STAGE 1A

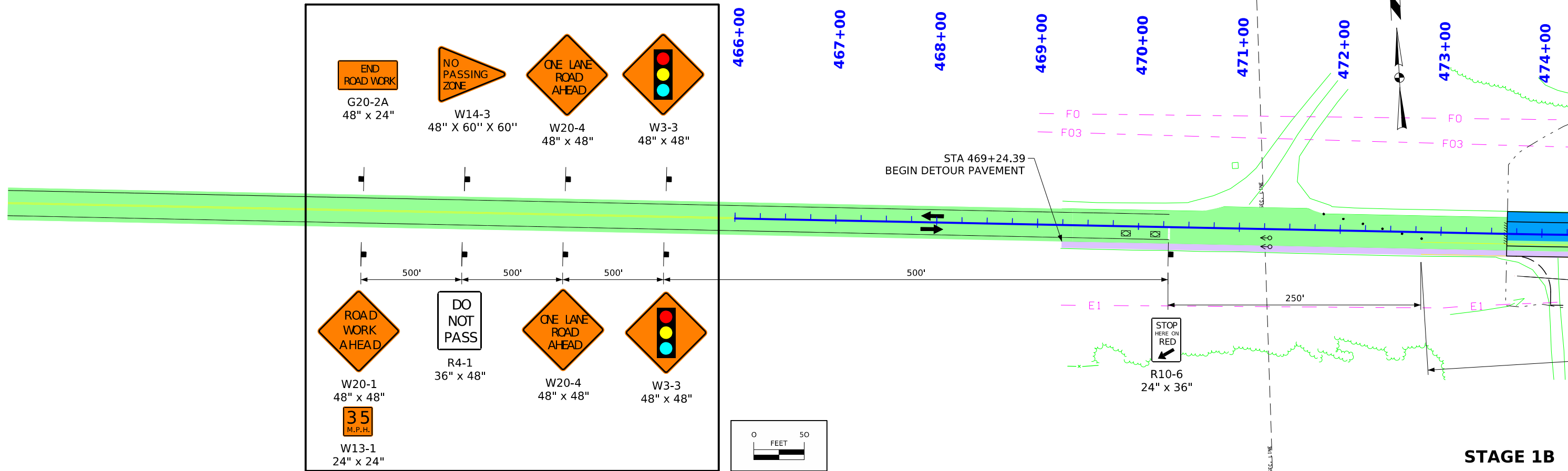


REFER TO IOWA DOT STANDARD ROAD PLANS TC-214 FOR SPACING INFORMATION
 WHEN TEMPORARY PAVEMENT IS INSTALLED OR CONSTRUCTION OFF HOURS. REFER TO IOWA DOT STANDARD ROAD PLANS TC-202
 FOR THE REMAINDER OF CONSTRUCTION OF STAGE 1A

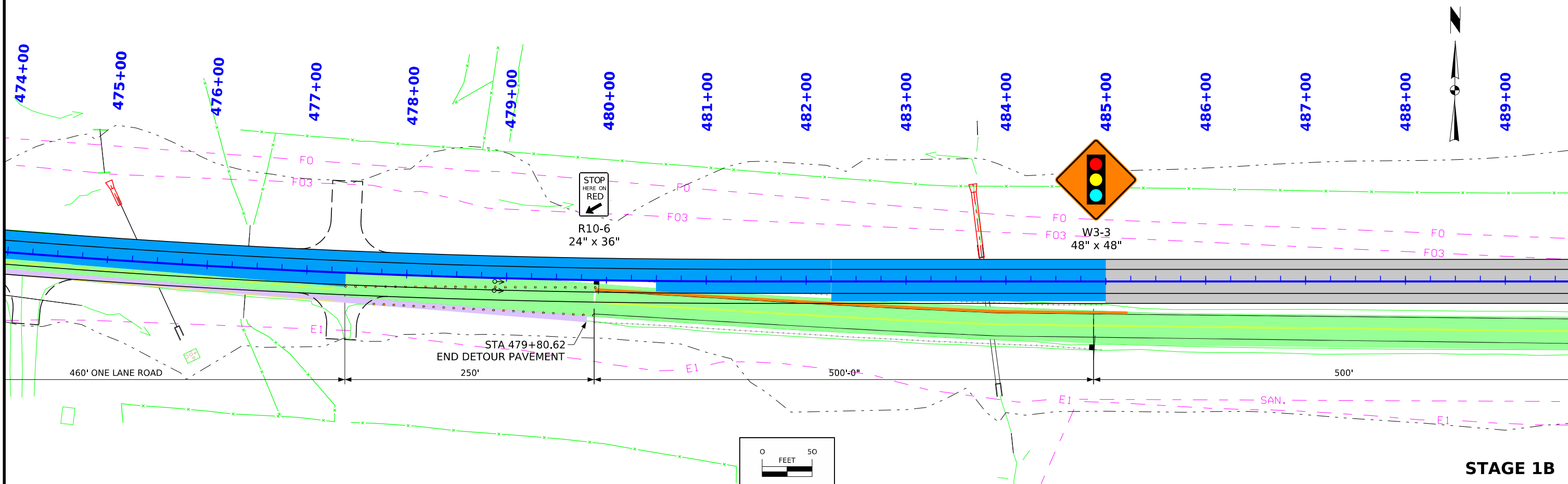


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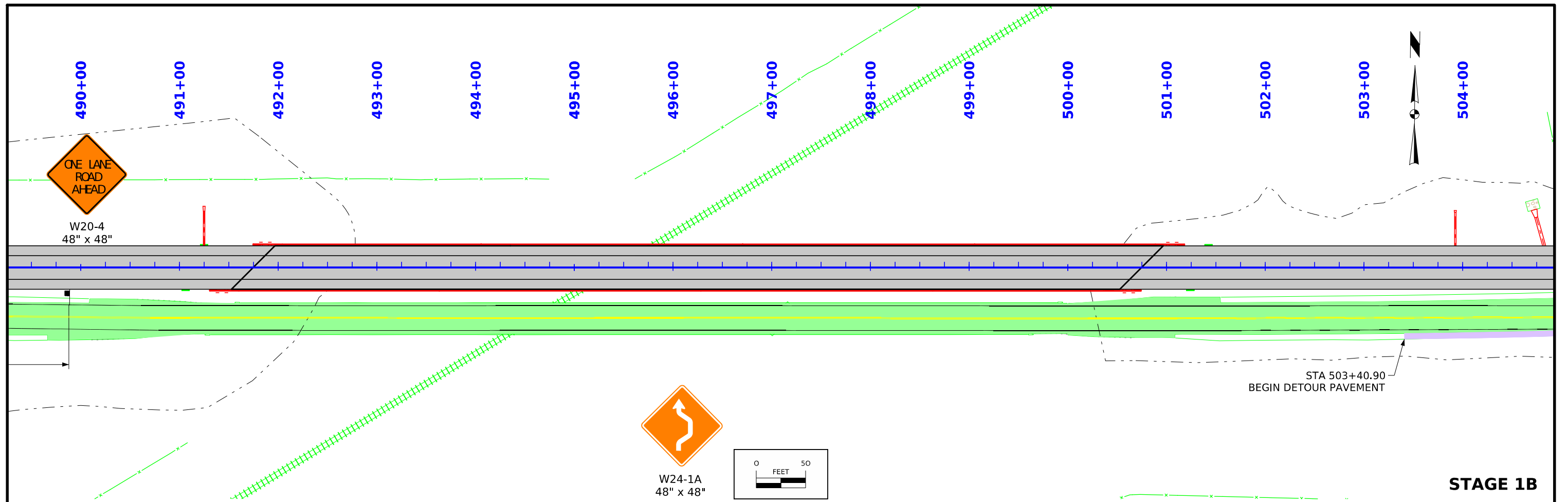
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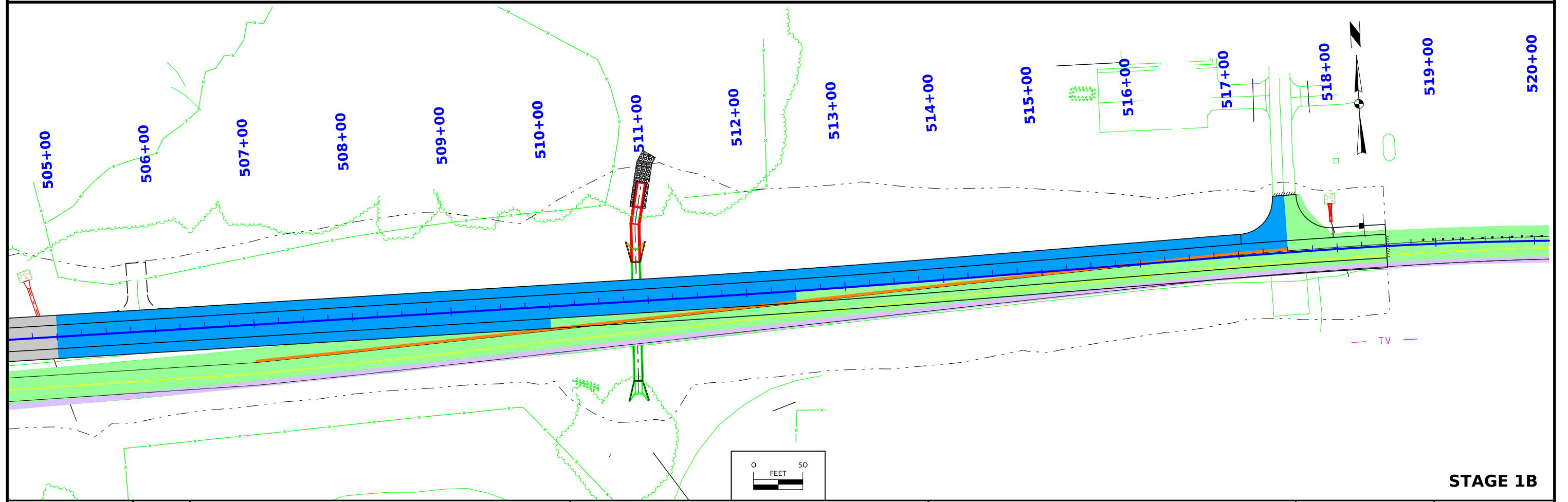
STAGE 1B



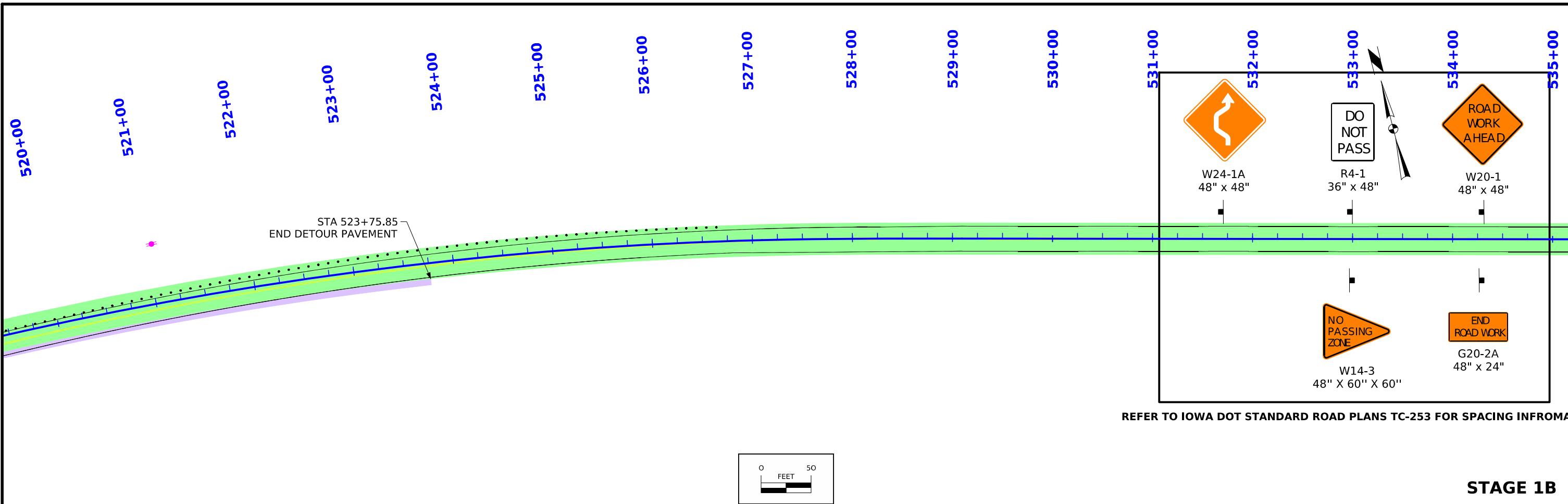
STAGE 1B



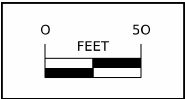
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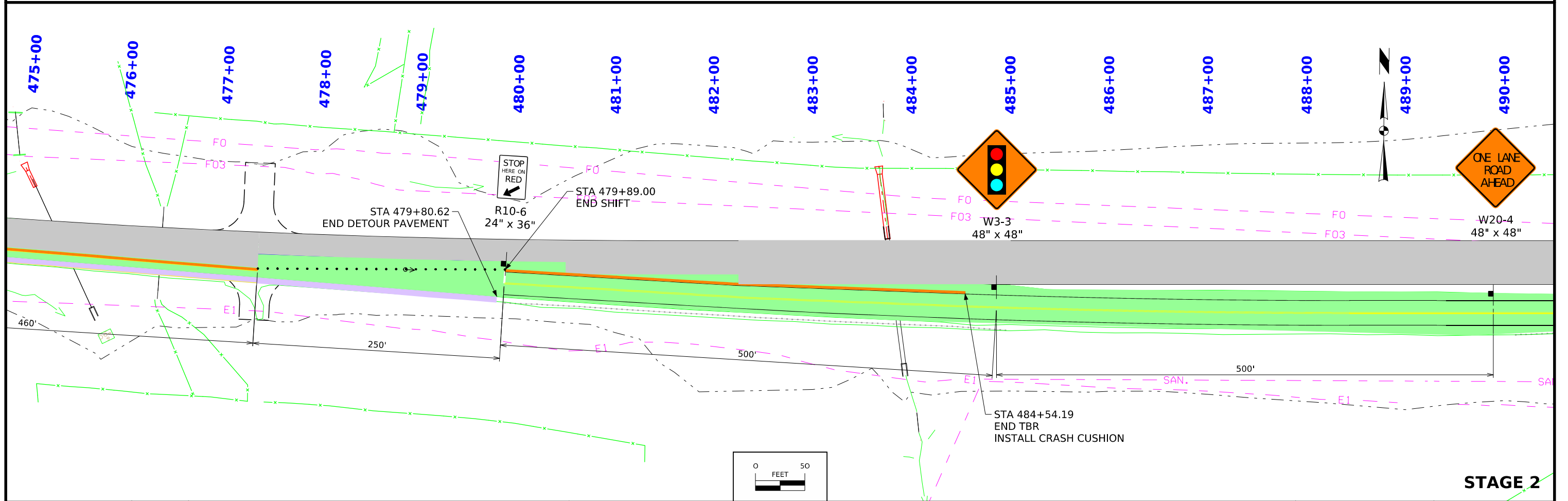
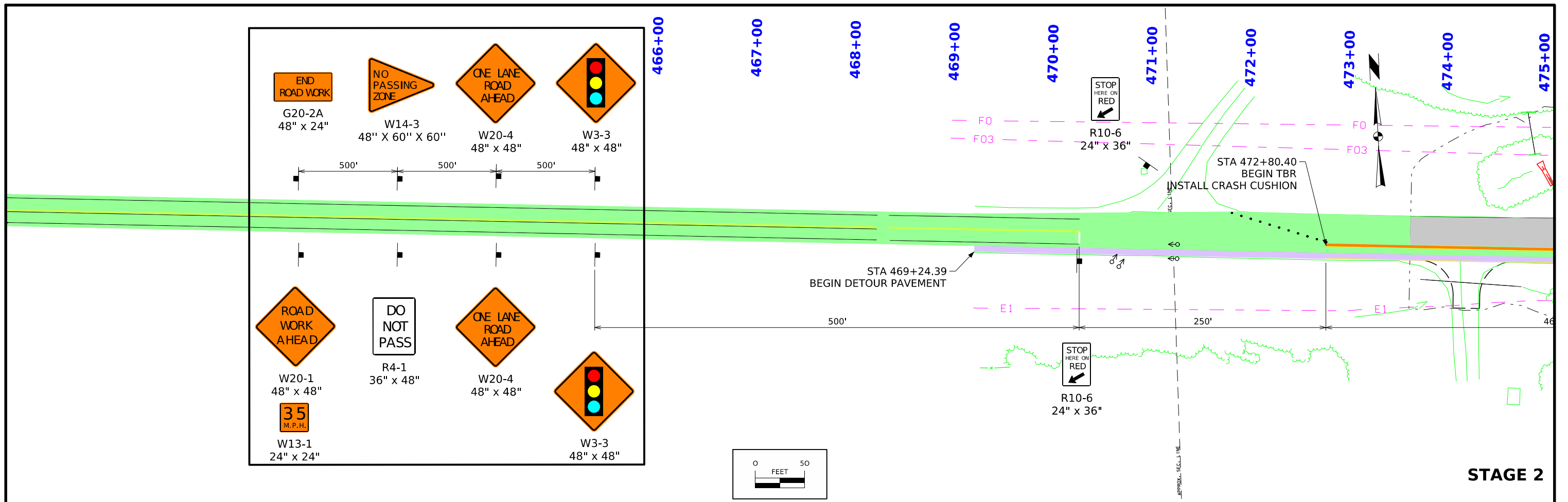
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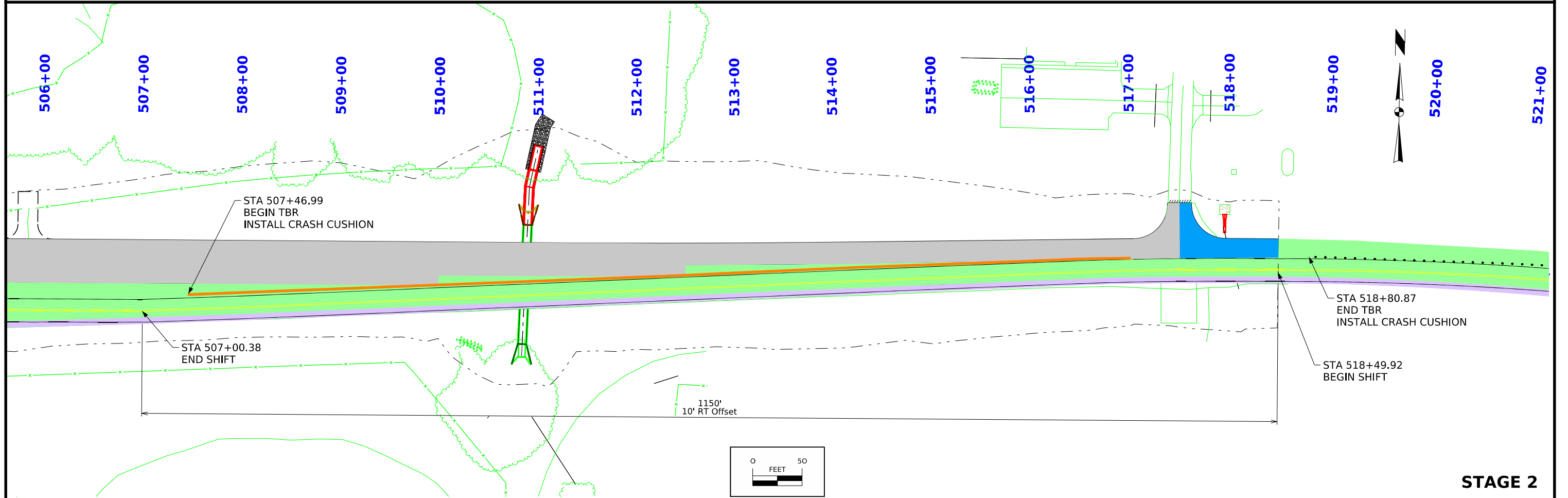
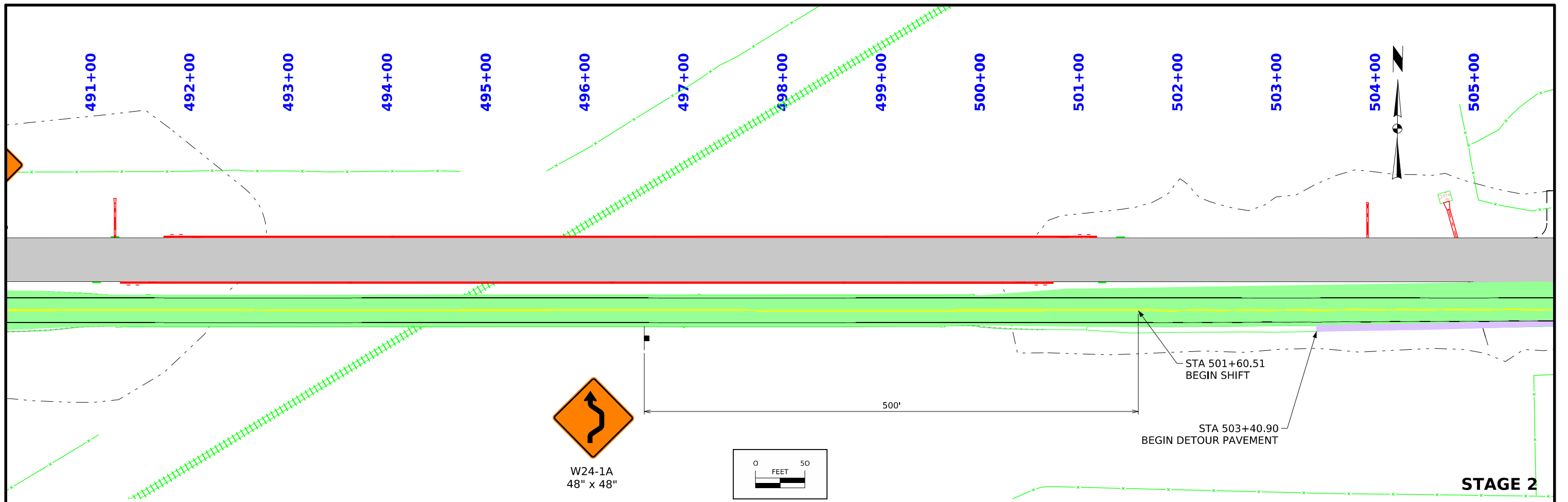


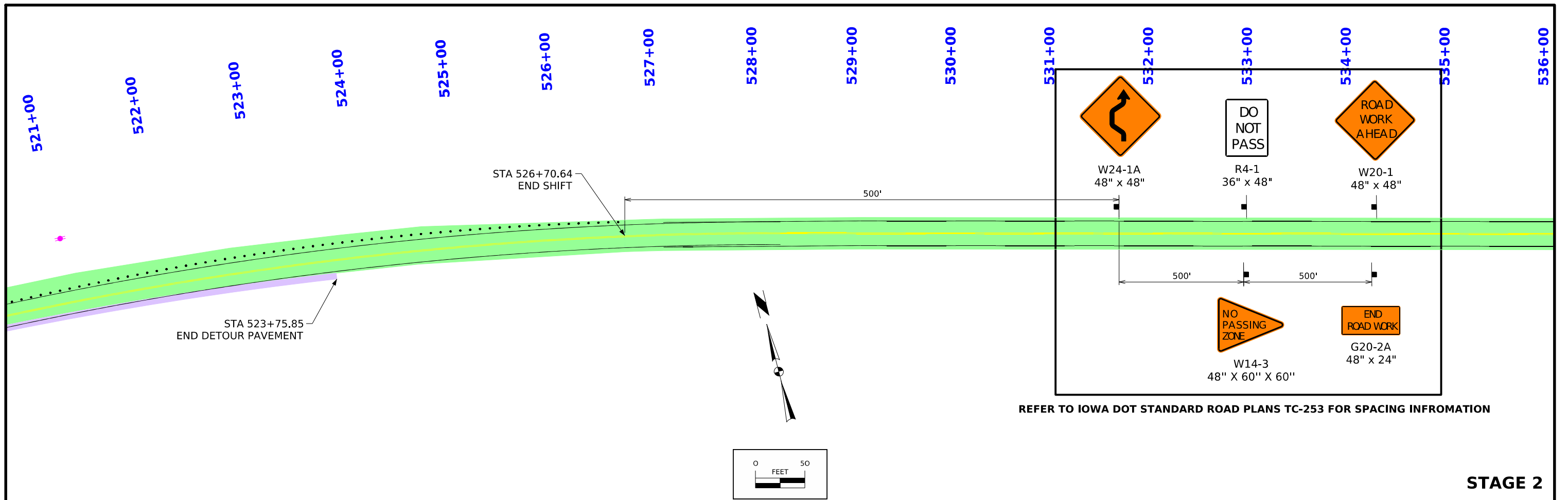
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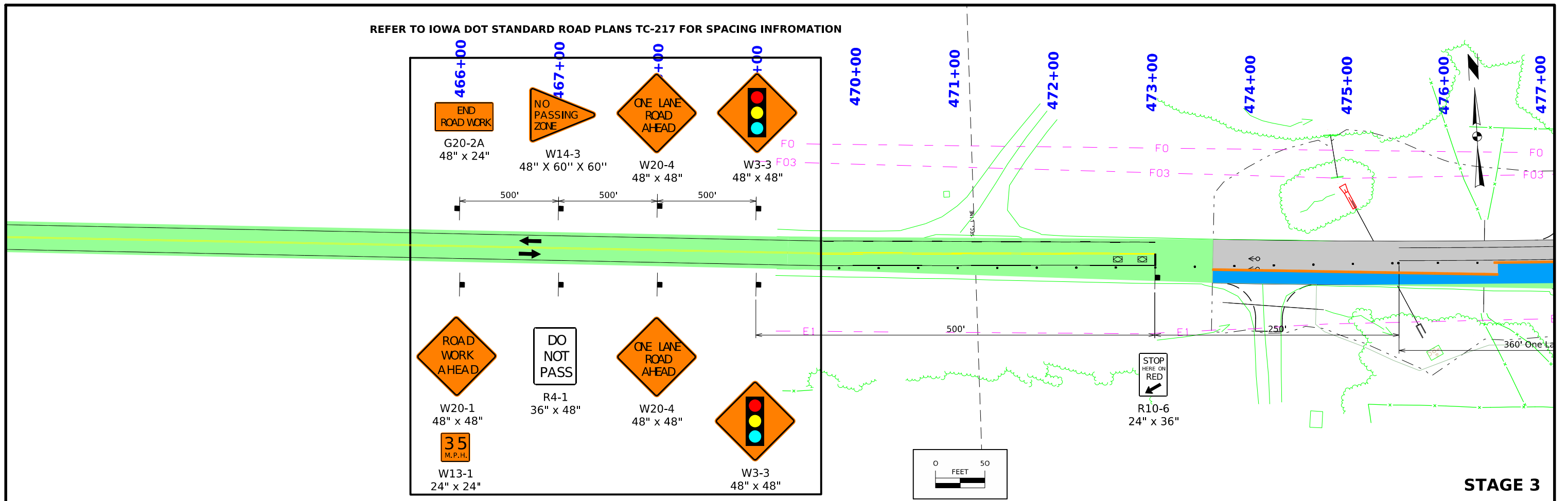
STAGE 1B



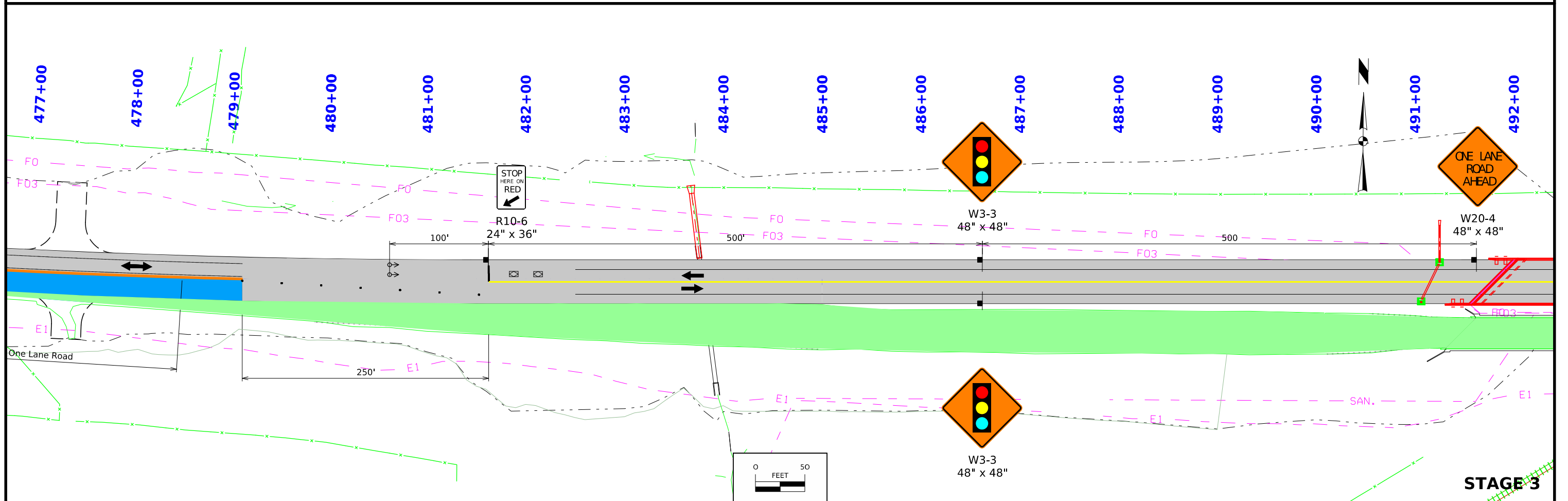




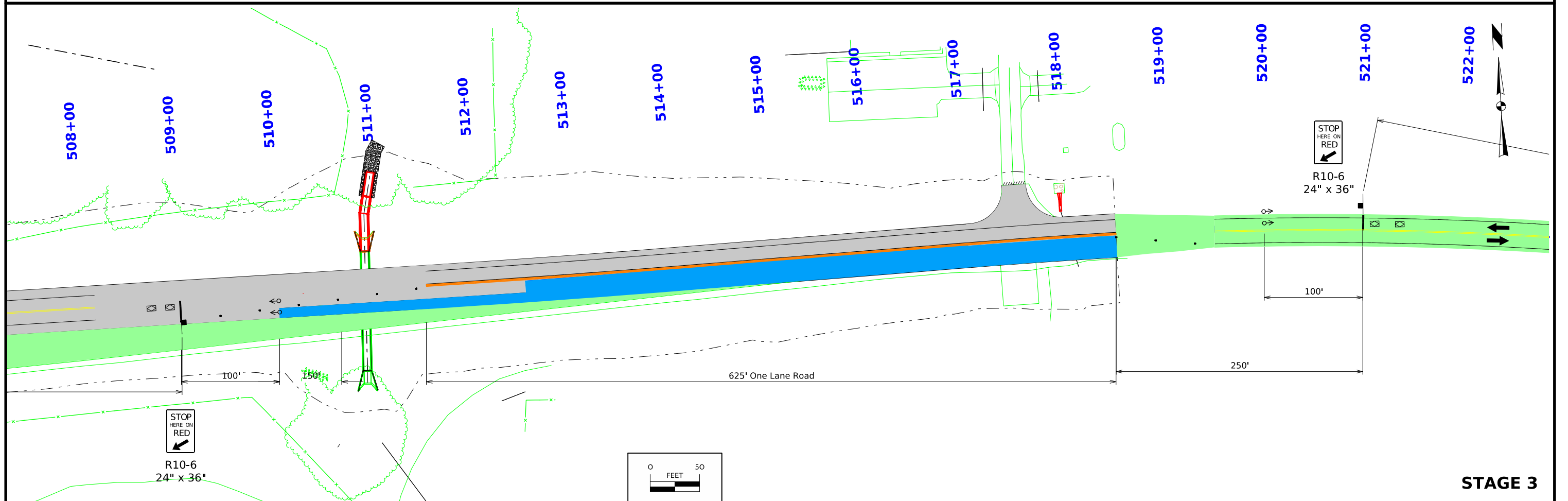
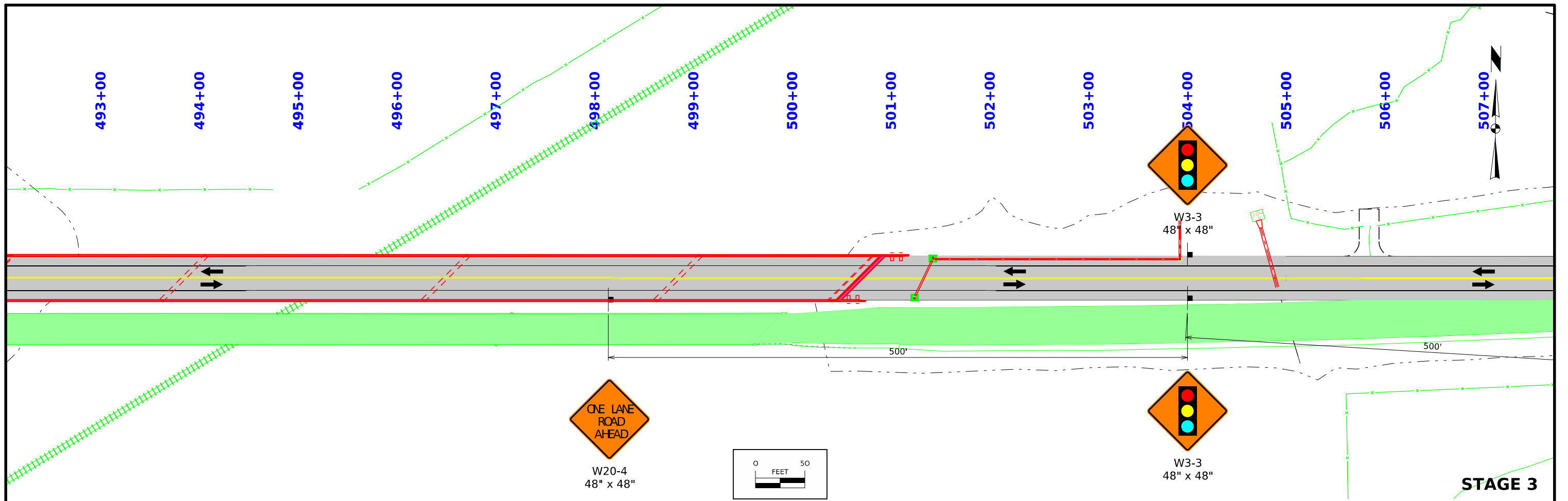
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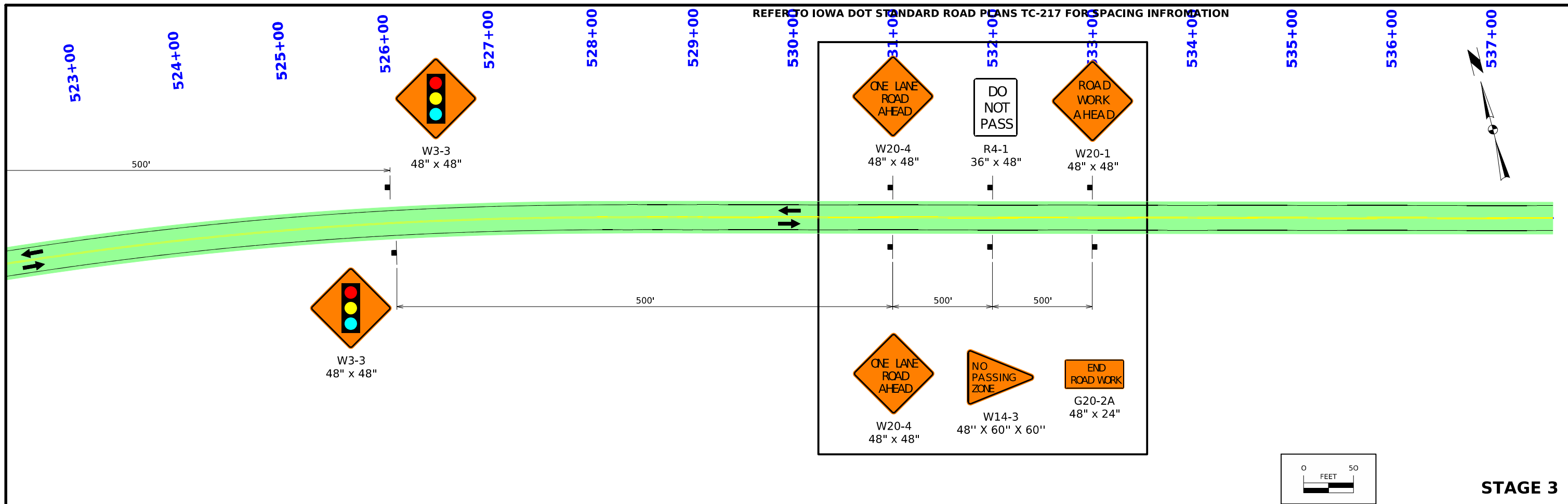


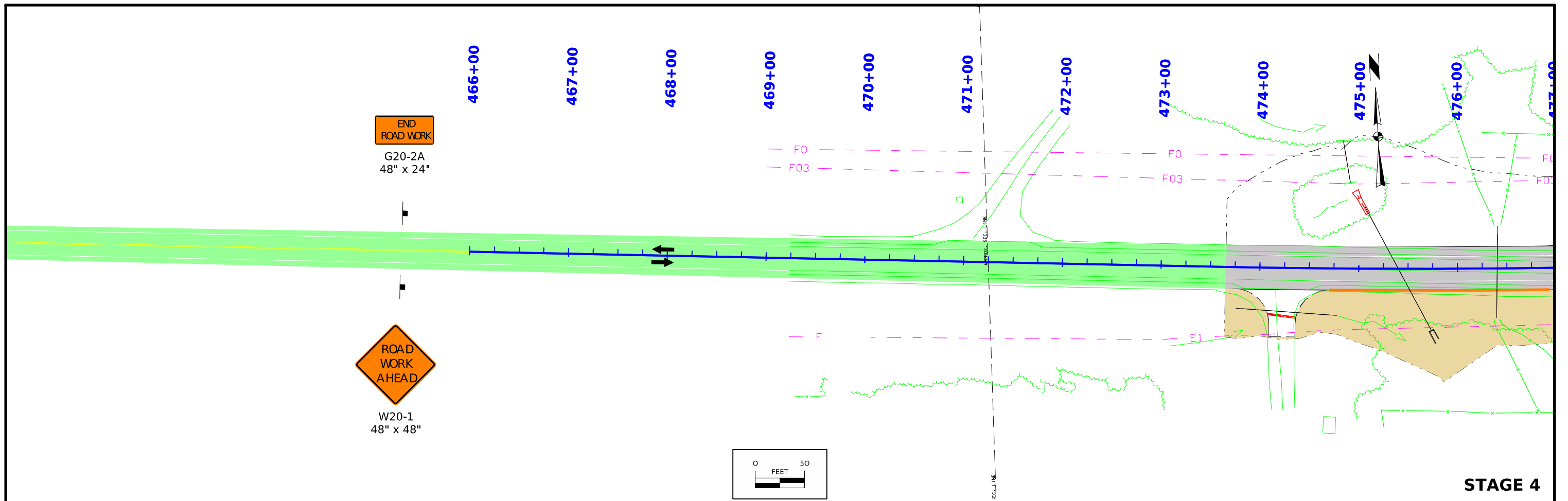
STAGE 3



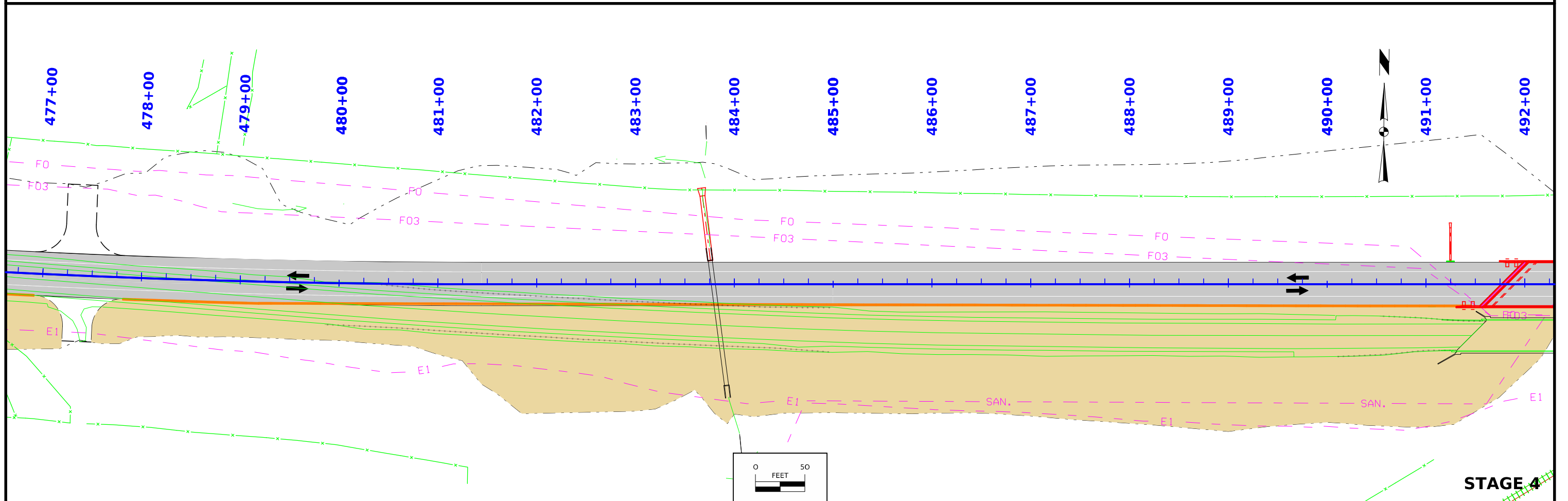
STAGE 3



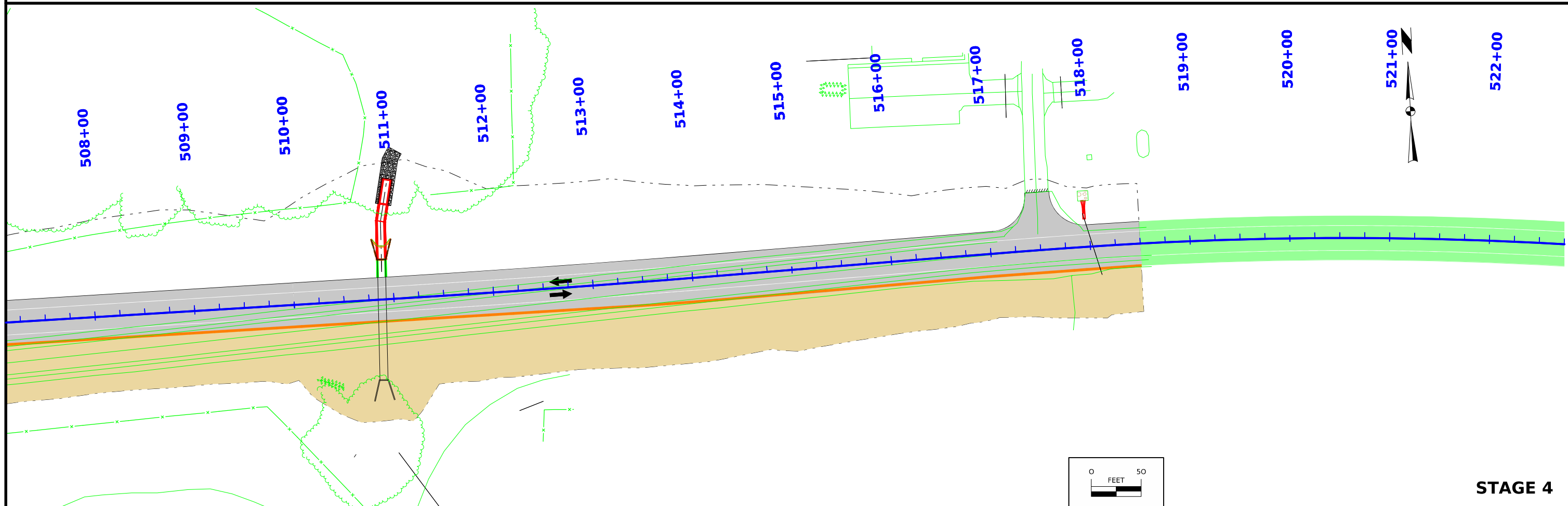
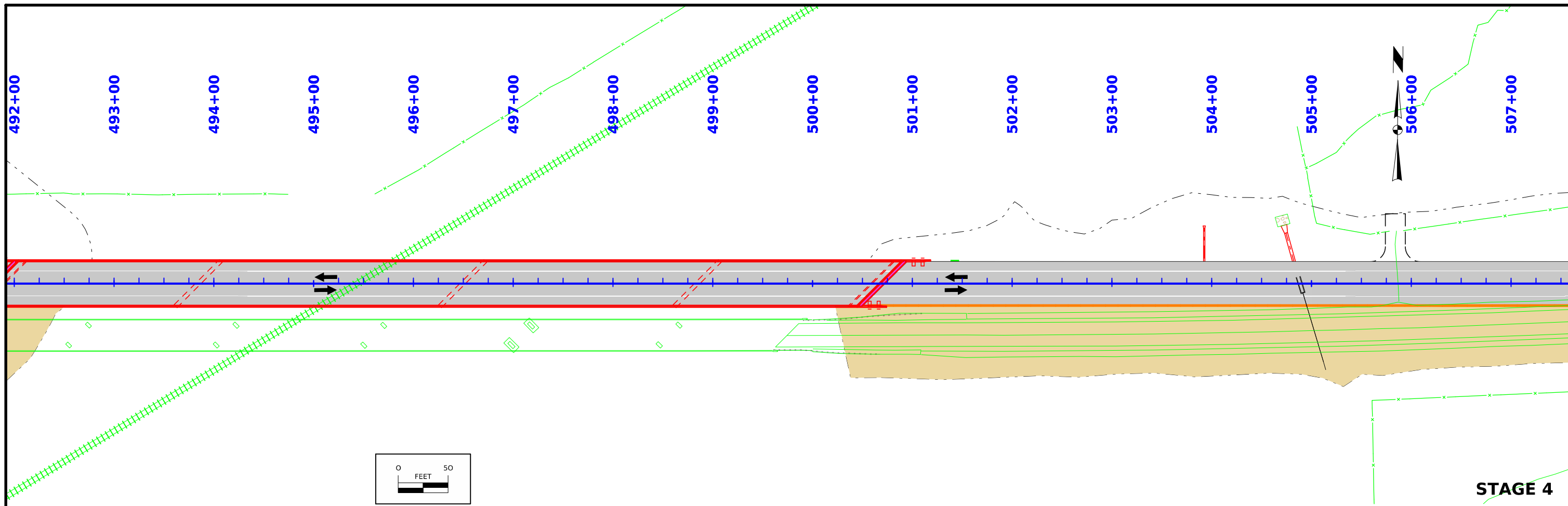


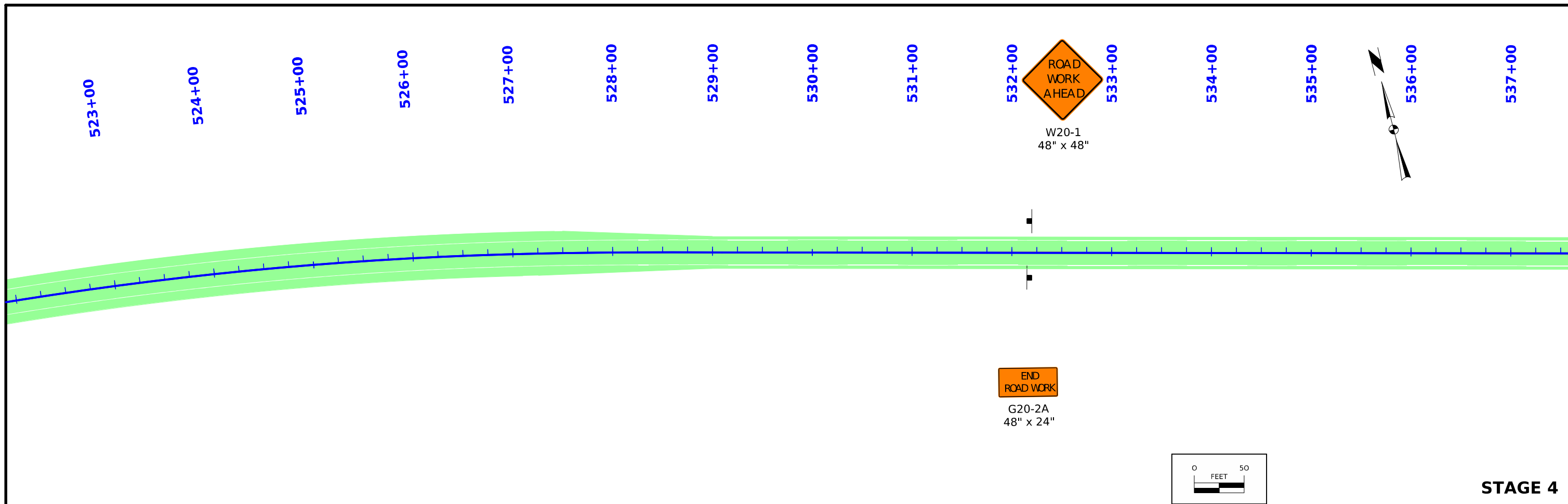


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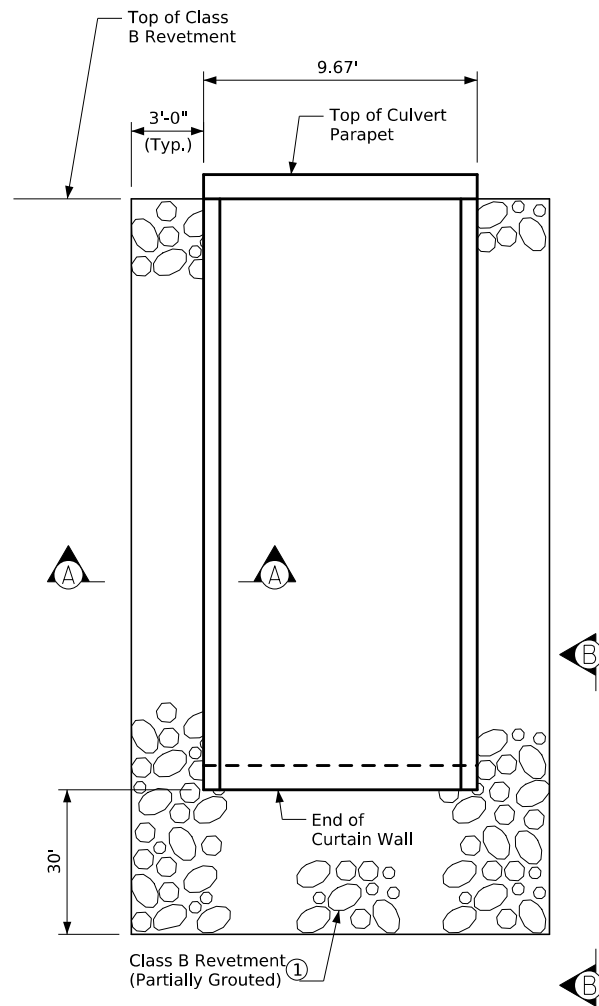


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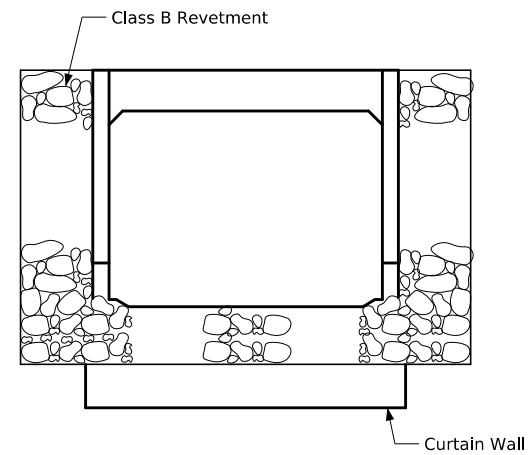




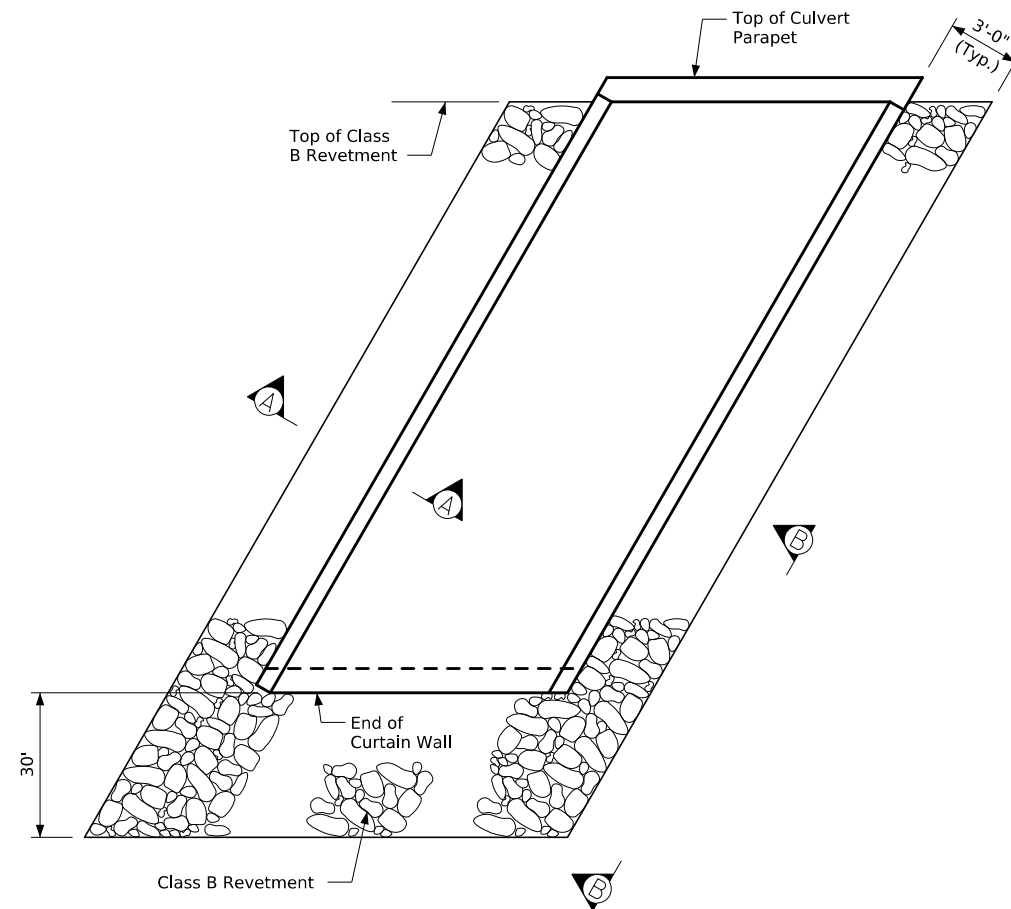
STAGE 4



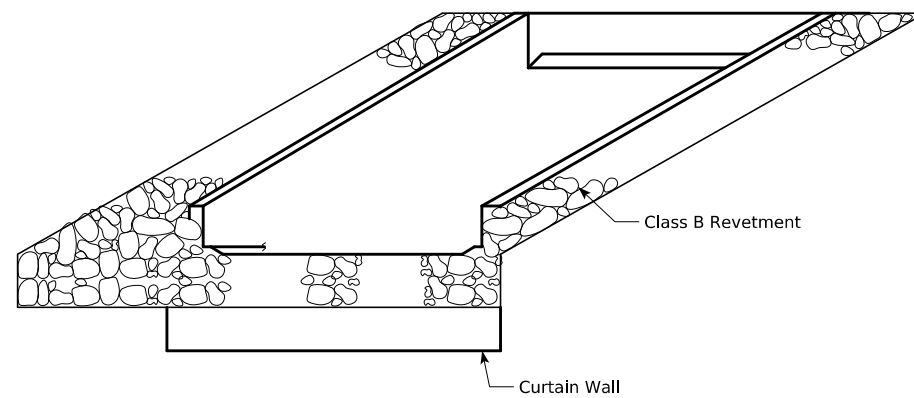
Plan View



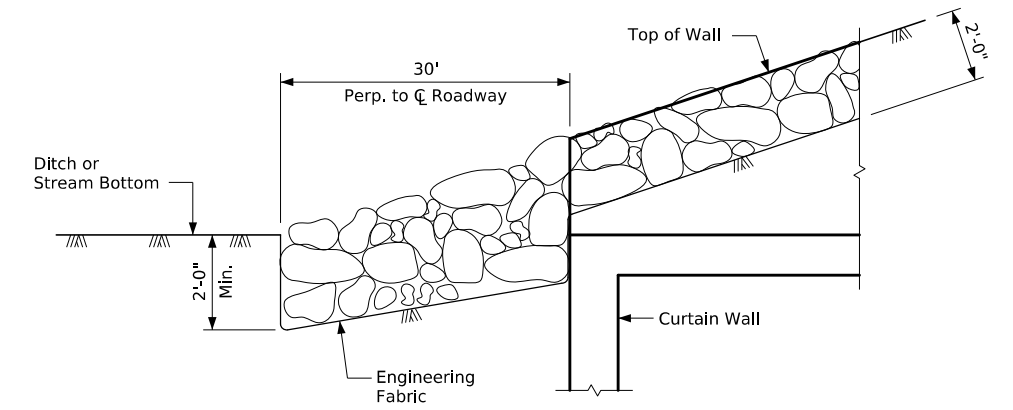
Elevation View
0° Skew Headwalls



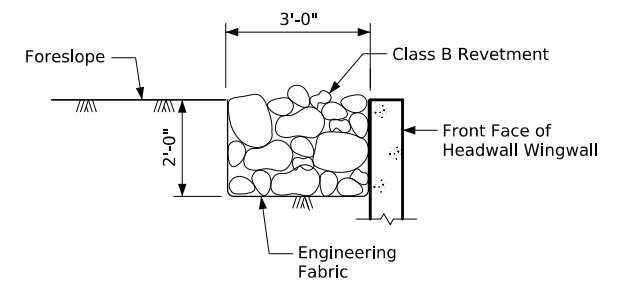
Plan View



Elevation View
___° Skewed Headwall



View B-B



Section A-A

Typical Details

Construction Notes:

Class B Revetment shall be used and placed according to Article 2507.03, of the Standard Specifications. The engineering fabric shall meet the material requirements in accordance with Article 4196.01,B,3, of the Standard Specifications.

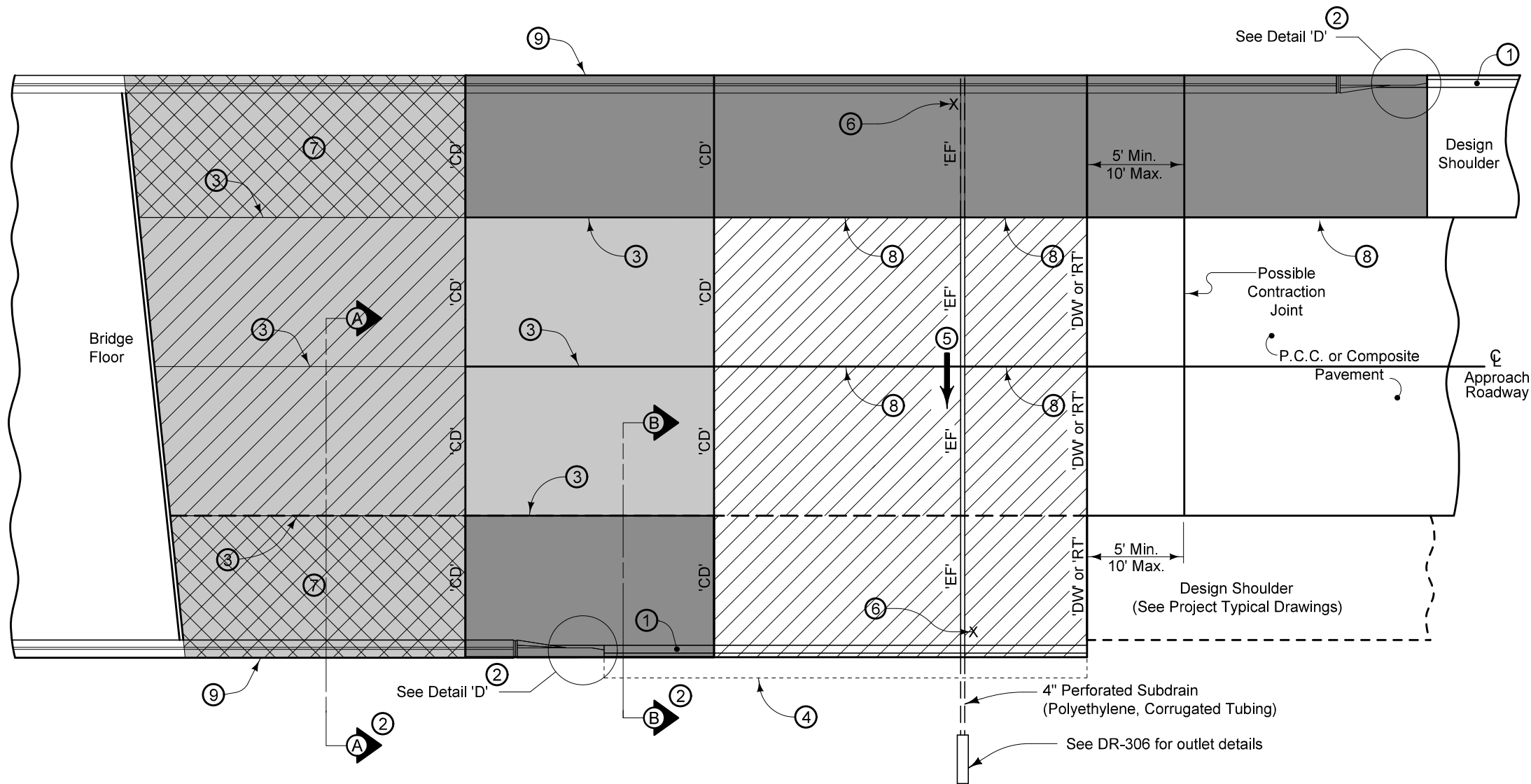
- ① Partially grout revetment using flowable mortar meeting the requirements of section 2506 of the Standard specification.

Revetment Protection Details

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. ___ OF ___ FILE NO. _____ DESIGN NO. _____

For joint details, see PV-101.

- ① Build 4 inch Sloped Curb to end of Double Reinforced Section. Refer to PV-102 for curb and runout details.
- ② See BR-203.
- ③ Longitudinal Joint (PV-101):
Single Pour - Saw cut joint per Detail B.
Two Pours - Use 'KS-1' joint (Single Reinforced Section).
Use 'KS-2' joint (Double Reinforced Section).
- ④ Polymer Grid and excavation limits of Modified Subbase 2 feet outside of pavement edge. See BR-201, BR-202, BR-203, or BR-204.
- ⑤ Slope subdrain to drain.
- ⑥ Place an "X" in the plastic concrete near the 'EF' joint at the outside edge of pavement.
- ⑦ Refer to Special Detail for double reinforced section with attached barrier rail.
- ⑧ L-2 or KT-2 joint.
- ⑨ Refer to BA-104 and sheet U.3 for barrier rail information. Refer to BA-107 for end section.

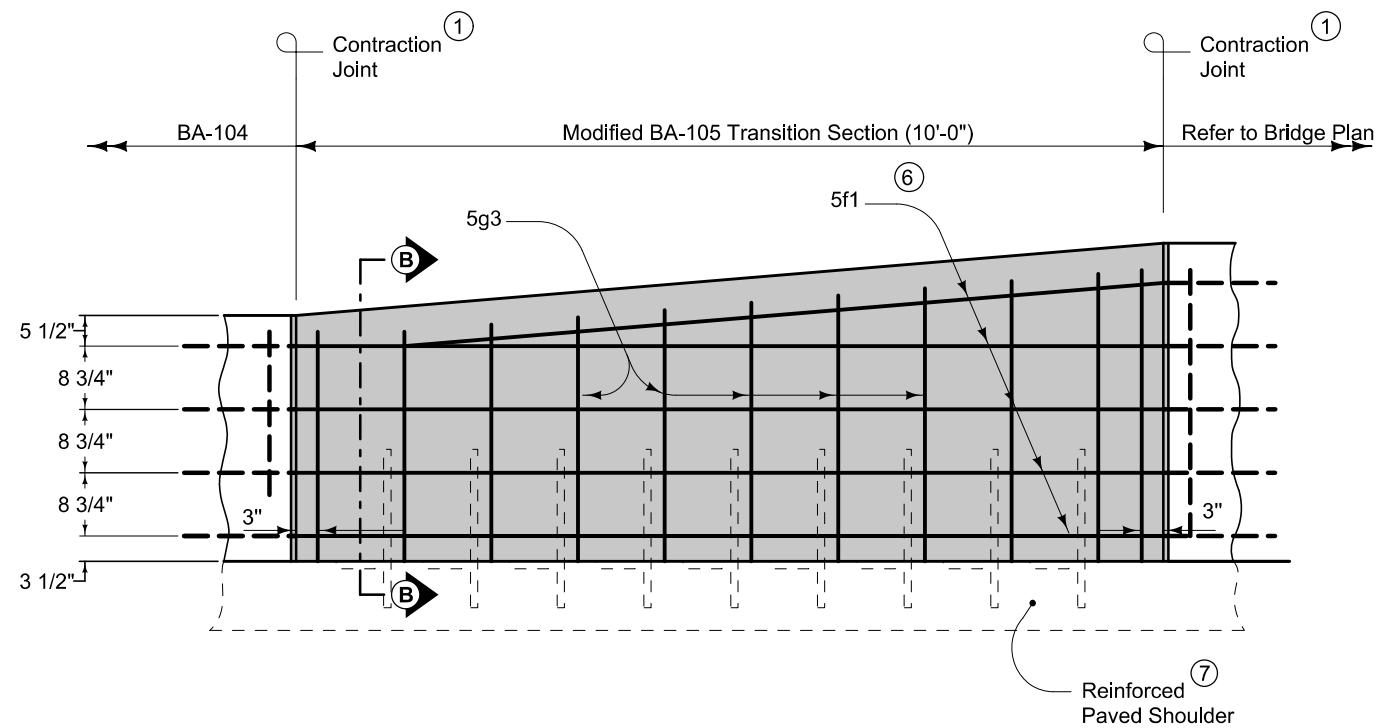


PLAN VIEW

Pay limits for contract item include the following areas:

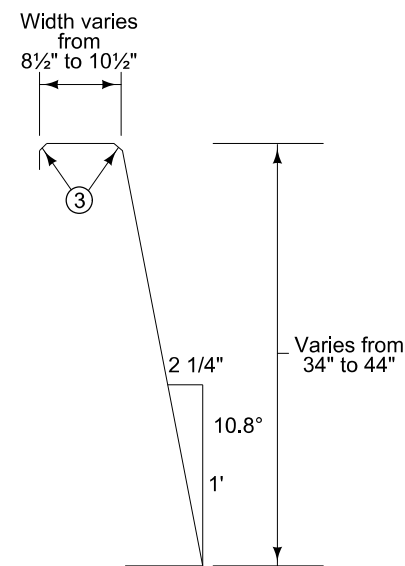
	Double Reinforced Section with Attached Barrier Rail
	Double Reinforced Section
	Single Reinforced Section
	Non-Reinforced Section
	Refer to BA-106 for Shoulder Information

**BRIDGE APPROACH
WHERE BARRIER RAIL
IS MOUNTED TO SHOULDER**



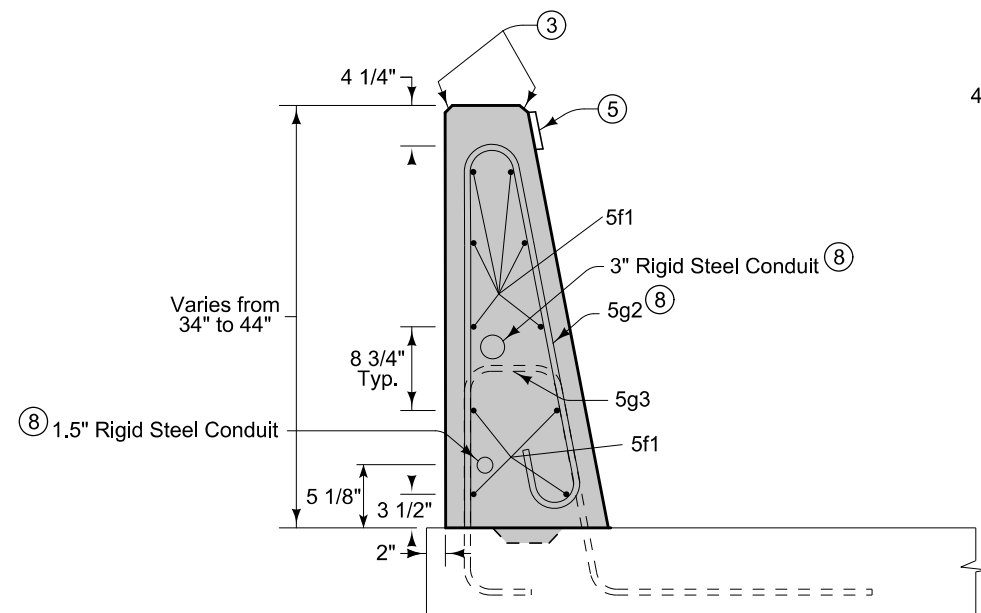
Use Grade 60 epoxy-coated reinforcing bars. Provide 2 inches minimum cover. Anchor all reinforcement to prevent movement. Secure each section at the front, back, and at 3'-6" intervals using a method approved by the Engineer.

- ① Where abutting sections are placed as separate pours, a butt joint may be used. Extend longitudinal reinforcement into the abutting section a minimum of 3 feet.
- ③ Fillet all exposed corners with a 3/4 inch dressed and beveled strip.
- ⑤ Place barrier markers at 100 foot increments in areas with non-continuous lighting, or 250 foot increments in areas with continuous lighting. Marker color to be the same as adjacent edge line.
- ⑥ When connecting to BA-107, include 6 additional #5 bars embedded a minimum of 3 feet into the BA-105 barrier. Evenly transition rebar over length of transition section.
- ⑦ Refer to BA-106 for details of 5g2 bars, 5g3 bars, and reinforced paved shoulder.
- ⑧ Install rigid steel conduit within barrier, match location shown in bridge plans.

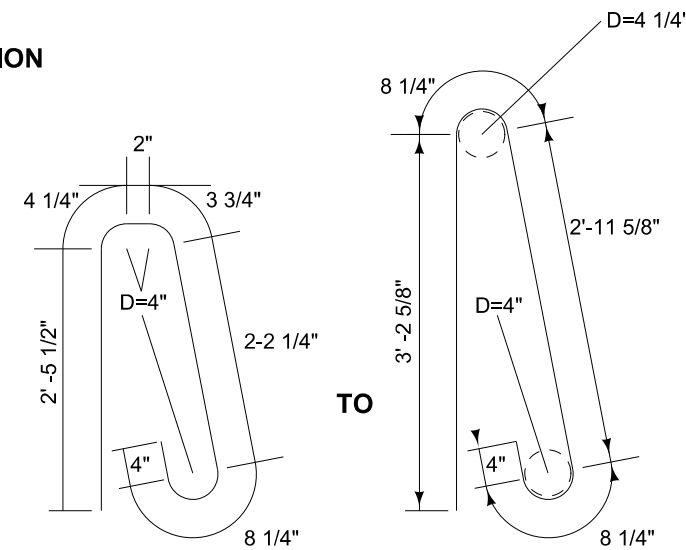


BARRIER FACE

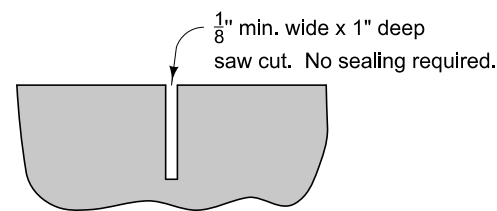
ELEVATION



TYPICAL SECTION

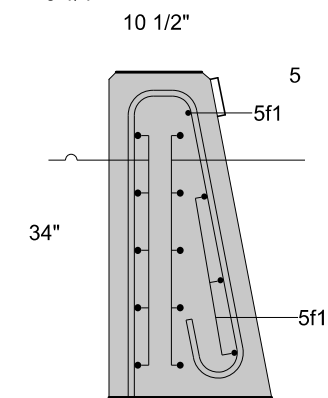


5g2 BENT BAR



SAWED CONTRACTION JOINT

Saw cut top and front face. Saw cut back if exposed.



SECTION B-B
When attaching to BA-107

Possible Contract Item:
Concrete Barrier, BA-105, Modified (Half Section)

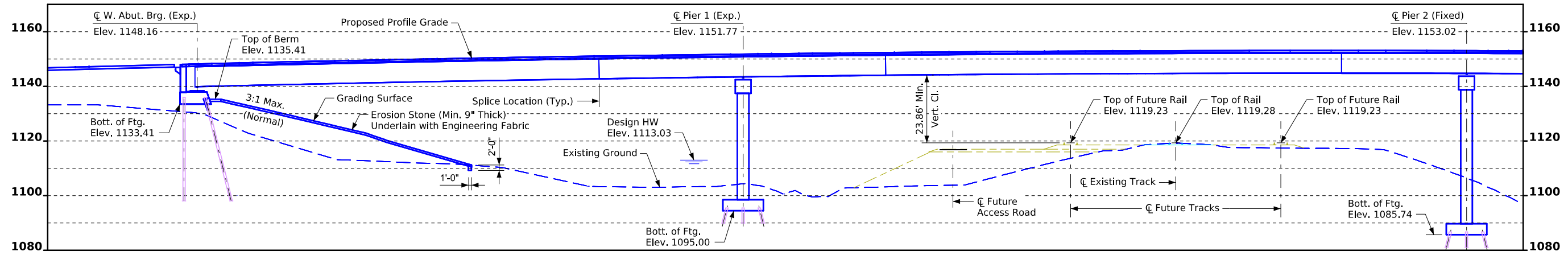
Possible Tabulation:
108-18B

REINFORCING BAR LIST for one Transition Section					
Bar	Size	Number of Bars	Length	Weight (lbs.)	Spacing
5g2	5	11	*	84	12"
5f1	5	10	10'-6"	104	—

* Varies from 6'-6" to 7'-10 1/4"

CONCRETE QUANTITIES for one Transition Section	
3.67 cy	

MODIFIED STANDARD ROAD PLAN	BA-105
	SHEET 1 of 1
34" TO 44" CONCRETE BARRIER TRANSITION (HALF SECTION)	

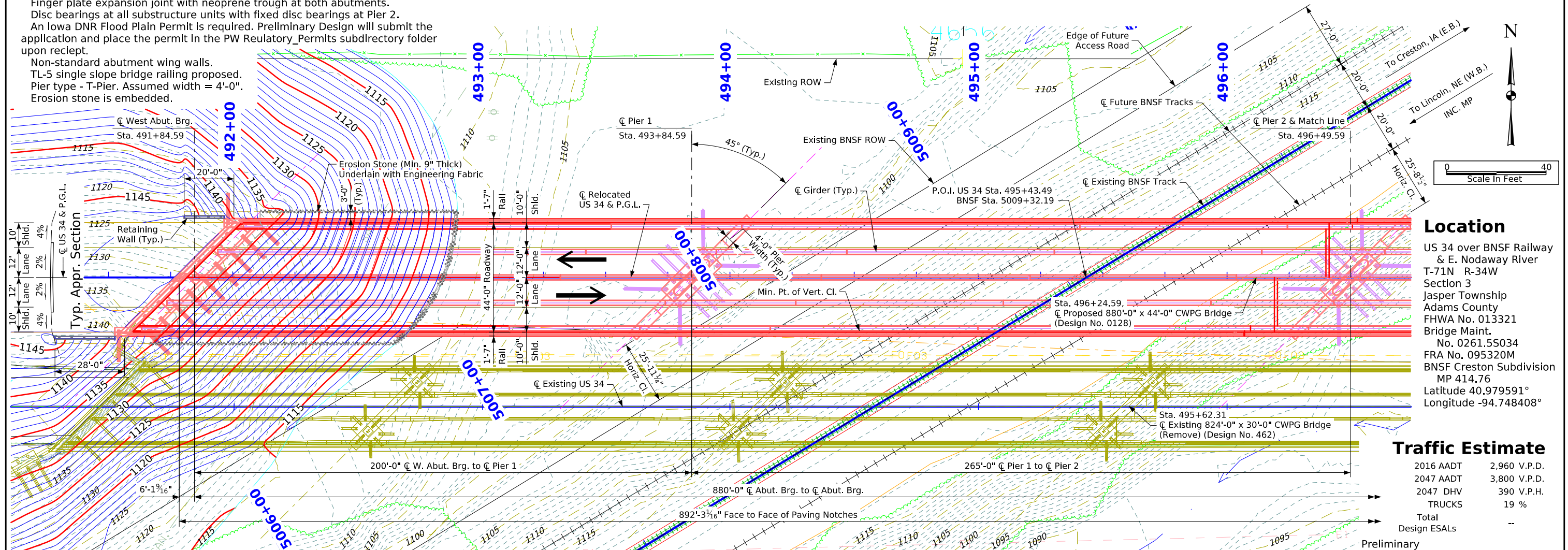


Notes:

This design is for the replacement of the existing 824'-0" x 30'-0" CWPG Bridge, Adams Design No. 462, FHWA No. 013320, Maint. No. 0261.55034. Finger plate expansion joint with neoprene trough at both abutments. Disc bearings at all substructure units with fixed disc bearings at Pier 2. An Iowa DNR Flood Plain Permit is required. Preliminary Design will submit the application and place the permit in the PW Regulatory Permits subdirectory folder upon receipt. Non-standard abutment wing walls. TL-5 single slope bridge railing proposed. Pier type - T-Pier. Assumed width = 4'-0". Erosion stone is embedded.

Longitudinal Section Along Approach Roadway
Top of bridge deck at centerline roadway is 0.03 ft. below the profile grade to account for parabolic crown.

Note: Total wing wall length (bridge wing wall plus retaining wall) is measured from back face of backwall.



Location
US 34 over BNSF Railway & E. Nodaway River
T-71N R-34W Section 3
Jasper Township Adams County
FHWA No. 013321
Bridge Maint. No. 0261.55034
FRA No. 095320M
BNSF Creston Subdivision MP 414.76
Latitude 40.979591°
Longitude -94.748408°

Traffic Estimate

2016 AADT	2,960 V.P.D.
2047 AADT	3,800 V.P.D.
2047 DHV	390 V.P.H.
TRUCKS	19 %
Total Design ESALS	--

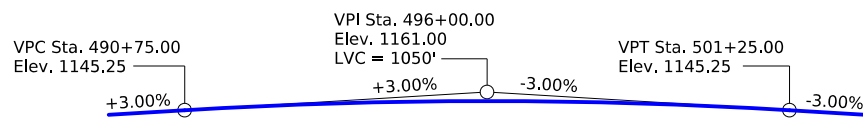
Preliminary

Minimum Vertical Clearance

Overhead Station = 494+53.93, Offset 21.50' Rt.
Overhead Elevation = 1152.11
Depth of Superstructure = 8.38 ft.
Deck Thickness = 8.5 in.
Estimated Haunch = 2 in.
Girder Depth = 90 in.
Max. Live Load Deflection = 0.27 ft.
Underpass Station = 5008+45.02, Offset 29.00' Lt.
Underpass Elevation = 1119.60
Minimum Vertical Clearance = 23.86 ft.

Situation Plan

All dimensions are in feet unless noted otherwise.



Proposed Profile Grade Relocated US 34

Utilities Note:

Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

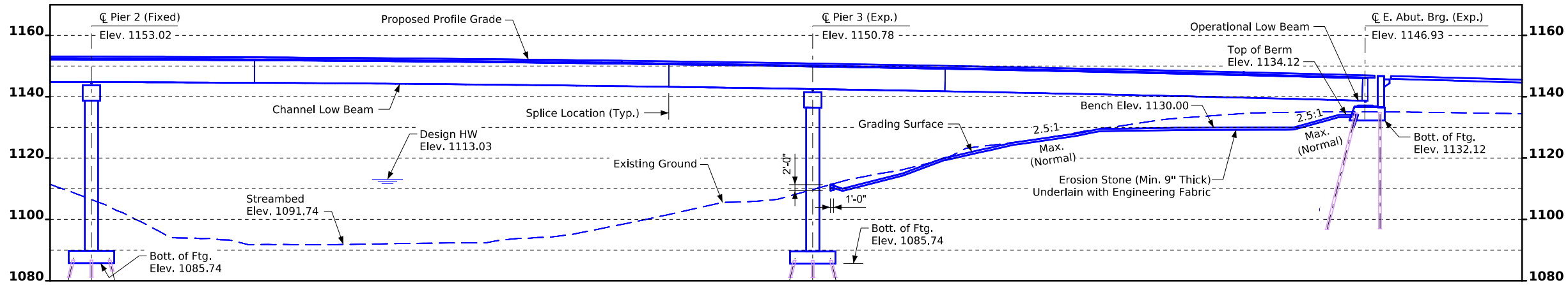
General Utility Symbols:

- E - Electric Line
- G - Gas Line
- SAN. - Sanitary Sewer
- T - Telephone Line
- W - Water Line
- FO - Fiber Optic Line
- GHP - Gas High Pressure
- ST S - Storm Sewer
- TV - TV
- Power Poles

880'-0" x 44'-0" Continuous Welded Plate Girder Bridge

200'-0" / 180'-0" End Spans 265'-0" / 235'-0" Interior Span
Design For 45° Skew LA
Situation Plan
STA. 496+24.59 (C Relocated US 34) Turn-in Date: August, 2024
Adams County
IOWA DEPARTMENT OF TRANSPORTATION
Design No. 0128 Design Sheet No. 1 of 4 FHWA No. 013321

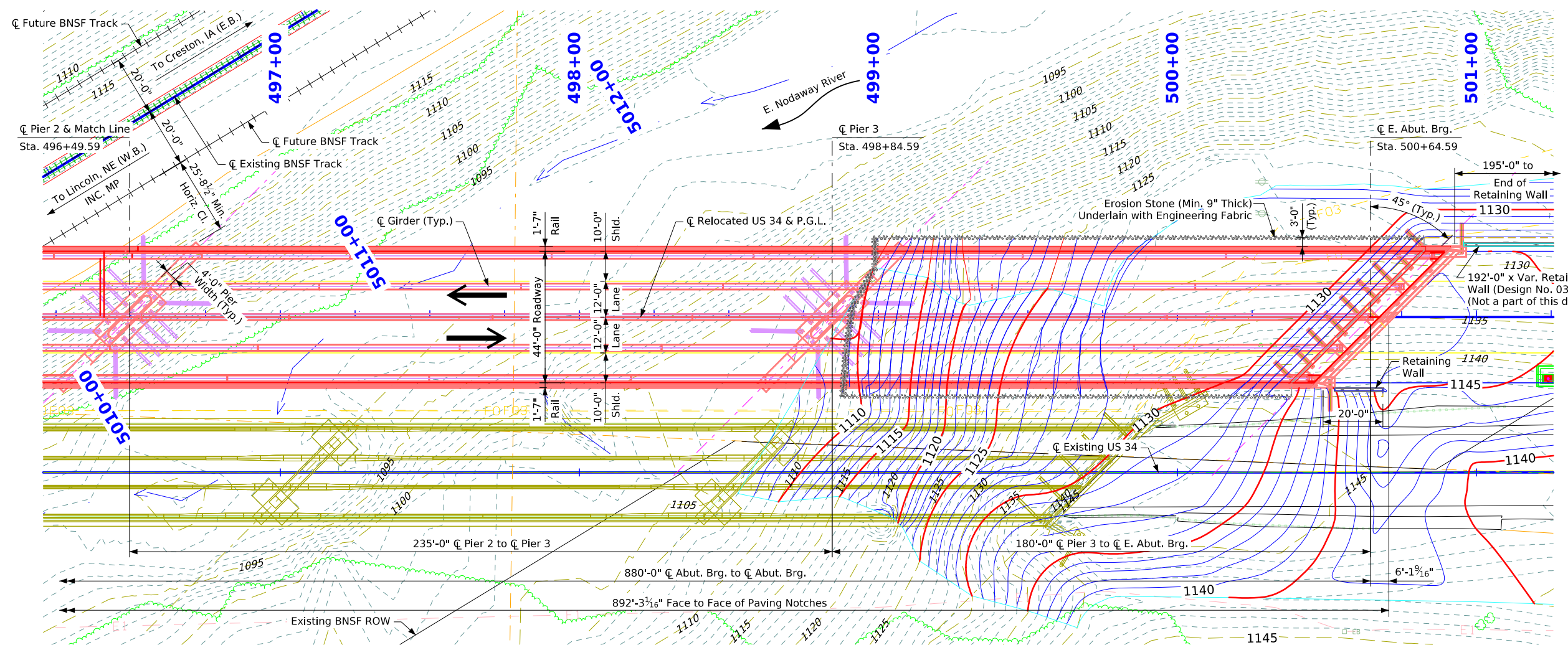




Longitudinal Section Along Approach Roadway

Top of bridge deck at centerline roadway is 0.03 ft. below the profile grade to account for parabolic crown.

Note: Total wing wall length (bridge wing wall plus retaining wall) is measured from back face of backwall.



Hydraulic Data

RIDB: "NodawayR_E_Adams_36.79"
 Drainage Area = 168 Sq. Mi.
 Stream Slope (HGL) = 6.7 ft./Mi.
 Avg. Low Water Stage = 1094.0

Q₅₀ = 18,000 cfs
 Stage = 1112.07
 Channel Low Beam = 1143.70
 Backwater = 0.24 ft.
 Avg. Bridge Velocity = 4.9 fps

Q₁₀₀ = 21,200 cfs
 Stage = 1113.03
 Operational Low Beam = 1137.62
 Backwater = 0.36 ft.
 Avg. Bridge Velocity = 5.2 fps

Q₂₀₀ = 24,500 cfs
 Stage = 1113.92
 Calculated Design Scour = 1079.20

Q₅₀₀ = 28,300 cfs
 Stage = 1114.90
 Avg. Bridge Velocity = 5.9 fps
 Calculated Check Scour = 1078.00

Roadway Overtop 1129.13
 Sta. 483+33.05

Extreme HW Stage = 1116.08
 Date = June 1947

Hydraulic Design

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

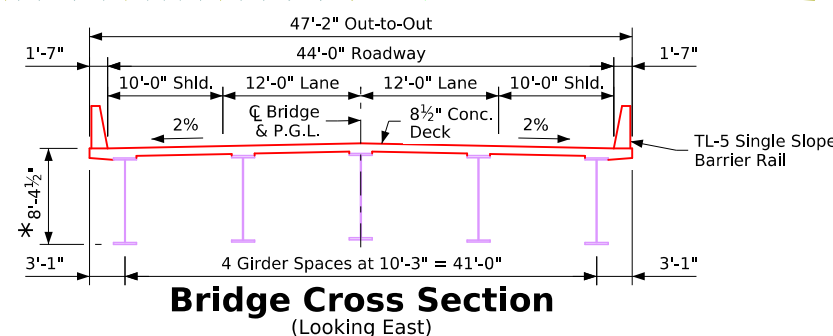
David J. Mulholland 07-23-2024
 Signature Date
 David J. Mulholland
 Printed or Typed Name
 My license renewal date is December 31, 2024

Pages or sheets covered by this seal: Sheets V.1 - V.3
 (Hydraulic Data, Channel Grading and Retention)

Situation Plan

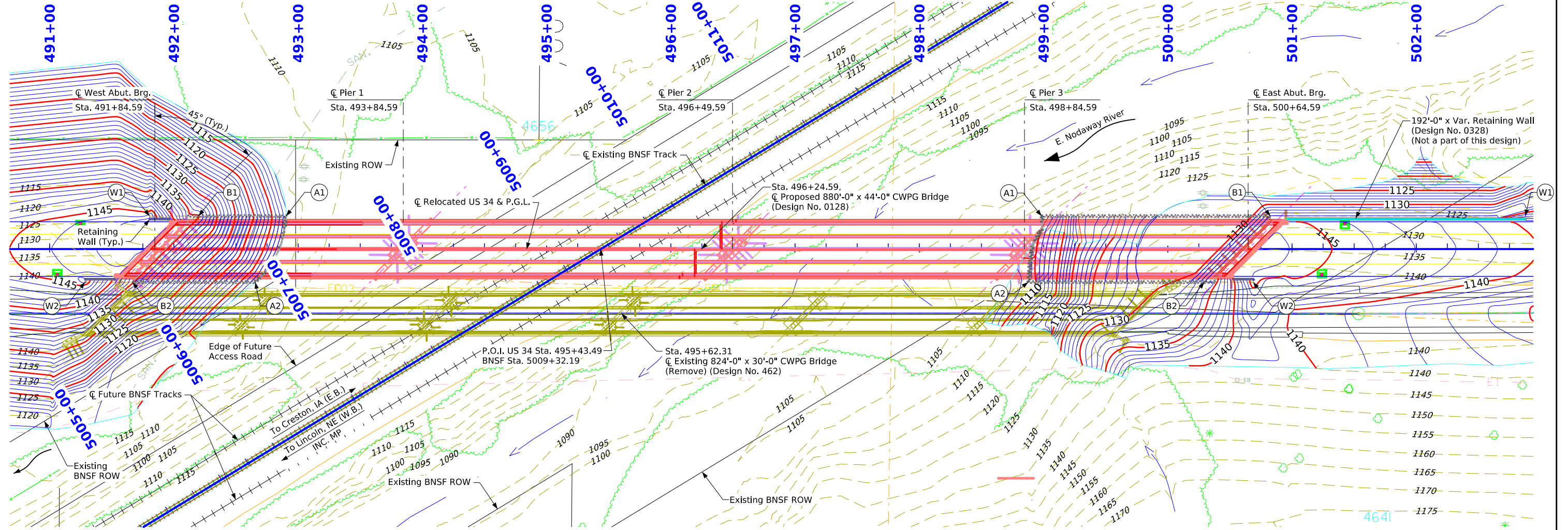
All dimensions are in feet unless noted otherwise.

* Structure depth of 8.38' assumes 8.5 in. deck, 2 in. concrete haunch, and 90 in. girder depth.



Design For 45° Skew LA
880'-0" x 44'-0" Continuous Welded Plate Girder Bridge
 200'-0" / 180'-0" End Spans 265'-0" / 235'-0" Interior Span
Situation Plan
 STA. 496+24.59 (Q Relocated US 34) Turn-in Date: August, 2024
Adams County
 IOWA DEPARTMENT OF TRANSPORTATION
 Design No. 0128 Design Sheet No. 2 of 4 FHWA No. 013321

Control Point: FENO3 Northing 6224262.67 Easting 22225760.64 Elev. 1127.91 Set Feno Monument 15'± West of Centerline Quarry Road, 5'± North of Sign with Arrow, 10'± East of Fence Line at End of Curve



Berm Slope Location Table

Points	West Abutment			East Abutment		
	Station	Offset	Elev.	Station	Offset	Elev.
A1	492+90.34	26.58' Lt.	1111.18	498+98.37	26.58' Lt.	1108.58
A2	492+65.54	26.58' Rt.	1112.06	498+87.61	26.58' Rt.	1112.03
B1	492+19.84	26.58' Lt.	1135.41	500+82.51	26.58' Lt.	1134.12
B2	491+66.67	26.58' Rt.	1135.41	500+29.34	26.58' Rt.	1134.12
W1	491+80.46	26.58' Lt.	1147.51	502+87.72	26.58' Lt.	1139.78
W2	491+28.46	26.58' Rt.	1146.18	500+68.72	26.58' Rt.	1146.26

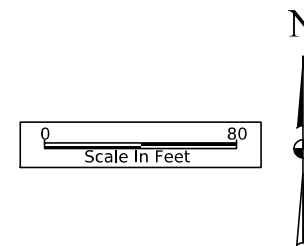
Berm slope elevations reflect the grading surface.

General Utility Symbols:

- E - Electric Line
- G - Gas Line
- SAN - Sanitary Sewer
- T - Telephone Line
- W - Water Line
- FO - Fiber Optic Line
- GHP - Gas High Pressure
- ST S - Storm Sewer
- TV - TV
- - Power Poles

Utilities Note:

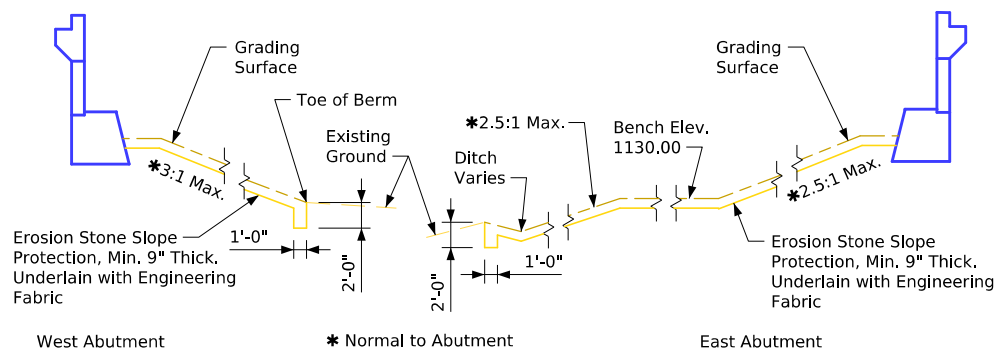
Utilities shown on this sheet are for information only. See Road Design sheets for utility information.



Estimated Berm Armoring Quantities

Location	Erosion Stone (Ton)	Engineering Fabric (SY)	CL. 10 Channel Excavation (CY)
Berm Lining - West Abutment	250	620	155
Berm Lining - East Abutment	433	1098	267
Totals	683	1718	422

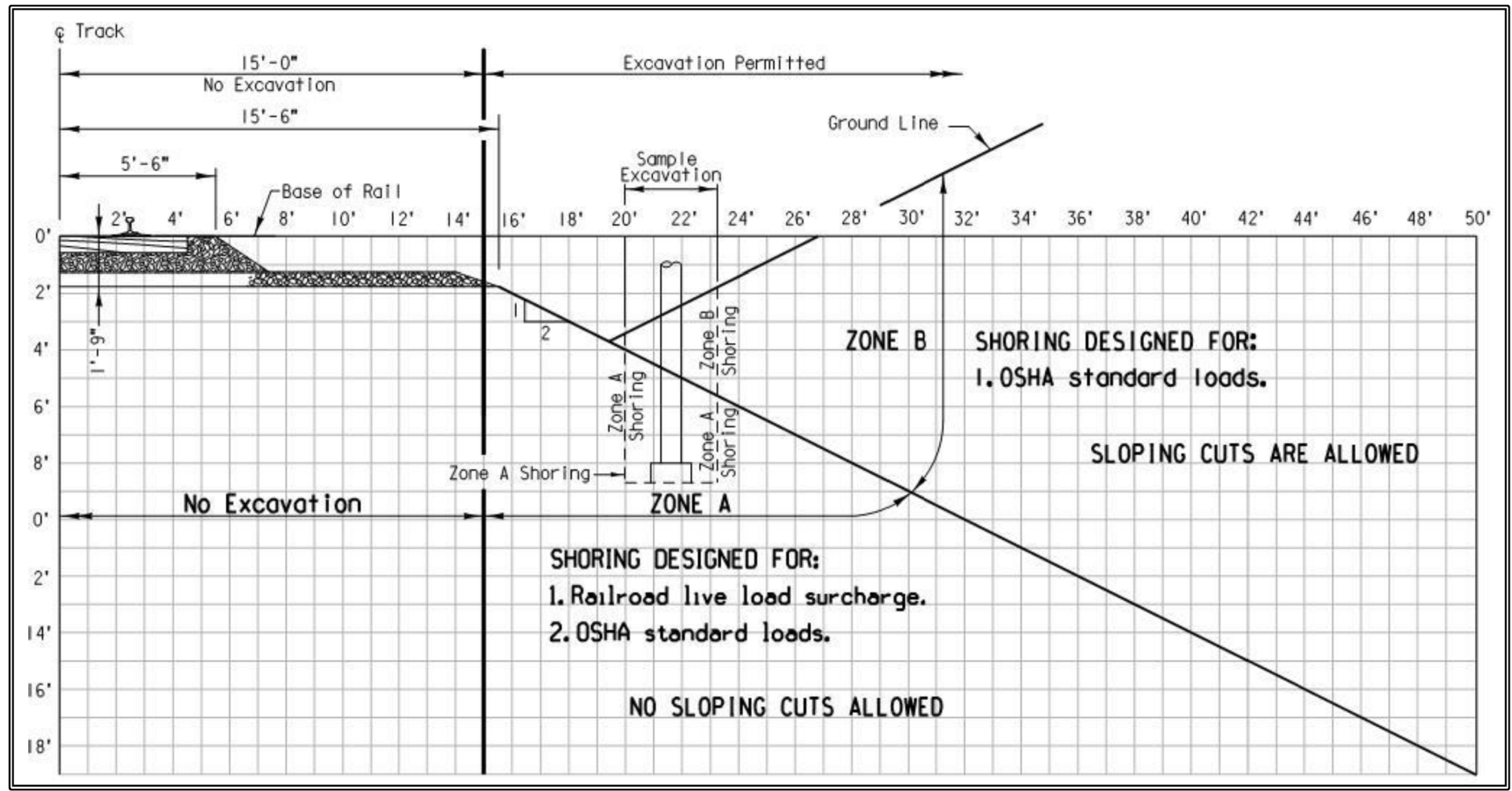
Excavation Quantity Calculated from Grading Surface. Erosion Stone Estimated at 1.6 Ton/CY.



Section Thru Embedded Erosion Stone

Preliminary
Design For 45° Skew LA
880'-0" x 44'-0" Continuous Welded Plate Girder Bridge
200'-0" / 180'-0" End Spans 265'-0" / 235'-0" Interior Span
Site Plan
STA. 496+24.59 (C Relocated US 34) Turn-in Date: August, 2024
Adams County
IOWA DEPARTMENT OF TRANSPORTATION
Design No. 0128 Design Sheet No. 3 of 4 FHWA No. 013321

Revised 07-2019: For UPRR, Changed Horizontal Distance of Minimum Construction Clearance Envelope to 15'-0" (Was 12'-0").
 Revised 03-2022: Updated "General Excavation Zones" Detail and "General Shoring Notes" to Follow the "UPRR & BNSF Guidelines for Temporary Shoring, 2021".
 Issued 12-2008.
 MiscellaneousBridges.dgn - 1067 - This Sheet Re-Issued 11-2023. Sheet Format Update.



Top of Rail Elevations

(Stations Increase with Milepost Decrease)

Main Line			
Alignment: Left Rail		Alignment: Right Rail	
Station	Elevation	Station	Elevation
5000+00.00	1119.53	5000+00.00	1119.53
5000+32.19	1119.50	5000+32.19	1119.50
5001+32.19	1119.55	5001+32.19	1119.55
5002+32.19	1119.59	5002+32.19	1119.59
5003+32.19	1119.53	5003+32.19	1119.53
5004+32.19	1119.39	5004+32.19	1119.39
5005+32.19	1119.28	5005+32.19	1119.28
5006+32.19	1119.06	5006+32.19	1119.06
5007+32.19	1119.05	5007+32.19	1119.05
5008+32.19	1119.24	5008+32.19	1119.24
① 5009+32.19	1119.28	① 5009+32.19	1119.28
5010+32.19	1119.26	5010+32.19	1119.26
5011+32.19	1119.25	5011+32.19	1119.25
5012+32.19	1119.28	5012+32.19	1119.28
5013+32.19	1119.25	5013+32.19	1119.25
5014+32.19	1119.23	5014+32.19	1119.23
5015+32.19	1119.31	5015+32.19	1119.31
5016+32.19	1119.38	5016+32.19	1119.38
5017+32.19	1119.47	5017+32.19	1119.47
5018+32.19	1119.61	5018+32.19	1119.61
5019+05.75	1119.57	5019+05.75	1119.57

Railroad General Notes:

Railroad review and approval of shoring, erection, demolition, and falsework is required. Allow a minimum of four weeks for the review and approval of each submittal.

The proposed grade separation project shall not increase the quantity and/or characteristics of the flow in the railroad's ditches and/or drainage structures.

The elevation of the existing top-of-rail profile shall be verified before beginning construction. All discrepancies shall be brought to the attention of the railroad prior to construction.

The Contractor must submit a proposed method of erosion and sediment control and have the method approved by the railroad.

All shoring systems that impact the railroad's operations and/or supports the railroad's embankment shall be designed and constructed per current railroad guidelines for temporary shoring.

All demolitions within the railroad's right-of-way and/or demolition that may impact the railroad's tracks or operations shall be in compliance with the railroad's demolition guidelines.

Erection over the railroad's right-of-way shall be designed to cause no interruption to the railroad's operation, enabling the track(s) to remain open to traffic per the railroad's requirements.

All construction phasing that may impact the railroad operations shall be designed to cause no interruption to the railroad's operation, enabling the track(s) to remain open to traffic per the railroad's requirements.

False-work clearances shall comply with minimum construction clearances.

All permanent clearances shall be verified before project closing.

For railroad coordination please refer to the railroad coordination requirements as part of special provisions.

General Excavation Zones

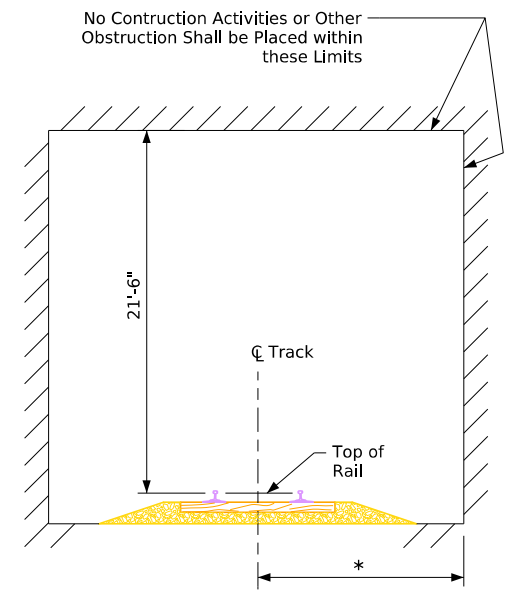
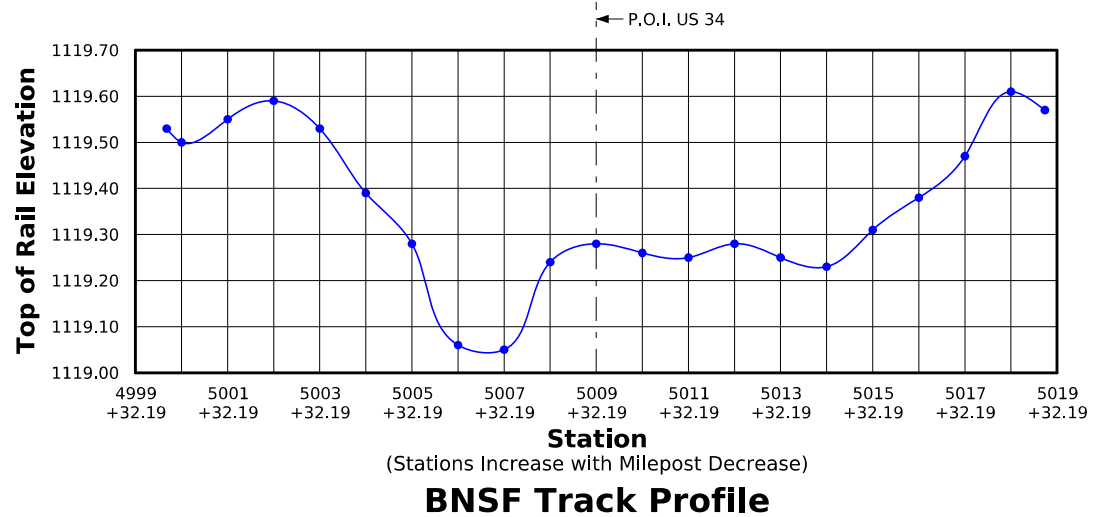
Source: UPRR & BNSF Guideline for Temporary Shoring, 2021

General Shoring Notes:

All dimensions are measured perpendicular to track.

Prior to commencing any work, the Contractor shall submit for approval by the Railroad detailed plans indicating the nature and extent of the track protection shoring proposed. The Contractor shall install the temporary shoring system per the approved plans. Design of the temporary shoring system to comply with UPRR & BNSF Guidelines for Temporary Shoring.

For excavations which encroach into Zone A or B, shoring plans shall be accompanied by design calculations. Plans and calculations must be signed and stamped by a Professional Engineer registered in the State of Iowa.



Minimum Construction Clearance Envelope
(Normal to Railroad)

* 15'-0" for BNSF and 15'-0" for UPRR

① \bar{C} of Mainline US 34 Along Track Stationing is 495+43.49
 \bar{C} of Tracks Along Mainline US 34 Stationing is 5009+32.19

Note:
 BNSF = Burlington Northern Santa Fe Railroad
 UPRR = Union Pacific Railroad

Preliminary

Design For 45° Skew LA

880'-0" x 44'-0" Continuous Welded Plate Girder Bridge

200'-0" / 180'-0" End Spans 265'-0" / 235'-0" Interior Span

BNSF General Notes and Shoring

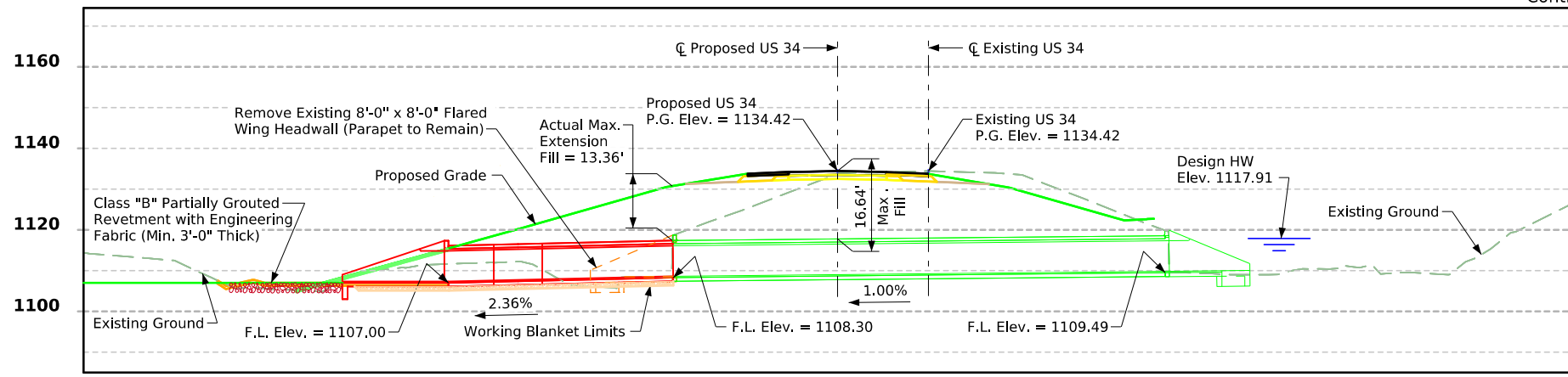
STA. 496+24.59 (\bar{C} Relocated US 34) Turn-in Date: August, 2024

Adams County

IOWA DEPARTMENT OF TRANSPORTATION

Design No. 0128 Design Sheet No. 4 of 4 FHWA No. 013321

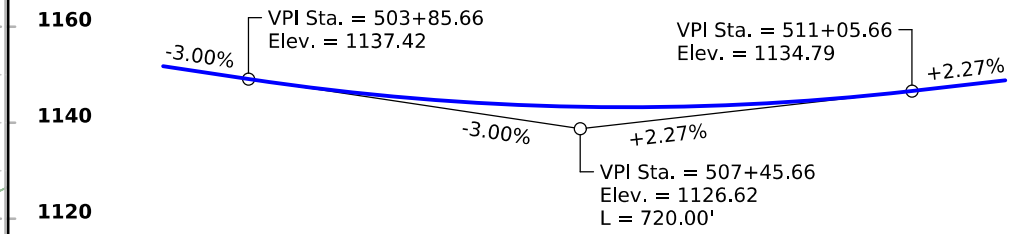
Control Point: FENO3 Northing 6224262.67 Easting 22225760.64 Elev. 1127.91 Set Feno Monument 15'± West of Centerline Quarry Road, 5'± North of Sign with Arrow, 10'± East of Fence Line at End of Curve



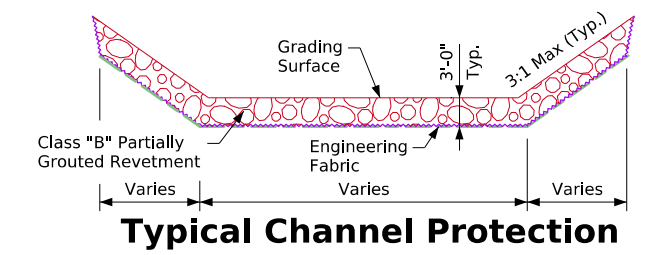
Longitudinal Section Along Centerline of Culvert

Anticipated Settlement = X"
 Extension Design Fill Height = 17'
 NBIS Structure Length = 8.0'

Note:
 Reinforced concrete box culvert extensions shall be cast-in-place. Cast-in-place details are intended to accommodate varying as-built conditions of the existing culvert.



Proposed Profile Grade Relocated US 34



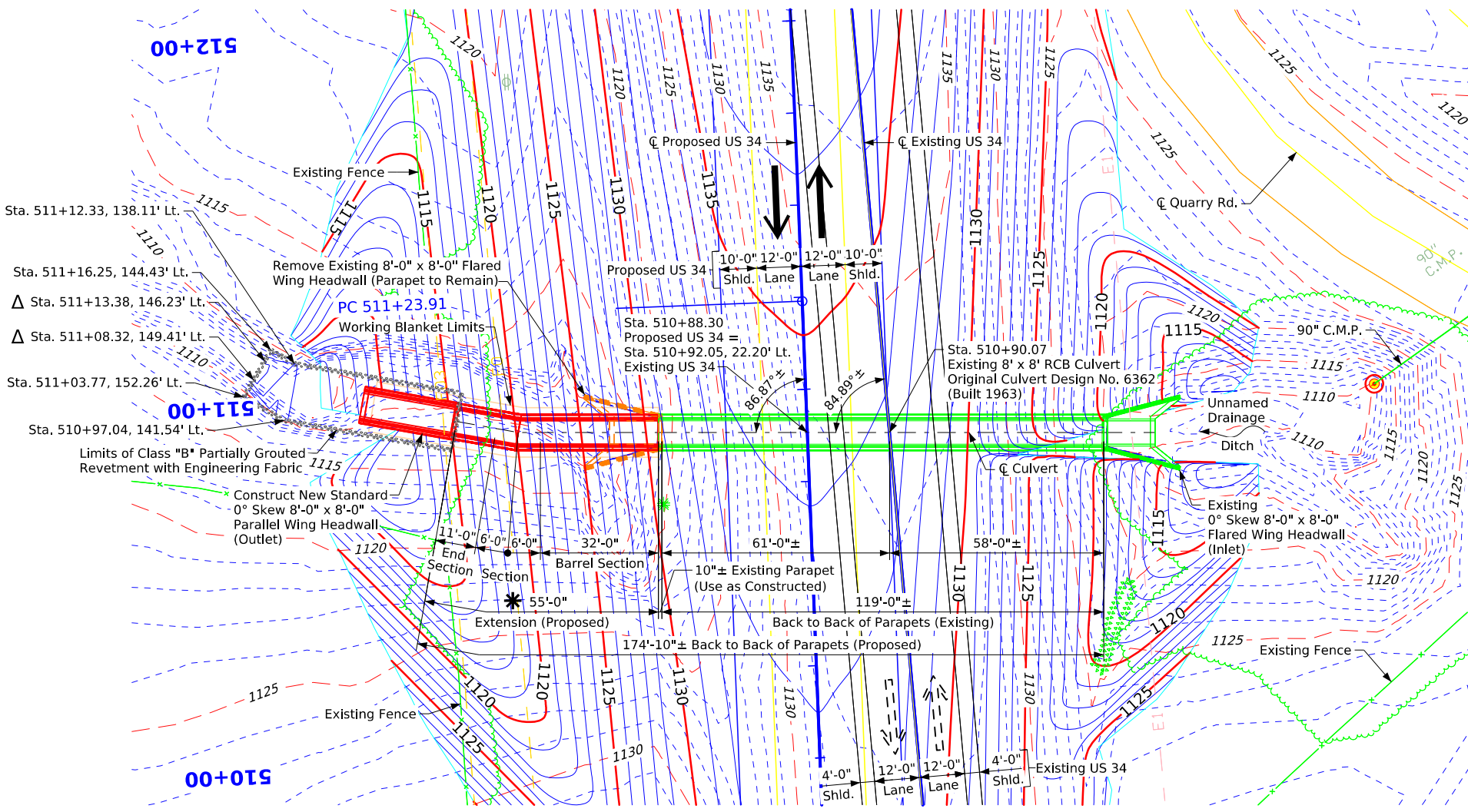
Typical Channel Protection

Estimated Revetment Quantities Included With Road Plans

Location	Revetment Class "B" (Ton)	Engineering Fabric (SY)	Cl. 10 Channel Excavation (CY)	Grout (CY)
Inlet	---	---	---	---
Outlet	108	115	70	11
Totals	108	115	70	11

Excavation quantity is calculated from grading surface. Excavation quantity is for embedded revetment core out only, and does not include excavation to the grading surface. The revetment details shown are a modification of Standard Sheet 1092 to accommodate velocities that exceed policy limits.

Δ Top of Revetment / Grading Surface
 Elev. 1106.00



Situation Plan

Existing US 34 granular shoulder not shown for clarity.

* Extension length from front of existing parapet to back of new parapet.

Hydraulic Data

RIDB: "Not Applicable"
 Drainage Area = 330.9 Acres (Very Hilly / Mixed Cover)
 Stream Slope = 68.6 Ft./Mi.
 Q₅₀ = 577 cfs
 HW Elev. = 1117.91
 Exit Velocity = 20.7 fps
 Q₁₀₀ = 693 cfs
 HW Elev. = 1119.01
 Exit Velocity = 21.7 fps

Location

US 34 over unnamed drainage ditch
 T-71N R-34W
 Section 3
 Jasper Township
 Adams County
 Asset ID No. 900755
 Latitude 40.979695°
 Longitude -94.743162°

Traffic Estimate

2016 AADT	2,960	V.P.D.
2047 AADT	3,800	V.P.D.
2047 DHV	390	V.P.H.
TRUCKS	19	%
Total Design ESALs	--	


Utilities Note:

Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

General Utility Symbols:

- E - Electric Line
- G - Gas Line
- SAN. - Sanitary Sewer
- T - Telephone Line
- W - Water Line
- FO - Fiber Optic Line
- GHP - Gas High Pressure
- ST S - Storm Sewer
- TV - TV
- - Power Poles

Hydraulic Design



I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

Signature: *David J. Mulholland* Date: 07-23-2024
 Printed or Typed Name: David J. Mulholland
 My license renewal date is December 31, 2024

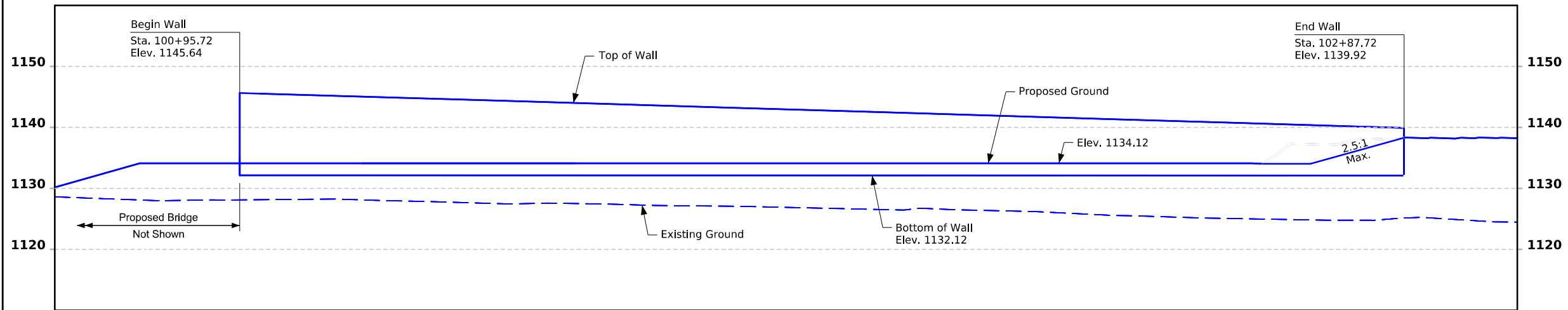
Pages or sheets covered by this seal: _____ Sheet V.5
 (Hydraulic Data, Channel Grading and Revetment)

Design For 3° Skew (LA) with 10° Bend Section
Single 8' x 8' Reinforced Concrete Box Culvert Extension

Situation Plan

STA. 510+88.30 (CL Relocated US 34) Turn-in Date: August, 2024
Adams County
 IOWA DEPARTMENT OF TRANSPORTATION
 Design No. 0228 Design Sheet No. 1 of 1 FHWA/Asset 900755

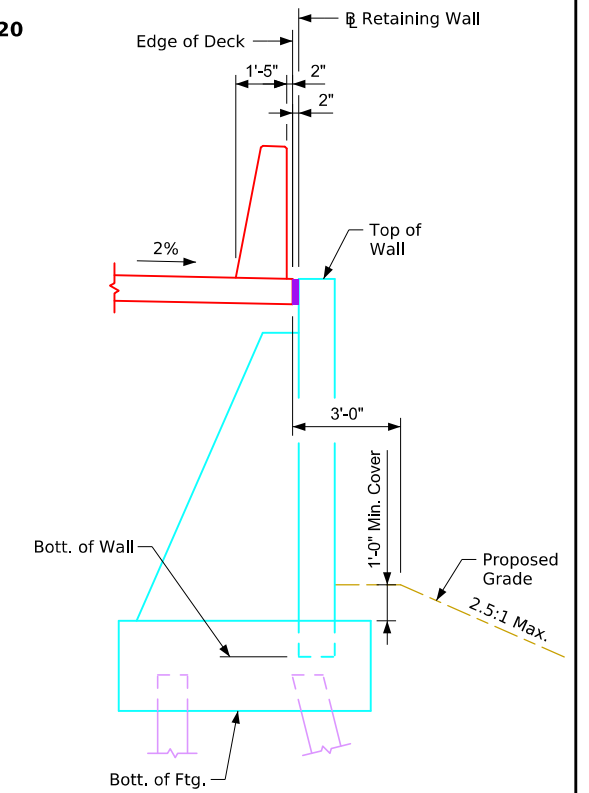




Longitudinal Section Along Retaining Wall

Note: Top of wall is 0.44 feet below the profile grade of US 34.
Counterfort spacing and contraction or expansion joints not shown.

Note:
During final design, the economics of constant footing vs. counterforts shall be determined.

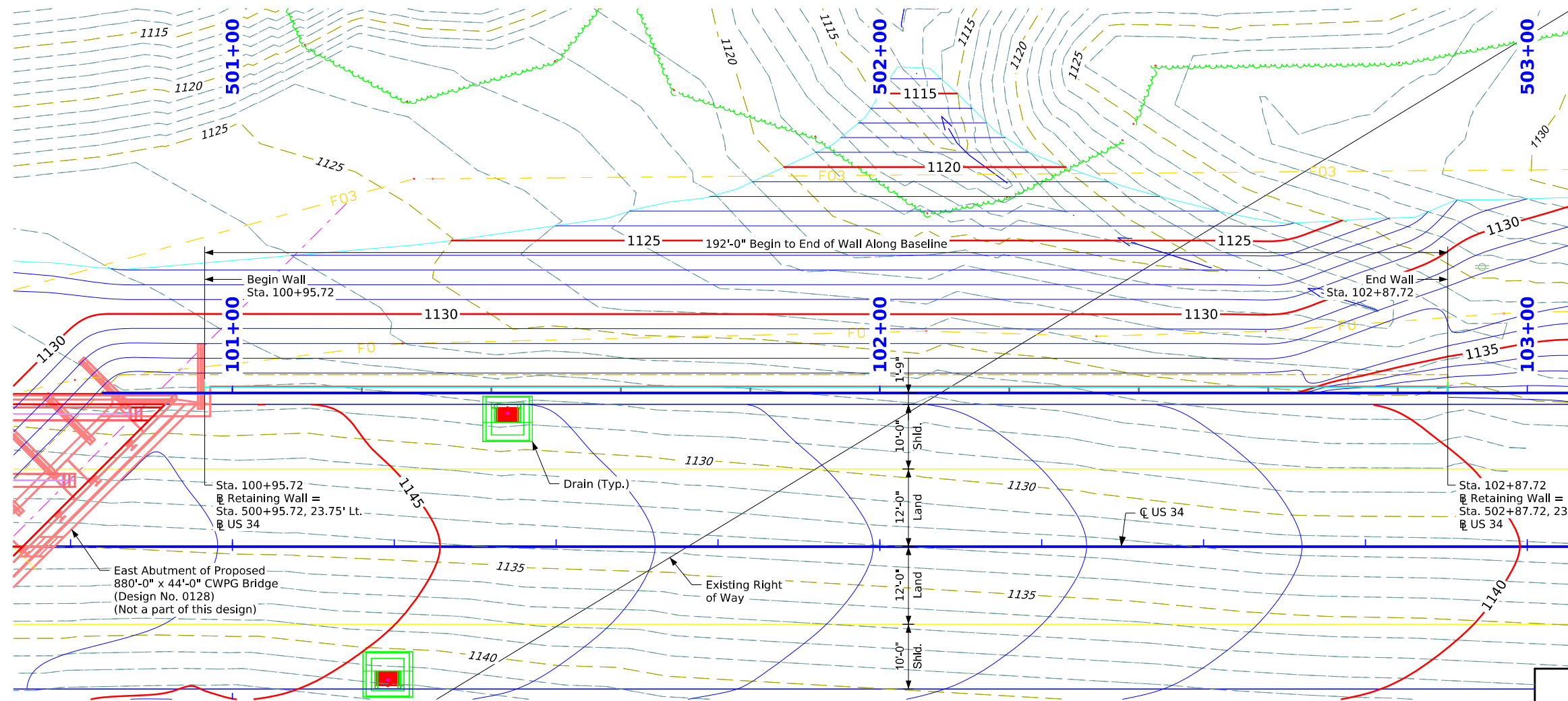


Typical Section thru Wall

(Counterfort Alternate Shown)

Location

Retaining wall adjacent to US 34
T-71N R-34W
Section 3
Jasper Township
Adams County
Asset ID No. MIS000049
Latitude 40.979714°
Longitude -94.746410°



Situation Plan

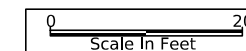
All dimensions are in feet unless noted otherwise.

General Utility Symbols:

- | | |
|-----------------------|-------------------------|
| E - Electric Line | FO - Fiber Optic Line |
| G - Gas Line | GHP - Gas High Pressure |
| SAN. - Sanitary Sewer | ST S - Storm Sewer |
| T - Telephone Line | TV - TV |
| W - Water Line | - Power Poles |

Utilities Note:

Utilities shown on this sheet are for information only. See Road Design sheets for utility information.



Design For A 192' x Variable Height Cast In Place Retaining Wall

Situation Plan

STA. 501+91.71 (US 34) Turn-in Date: August, 2024
Adams County
IOWA DEPARTMENT OF TRANSPORTATION
Design No. 0328 Design Sheet No. 1 of 1 Asset ID No. MIS000049

CROSS SECTION VIEW COLOR LEGEND

Design Color No.	Feature	Design Color No.	Feature
Aggregate			
(64)	Choke Stone	(112)	Noise Wall
(42)	Engineering Fabric	(112)	Noise Wall Footing
(8)	Flooded Backfill	(112)	Retaining Wall Back
(92)	Macadam Stone	(112)	Retaining Wall Back Excavate
(20)	Modified	(112)	Retaining Wall Face
(12)	Plowing Shaping	(112)	Retaining Wall Front Excavate
(14)	Porous Backfill	(112)	Retaining Wall Front Footing
(8)	Revetment Class A	(112)	Retaining Wall MSE Gutter
(6)	Revetment Class B	(112)	Retaining Wall Reinforced Earth
(62)	Revetment Class C	Grading	
(188)	Revetment Class D	(8)	Behind Curb Cut
(28)	Revetment Class E	(6)	Granular
(12)	Shoulder Special Backfill	(13)	Granular Back Fill
(12)	Special Backfill	(48)	Rock Undercut
(20)	Subbase	(8)	Shoulder Earth Fill
(20)	Subbase Lower	(2)	Side Slopes
(20)	Subbase Upper	(226)	Side Slopes Dressing
(118)	Subgrade Treatment	Substrata	
Asphalt			
(207)	HMA Base Course	(128)	Boulder Substrata
(207)	HMA Interim Course	(48)	Broken Weathered Substrata
(207)	HMA Surface Course	(3)	Core Out Substrata
Concrete			
(0)	Barrier Concrete	(203)	Existing Pavement Substrata
(0)	Barrier Concrete Footing	(6)	Loam Substrata
(0)	Curb Gutter	(80)	Rock Substrata
(48)	Flowable Mortar	(4)	Select Sand Substrata
(0)	Median Concrete	(3)	Shale Substrata
(0)	PCC Pavement	(10)	Topsoil Substrata
(0)	Sidewalk	Unsuitable / Waste	
Shoulder			
(209)	Shoulder HMA	(3)	Unsuitable Type A
(0)	Shoulder PCC	(13)	Unsuitable Type B
(6)	Shoulder Granular	(11)	Unsuitable Type C
(6)	Shoulder Granular	(3)	Waste
Existing			
(0)	Existing Pavement		

NOTES:

Text

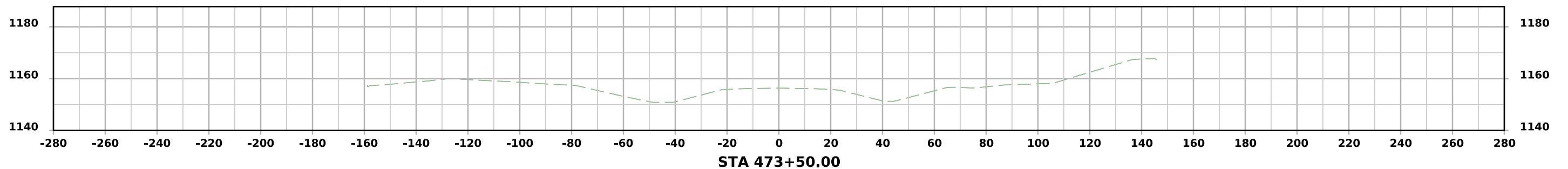
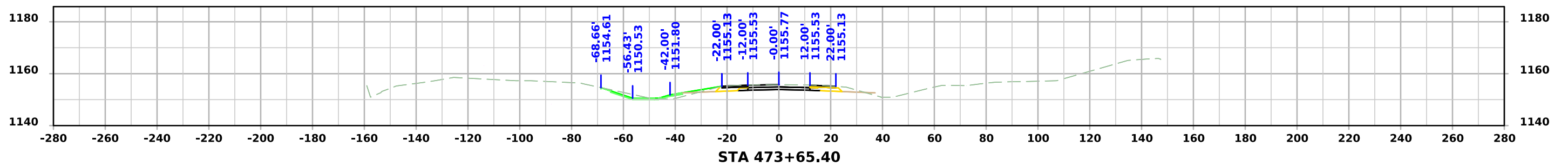
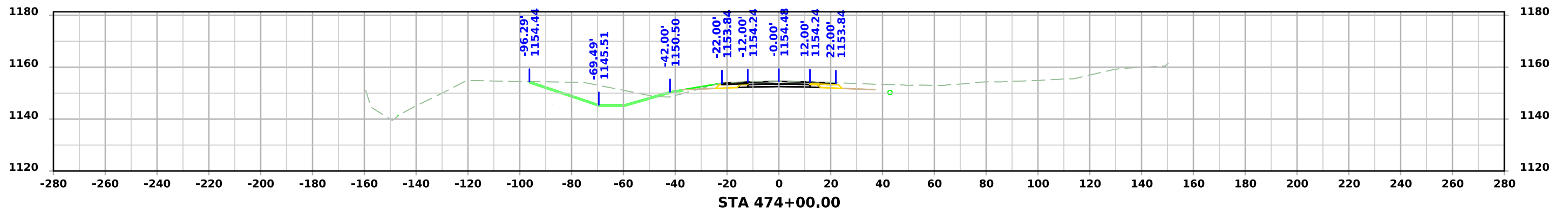
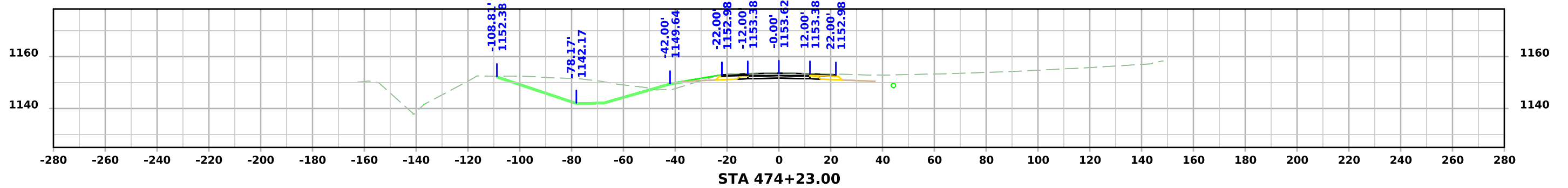
NOTES:

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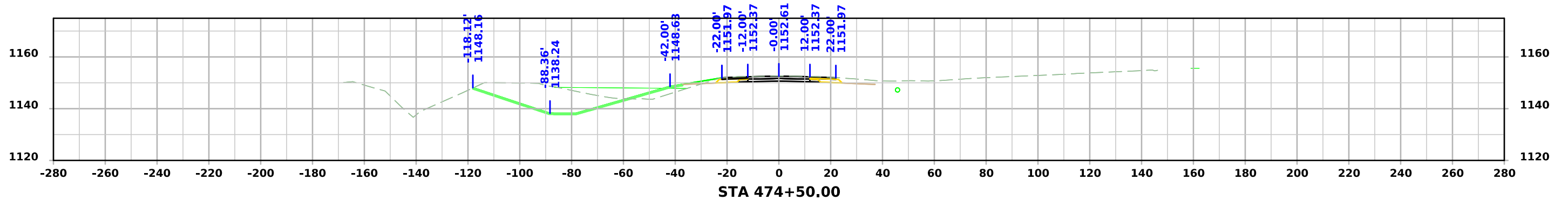
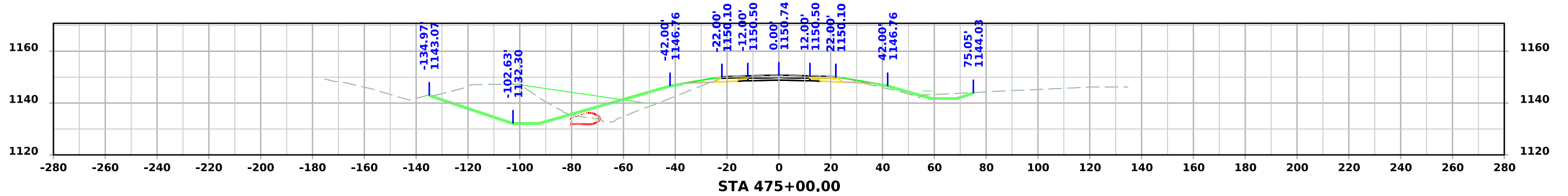
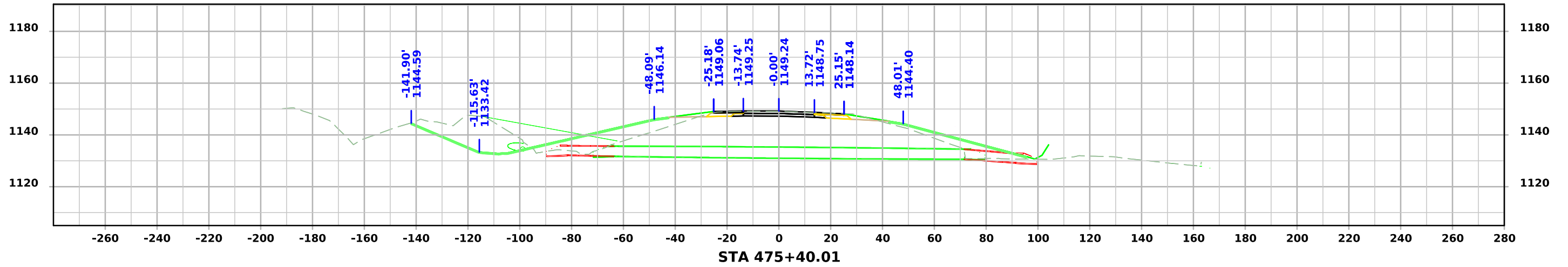
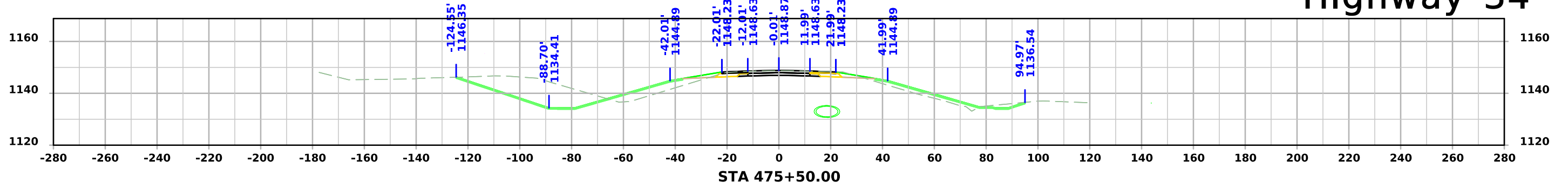
CROSS SECTIONS LEGEND AND INFORMATION SHEET

(COVERS SHEET SERIES W, X, Y, & Z)

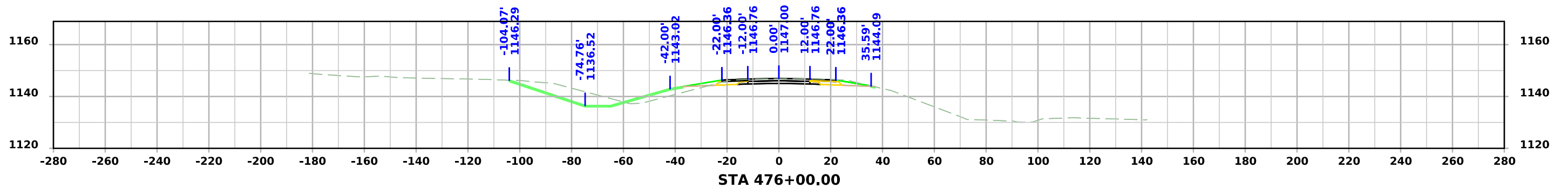
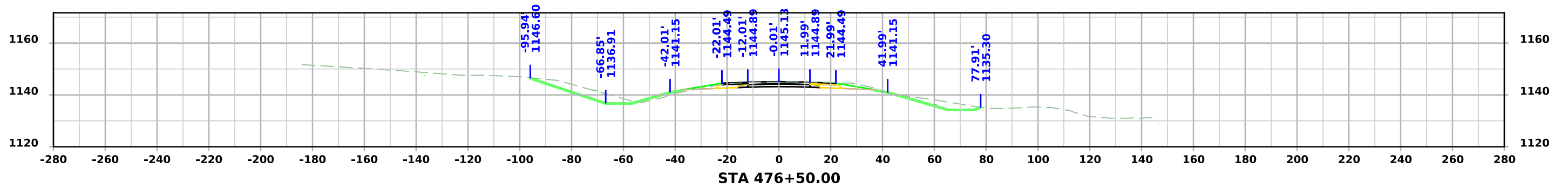
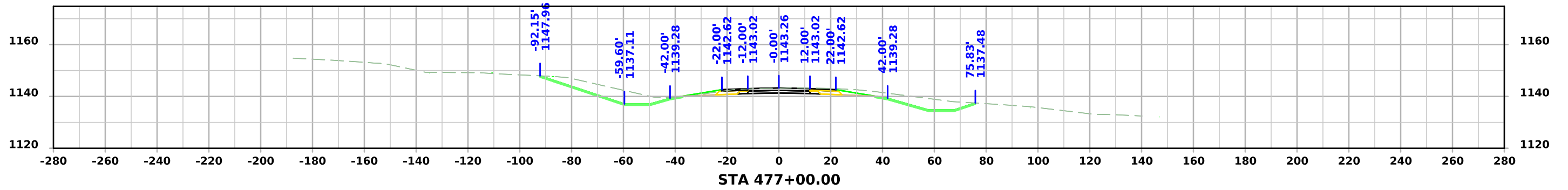
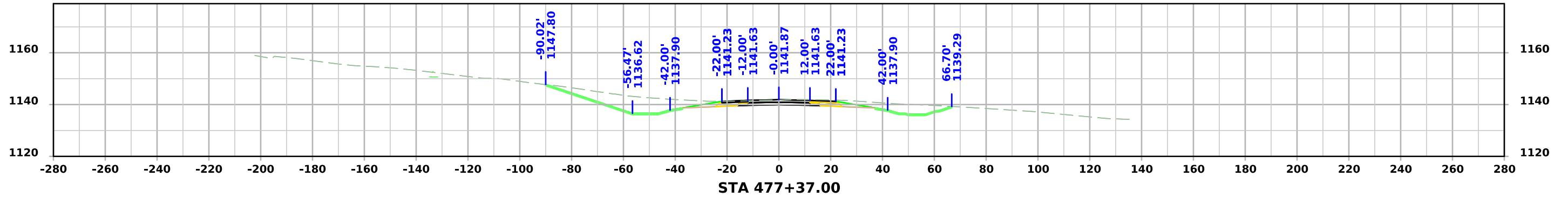
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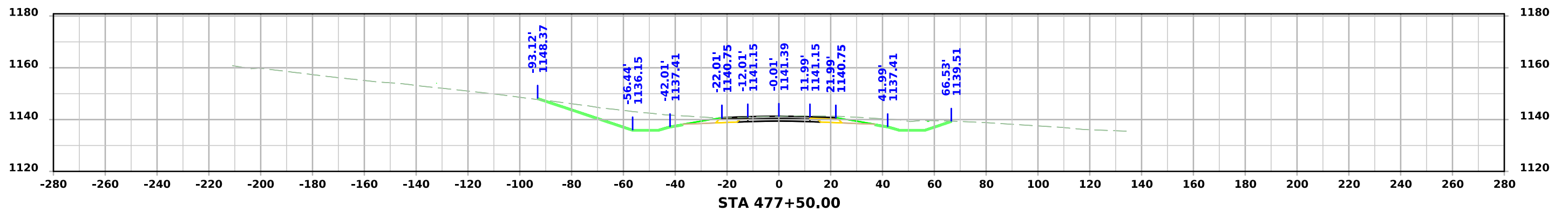
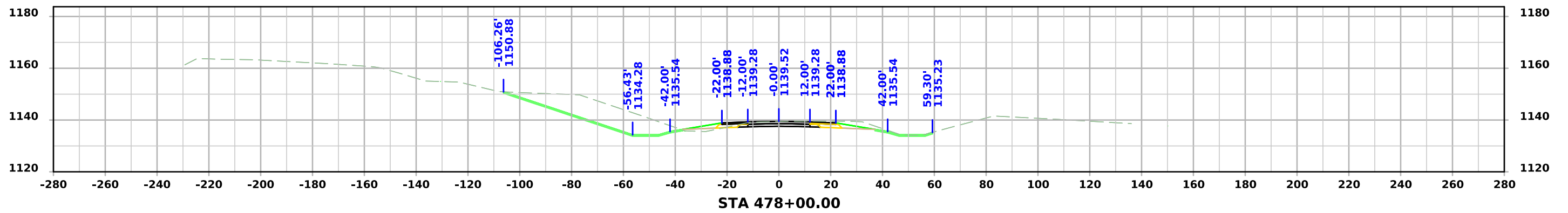
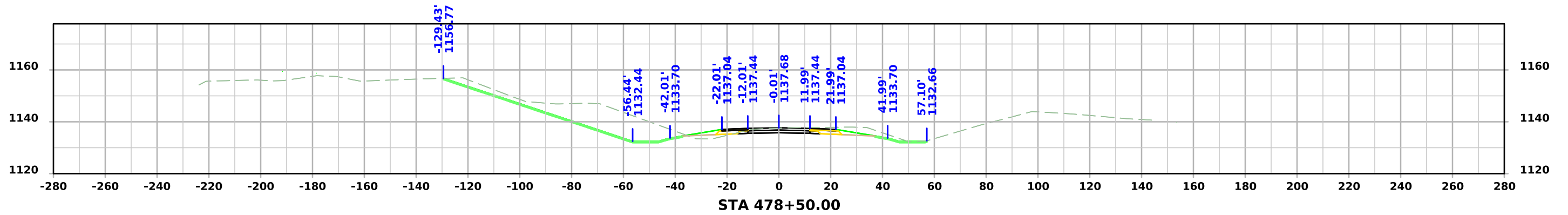
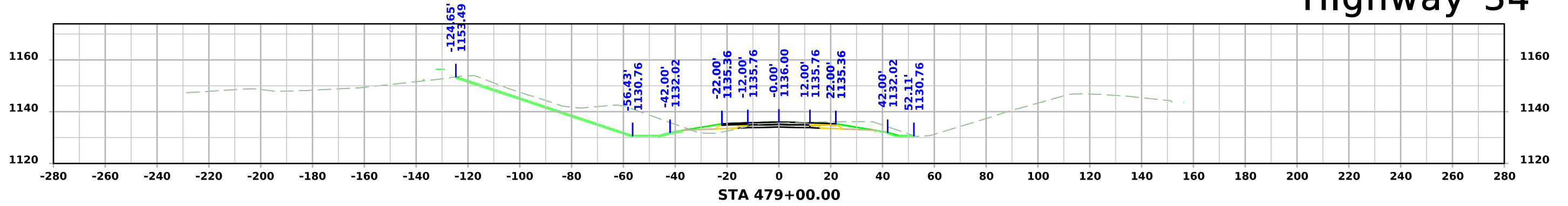
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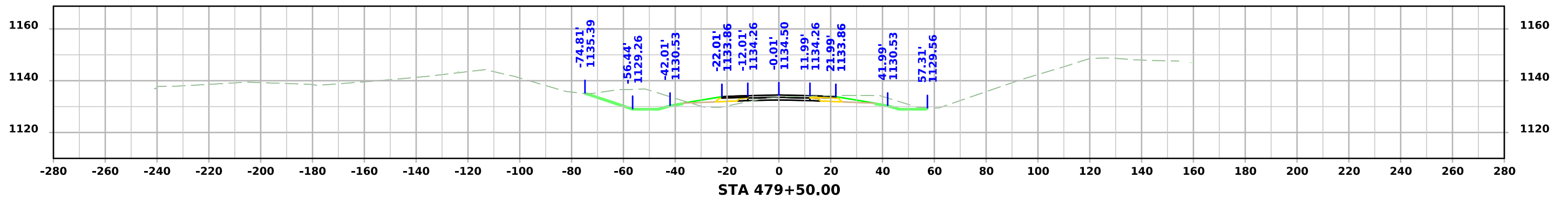
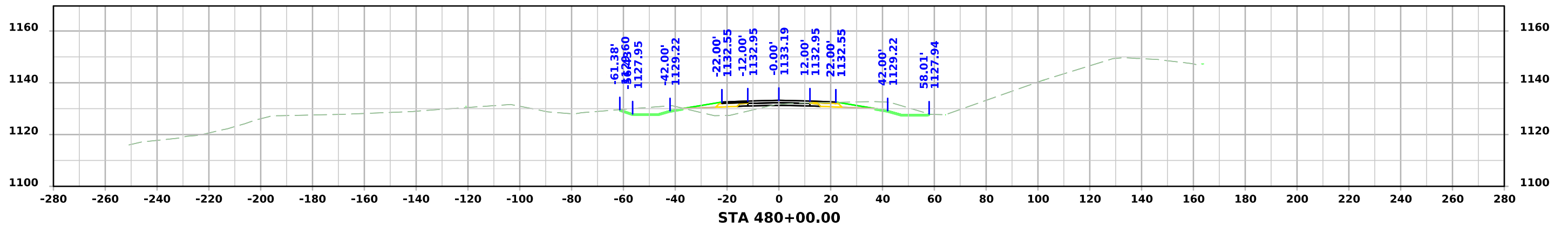
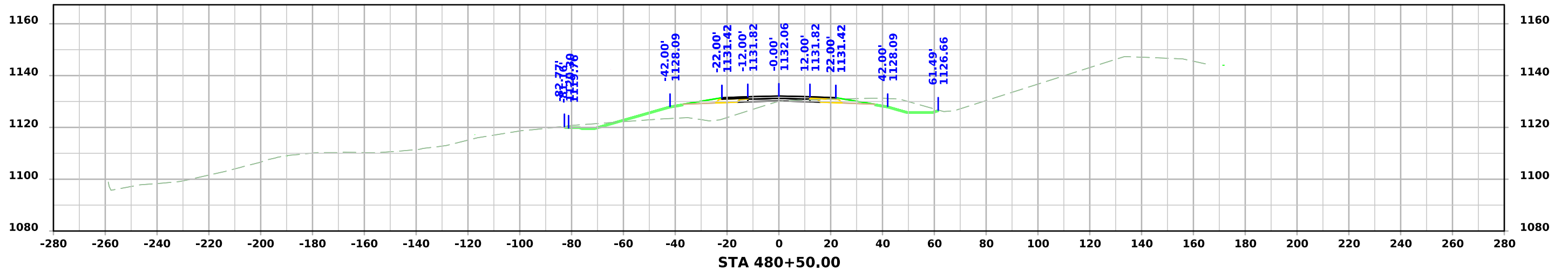
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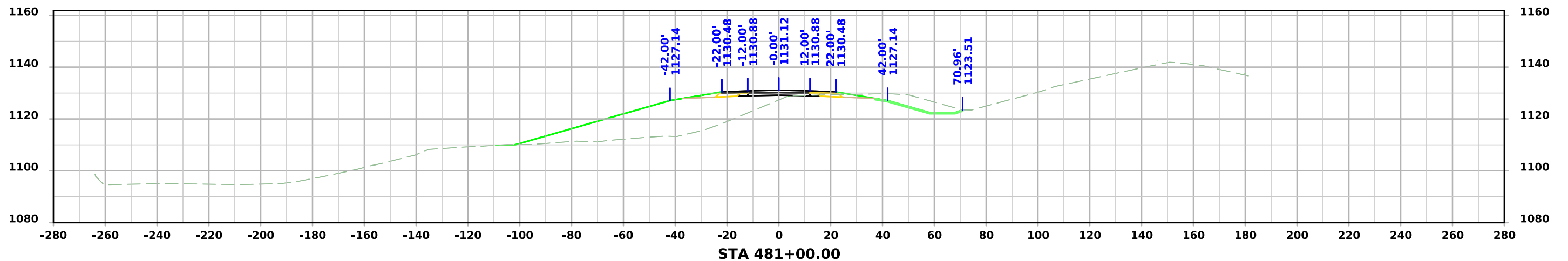
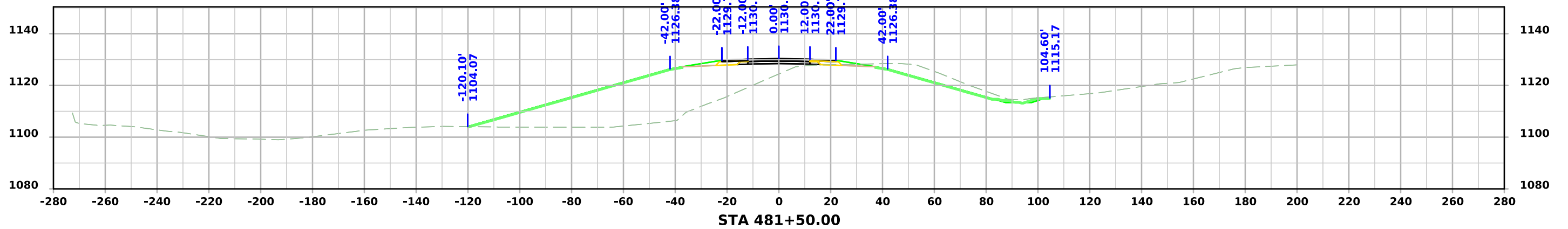
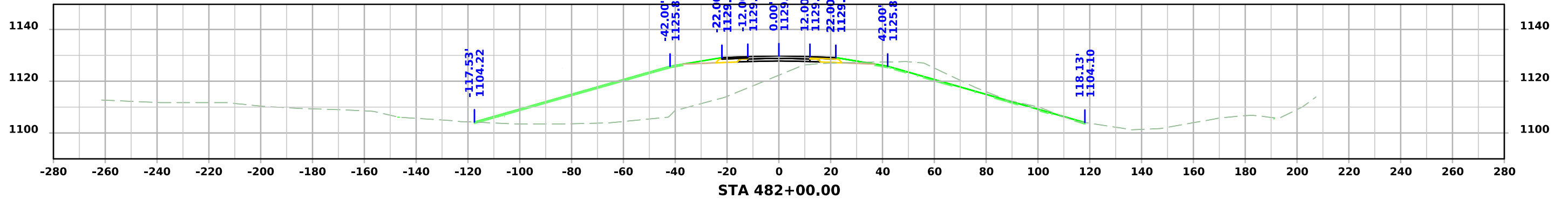
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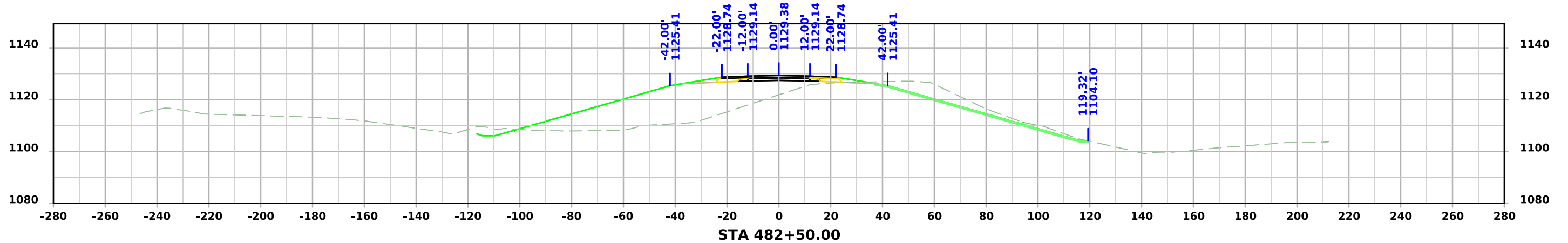
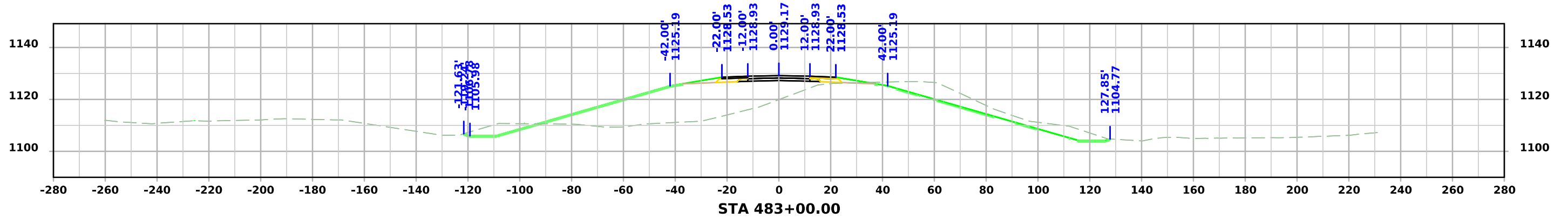
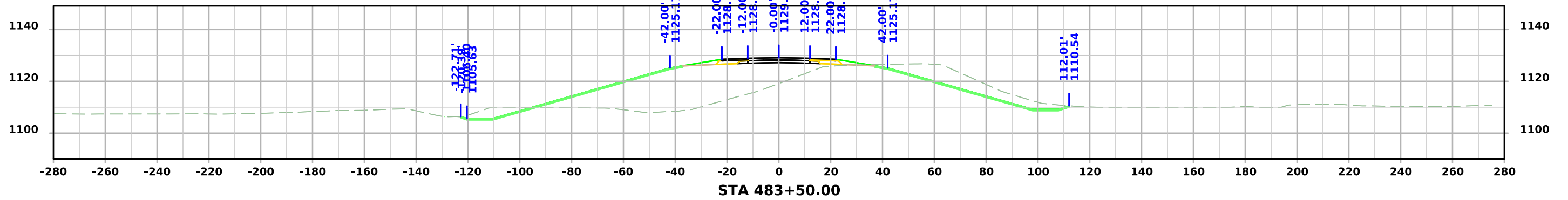
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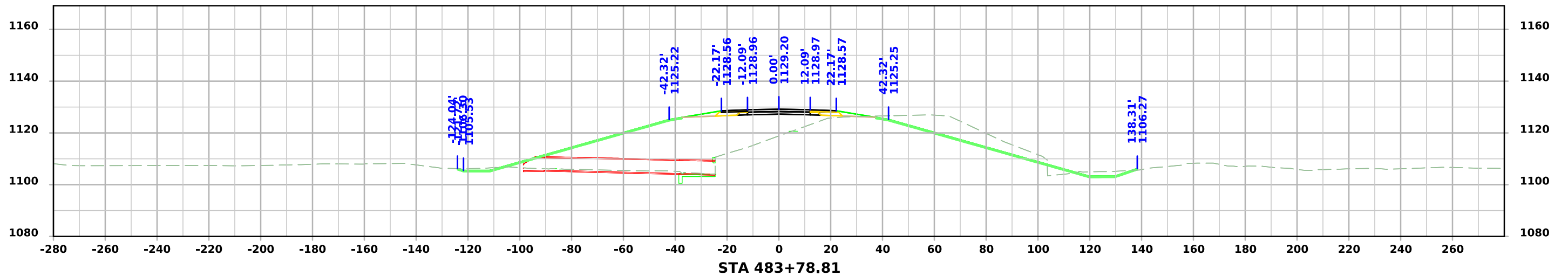
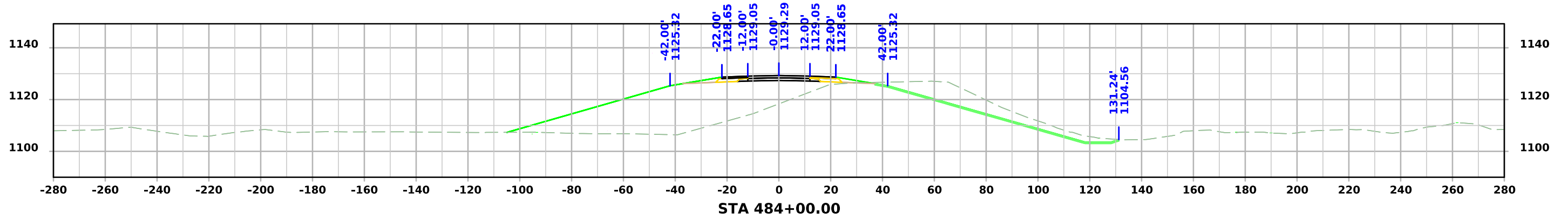
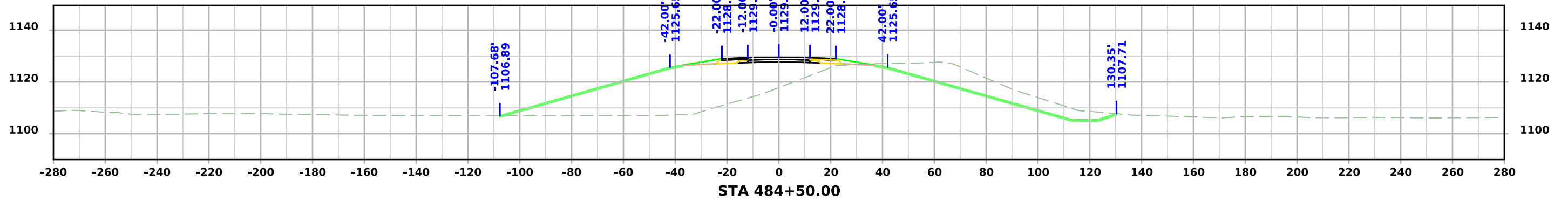
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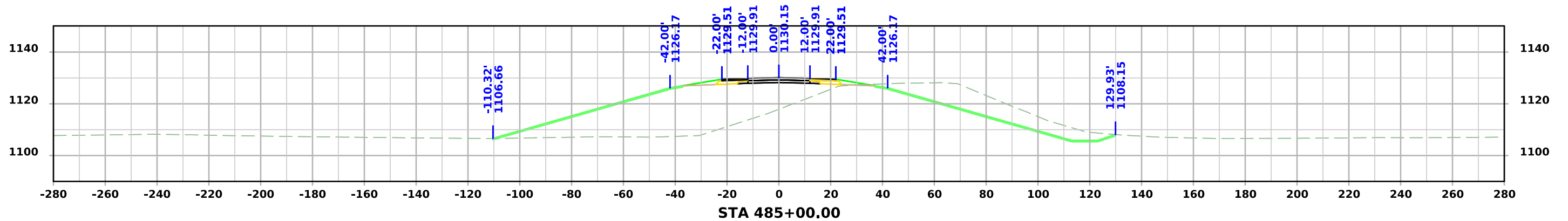
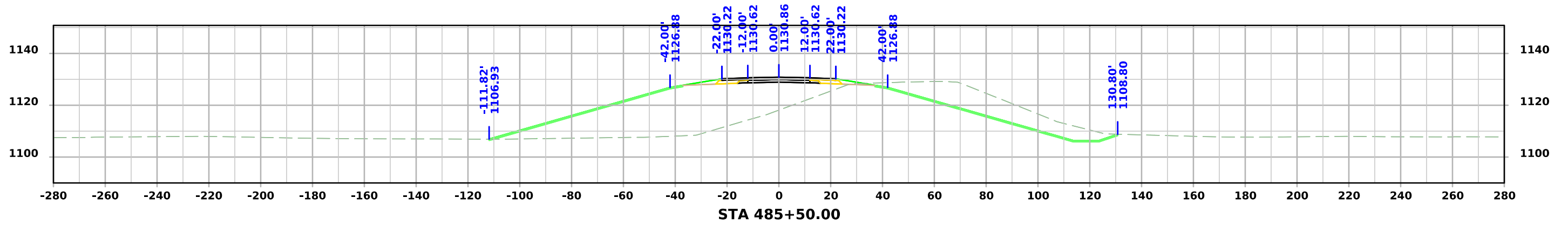
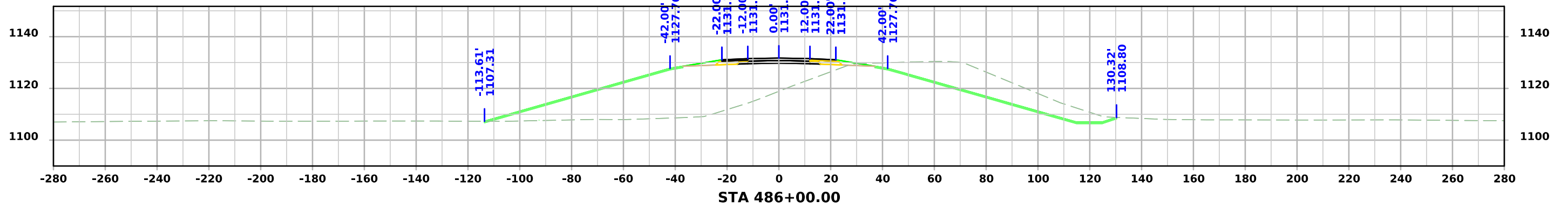
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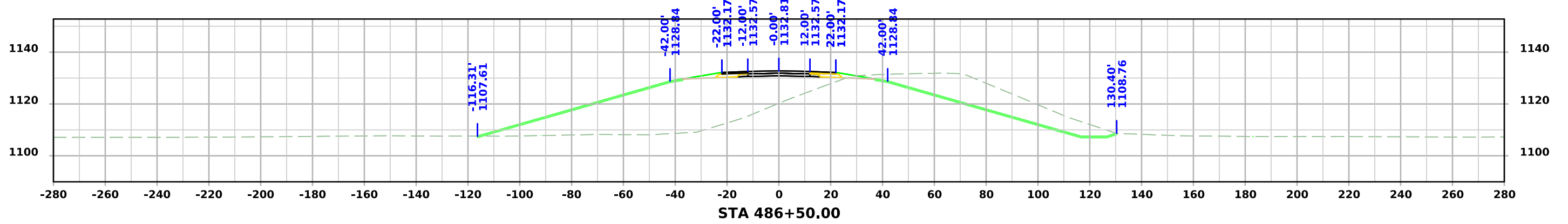
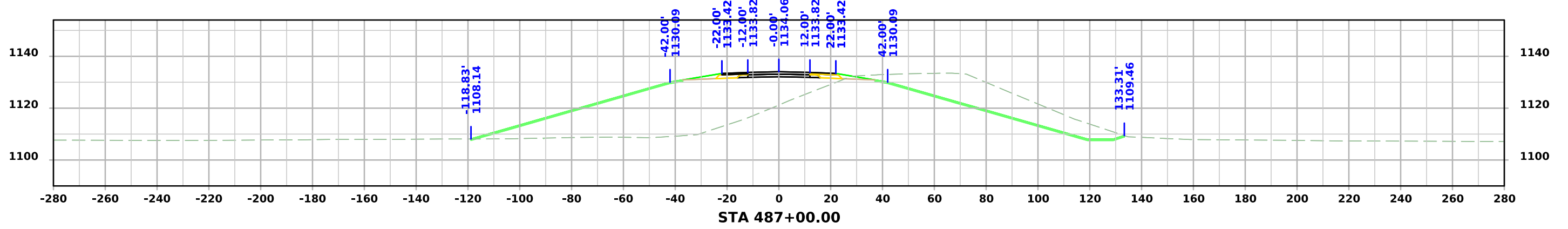
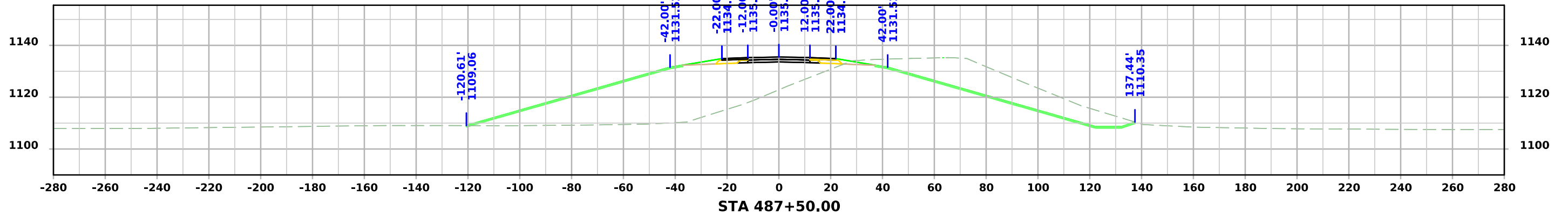
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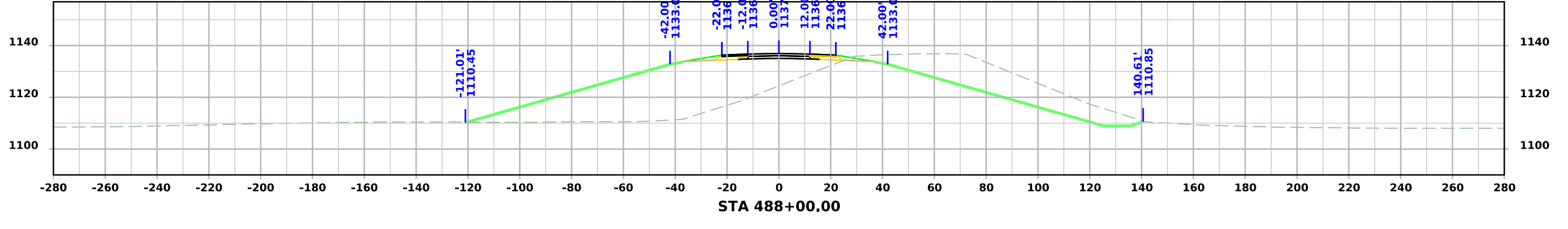
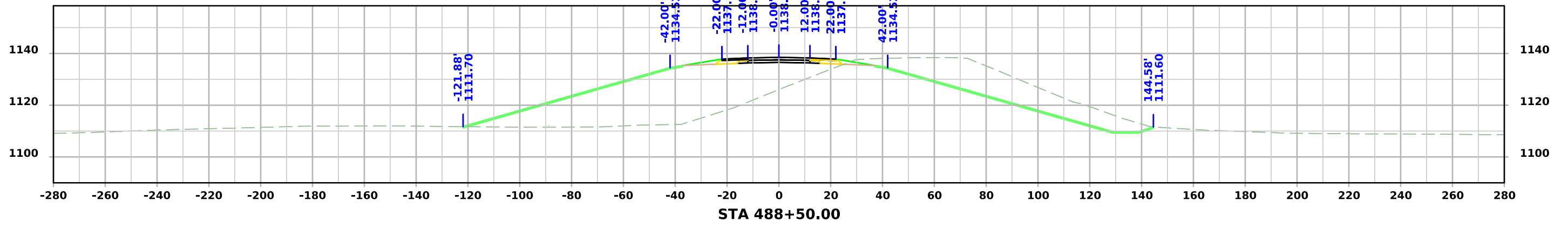
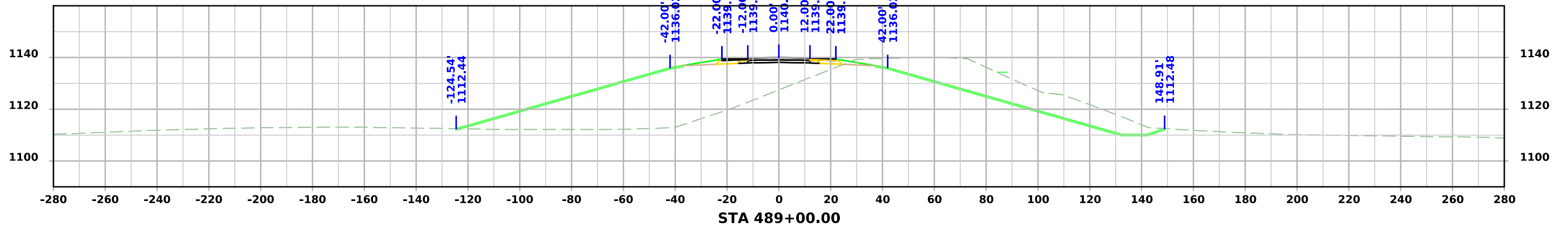
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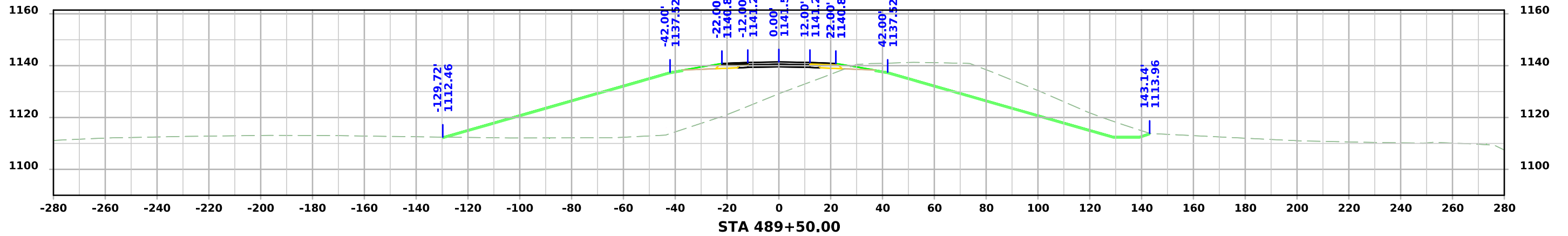
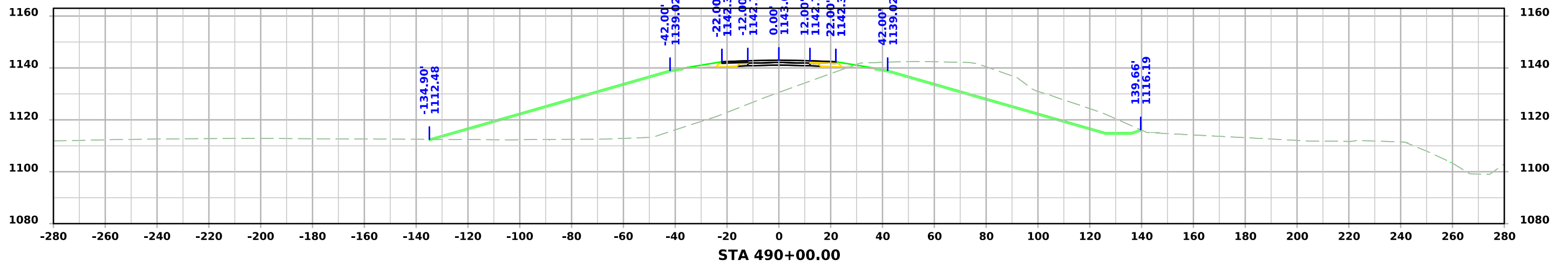
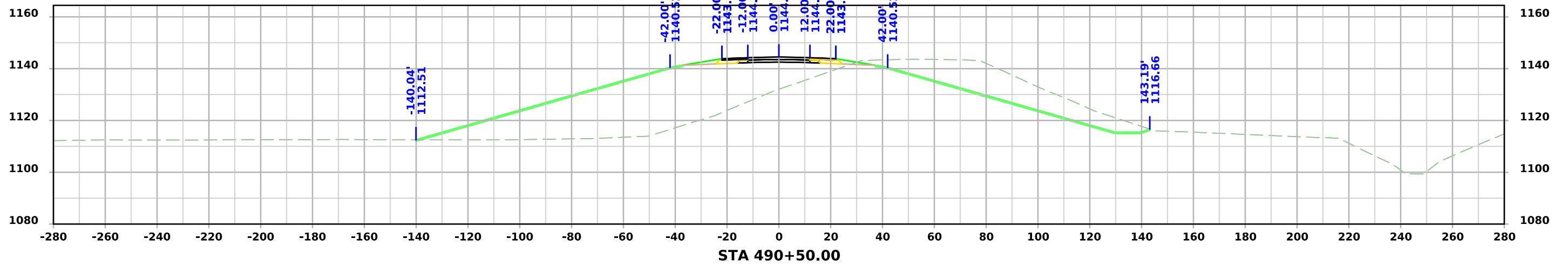
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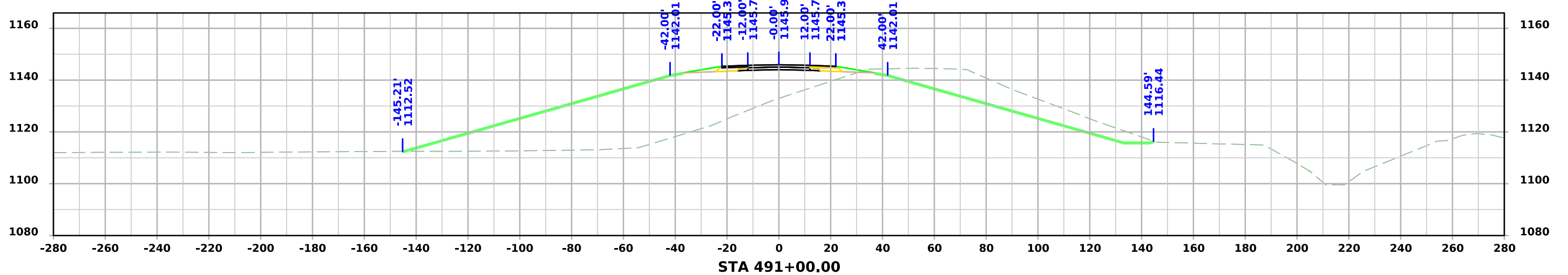
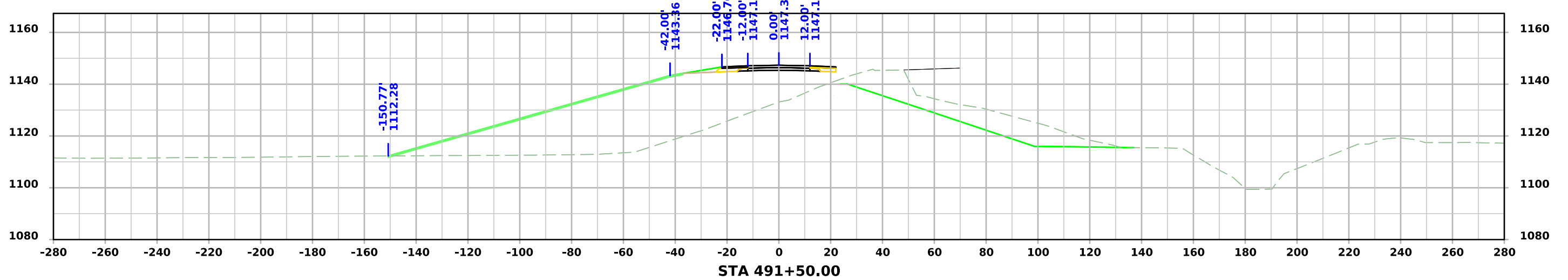
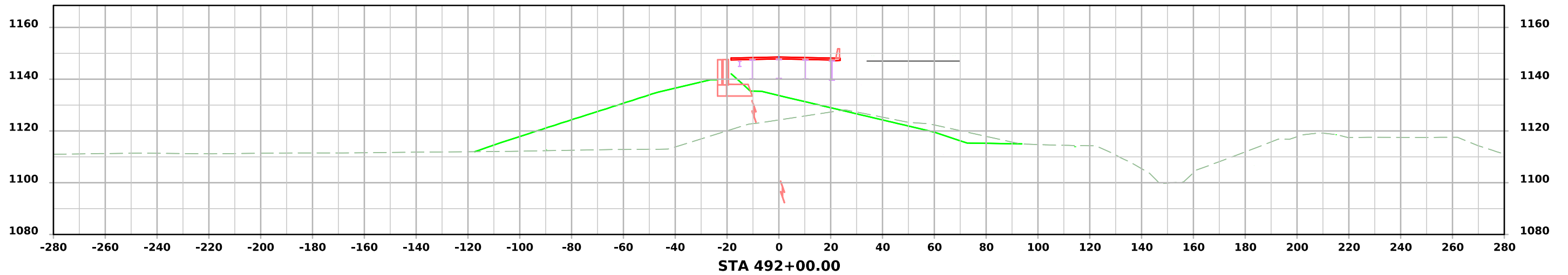
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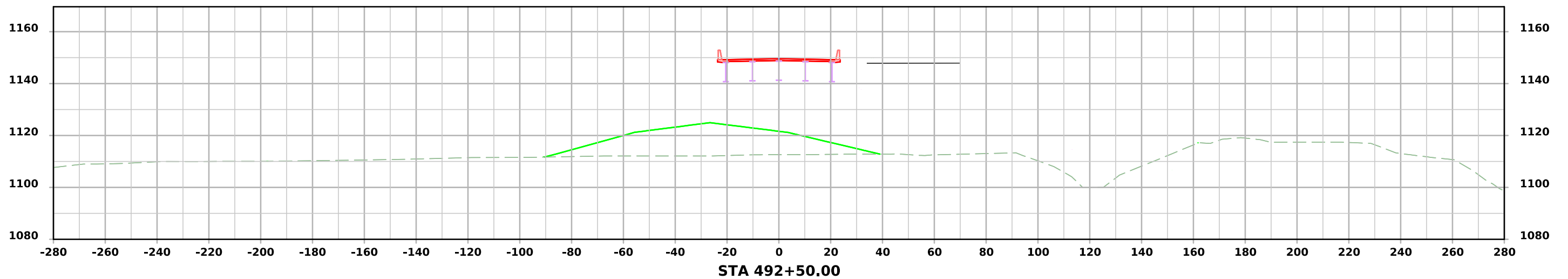
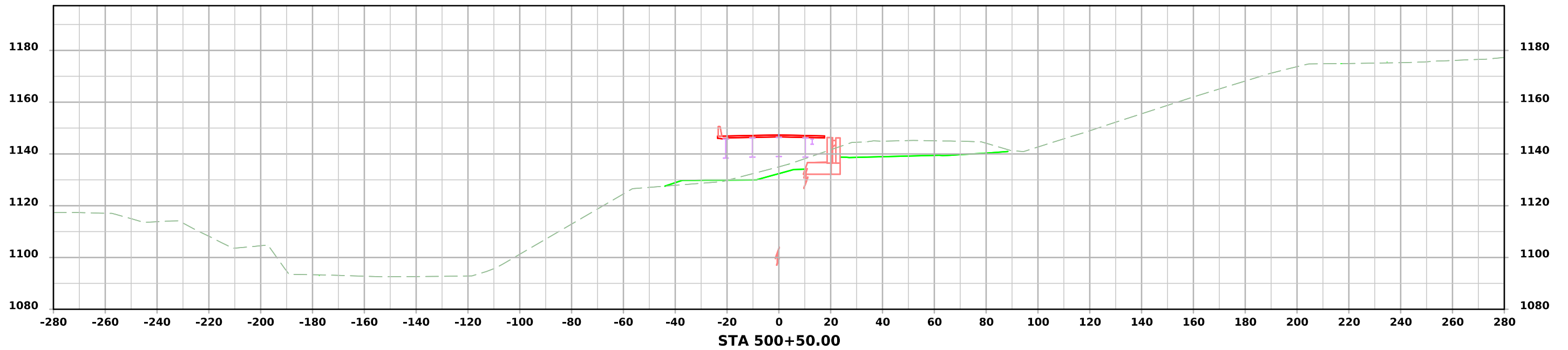
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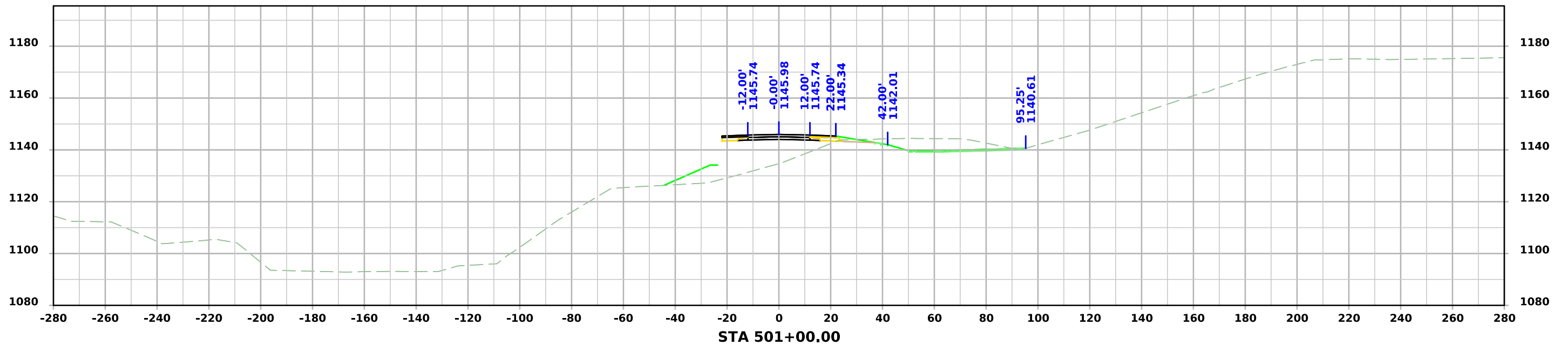
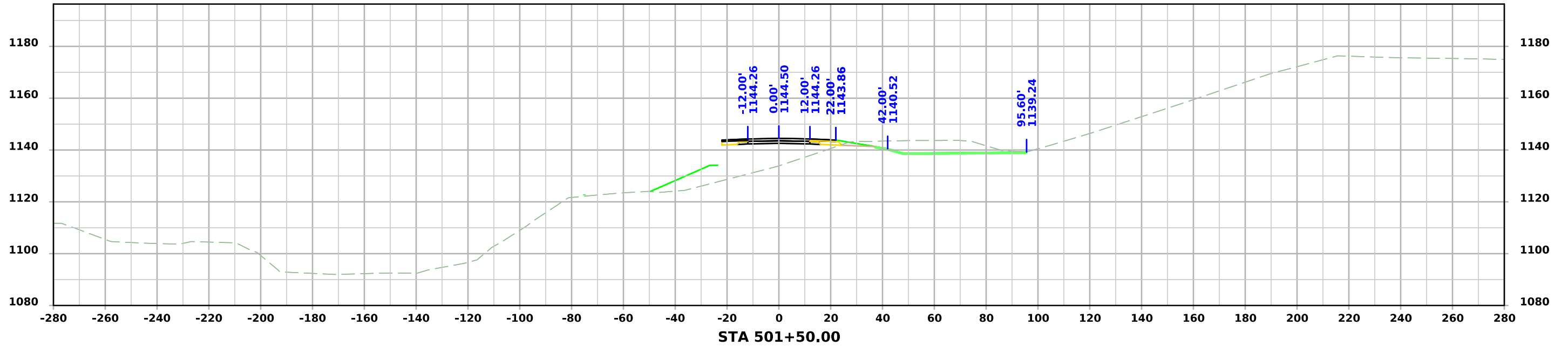
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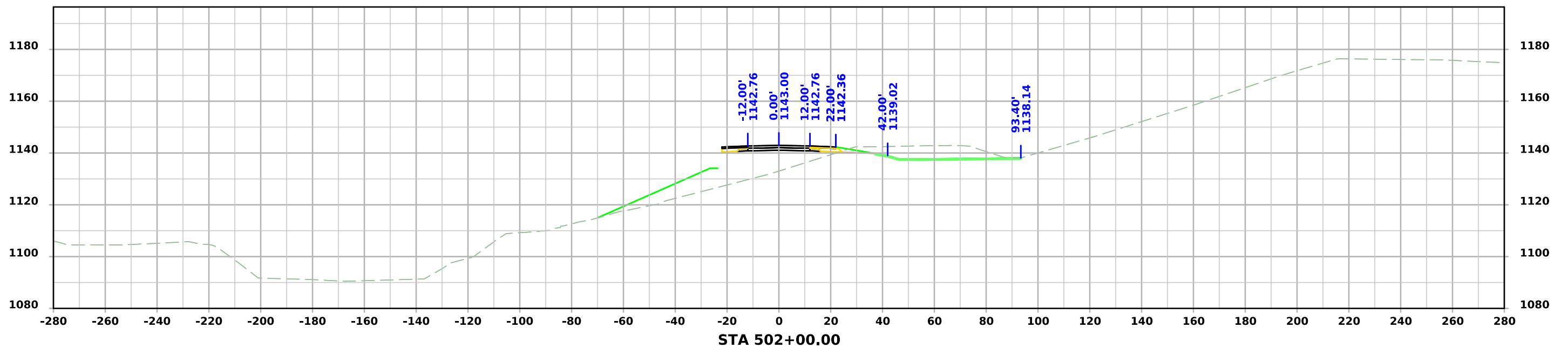
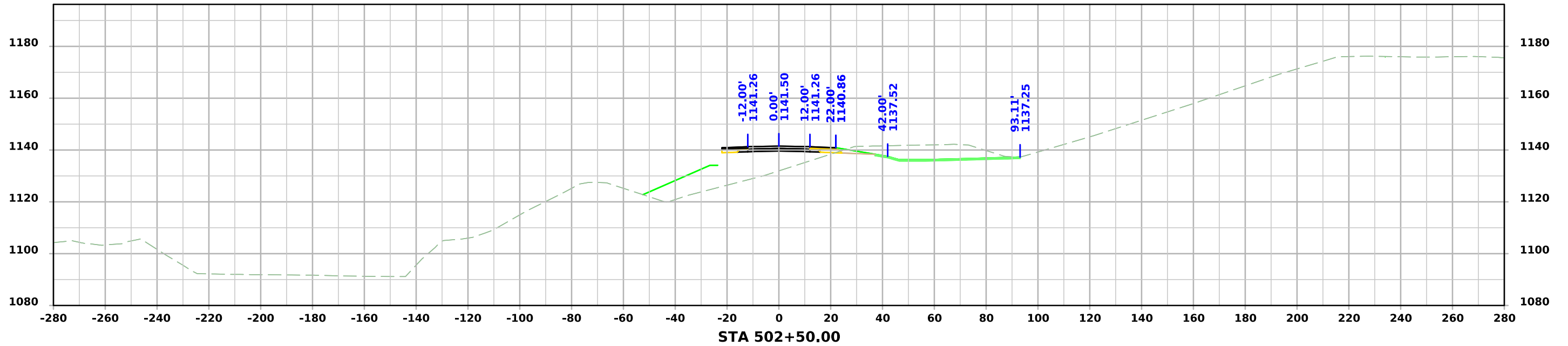
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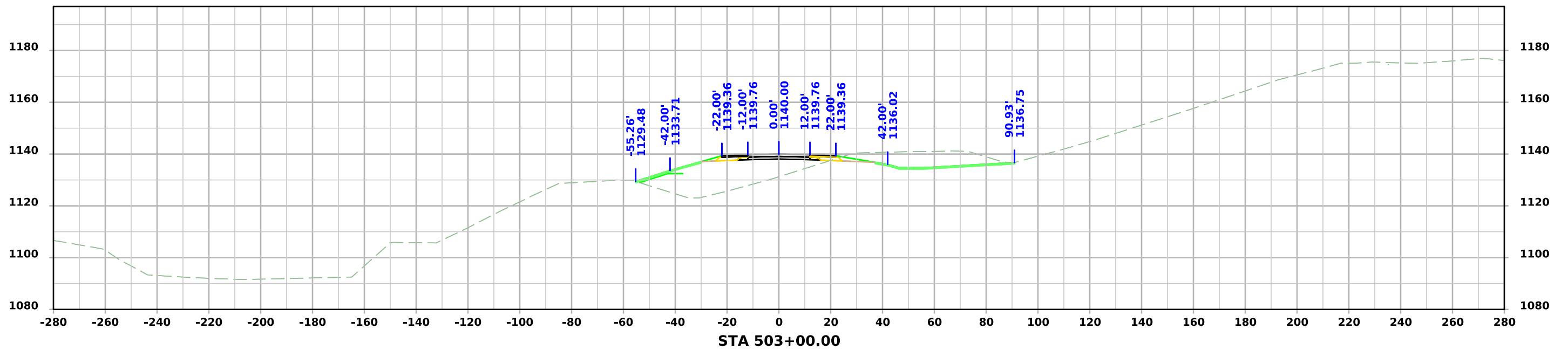
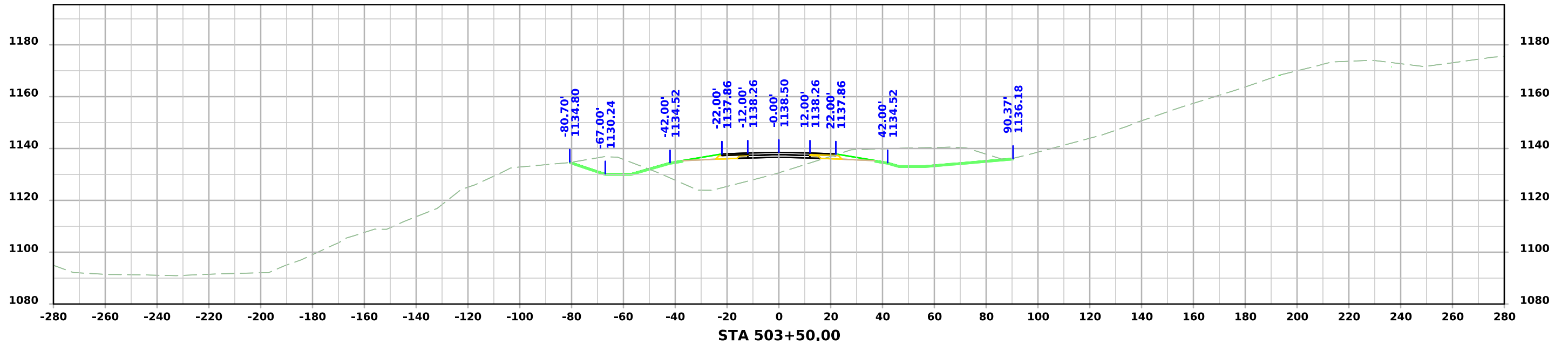
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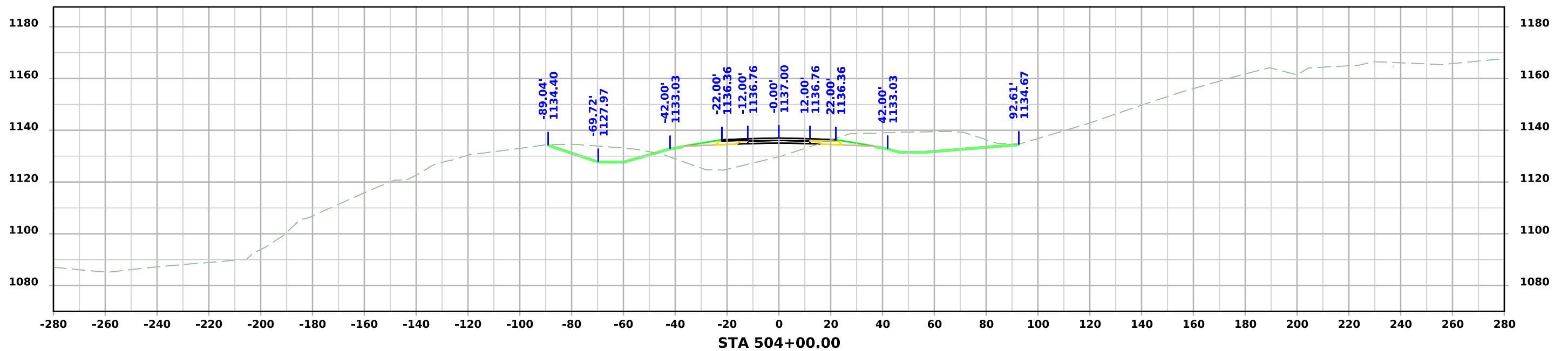
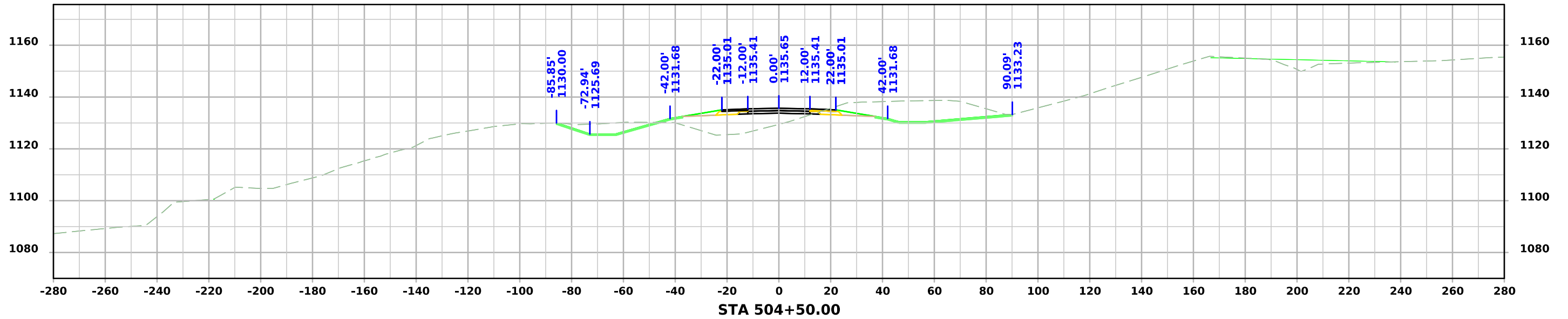
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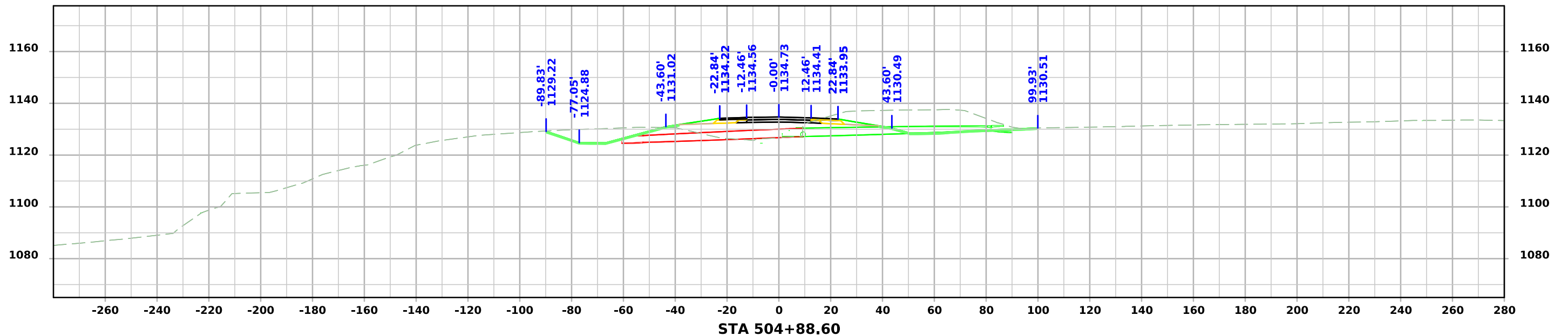
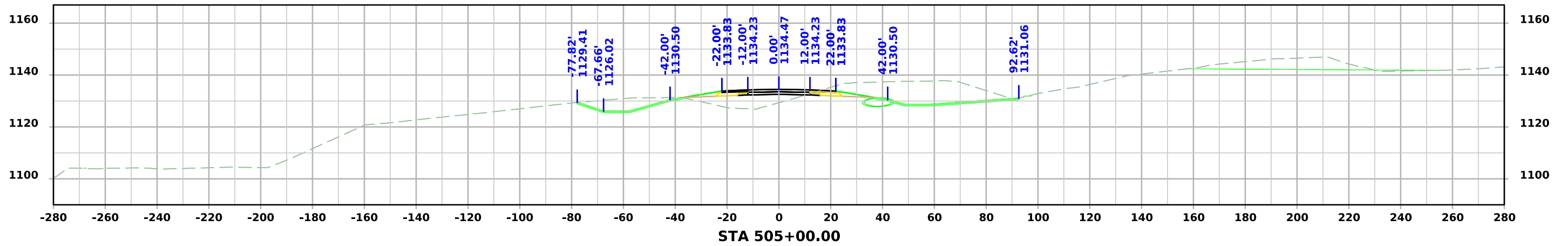
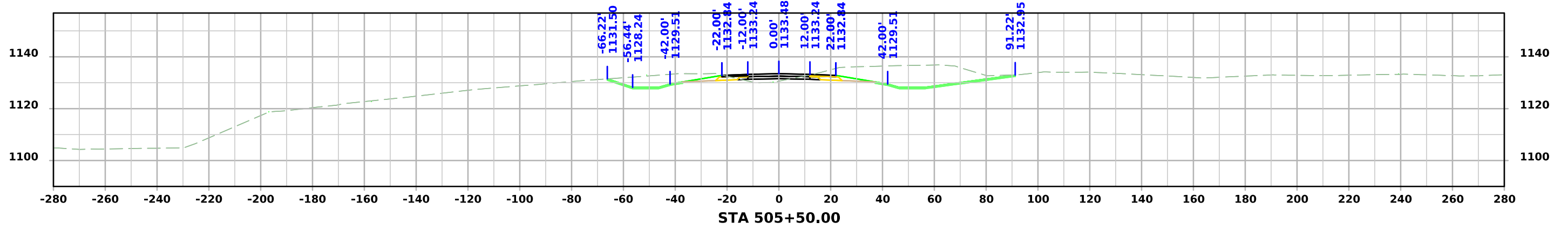
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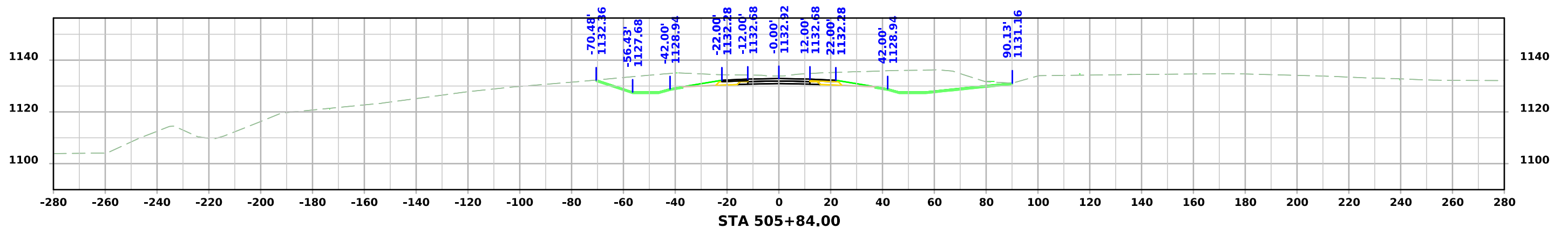
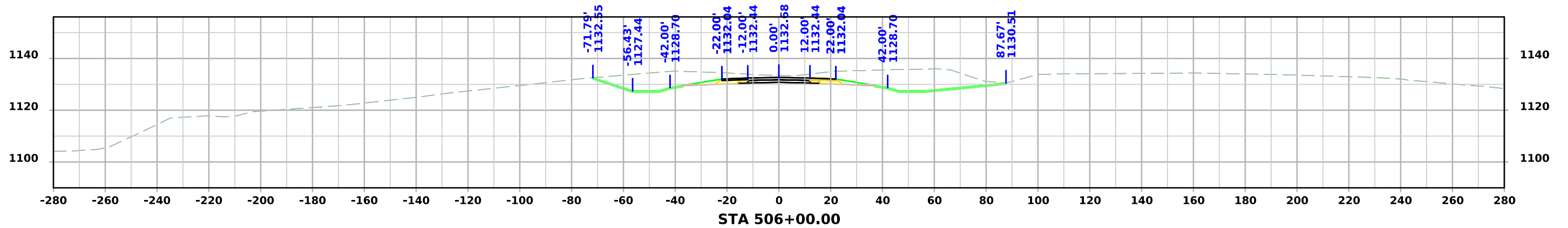
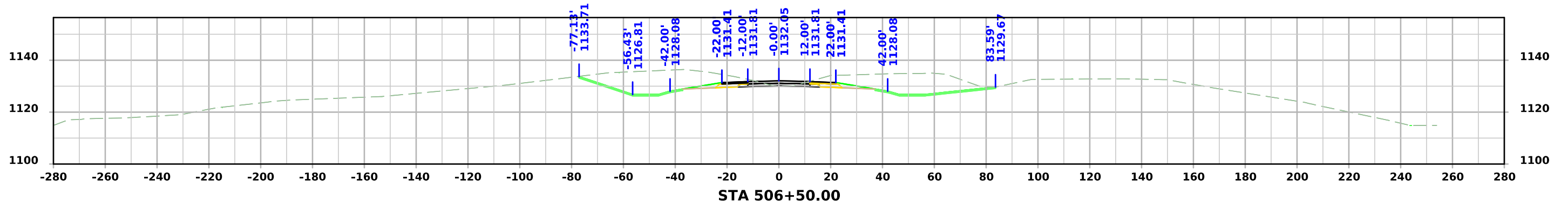
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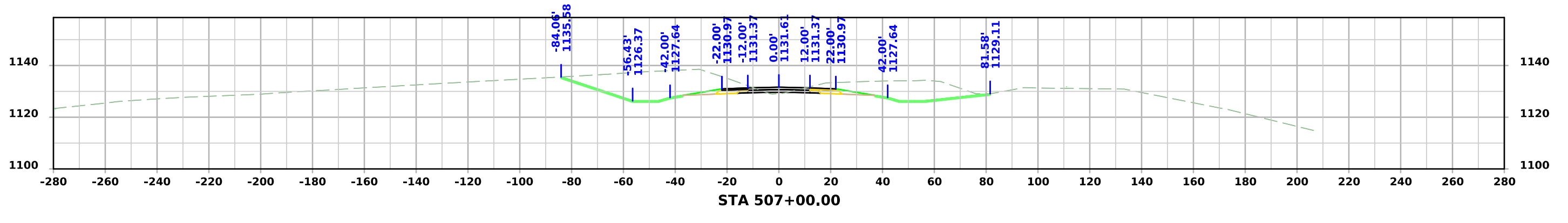
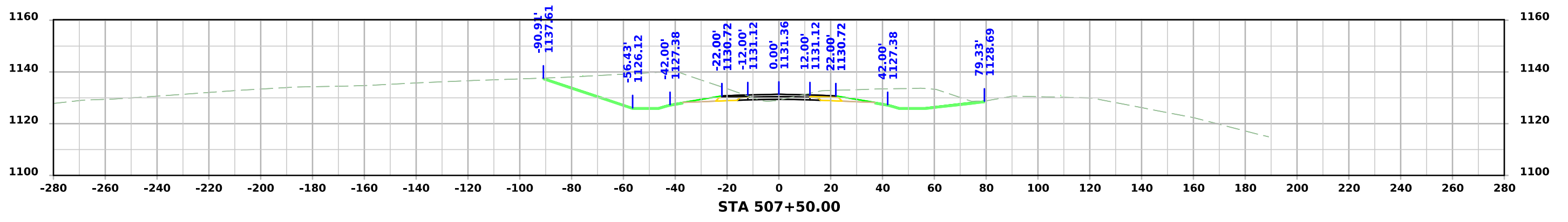
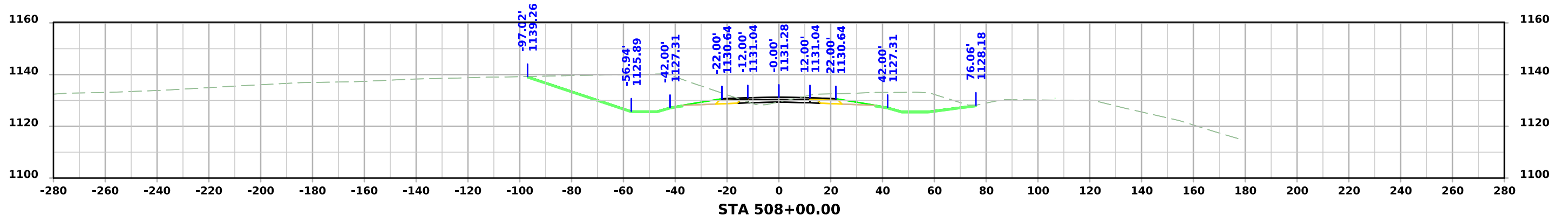
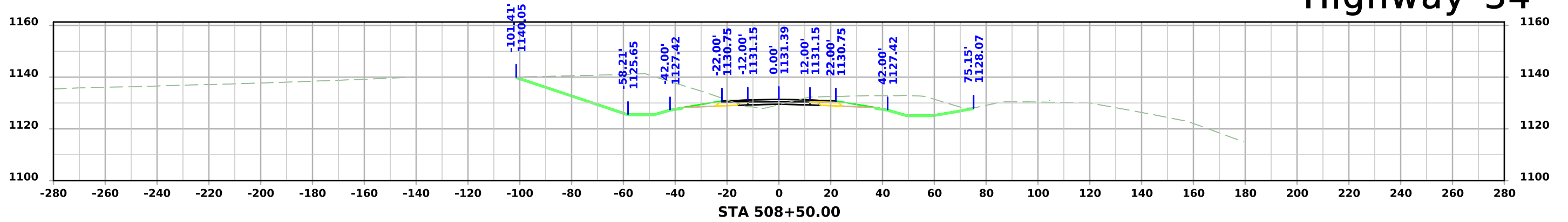
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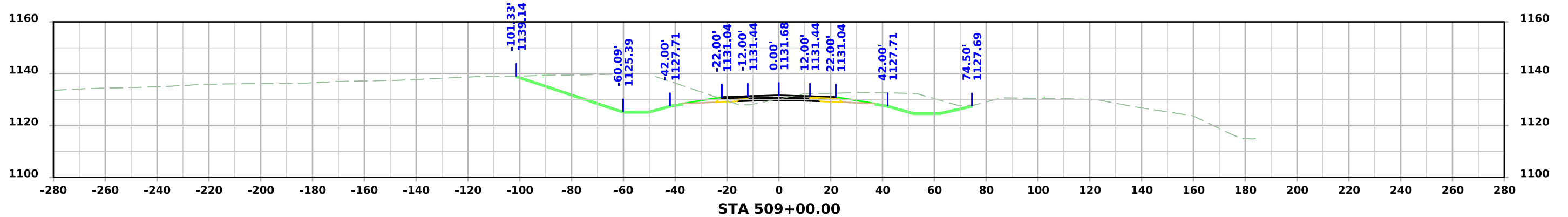
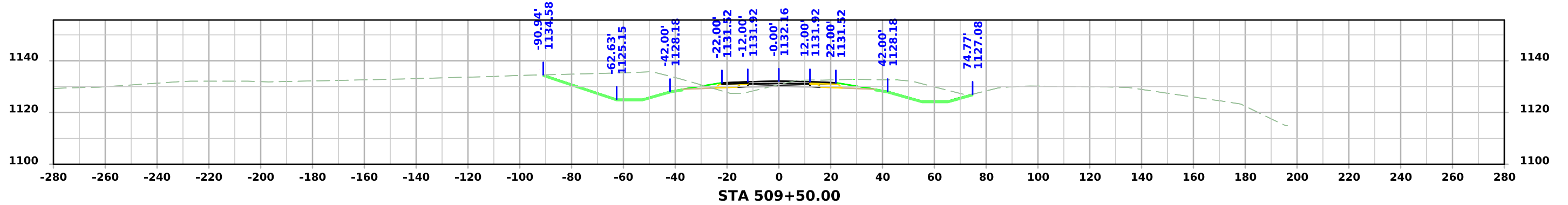
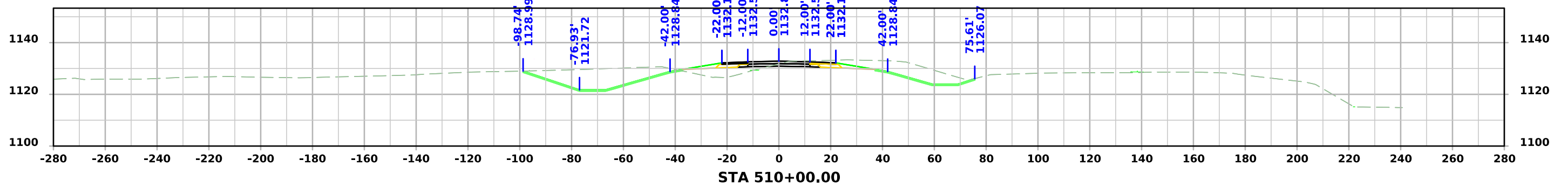
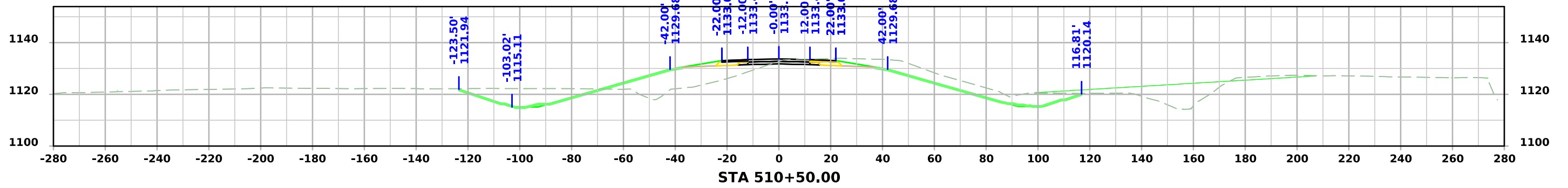
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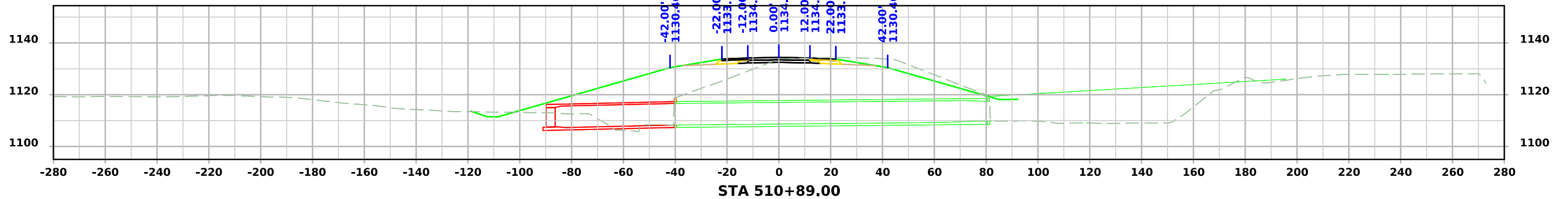
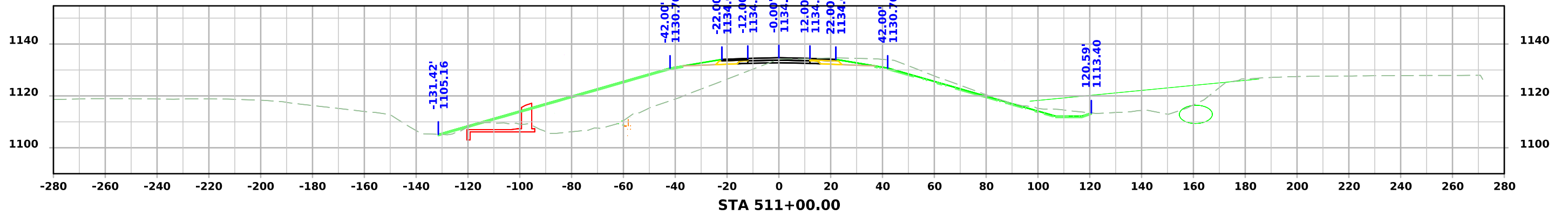
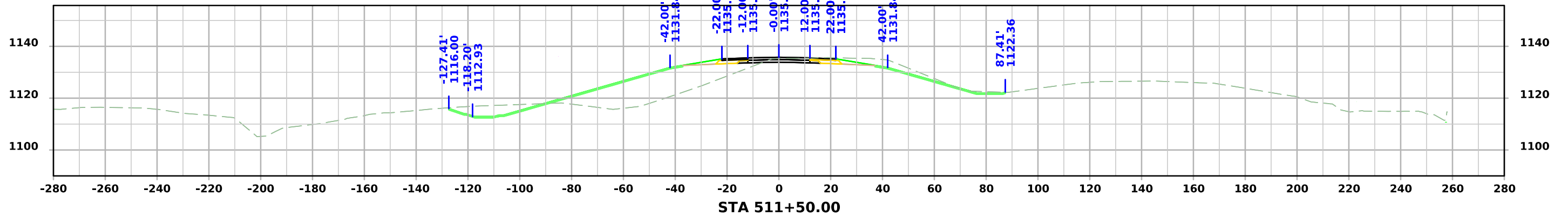
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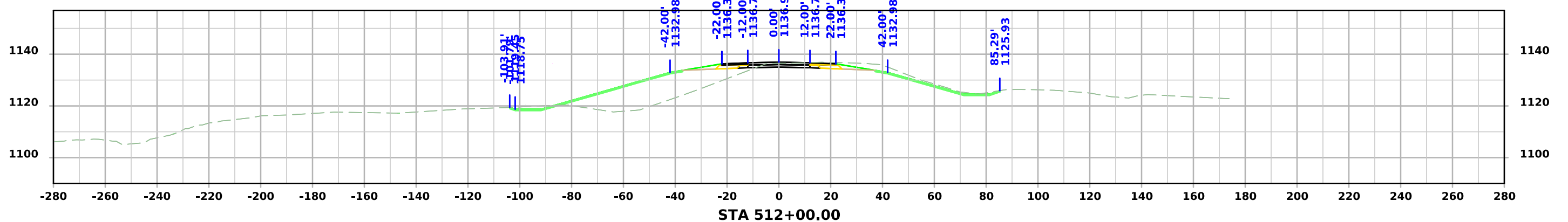
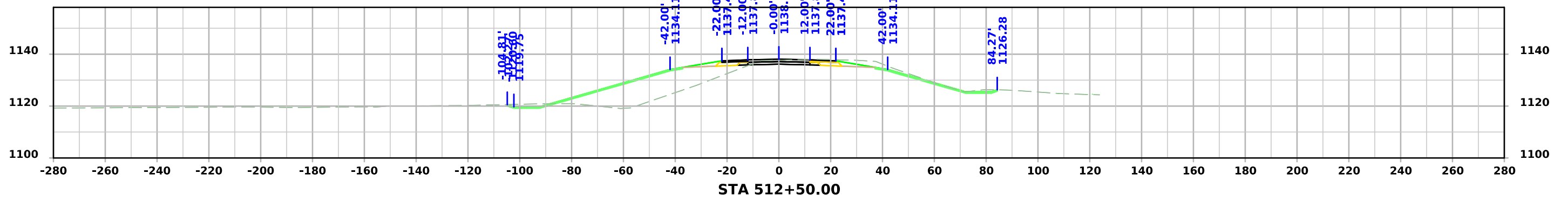
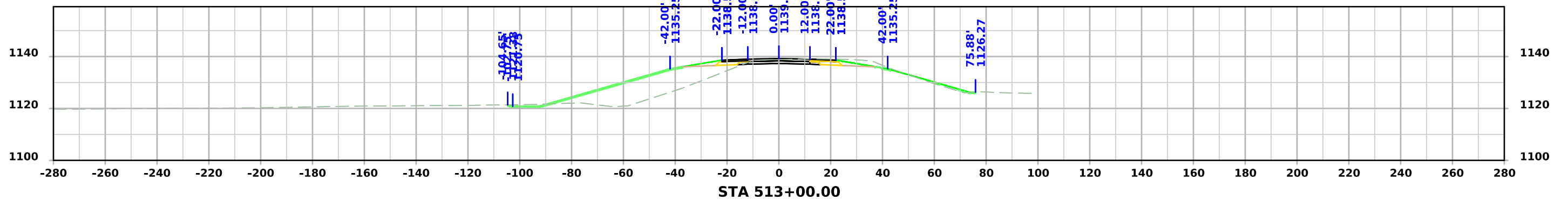
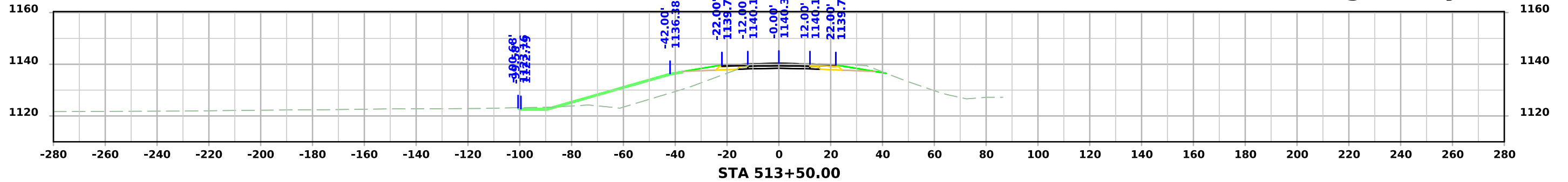
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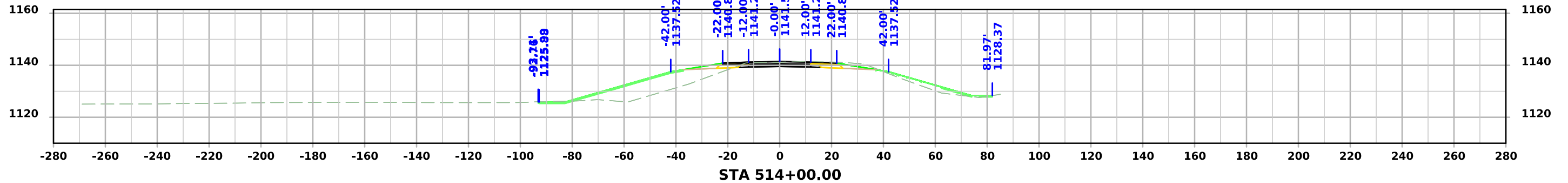
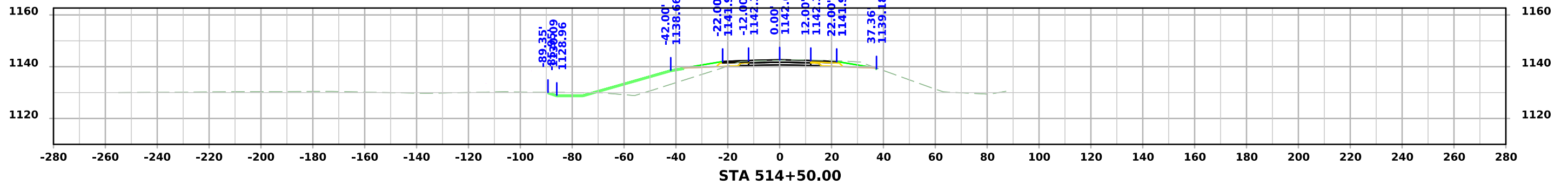
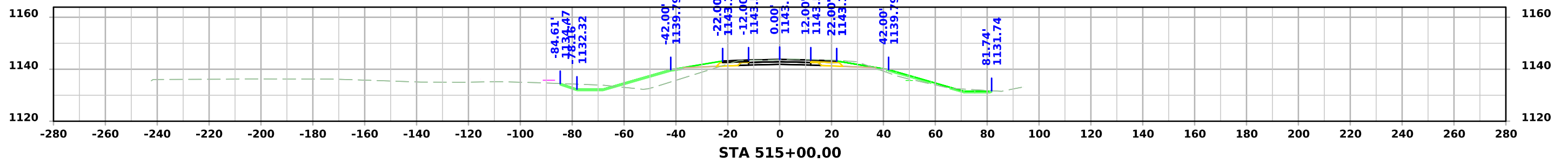
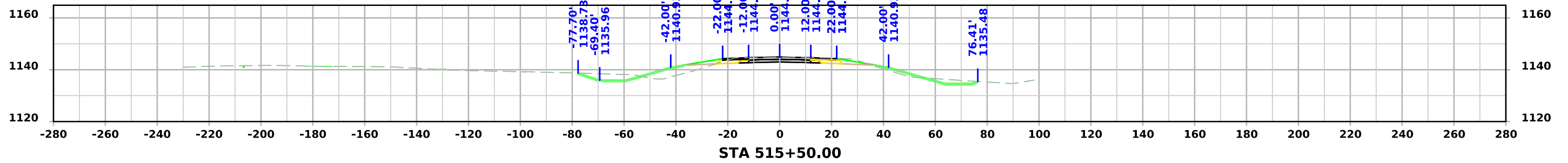
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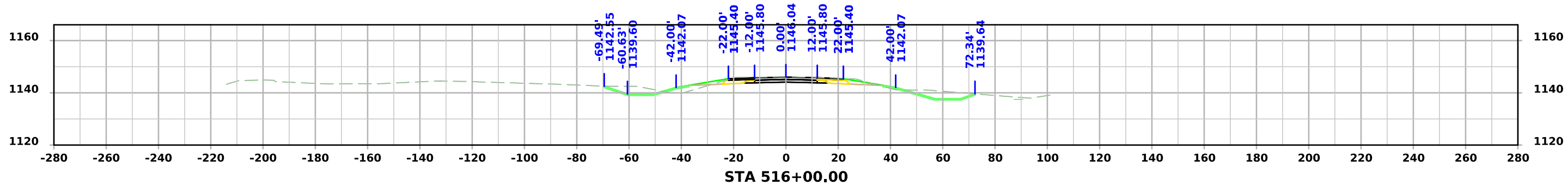
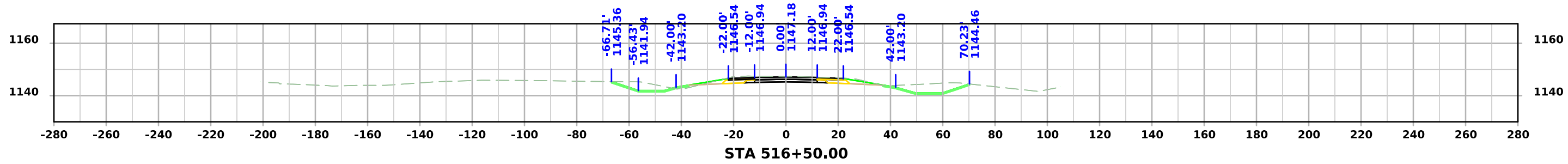
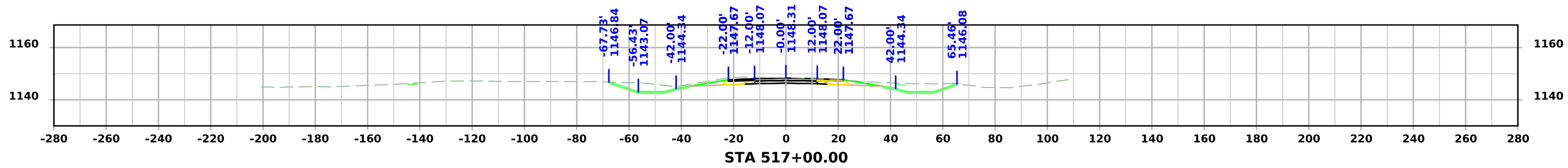
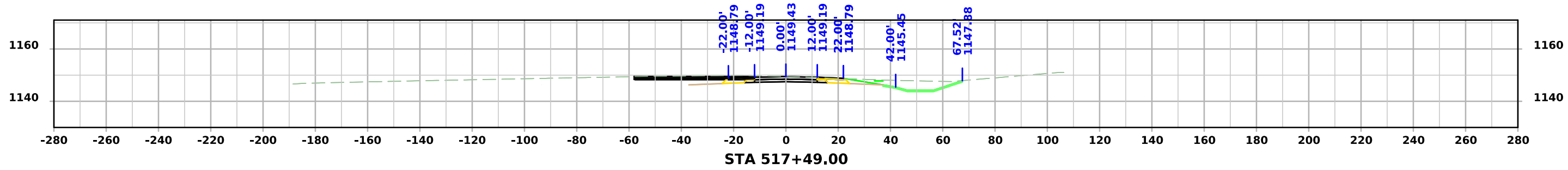
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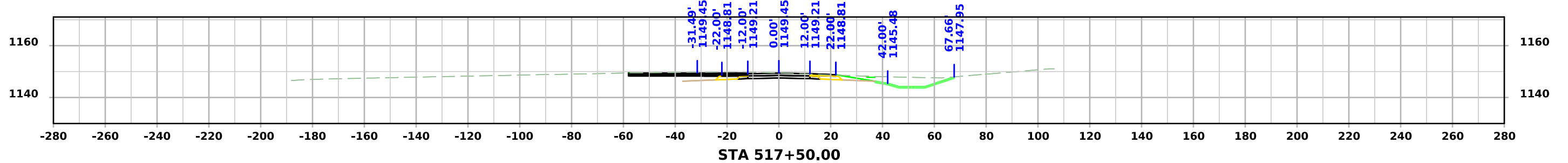
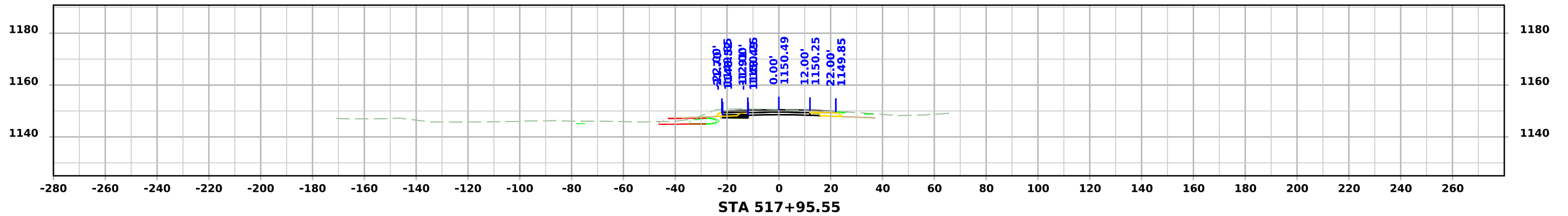
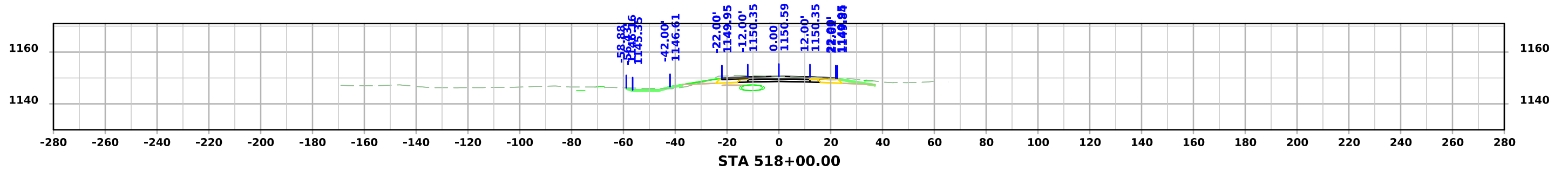
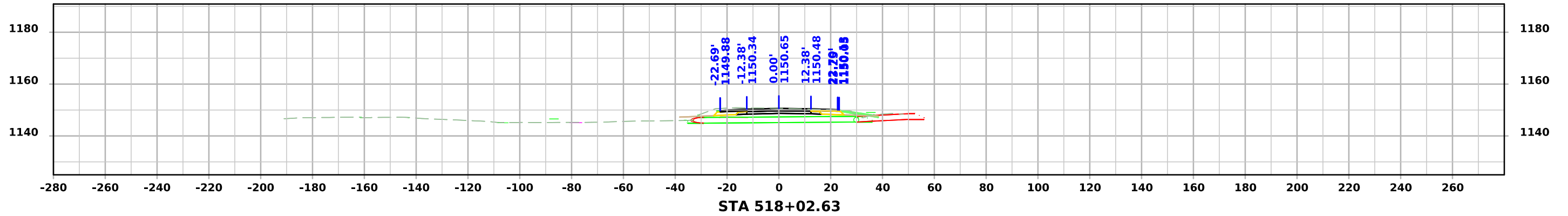
Highway 34



Highway 34



Highway 34



Highway 34

