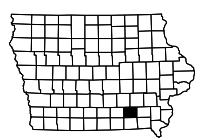


WAPELLO COUNTY

Bridge - Unspecified
BRF-149-1(096)--38-90

LETTING DATE
10/19/2027



INDEX OF SHEETS	
No.	DESCRIPTION
A Sheets	Title Sheets
* A.1	Title Sheet
* A.2	Location Map Sheet
* A.3 - 10	Concept Statement
A.11	Field Exam Notes
B Sheets	Typical Cross Sections and Details
B.1	Typical Cross Sections and Details
D Sheets	Mainline Plan and Profile Sheets
* D.1	Plan & Profile Legend & Symbol Information Sheet
* D.2	NB IA 149
* D.3	SB IA 149
F Sheets	Detour or Temporary Pavement Sheets
* F.1 - 2	Temporary Pavement Plan and Profile Sheets
G Sheets	Survey Sheets
G.1 - 3	Survey Information
G.4 - 5	Survey Alignments
G.6	Alignment Coordinates
G.7	Circular Curve Data
H Sheets	Right-of-Way Sheets
* H.1 - 2	IA 149
J Sheets	Traffic Control and Staging Sheets
J.1	Traffic Control Plan
J.2	511 Travel Restrictions
J.3	Staging Notes
J.4	Coordinated Operations
V Sheets	Bridge and Culvert Situation Plans
V.1 - 2	Dual Bridge Situation Plans
W Sheets	Mainline Cross Sections
* W.1	Cross Sections Legend & Symbol Information Sheet
* W.2 - 13	Mainline Cross Sections
	* Color Plan Sheets



PLANS OF PROPOSED IMPROVEMENT ON THE
PRIMARY ROAD SYSTEM
WAPELLO COUNTY
Bridge - Unspecified
DME RR 3.9 mi N of US 34 (NB/SB)

SCALES: As Noted

Refer to the Proposal Form for list of applicable specifications.

Value Engineering Saves. Refer to Article 1105.14 of the Specifications.



MILEAGE SUMMARY			
		105-1	
		09-27-94	
Div.	Location	Lin. Ft.	Miles
1	IA 149 Sta. 1203+89.58 to 1214+00.81	1,011.23	0.192

DESIGN DATA URBAN	
20 27	AADT 6,200 V.P.D.
20 47	AADT 6,800 V.P.D.
20 -	DHV - V.P.H.
	TRUCKS 11 %
	Total
	Design ESALs -

REVISIONS

TOTAL
45

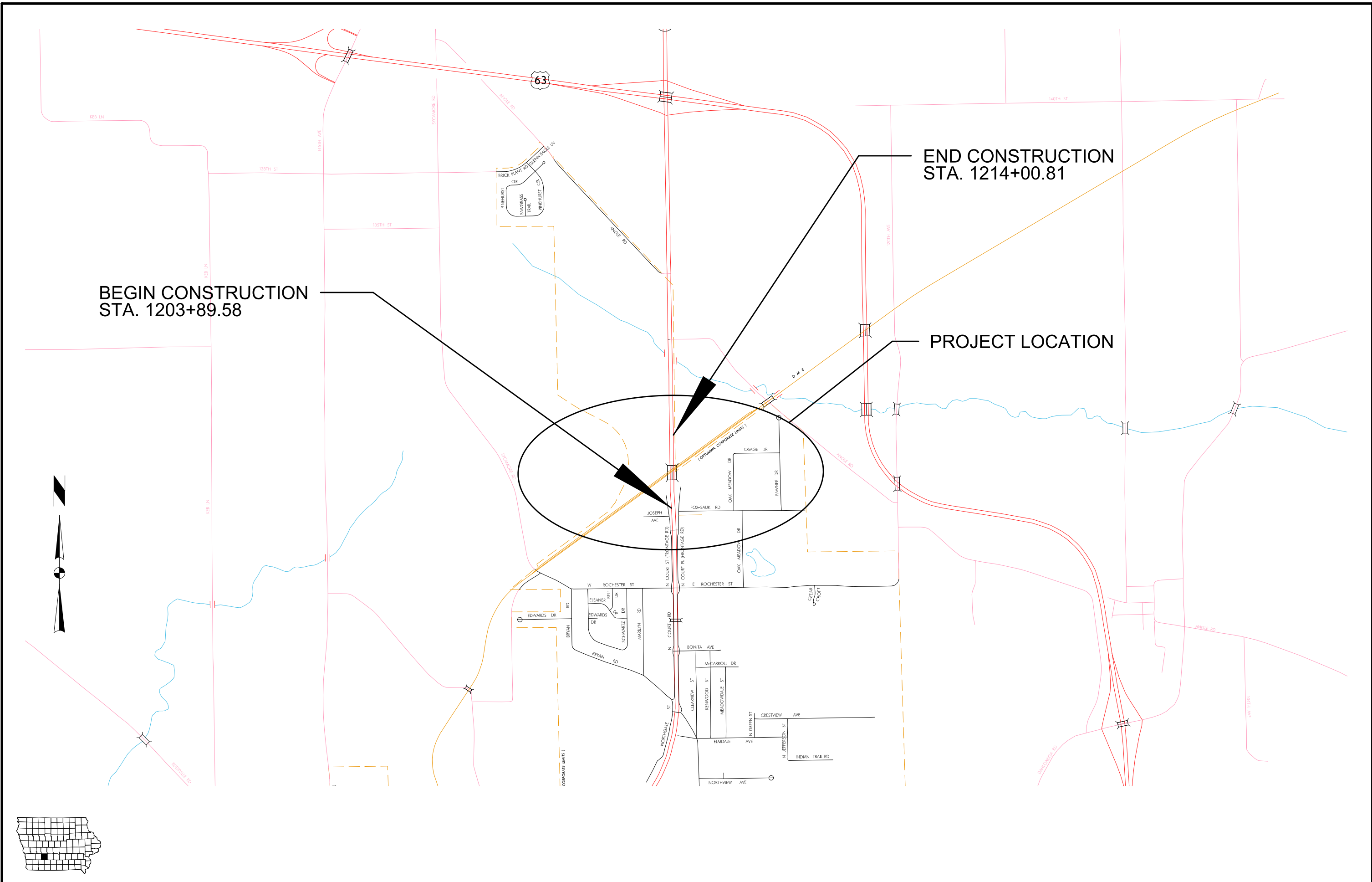
PROJECT IDENTIFICATION NUMBER
22-90-149-030
PROJECT NUMBER
BRF-149-1(096)--38-90
R.O.W. PROJECT NUMBER
STPN-149-1(097)-2J--90

- Attendees:**
- Brandy Beavers - Iowa DOT
 - Bonnie Clancy - Iowa DOT
 - Nikki Cuva - Iowa DOT
 - Jim Ellis - Iowa DOT
 - Liz Finarty - Iowa DOT
 - Jeremy Harris - Iowa DOT
 - Steve McElmeel - Iowa DOT
 - Phil Mescher - Iowa DOT
 - Kevin Patel - Iowa DOT
 - Tami Quan - Iowa DOT
 - Brian Smith - Iowa DOT
 - Hector Torres-Cacho - Iowa DOT
 - Bob Younie - Iowa DOT
 - Scott Sweet - WHKS
 - Josh Oheim - WHKS
 - Chase Holien - WHKS

PRELIMINARY PLANS

Subject to change by final design.

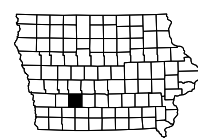
D2 PLAN - Date: 04/17/2024



BEGIN CONSTRUCTION
STA. 1203+89.58

END CONSTRUCTION
STA. 1214+00.81

PROJECT LOCATION



Analysis of these alternatives is described in the "Concept Analysis and Supporting Data" section following.

RECOMMENDATIONS:

Proceed with the Proposed Alternative. Replace existing bridges with dual 274' x 40' PPCB three-span bridges. Total estimated project cost is \$6,886,000 in current-year funds. Adjusted to FY 2028 funds, the cost estimate is \$8,212,000.

FUNDS PROGRAMMED:

This project is currently listed in the 2024-2028 Iowa Transportation Improvement Program as a bridge replacement project scheduled for FY 2028 with an estimated cost of \$8,127,000.

PROJECT IMPACTS:

Designed by: WHKS & Co. thru D5, Final Design Undetermined.

Design Impact	Assistance Requested (Y/N)	Remarks
ADA:	N	
Agreements/Notification Letters:	Y	City of Ottumwa and Wapello County
Bridges and Structures:	Y	
Consultant:	Y	WHKS & Co. thru D5
Contracts:	N	
Design/Methods:	N	
Location and Environment:	N	
Maintenance: (Shop Location)	N	Ottumwa Maintenance Garage
Project Management:	N	
Railroad:	Y	FRA # 375788T, DME / CP
RCE: (Office Name)	N	Fairfield RCE
Right of Way:	N	District 5 has assisted with confirming the RR right-of-way
Soils:	Y	Foundation Design, analysis of berm foreslope stability given raise in profile grade
Survey/Photogrammetry:	N	Survey by WHKS & Co.
Systems Planning:	N	
Traffic and Safety:	N	
Utilities:	Y	
Other:	N	

CC: S. Anderson, B. Azeltine, J. Bartholomew, H. Beach, C. Brakke, D. Biesler, K. Brink, M. Buttz, G. Cagle, M. Chambers, M. Claeys, D. Claman, B. Clancy, N. Cuva, M. Dell, B. Dolan, E. Engle, E. Gansen, J. Garton

S. Gent, R. Harris, J. Harris, J. Hart, M. Hobbs, B. Hofer, A. Karimpour, G. Karssen, M. Kennerly, J. Klein, J. Laaser-Webb, R. Larsen, B. Lauderman, N. Lind, D. Maifield, S. Majors, S. McElmeel, W. Musgrove, J. Nelson, D. Newell, K. Nicholson, T. Nicholson, S. Nielsen, M. Nop, K. Olson, M. Ortiz-Pagan, K. Patel, G. Pedersen, A. Poole, C. Poole, B. Porter, C. Purcell, K. Rauk, R. Reichter, J. Ridlen, M. Ross, M. Schmitt, M. Serio, W. Sorenson, D. Sprengeler, C. Steffensmeier, M. Swenson, B. Thede, H. Torres-Cacho, C. VanBuskirk, B. Walls, J. Weber, A. Welch, E. Wright, B. Younie, G. Zittergruen

CONCEPT ANALYSIS & SUPPORTING DATA:

Necessary supporting data may be linked in the analysis to ProjectWise.

Date of Field Review:

A combined DO/D2 field review is proposed at the time of the D2 (Field Exam) event.

Participants:

No concept review held in the field.

PAVEMENT:

Existing Conditions:

IA 149 is a dual-roadway facility. Each roadway has two 12-foot lanes, 4-foot paved outside and median shoulders separated by a 50-foot wide median (edge of travel lane to edge of travel lane, inclusive of median shoulders). A 2022 project, Wapello STP-149-1(90)- -2C-90, Mill/HMA Overlay, addressed pavement conditions on both roadways north and south of the dual bridges.

The existing SB pavement consists of a 10-7-10 PCC pavement, overlaid with 3 inches of HMA. The existing NB roadway consists of a 10-inch PCC pavement. Both roadways were later milled 0.75 inches and resurfaced with 3.5 inches of HMA. The 2022 project milled 2-inches of existing HMA, added a 1-inch interlayer course, a 1.5-inch HMA strengthening course north of the bridges only, and a 1.5-inch HMA surface course.

Pavement History [2023 Test Sections by Milepost]:

MP 003.53 to 004.21

- Original Pavement: 1927, PCC, 10" – 7" – 10" inside lanes, Linwood C. LST.
- Improvements: 1963, PCC, 10" outside Lanes, Douds, C. LST
- 1963, HMA, 3", inside lanes, 1963, Douds, C. LST
- 2004, HMA, 3.5", both roadways
- 2019, PCC Patching
- 2022, Mill 2", place 1" HMA interlayer and 1.5" HMA surface course

Pavement Design & dTIMS Recommendation:

This concept assumes a 10" PCC pavement for the new pavement. Bridge approach sections to be constructed with subbase and drainage provisions per Standard Road Plan BR-106.

Patching/Curb Repairs:

No patching anticipated.

ADA/Sidewalk/Trails:

No pedestrian or trail facilities on existing facility. No such improvements are proposed.

SAFETY:

Roadway Design Criteria:

Proposed roadway design criteria were developed consistent with Section 1C-1 of the Design Manual for urban expressways. See Exhibit C for specific criteria and project values.

Crash Analysis

The safety study period and limits for this evaluation was determined to be January 1, 2019 through December 31, 2023 (5 year period), extending from the intersection with the N. Court Road Connectors on the south and the intersection with the Memorial Lawn Cemetery access on the north, inclusive of both intersections.

There were 12 total crashes within the study period and limits, resulting in zero (0) fatal injuries, zero (0) serious injuries, four (4) minor injuries, one (1) possible injury. Four (4) crashes produced injuries, and eight (8) crashes were property-damage-only.

Nine (9) of the 12 crashes, resulting in all five (5) of the reported injuries, were located at the intersection with Joseph Avenue and Fox-Salk Road south of the proposed bridge replacements.

One south-bound (SB), property-damage-only, fixed-object crash occurred at the bridge location, involving a single-vehicle impact with the face of a guardrail. Roadway conditions were reported as dry. The crash occurred on an April 2021 Thursday between 2:00 AM and 4:00 AM. Driver was female, age 31, and was reported as being asleep/fatigued. Two other crashes involved the north bridge berm foreslopes, both animal-related, property-damage-only crashes occurring in 2019 and 2023.

The one crash at the bridge location will be addressed by this project by providing updated bridge approach guardrails and full-shoulder-width bridges.

Intersection Analysis:

The project area, including the intersection south of the bridges at the N. Court Road Connection, was addressed by a 2002 HMA resurfacing project (Wapello STP-149-1(90)- -2C-90/HSIPX-149-1(91)- -3L-90) so no further safety improvements at this intersection will be considered in this bridge replacement project.

Railroads:

The dual bridges being replaced take IA 149 over the Dakota, Minnesota, and Eastern Railroad (DME), operating on a line owned by the Canadian Pacific Railroad. FRA # 375788T.

Additional Safety & Operation Considerations:

Major City Events include:

- American Gothic Performing Arts Festival – mid-June
- Wapello County Fair – mid-June
- Elks 347 Pro Balloon Races – Mid- to late June
- Wapello County 4-H Expo – Mid-July
- Ottumwa Oktoberfest – Early October

Utilities:

Alliant Energy

- Overhead electric at the west ROW line.
- Overhead electric between IA 149 and N. Court Street, SE quadrant.
- Underground gas crossing IA 149 ~300 ft south of bridges.

Aureon Network Services

- Fiber optic east of N. Court Street – No Impact expected.

Century Link

- Multiple lines in both west and east ditches. One line crossing IA 149 diagonally under bridges.

Iowa Communications Network

- One underground fiber optic cable east ditch.

LISCO/LTDS

- One buried fiber optic cable east ditch south of bridges.
- One buried fiber optic cable crossing IA 149 ~1200 ft south of bridges.
- One aerial fiber optic cable east ditch south of bridges.

MidAmerican

- One underground gas crossing IA 149 ~300 ft south of bridges.

City of Ottumwa

- Sanitary Sewer under IA 149 NB lanes, beginning at manhole ~700 feet south of bridges and extending south.

STRUCTURES and DRAINAGE:

Bridges:

- 1) **Northbound:** Maint # 9003.9R149; FHWA #50670; 220' x 30' steel girder bridge. Built in 1963; design No. 260. Bridge Condition Index is 68.9.

The NB bridge is a 3-span continuous steel welded-girder bridges with a 35° RA skew. The abutments are stub concrete, supported on treated wooden friction piling. The piers are open two-column concrete cantilever piers, supported on steel H-piling founded on rock.

The deck is original PC concrete and was overlaid with dense low-slump concrete in 2002. Retrofit rectangular concrete bridge rails were installed in 1981. In 2008, the bridge was retrofitted by loosening diaphragm bolts to address the potential for fatigue cracking caused by out-of-plane bending.

Among the distresses noted in the 2022 In-Depth Inspection report, are: cracks and spalls in both curbs adjacent to old post connections; crack in Beam 4, Span 3, Diaphragm 3 drilled in 2019; beam end deterioration for Beams 1, 2, 3, and 4; light to severe rust for the end diaphragms over the

abutments; moderate to severe erosion below deck drains; bottom of deck has several cracks, a few hollow areas and areas of light to heavy leaching; beams have some scattered light rust and areas with blistering paint; pier bearings with loose or protruding anchor bolts; abutments have cracks, light to heavy leaching, and some patches and hollow areas; abutment bearings have rust, light to heavy debris and some bent anchor bolts.

- 2) **Southbound:** Maint # 9003.9L149; FHWA #50680, 220' x 30' steel girder bridge. Built in 1963; design No. 260. Bridge Condition Index is 60.5.

The bridge is a 3-span continuous steel welded-girder bridges with a 35° RA skew. The abutments are stub type, supported on treated wooden friction piling. The piers are open two-column concrete cantilever piers, supported on steel H-piling founded on rock.

The deck is original PC concrete and were overlaid with dense low-slump concrete in 1981 and re-overlaid in 2004. In 2008, the bridge was retrofitted by loosening diaphragm bolts to address the potential for fatigue cracking caused by out-of-plane bending.

Among the distresses noted in the 2022 In-Depth Inspection report, are: cracks, spalls, and hollow areas in both curbs, moderate erosion below deck drains; bottom of deck has numerous map and random cracks, a few hollow areas with exposed rebar and areas of light to heavy leaching; beams have some scattered light rust and areas with blistering paint, confirmed fatigue cracks with drilled holes; pier bearings with loose or protruding anchor bolts; abutments have cracks, light to heavy leaching, and some patches and hollow areas; abutment bearings have light to heavy rust.

Bridge Berm Foreslopes:

The existing bridge berm foreslopes are 3V:1H or steeper. North of the bridges, the foreslopes end in a non-ditch condition, i.e., sheet flow onto the adjacent properties. South of the bridges, the foreslopes end in defined ditches. Commercial developments adjacent to the SE and SW berm foreslopes may constrain the opportunity to flatten the slopes. Adjacent to the NE berm slope is the Memorial Lawn Cemetery where a circulating cemetery roadway and gravesites may preclude any flattening of the berm foreslope.

Culverts/Pipes: Existing 30" diameter reinforced concrete pipe culverts are located under the bridge berms, within the railroad right-of-way. An RCP culvert may be in conflict with a proposed pier. The conflict can be resolved by rerouting the RCP or performing hydraulic calculations to determine if it can be removed and the water handled with a ditch. This potential conflict will be further evaluated in Final Design.

Guardrail: Bridge approach guardrails were updated with the 2002 HMA resurfacing project. The 2022 HMA resurfacing project shielded the SE Berm foreslope (NB direction of travel, approaching the bridge) and a portion of the NW berm foreslope (SB direction of travel, approaching the bridge). These existing guardrail materials will be removed and reinstalled, as possible, with this bridge replacement project.

Drainage District:

None

PROJECT IMPACTS:

Impacts Map:

Memorial Lawn Cemetery is in the immediate NE quadrant of the project location.

Environmental:

No impacts identified during concept development.

TSMO/Traffic Control:

Posted speed limit is 55 mph. The regulatory speed drops to 45 mph approximately 850 feet south of the dual bridges. 2022 AADT is approximately 7,000 vehicles per day with 6% trucks.

Recommended is construction of the dual bridges one-at-a-time utilizing a two-lane, two-way operation (TWTLO) with mainline crossovers constructed north and south of the bridges' location. Workzone will comply with provisions of Iowa DOT Standard Road Plan TC-61 (55 mph, 50-foot median), with potential for a special crossover layout south of the dual bridges reflecting a variable width median, a reduced regulatory speed limit south of the dual bridges, and the presence of a minor intersection.

Possible detours routes (see map of routes considered in Exhibit F) were reviewed and found to be not recommended, as follows:

West Detour

A. IA 149 to 2nd Street to N Forest Avenue within Ottumwa (becoming 145th Avenue in Wapello County) to US 63 to IA 149. 2nd Street was former IA 23 (Eddyville Road), jurisdiction transferred to the local governments in 1997. 2nd Street is a two-lane facility, some portions with on-street parking. 2nd Street has an aging HMA surface for most of its length. 2nd Street passes through a downtown commercial area and transitions to residential developments. N. Forest Avenue (145th Avenue in the county) is a two-lane county road and parallels IA 149. In the rural portion, this roadway generally has an HMA surface showing some age and distress, narrow- to moderate-width granular shoulders, some horizontal and vertical geometry marked with no-passing zones. Out-of-distance travel for thru IA 149 motorists would be approximately 2.1 miles. Detour agreements with the City of Ottumwa and Wapello County would be needed. This detour option has many disadvantages and was dismissed from further consideration.

East Detour: All detour routes to the east of IA 149 involve a common section of US 63 between the IA 149 interchange and the H25 interchange.

B. IA 149 to West Woodland Avenue to N. Court Street to E Alta Vista Ave (becoming H25) to US 63 to IA 149. The county road/city street portions are two-lane roadways, portions with on-street parking, and aging HMA surfaces. Intersection geometry would need to be evaluated closely. This route would pass thru predominantly residential areas, and past Horace Mann Elementary School, the Ottumwa Cemetery, the Ottumwa Golf and Social Club, and Indian Hills Community College. Out-of-distance travel for thru IA 149 motorists would be approximately

2.6 miles. Detour agreements with the City of Ottumwa and Wapello County would be needed. This detour option has many disadvantages and was dismissed from further consideration.

- C. IA 149 to Woodland Avenue to N. Court Street to E. Pennsylvania Ave (becoming 100th Street) to US 63 to IA 149. Much of E. Pennsylvania Avenue is a three-lane with a newer PCC pavement. This route passes the Ottumwa Regional Health Center, and otherwise mixed residential and commercial developments. Out-of-distance travel for thru IA 149 motorists would be approximately 3.39 miles. Detour agreements with the City of Ottumwa and Wapello County would be needed. Geometry and pavement conditions appear the most promising of the possible local road detour routes considered.
- D. IA 149 to US 34 to US 63 to IA 149. This route has the advantage of being an all Primary Highway route, and pavement conditions and geometry are the best among the routes. This route also avoids residential and significant local developments to a large extent. Out-of-distance travel for thru IA 149 motorists would be approximately 4.58 miles. No detour agreements would be needed with local governments.

Detouring IA 149 is not recommended. Any of these detour routes would result in significant out-of-distance travel for thru IA 149 traffic, and significantly disrupt traffic operations for the large part of the City of Ottumwa that lies north of the Des Moines River.

ROW:

No ROW is anticipated for the roadway.

Agreements/Notification Letters:

Notification letters to City of Ottumwa and Wapello County. Construction agreements with DME/CP railroads.

Project Coordination

No other known projects with which to coordinate.

Previous Projects List:

- U-UG-F 159(6) – Grading/PCC Paving, 1963
- U-UG-F 159(6 – Bridges and Culverts, 1963
- Wapello STP-149-1(90)- -2C-90/HSIPX-149-1(91)- -3L-90, HMA Resurfacing with Milling/HMA Paved Shoulder, 2022.

FEASIBLE ALTERNATIVES & RECOMMENDATION:

Proposed Alternative:

Remove existing dual 215' x 30' bridges and replace with dual 274' x 40' PPCB bridges, three-span, skewed 35 degrees right-ahead, with BTB beams, frame piers, and integral abutments. Proposed span

lengths are 71' – 132' - 71'. See draft Type, Size, and Location (TS&L) drawings attached as Exhibit B1 and B2.

The profile grade will be raised ~16-inches to provide 23'-4" clearance over the existing two railroad tracks in the center span, and to account for the proposed beam depth. Proposed main span length provides greater than 25.0 ft horizontal clearance to the centerline of the nearest track so "heavy" pier construction requirements do not apply and clear spans the 100-ft- wide railroad right-of-way east of the IA 149 centerline. 2.5H:1V berm slopes (normal to the bridge) are proposed.

Roadway reconstruction limits are shown on the attached plan and profile sheets (See Exhibits D1 and D2). For the purposes of this concept, 10" PCC pavement is assumed for the non-reinforced bridge approach panels.

It is recommended to maintain as originally constructed the bridge berm foreslopes (ranging from 2.5H:1 to 3H:1V), due to the minimal rise in the profile grade, the limited pavement reconstruction needed, a lack of crash history associated with these slopes, and the many likely utility conflicts that may occur if the foreslopes were to be flattened. Existing bridge approach guardrails and foreslope-protection cable barriers, updated in 2022, will be removed and reinstalled/replaced.

Bridges will be replaced one-at-a-time utilizing Two-Lane, Two-way Operations (TLTWO) with crossovers constructed south and north of the bridge locations. See attached plan for the proposed location of the south median crossover (Exhibit E).

Alternatives Considered, but Not Proposed.

- 1) Dual bridges providing a 25-foot minimum horizontal clearance from the two existing railroad tracks with piers located within the railroad right-of-way. Alternative 1 was considered for baseline comparison purposes. This is the minimum bridge length determined by the FHWA method for full participation setting the bridge berms at the top of rail elevation 26' from centerline of nearest track (BDM 3.4.1.4). Piers are located at least 25' from centerline of nearest track. This option is not preferred due to potential for objection to construction of piers within railroad ROW.
- 2) Dual bridges clear-spanning the 100 foot railroad right-of-way, providing the minimum 23'-4" vertical clearance over a third non-existing track north of the two existing tracks. Originally, the existing bridges cleared 3 tracks. The railroad has since then removed one set of tracks. This option would not require any additional bridge length but would require a higher profile grade resulting in an estimated additional 1100 SY of pavement removal and replacement, along with undetermined additional earthwork quantities.
- 3) Dual bridges clear-spanning the 200 ft railroad right-of-way west of IA 149. Clear-spanning this 200 foot right-of-way is not feasible due to the required grade raise of 3.5' or greater. The 35 degree skew requires a minimum center span length of 250 ft. Welded plate girders would be required because prestressed concrete beams are unable to span that distance. Structure depth of 8' (min.) would be required. Per BDM 3.6.1.7, end span lengths of 75% of the center span are desired,

resulting in a bridge length of 625'. Profile runout to touchdown points would be hundreds of feet requiring a large amount of grading and pavement replacement. Roadway foreslopes are currently approximately 2H:1V and may require slope stability analysis and/or soil improvements due to the grade raise. Adjacent properties including a cemetery at the northeast corner prevent obtaining additional ROW for grading to flatten foreslopes.

Recommendation:

Proceed with the Proposed Alternative. Replace existing bridges with dual 274' x 40' PPCB three-span bridges.

Estimate:

Estimate Items Report						
Version D00-Concept Cost Estimate						
Project PRJ-54217 PHASE-1						
Item Number	Item Description	Units	Quantity	Cost Used	Suggested Cost	Line Total
Roadway Items						
2102-0425070	SPECIAL BACKFILL	TON	289.000	\$39.48	\$39.48	\$11,408.54
2102-2625000	EMBANKMENT-IN-PLACE	CY	1,000.000	\$21.48	\$21.48	\$21,478.80
2115-0100000	MODIFIED SUBBASE	CY	492.000	\$59.02	\$59.02	\$29,037.99
2121-7425020	GRANULAR SHOULDERS, TYPE B	TON	80.000	\$40.24	\$40.24	\$3,219.26
2122-5190095	PAVED SHOULDER, P.C. CONCRETE, 9.5 IN.	SY	464.000	\$85.52	\$85.52	\$39,681.28
2301-0690203	BRIDGE APPROACH, BR-203	SY	1,067.000	\$199.43	\$199.43	\$212,794.90
2301-1033095	STANDARD OR SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 9.5 IN.	SY	3,193.000	\$113.11	\$113.11	\$361,160.23
2304-0100000	DETOUR PAVEMENT	SY	4,135.000	\$34.90	\$34.90	\$144,314.39
2505-4008120	REMOVAL OF STEEL BEAM GUARDRAIL	LF	600.000	\$8.30	\$8.30	\$4,981.20
2505-4008300	STEEL BEAM GUARDRAIL	LF	200.000	\$27.76	\$27.76	\$5,551.66
2505-4008410	STEEL BEAM GUARDRAIL BARRIER TRANSITION SECTION, BA-201	EA	8.000	\$2,683.06	\$2,683.06	\$21,464.49
2505-4021010	STEEL BEAM GUARDRAIL END ANCHOR, BOLTED	EA	8.000	\$299.20	\$299.20	\$2,393.61
2505-4021720	STEEL BEAM GUARDRAIL TANGENT END TERMINAL, BA-205	EA	8.000	\$2,850.50	\$2,850.50	\$22,804.04
2505-6000111	HIGH TENSION CABLE GUARDRAIL	LF	2,000.000	\$20.00	\$0.00	\$40,000.00
2505-6000121	HIGH TENSION CABLE GUARDRAIL, END ANCHOR	EA	8.000	\$4,000.00	\$0.00	\$32,000.00
2510-6745850	REMOVAL OF PAVEMENT	SY	6,252.000	\$11.65	\$11.65	\$72,835.80
PCT-000-000	MOBILIZATION (000-000)	% of Project	1,518,705.470	10.00%	8.44%	\$151,870.55
PCT-000-030-020	TEMPORARY TRAFFIC CONTROL (000-030-020)	% of Project	1,518,705.470	2.50%	0.57%	\$37,967.64
PCT-999	UNQUANTIFIED	% of Project	1,518,705.470	20.00%		\$303,741.09
Southbound Bridge Items						
2401-6745625	RMML OF EXISTING BRIDGE	LS	1.000	\$159,000.00		\$159,000.00
PARA-020-020	Bridges	Parametric	11,094.000	\$155.00	\$135.94	\$1,719,570.00
PCT-000-000	MOBILIZATION (000-000)	% of Project	2,683,671.430	10.00%		\$268,367.14
PCT-999	UNQUANTIFIED	% of Project	2,683,671.430	20.00%		\$536,734.29
Northbound Bridge Items						
2401-6745625	RMML OF EXISTING BRIDGE	LS	1.000	\$159,000.00		\$159,000.00
PARA-020-020	Bridges	Parametric	11,094.000	\$155.00	\$135.94	\$1,719,570.00
PCT-000-000	MOBILIZATION (000-000)	% of Project	2,683,671.430	10.00%	8.44%	\$268,367.14
PCT-999	UNQUANTIFIED	% of Project	2,683,671.430	20.00%		\$536,734.29
Total:						\$6,886,048.33

The estimate is calculated in 2024 dollars. Accounting for 4.5% annual inflation from 2024 to 2028, the estimated project cost is \$8,212,000 for FY 2028.

Funds Programmed:

This project is currently listed in the 2024-2028 Iowa Transportation Improvement Program as a bridge replacement project scheduled for FY 2028 with an estimated cost of \$8,127,000.

Development Schedule:

D01 Prel. Survey	01/26/2024
D00 Concept	02/23/2024
D02 Field Exam	03/29/2024
B01 Prel. Bridge	07/26/2024
D05 ROW Plans	09/26/2024
L05 Letting	10/19/2027

D2 is behind schedule, but WHKS will get the project back on schedule for the D5 event

The posted speed limit is 55 mph over the bridges, so the design speed would be 60 mph; Refer to Sheet A.11 for more information regarding design speed

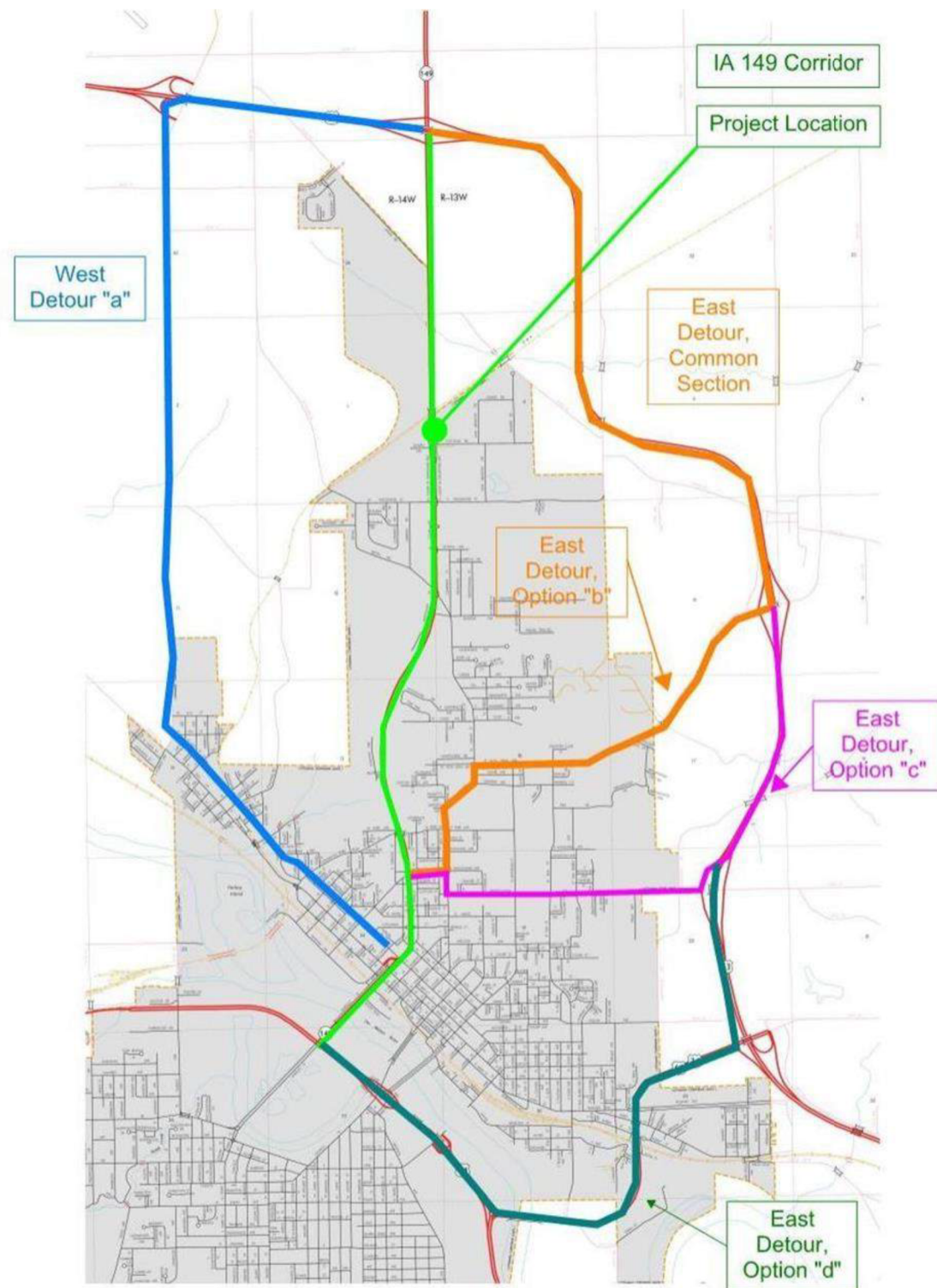
The Design Criteria was changed from the Concept. The new Design Criteria is included on the next sheet

Exhibit C: Project Design Criteria

Roadway				
IA 149 over the Dakota, Minnesota, and Eastern Railroad in Ottumwa.				
PIN Number	PIN 22-90-149-030			Submittal Date
Project Number	BRF-149-1(096)- -38-90			Approval Date
District	District 5	Assistant District Engineer		
County	WAPELLO	or		
Route	IA 149	Office Director		
Location	over the Dakota, Minnesota, and Eastern Railroad in Ottumwa. MP 3.90			
Work Type	Bridge Replacement			
Segment Manager				
Designer	WHKS & Co.			
Design Manual Section 1C-1 Last Updated: 04-29-19				
Design Element	Proposed	Acceptable Criteria	Project Values	
Design speed (mph)	The anticipated posted speed limit	55	35	
Maximum superelevation rate (Refer to Section 2A-2)	4%	8%	N/A, Tangent Section	
Design lane width (ft)	12	11	12	
Full depth paved width (ft)	Outside lane	Design lane width + curb and gutter unit or 12 feet for roadways with shoulders	Match design lane width	
	Inside lane(s)	Design lane width + curb and gutter unit. 12' for roadways without a curb and gutter unit	Match design lane width	
Right turn lane or an auxiliary lane (ft)	12	10	N/A	
Left turn lane (ft)	With raised or painted median	12 ft + median	10 ft + median	
	With depressed median	12	10	
Two-way left turn lane (ft)	14	11	N/A	
Parking lane width (ft)	10	7	N/A	
Pavement cross-slope (on tangent sections)	Through lanes	2%. However, when adjacent lanes slope in the same direction, increase slope by 0.5% per lane up to 3%	1.5% minimum, 3% maximum	
	Auxiliary and turn lanes	3%	3% maximum	
	Crown break at centerline	4%	4% maximum	
Shoulder cross-slope (on tangent sections)	Shoulders	4%	Shoulder cross-slope cannot be less than the adjacent lane, 6% max for paved or granular shoulders, 8% max for earth shoulders	
	Curb and gutter units	Match pavement cross-slope	8% maximum	
	Parking lanes	1% greater than pavement cross-slope	6% maximum	
Curb type (Refer to Section 3C-2)	Design speed ≤ 45 mph	6-inch standard	any shape	
Foreslope (For fill areas greater than 40 ft. Beyond standard ditch depth and design clear zone contact the Soils Design Section for assistance)	Adjacent to shoulder	10:1 for 4' then 8:1	3:1	
	Beyond standard ditch depth and design clear zone	3.5:1	3:1	
	Curbed roadways	2%	not steeper than 3:1	
Backslope (For cut areas greater than 25 feet, contact the Soils Design Section for assistance with backslope benches.)		3:1	2.5:1	
			2.5V:1V	
Transverse Slopes	w/ drainage structures	8:1	6:1	
	w/o drainage structures	10:1	6:1	
Ditches (Refer to Section 3G-1)	Outside ditch (depth x width) (ft)	5 x 10	-	
Median width (ft) (Refer to Section 3E-1)		See Section 3E-1	0	
Bridge width—new*	Bridge length ≤ 200 ft	design lane widths + effective shoulder widths or design lane width + 3 ft each side in curb and gutter section	design lane widths + effective shoulder widths or curb-to-curb width in curb and gutter section**	
	Bridge length > 200 ft	design lane widths + effective shoulder widths or design lane width + 3 ft each side in curb and gutter section	design lane widths + 4 ft offset each side for roadways with shoulders or curb-to-curb width in curb and gutter section**	
Bridge width—existing*		design lane widths + no less than 2 ft left and right	design lane widths + 2 ft left and right of the design widths	
Vertical clearance (ft) (above lanes, shoulders and 25 feet left and right of the center of railroad tracks)	Over primary	16.5	16	
	Over non-primary	16.5 at interchange locations, 15 at all other locations	14	
	Over railroad	23.3	23.3	
	Sign truss and pedestrian crossings	17.5	17	
Structural Capacity	Contact Office of Bridges and Structures			Contact Office of Bridges and Structures
Level of Service	C			D

Urban Multilane Roadways (Urban Arterials)

Exhibit F: Possible Detour Routes Reviewed



Field Exam Notes

After the Field Exam, WHKS evaluated profiles using design speeds of 50 mph and 60 mph:

The 50 mph vertical geometry expanded the profile by about 520'; The 60 mph vertical geometry expanded the profile by about 2700'

The existing vertical geometry correlates to a design speed of 45 mph

The 5-year crash history of this section of IA 149 includes 12 crashes, including 4 minor injuries and 1 possible injury; 9 of the 12 crashes occurred at the intersection just south of the bridges
AASHTO Green Book Section 3.2.2.5.1 pertains to this situation:

3.2.2.5.1 New Construction vs. Projects on Existing Roads

The stopping sight distance criteria in Tables 3-1 and 3-2 are appropriate for use in new construction projects where no constraints are present, since stopping sight distances that meet these criteria can typically be readily implemented. Sight distance improvements for projects on existing roads are often very costly. Recent research (35) has found little or no difference in crash experience between crest vertical curves that meet the stopping sight distance criteria in Tables 3-1 and 3-2 and those that do not, except where a design feature where drivers may need to change direction or speed is hidden from the driver's view. Therefore, in most cases, design elements at which the stopping sight distance is less than shown in Tables 3-1 and 3-2 may be left in place. However, where a roadway feature such as a horizontal curve, an intersection, a driveway, or a ramp terminal is hidden from the driver's view by the sight distance limitation or where a crash history review as part of the project development process finds a documented crash pattern that may be correctable by a sight distance improvement, improvement of stopping sight distance to the criteria presented in Tables 3-1 and 3-2 should be considered.

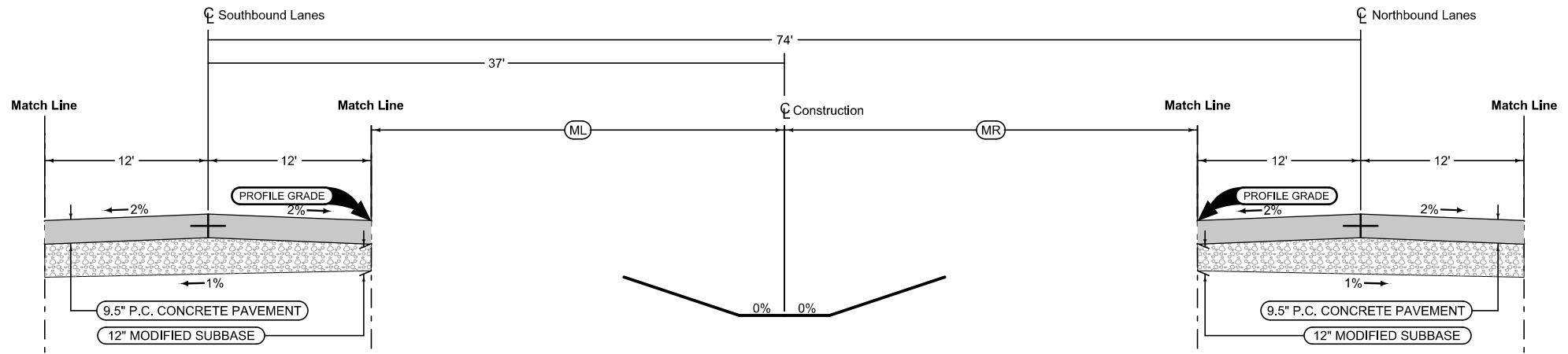
A meeting was held on 5/15, with staff from the Design Bureau, District 5 and WHKS, to discuss the design speed to be used going forward

Due to the lack of crash history and AASHTO Green Book Section 3.2.2.5.1, DOT staff decided to keep the profile shown in the D2 plans, 45 mph design speed (with minor tweaks)

The 45 mph speed limit sign will be moved from just south of the bridges to just north of the bridges with the project

A memo with more information has been uploaded to Projectwise in the following folder:

pw:\projectwise.dot.int.lan:PWMMain\Documents\Projects\9014903022\Design\Correspondence\Decisions\



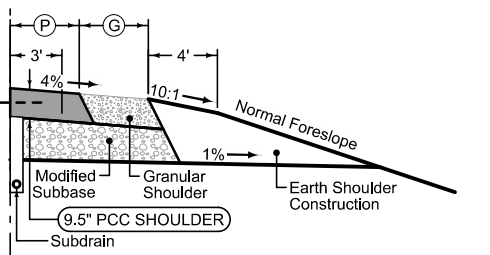
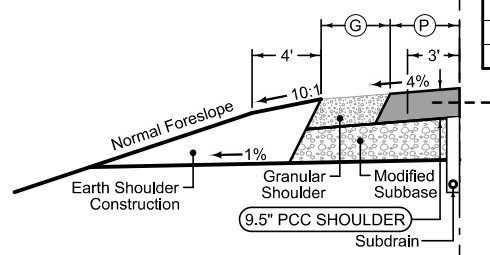
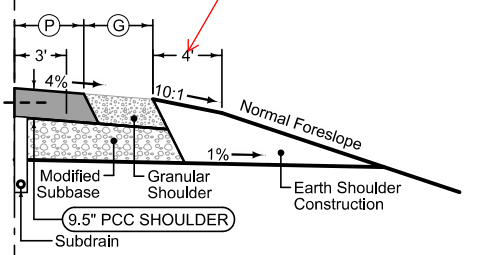
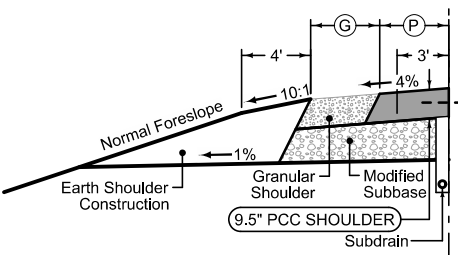
Mainline Jointing:
 Transverse joints: CD at 17' spacing
 Longitudinal joint: L-2

Mainline Jointing:
 Transverse joints: CD at 17' spacing
 Longitudinal joint: L-2

This 4' of 10:1 should be left for guardrail and cable guardrail

4DP_04-21-20			
Direction of Travel	BEGIN STATION	END STATION	(ML) Feet
SB	1204+36.20	1206+51.90	25
SB	1210+14.56	1211+66.51	25

4DP_04-21-20			
Direction of Travel	BEGIN STATION	END STATION	(MR) Feet
NB	1203+89.58	1207+03.72	25
NB	1210+66.38	1214+00.81	25



Combination Shoulder
 Shoulder Jointing:
 Longitudinal joint: BT-2, L-2 or KT-2
 Transverse joints: C at 17' spacing

Combination Shoulder
 Shoulder Jointing:
 Longitudinal joint: BT-2, L-2 or KT-2
 Transverse joints: C at 17' spacing

Combination Shoulder
 Shoulder Jointing:
 Longitudinal joint: B

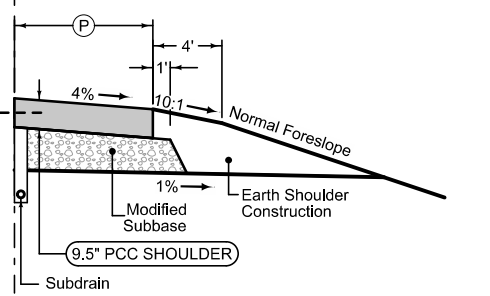
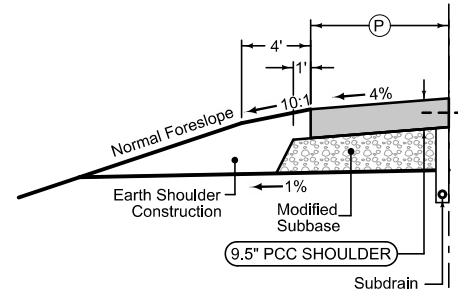
Combination Shoulder
 Shoulder Jointing:
 Longitudinal joint: B

4_C_04-21-20				
Direction of Travel	BEGIN STATION	END STATION	(P) Feet	(G) Feet
SB	1204+36.20	1205+88.82	4	4

4_C_04-21-20				
Direction of Travel	BEGIN STATION	END STATION	(P) Feet	(G) Feet
SB	1204+36.20	1206+16.97	4	4

4_C_04-21-20				
Direction of Travel	BEGIN STATION	END STATION	(P) Feet	(G) Feet
NB	1203+89.58	1206+43.46	4	4
NB	1211+01.47	1214+00.81	4	4

4_C_04-21-20				
Direction of Travel	BEGIN STATION	END STATION	(P) Feet	(G) Feet
NB	1203+89.58	1206+71.53	4	4
NB	1211+29.47	1214+00.81	4	4



Full Depth PCC Shoulder
 Shoulder Jointing:
 Longitudinal joint: BT-2, L-2 or KT-2
 Transverse joints: C at 17' spacing

Full Depth PCC Shoulder
 Shoulder Jointing:
 Longitudinal joint: BT-2, L-2 or KT-2
 Transverse joints: C at 17' spacing

4_P_FullPCC_04-20-21			
Direction of Travel	BEGIN STATION	END STATION	(P) Feet
SB	1210+46.84	1211+66.51	5.3

4_P_FullPCC_04-20-21			
Direction of Travel	BEGIN STATION	END STATION	(P) Feet
SB	1210+74.85	1211+66.51	4

IA Highway 149

SURVEY SYMBOLS

	Interstate Highway Symbol		Septic Tank
	U.S. Highway Symbol		Cistern
	Iowa Highway Symbol		L.P. Gas Tank (No Footing)
	County Road Highway Symbol		Underground Storage Tank
	Evergreen Tree		Latrine
	Deciduous Tree		Satellite TV Dish
	Fruit Tree		Water Hook Up
	Shrub (Bushes)		Radio Tower
	Timber		Tower Anchor
	Hedge		Guardrail (Beam or Cable)
	Stump		Guard Post (one or two)
	Swamp		Guard Post (over two)
	Rock Outcrop		Filler Pipe
	Broken Concrete		Gas Valve
	Revetment (Rip Rap)		Water Valve
	Cemetery		Speed Limit Sign
	Grave		Mile Marker Post
	Cave		Sign
	Sink Hole		Traffic Signal Control Box
	Board Fence		Rail Road Signal Control Box
	Chain Link or Security Fence		Telephone Switch Box
	Wire Fence		Electric Box
	Terrace		
	Earth Dam or Dike (Existing)		
	Tile Outlet		
	Edge of Water		
	Existing Drainage		
	Right of Way Rail or Lot Corner		
	Concrete Monument		
	Well		
	Windmill		
	Beehive Intake		
	Existing Intake		
	Existing Utility Access (Manhole)		
	Fire Hydrant		
	Water Hydrant (Rural)		

UTILITY LEGEND

	E1	EL1D, Alliant Energy - Quality D
	F0	FO1D, LTSD / LISCO - Quality D
	F02	FO2D, Iowa Communications Network - Quality D (From Maps)
	F03	FO3D, Qwest - Quality D
	F04	FO4D, Aureon - Quality D (From Maps)
	GHP	GH1D, MidAmerican Energy - Quality D
	G	GL1D, MidAmerican Energy - Quality D
	SAN. (C)	SA1C, City of Ottumwa - Quality C
	T1	TL1D, Century Link - Quality D (From Maps)
	TV	TV1D, Mediacom - Quality D
	W	WL1D, City of Ottumwa - Quality D
	PPA	Alliant Energy

PLAN VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

LINEWORK		Design Color No.
Green	(2)	Existing Topographic Features and Labels
Blue	(1)	Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation
Magenta	(5)	Existing Utilities
SHADING		Design Color No.
Lavender	(9)	Temporary Pavement Shading
Yellow	(4)	Proposed Pavement Shading
Orange	(6)	Proposed Granular Shading
Orange	(70)	Proposed Shoulder Granular Shading
Yellow	(68)	Proposed Shoulder Paved Full Depth Shading
Yellow	(132)	Proposed Shoulder Paved Partial Depth Shading
Gray, Dark	(112)	Proposed Grade and Pave Shading "In conjunction with a paving project"
Brown, Light	(236)	Grading Shading
Orange, Light	(134)	Proposed Granular Entrance Shading
Yellow	(220)	Proposed Paved Entrance Shading
Tan	(8)	Proposed Sidewalk Shading
Blue, Light	(230)	Proposed Sidewalk Landing Shading
Pink	(11)	Proposed Sidewalk Ramp Shading
Green, Light	(225)	Existing Pavement Shading
Red	(3)	Proposed Structure Shading
Red	(3)	Delineates Restricted Areas

PROFILE VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

LINEWORK		Design Color No.
Green	(10)	Existing Ground Line Profile
Blue	(1)	Proposed Profile and Annotation
Magenta	(5)	Existing Utilities
Blue, Light	(230)	Proposed Ditch Grades, Left
Black	(0)	Proposed Ditch Grades, Median
Rust	(14)	Proposed Ditch Grades, Right

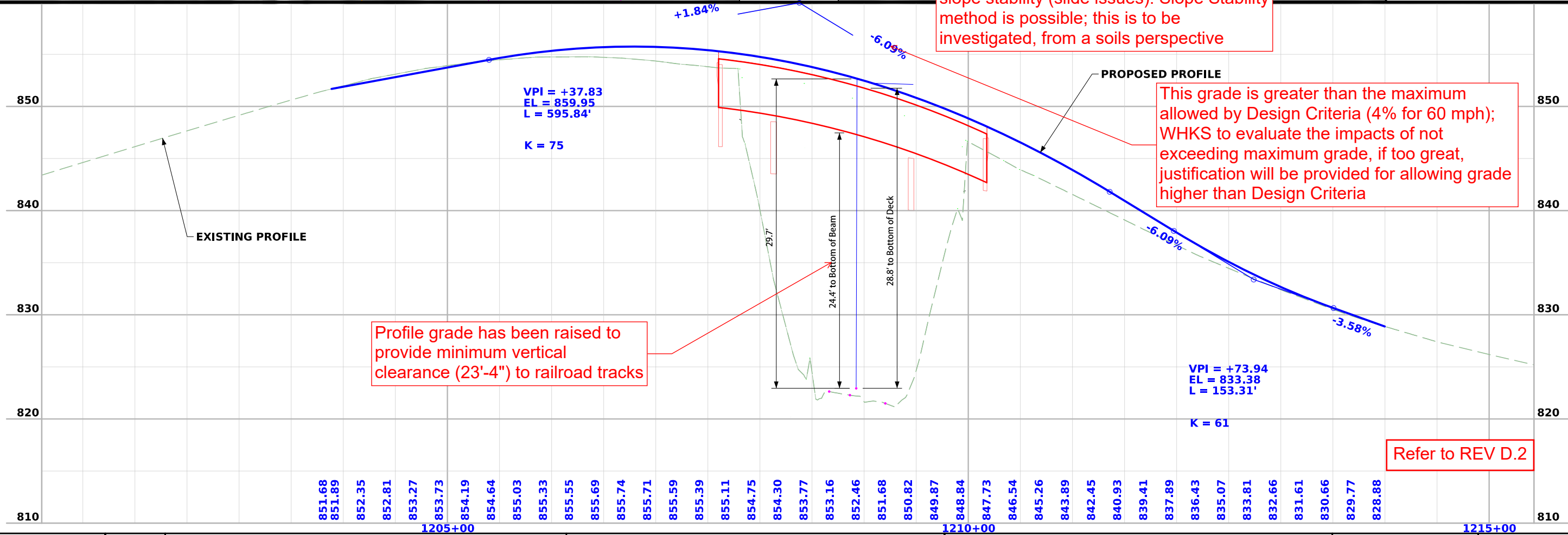
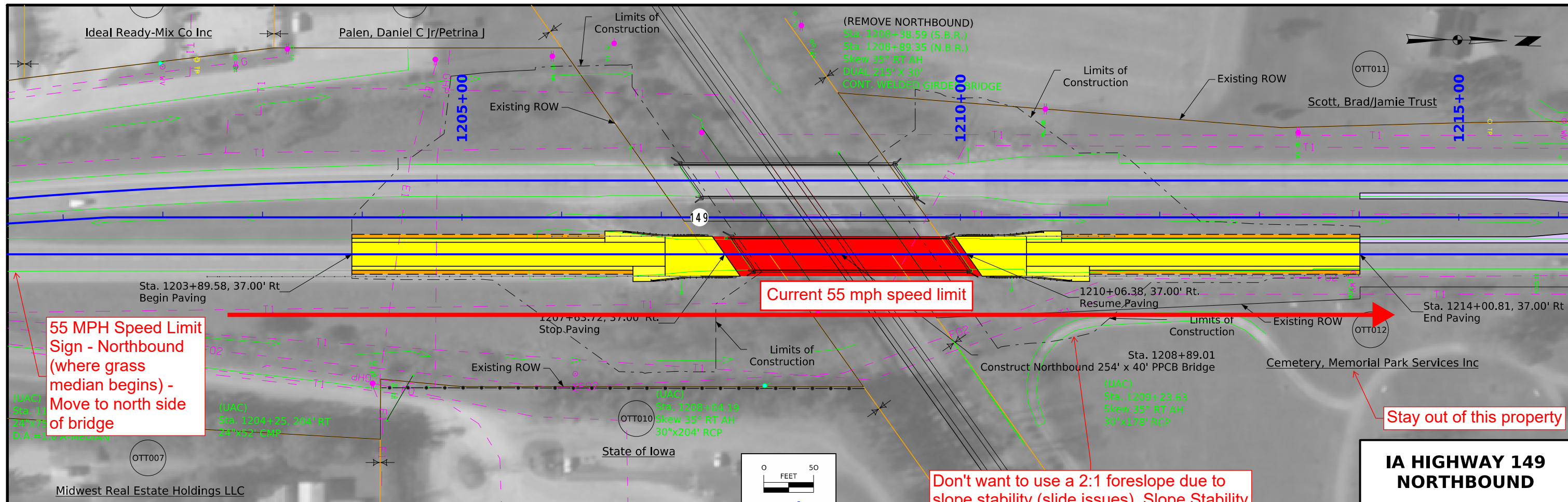
	Reference Point	Survey Line
	Station	
	Section Corner	
	Ground Line Intercept	
	Saw Cut	
	Guardrail	
	Trench Drain	
	HighTension Cable Guardrail	
	Sheet Pile	
	Pavement Removal	Clearing & Grubbing Area

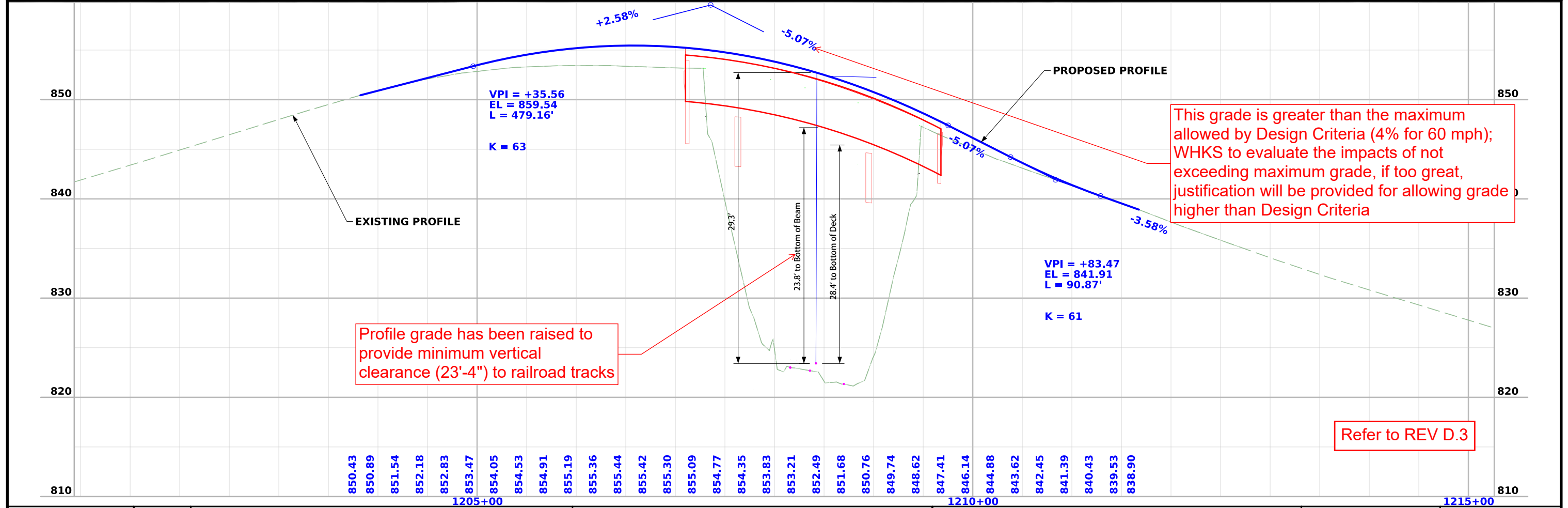
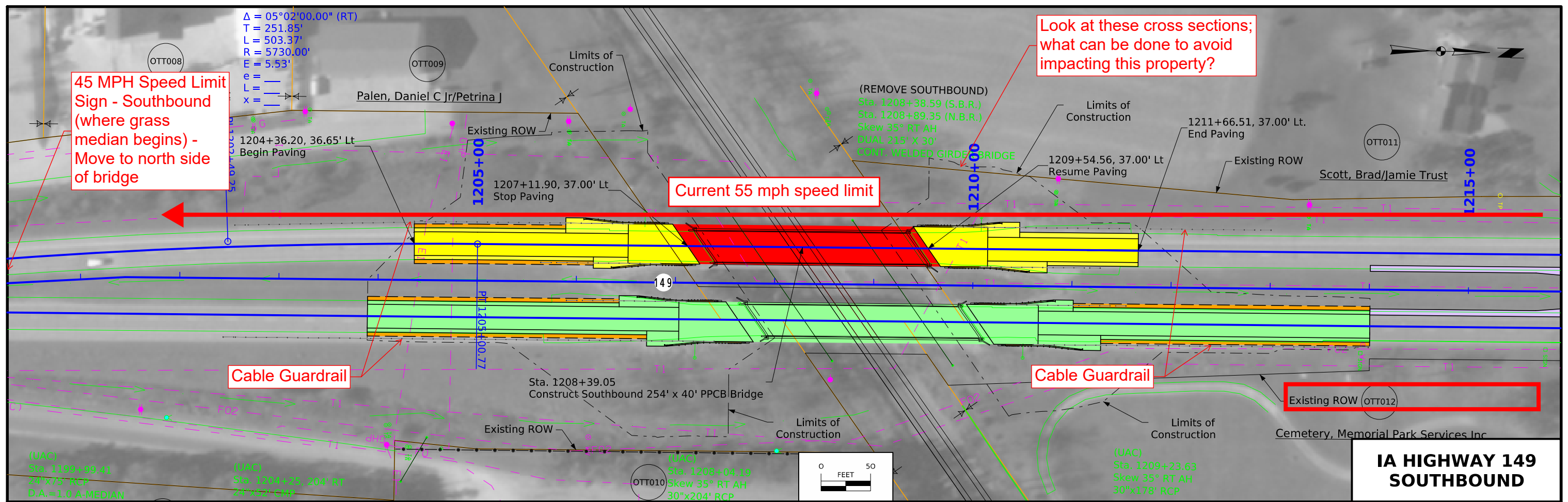
RIGHT-OF-WAY LEGEND

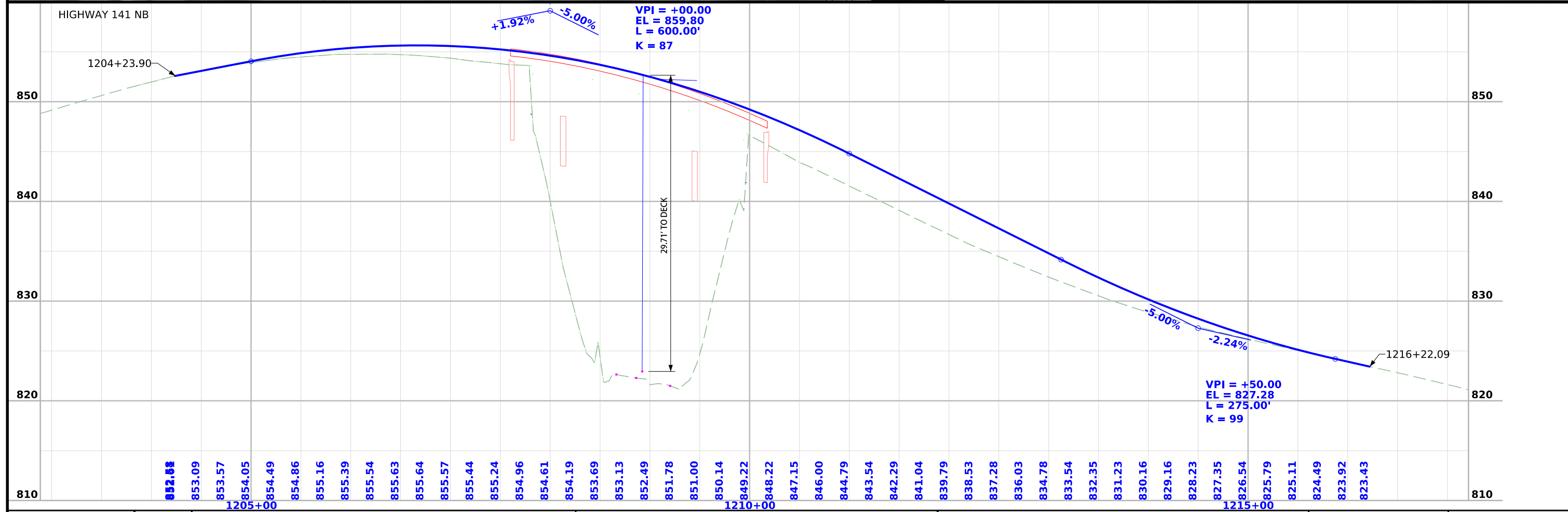
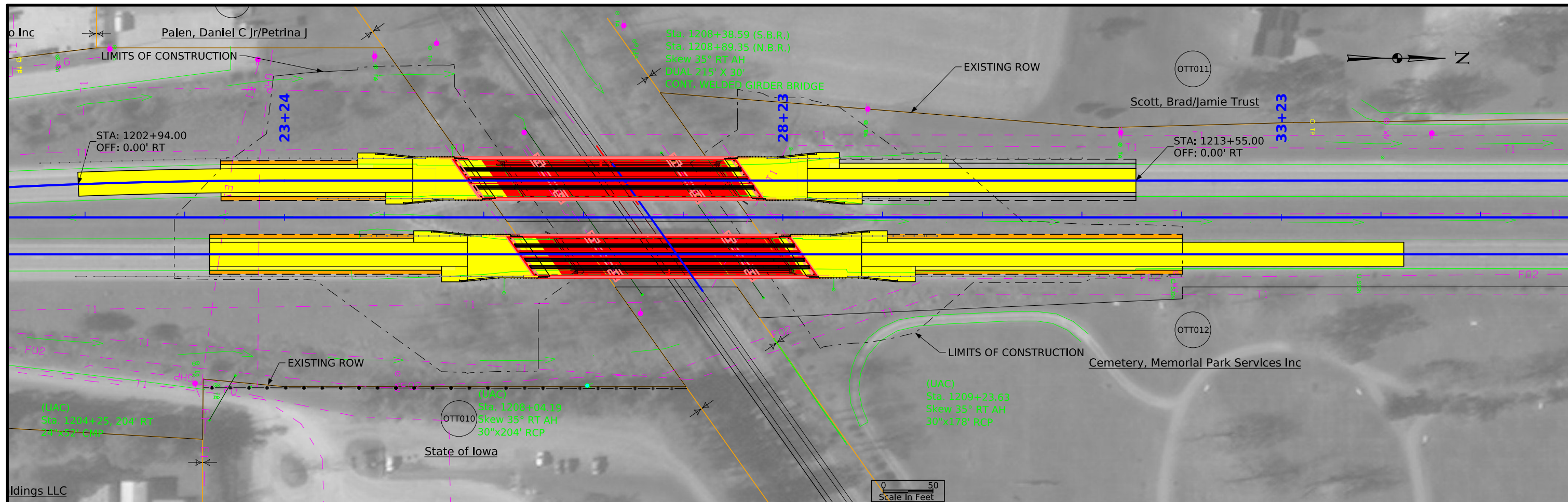
	Proposed Right-of-Way
	Existing Right of Way
	Existing and Proposed Right-of-Way
	Easement and Existing Right-of-Way
	Easement (Temporary)
	Easement
	Access Control
	Property Line
	ROW Line

PLAN AND PROFILE LEGEND AND SYMBOL INFORMATION SHEET

(COVERS SHEET SERIES D, E, F, & K)

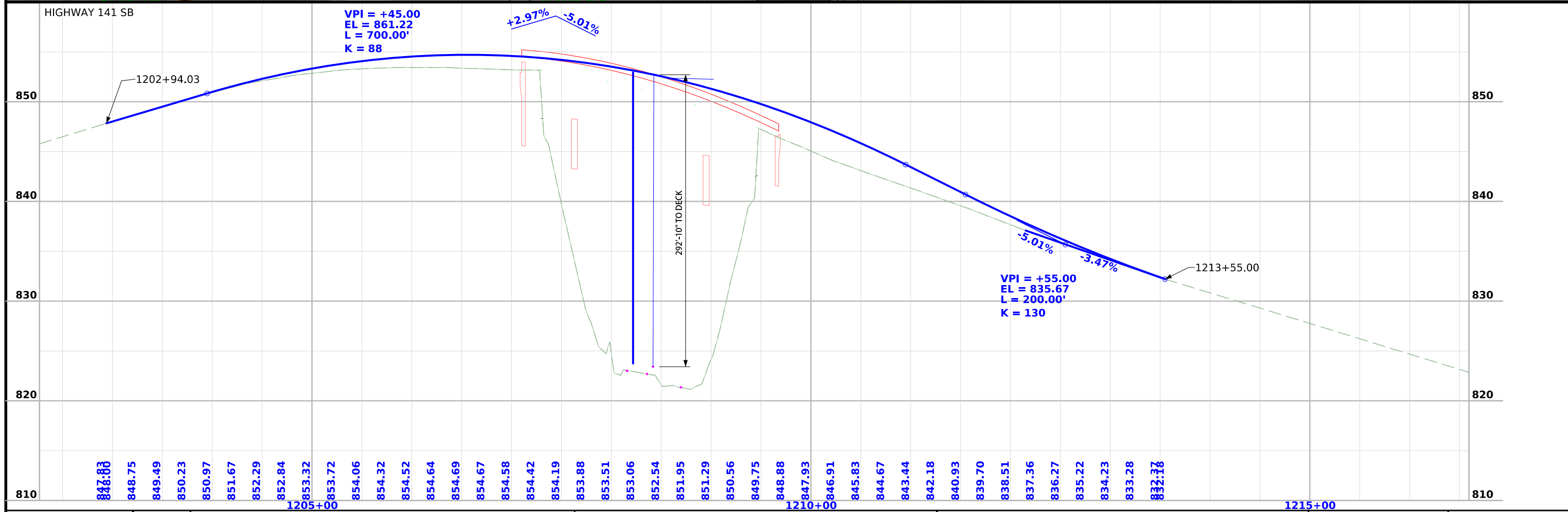
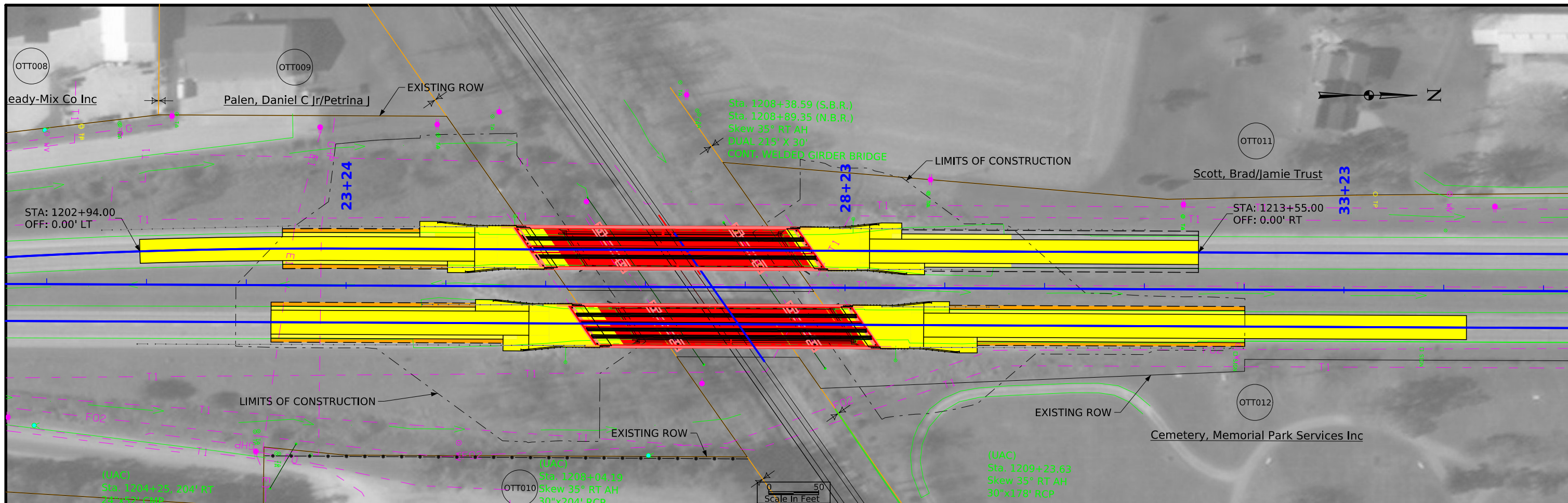






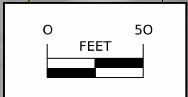
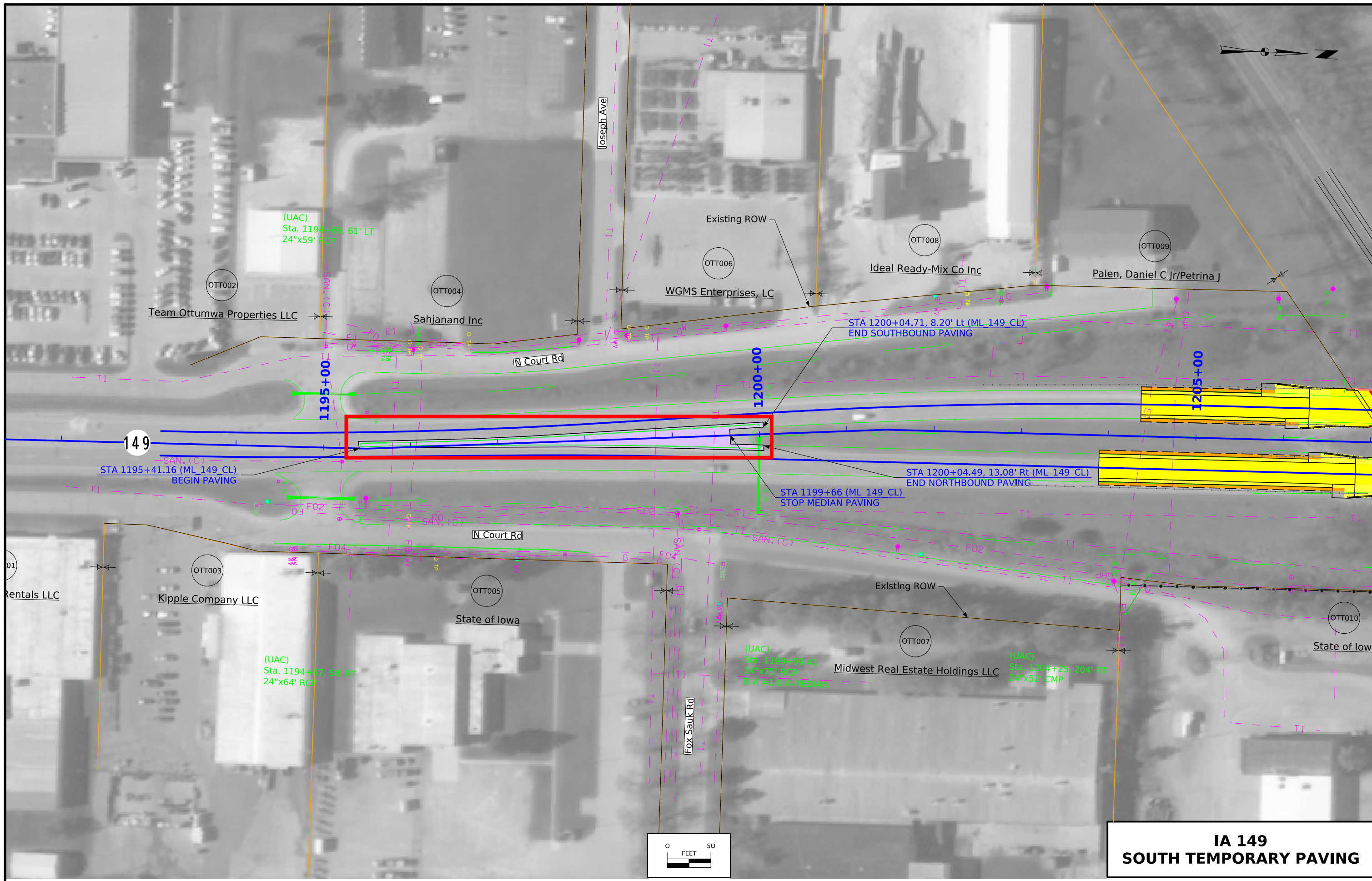
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Revised



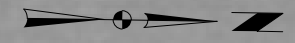
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Revised



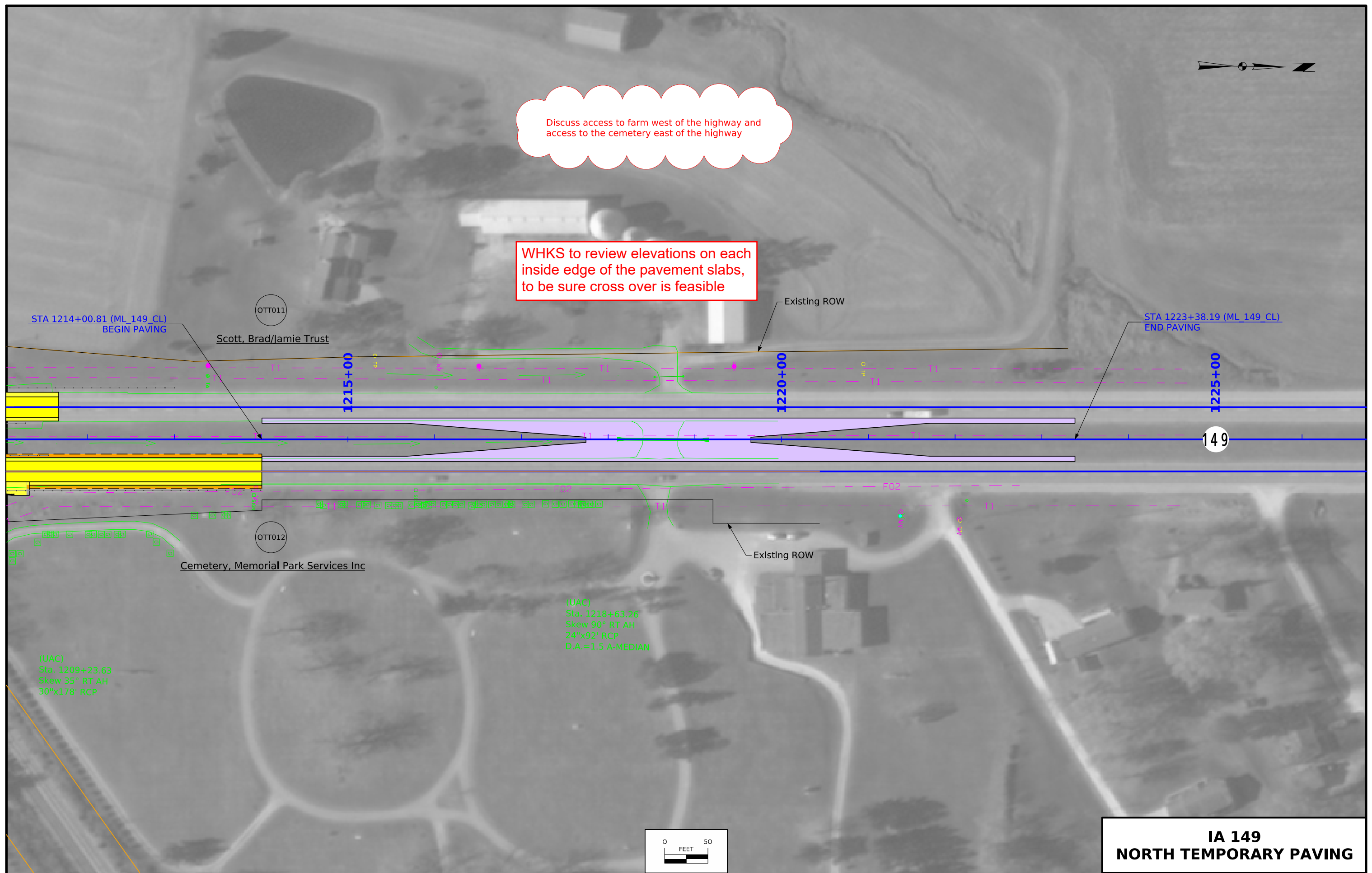
**IA 149
SOUTH TEMPORARY PAVING**

FILE NO.	ENGLISH	DESIGN TEAM WHKS & CO.	WAPELLO COUNTY	PROJECT NUMBER BRF-149-1(096)--38-90	SHEET NUMBER F.1
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Discuss access to farm west of the highway and access to the cemetery east of the highway

WHKS to review elevations on each inside edge of the pavement slabs, to be sure cross over is feasible



STA 1214+00.81 (ML 149 CL) BEGIN PAVING

OTT011
Scott, Brad/Jamie Trust

STA 1223+38.19 (ML 149 CL) END PAVING

1215+00

1220+00

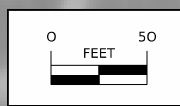
1225+00

149

OTT012
Cemetery, Memorial Park Services Inc

(UAC)
Sta. 1209+23.63
Skew 35° RT AH
30"x178' RCP

(UAC)
Sta. 1218+63.26
Skew 90° RT AH
24"x92' RCP
D.A.=1.5 A-MEDIAN



**IA 149
NORTH TEMPORARY PAVING**

Survey Information

SURVEY INDEX

County: Wapello

PIN: 22-90-149-030

Project Number: BRF-149-1(96)—38-90

Location: DME Railroad 3.9 miles North of US 34 (NB/SB)

Type of Work: Bridge Replacement

Project Directory: 9014903022

Survey Personnel

Jeremy Leemon Survey Project Manager / Party Chief
Chris Ries Assistant Survey Project Manager
Jacob Powers Instrument

Date(s) of Survey

Begin Date 01/08/2024
End Date 01/22/2024

General Information

This survey is for replacement of dual bridges on Iowa Highway 149 over Dakota, Minnesota and Eastern Railroad in Ottumwa. This project is a Full Field DTM survey. Measurement units for this survey are US survey feet.

Utility Information

For logging data and other utility details see Utility Survey and Ownership Report in the Utility folder of the PrelimSurvey project directory.

Project Control

Nearby Iowa Real Time Network reference stations were utilized to obtain horizontal and vertical control on primary project control points. Two 3-minute observations were taken with appropriate time spans between and used in a weighted average to obtain final coordinate values. The Horizontal Scale Factor is: 0.9999992580. Additional control was established from the calibration along the site. For additional details of the control survey, contact the Preliminary Survey

PROJECT DATUM: NAD83(2011) for EPOCH 2010.00 (IaRTN 2019 ADJUSTMENT)
COORDINATE SYSTEM: IOWA REGIONAL COORDINATE SYSTEM ZONE 12
(U.S. SURVEY FOOT)
VERTICAL DATUM: NAVD88
GEOID MODEL: 2018u3

Alignment Information

The horizontal alignment for IA Hwy 149 this survey is a retrace of As-built Plans Project No. U-U.G-F-159(6). Survey stationing was equated to the plan POT at Sta. 1207+39.4 and carried back and ahead without equation throughout the survey.

Survey stationing relates to as built plan stationing as follows:

SURVEY ALIGNMENT

PI Sta. 1181+76.5 As-built Plans Project No. U-U.G-F-159(6)
=Survey POT Sta. 1181+76.50

PI Sta. 1195+50 As-built Plans Project No. U-U.G-F-159(6)
=Survey PI Sta. 1195+50.00

POT Sta. 1207+39.4 As-built Plans Project No. U-U.G-F-159(6)
=Survey POT Sta. 1207+39.40

POT Sta. 1229+61.3 As-built Plans Project No. U-U.G-F-159(6)
=Survey POT Sta. 1229+61.30

NORTH BOUND ROAD ALIGNMENT

PC N.B.R. Sta. 1193+13.9 As-built Plans Project No. U-U.G-F-159(6)
=Survey N.B.R. PC Sta. 1193+13.90

PT N.B.R. Sta. 1195+13.9 As-built Plans Project No. U-U.G-F-159(6)
=Survey N.B.R. PT Sta. 1195+13.90

PT N.B.R. Sta. 1199+15.0 As-built Plans Project No. U-U.G-F-159(6)
=Survey N.B.R. PT Sta. 1199+15.00

SOUTH BOUND ROAD ALIGNMENT

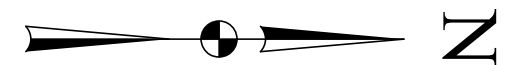
PC S.B.R. Sta. 1193+13.9 As-built Plans Project No. U-U.G-F-159(6)
=Survey S.B.R. PC Sta. 1193+13.90

PT S.B.R. Sta. 1198+17.2 As-built Plans Project No. U-U.G-F-159(6)
=Survey S.B.R. PT Sta. 1198+17.20

PT S.B.R. Sta. 1205+00.7 As-built Plans Project No. U-U.G-F-159(6)
=Survey S.B.R. PT Sta. 1205+00.70

CONTROL POINT VICINITY MAP

This map is a guide to the vicinity of the primary project control points. Primary control is for use with RTK base stations and for RTN validation. Future surveys will use primary project control to establish temporary control as needed for construction or other surveying applications.



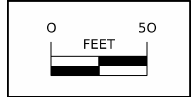
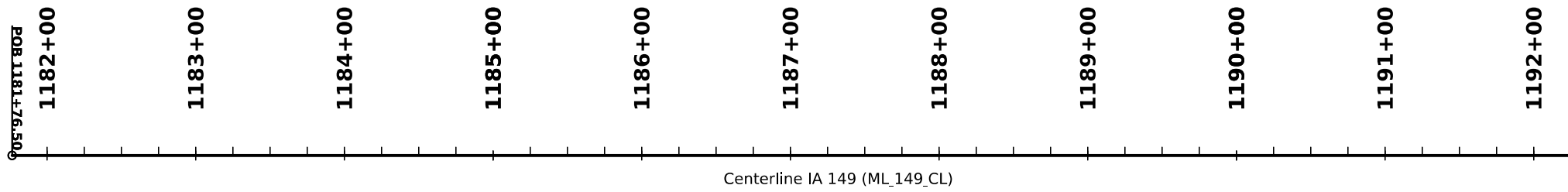
HORIZ. DATUM: NAD83(2011) for EPOCH 2010.00 (IaRTN 2019 Adjustment) - Iowa RCS Zone 12 (U.S. Survey Foot)

VERT. DATUM: NAVD88 - Geoid Model: 2018u3

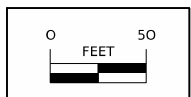
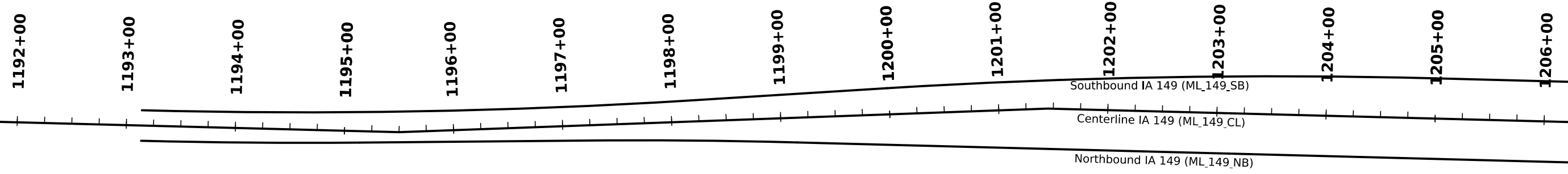
Coordinate listing from next sheet will be used with IaRTN for monument recovery. No other reference ties are given.

HORIZONTAL AND VERTICAL PROJECT CONTROL COORDINATE LISTING
 HORIZ. DATUM: NAD83(2011) for EPOCH 2010.00 (IaRTN 2019 Adjustment)
 Ia. Regional Coordinate System Zone 12 (U.S. Survey Foot)
 VERT. DATUM: NAVD88
 Geoid Model: 2018u3

Point Name	Northing	Easting	Elevation	Feature Definition - Description
101	6256278.66	22869467.99	832.42	Set Feno Style Monument 1000mm W/ Brass Cap 28' West of N Ct St CL
502	6257623.04	22869500.84	853.04	Fd IHC Monument SE Wing of Bridge
501	6257655.58	22869536.64	853.72	Fd IHC Monument SW Wing of Bridge
504	6257840.78	22869497.35	847.01	Fd IHC Monument NE Wing of Bridge
503	6257872.85	22869533.28	847.13	Fd IHC Monument NW Wing of Bridge
102	6259035.43	22869584.71	805.44	Set Feno Style Monument 1000mm W/ Brass Cap 20' South of private entrance pavement



IA 149 SURVEY ALIGNMENTS

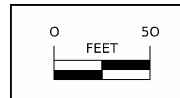


1206+00 1207+00 1208+00 1209+00 1210+00 1211+00 1212+00 1213+00 1214+00 1215+00 1216+00 1217+00 1218+00 1219+00 1220+00 1221+00

Southbound IA 149 (ML_149_SB)

Centerline IA 149 (ML_149_CL)

Northbound IA 149 (ML_149_NB)



IA 149 SURVEY ALIGNMENTS

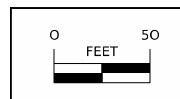
1220+00 1221+00 1222+00 1223+00 1224+00 1225+00 1226+00 1227+00 1228+00 1229+00

Southbound IA 149 (ML_149_SB)

Centerline IA 149 (ML_149_CL)

Northbound IA 149 (ML_149_NB)

POE 1229+61.309

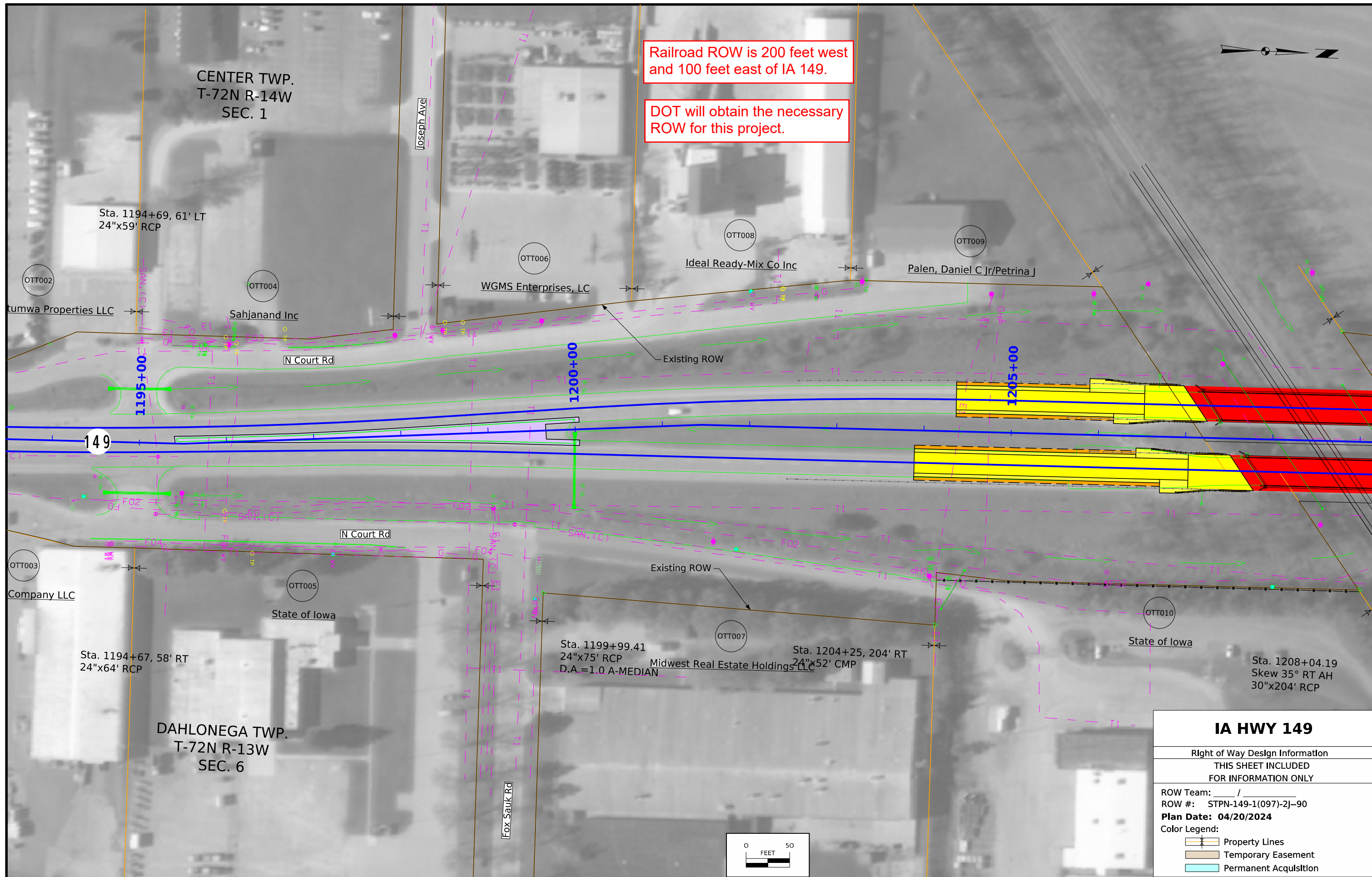


ALIGNMENT COORDINATES

Line No.	Name	Location	Poin on Tangent Station	Point on Tangent Y Northing	Point on Tangent X Easting	Begin Spiral Station	Begin Spiral Y Northing	Begin Spiral X Easting	Begin Curve Station	Begin Curve Y Northing	Begin Curve X Easting	Simple Curve PI or Master PI Station	Simple Curve PI or Master PI Y Northing	Simple Curve PI or Master PI X Easting	End Curve Station	End Curve Y Northing	End Curve X Easting	End Spiral Station	End Spiral Y Northing	End Spiral X Easting
1.1	Mainline IA 149	ML_149_CL	1181+76.50	6255062.643	22869598.749															
1.2	Mainline IA 149	ML_149_CL	1195+50.00	6256435.951	22869575.806															
1.3	Mainline IA 149	ML_149_CL	1201+44.80	6257028.898	22869528.892															
1.4	Mainline IA 149	ML_149_CL	1229+61.30	6259845.005	22869481.8456															
2.1	NB IA 149	ML_149_NB							1193+13.90	6256200.118	22869593.748	1194+13.91	6256300.113	22869592.077	1195+13.90	6256399.988	22869586.917			
2.2	NB IA 149	ML_149_NB							1197+15.08	6256600.906	22869576.539	1198+15.10	6256700.791	22869571.379	1199+15.10	6256800.795	22869569.708			
2.3	NB IA 149	ML_149_NB	1229+60.35	6259845.623	22869518.840															
3.1	SB IA 149	ML_149_SB							1193+13.90	6256199.650	22869565.751	1195+65.71	6256451.427	22869561.543	1198+17.20	6256701.864	22869535.264			
3.2	SB IA 149	ML_149_SB							1199+97.40	6256881.111	22869516.455	1202+49.25	6257131.583	22869490.172	1205+00.77	6257383.395	22869485.965			
3.3	SB IA 149	ML_149_SB	1229+62.21	6259844.387	22869444.851															

SPIRAL OR CIRCULAR CURVE DATA

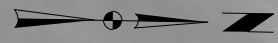
Line No.	Name	Location	SCS	S	Ls	Ts	Es	Xc	Yc	L.T.	S.T.	C	T	L	R	E	Remarks
1.1	C1	ML_149_NB										2.000	100.009	199.997	55730.000	0.873	
1.2	C2	ML_149_NB										2.000	100.018	200.016	5730.037	0.873	
2.1	C1	ML_149_SB										5.033	251.847	503.300	5730.000	5.530	
2.2	C2	ML_149_SB										5.033	251.847	503.370	5730.000	5.532	



Railroad ROW is 200 feet west and 100 feet east of IA 149.

DOT will obtain the necessary ROW for this project.

IA HWY 149	
Right of Way Design Information	
THIS SHEET INCLUDED FOR INFORMATION ONLY	
ROW Team: _____ / _____	
ROW #: STPN-149-1(097)-2J-90	
Plan Date: 04/20/2024	
Color Legend:	
	Property Lines
	Temporary Easement
	Permanent Acquisition



CENTER TWP.
T-72N R-14W
SEC. 1

Sta. 1208+38.59 (S.B.R.)
Sta. 1208+89.35 (N.B.R.)
Skew 35° RT AH
DUAL 215' X 30'
CONT. WELDED GIRDER BRIDGE

OTT011
Scott, Brad/Jamie Trust

18"x33'
C.M.P.

1210+00

1215+00

1220+00

149

OTT012
Cemetery, Memorial Park Services Inc

Sta. 1218+63.26
Skew 90° RT AH
24"x92' RCP
D.A.=1.5 A-MEDIAN

Sta. 1208+04.19
Skew 35° RT AH
30"x204' RCP

Sta. 1209+23.63
Skew 35° RT AH
30"x178' RCP

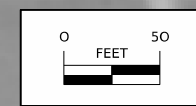
DAHLONEGA TWP.
T-72N R-13W
SEC. 6

IA HWY 149

Right of Way Design Information
THIS SHEET INCLUDED
FOR INFORMATION ONLY

ROW Team: ____ / ____
ROW #: STPN-149-1(097)-2J--90
Plan Date: 04/20/2024

- Color Legend:
-  Property Lines
 -  Temporary Easement
 -  Permanent Acquisition



FILE NO.	ENGLISH	DESIGN TEAM WHKS & CO.	WAPELLO COUNTY	PROJECT NUMBER BRF-149-1(096)--38-90	SHEET NUMBER H.2
10:53:07	10/04/2024	skoontz	pw:\projectwise.dot.int.lan:PWMMain\Documents\Projects\9014903022\Design\CADD_Files\Sheet_Files\SHT_90149096_H01.dgn		

TRAFFIC CONTROL PLAN

108_23A
8/15/22

1. Through traffic on IA 149 shall be maintained at all times. Maintain a minimum of one lane of traffic, in each direction, at all times during construction.
2. Access to all properties shall be maintained at all times.

Discuss access to farm north of the bridge on the west side of the Highway and access to the cemetery north of the bridge on the east side of the highway

Special traffic control layouts will be needed for all stages, to allow access to each of these properties.

Discuss access to North Ct St.; Advanced warning signs and tapers will extend into intersection

This intersection will be closed during construction, due to the advanced warning layout. Traffic will use the intersection to the south. All lanes will be left open at this intersection. If lane closures are located in this area, the additional lane can be used for a left turn lane.

511 TRAVEL RESTRICTIONS

Line No.	Route	Direction	County	Location Description	Feature Crossed	Object Type	Maint. Bridge No. or Structure ID or FHWA No.	Type of Restriction	Existing Measurement	Construction Measurement	Construction Measurement as Signed	Projected As Built Measurement	Remarks
1.0	IA 149	NB	Wapello	In Ottumwa	DME Railroad	Traffic Control Device	9003.9R149	Horizontal	N/A	14.5	13.5	N/A	
2.0	IA 149	SB	Wapello	In Ottumwa	DME Railroad	Traffic Control Device	9003.9L149	Horizontal	N/A	14.5	13.5	N/A	

STAGING NOTES

Any requests to change the staging shall follow Article 1105.15 of the Standard Specifications. Maintenance of traffic shall be per Tab. 108-23A and the traffic control details described within. No Value Engineering proposals will be accepted that deviate from the intent of the traffic control shown in the plans.

Stage 1 Construction

- Construct temporary pavement south of the bridge and north of the bridge.

Stage 1 Traffic Control

- Close inside lanes per TC-418 and TC-419.

Stage 2 Construction

- Remove bullnose guardrail on both sides of the existing bridges. Install TBR and temporary crash cushions for approach side of southbound bridge.

- Construct northbound bridge and associated roadway work.

Stage 2 Traffic Control

- Shift southbound traffic to the outside, southbound lane using TC-418.
- Shift northbound traffic to the inside, southbound lane using TC-61.

Stage 3 Construction

- Construct southbound bridge and associated roadway work.

Stage 3 Traffic Control

- Shift northbound traffic to the outside, northbound lane using TC-418.
- Shift southbound traffic to the inside, northbound lane using TC-61.

Stage 4 Construction

- Remove temporary pavement south of the bridge and north of the bridge.

Stage 4 Traffic Control

- Close inside lanes per TC-418 and TC-419.

111_01
10/14/22

COORDINATED OPERATIONS

Other work in progress during the same period of time will include the construction of the projects listed. Coordinate operations with those of other contractors working within the same area.

Project	Type of Work
District to	Provide

STP-149-1(88)--2C-90 PCC Pavement - Grade and Replace
Potential City RISE project south of the IA 149 project over DME Railroad

Possible Revision:
 WHKS will shift the bridge to the north, to avoid the existing pier foundations. The interior span could not be extended, because the beam depths would get deeper, and the profile would need to be raised to maintain minimum vertical clearance thus increasing the length of reconstruction.

BTD Beam Depth = 54" Depth (Current proposal)
BTE Beam Depth = 63" Depth

The south pier will encroach on the railroad ROW, but will still provide a greater horizontal clearance than what is provided today. WHKS will provide a revised drawing for the Railroad Bureau to discuss with the railroad

The bridge will span the 100' wide railroad ROW, but not the 200' wide railroad ROW.

Conduit will be installed in the outside barrier rails (2 in each outside barrier rail), in case it is needed in the future.

No need for aesthetics due to location and spanning the railroad

$g_1 = 2.60\%$ $g_2 = -5.1\%$
 VPI Sta. = 1207+35.56
 VPI Elev. = 859.541
 VC = 479.159'
 Proposed Profile Grade \bar{C} SB IA 149

30" RCP will need to be check, especially if the pier is shifted to the north

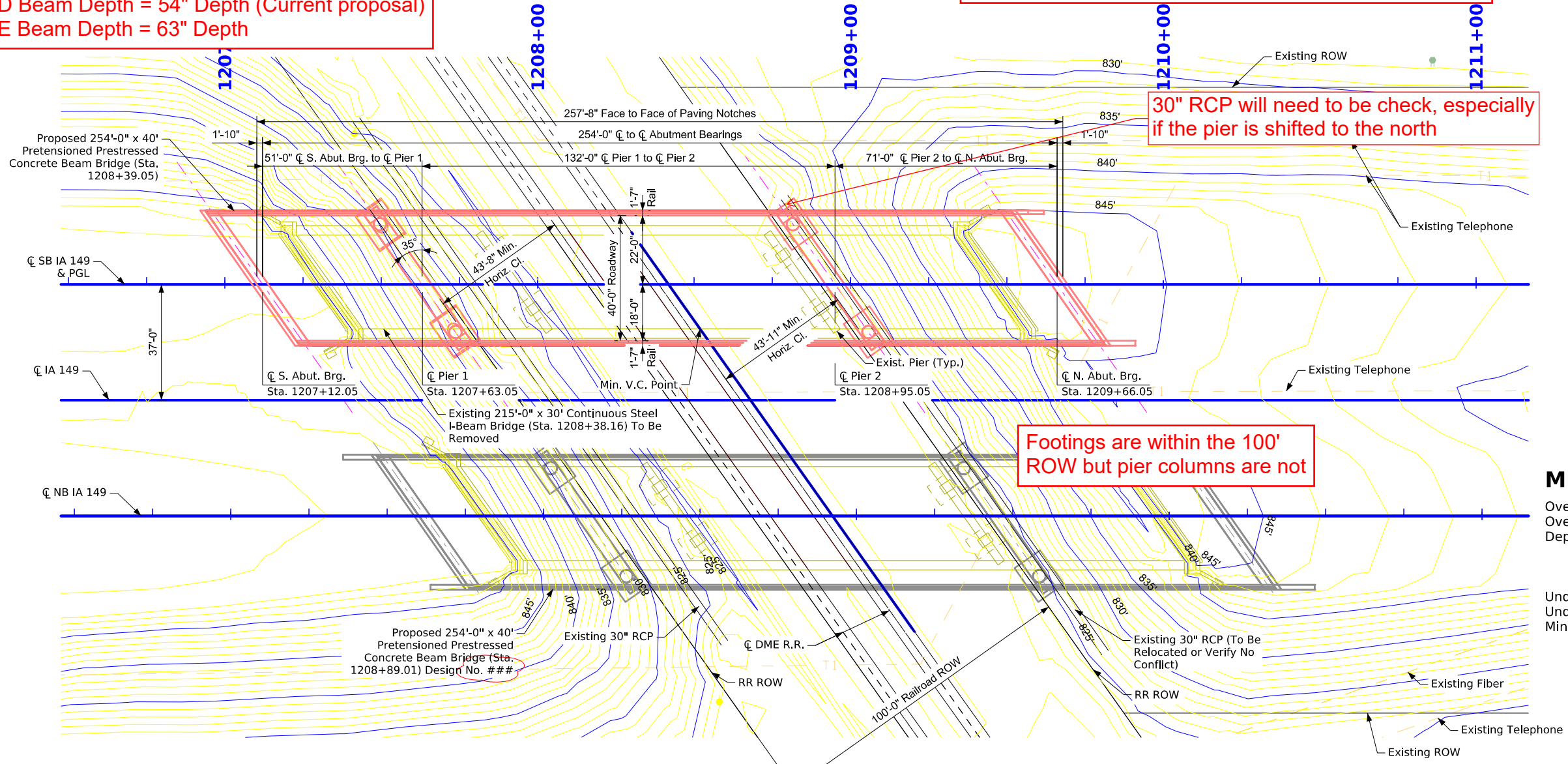
The FRA number should be included on the V Sheets

The Bridges and Structures Bureau can provide the Design and FHWA #'s

Footings are within the 100' ROW but pier columns are not

Minimum Vertical Clearance

Overhead Station = 1208+53.29, Offset 16.08' Rt.
 Overhead Elevation = 852.02
 Depth of Superstructure = 64.5 in.
 Deck Thickness = 8.5 in.
 Estimated Haunch = 2 in.
 Beam Depth = 54 in.
 Underpass Station = 3+57.41, Offset 0
 Underpass Elevation = 823.29
 Minimum Vertical Clearance = 23.36'



Situation Plan

General Notes:
 --This design is for the replacement of the existing 215'-0" x 30'-0" Continuous I-Beam Bridge, Wapello County Design 260, FHWA No. 50680, Maint. No. 9003.9L149.

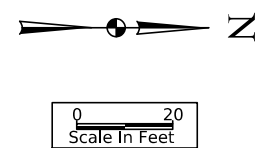
Design Notes:
 --TL-5 Bridge Railing Proposed
 --Pier Type - Frame (4' Assumed Column Diameter)

Plan Notes:
 --Top of bridge deck at centerline roadway is 0.03' below the profile grade to account for deck cross slope and parabolic crown.
 --Proposed bridge will not effect flow in existing railroad ditches or drainage structures.

Utilities Note:
 Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

General Utility Symbols:
 T - Telephone Line FO - Fiber Optic Line
 ● - Power Poles

Location
 SB IA 149 over DME RR
 T-72N R-13 & 14W
 Section 1 & 6
 Center Township
 Wapello County
 City of Ottumwa
 FHWA No. 50680 (Existing)
 Bridge Maint. No. 9003.9L149
 Latitude 41.067479°
 Longitude -92.410135°



Traffic Data
 2022 AADT 3,500
 TRUCKS 6 %

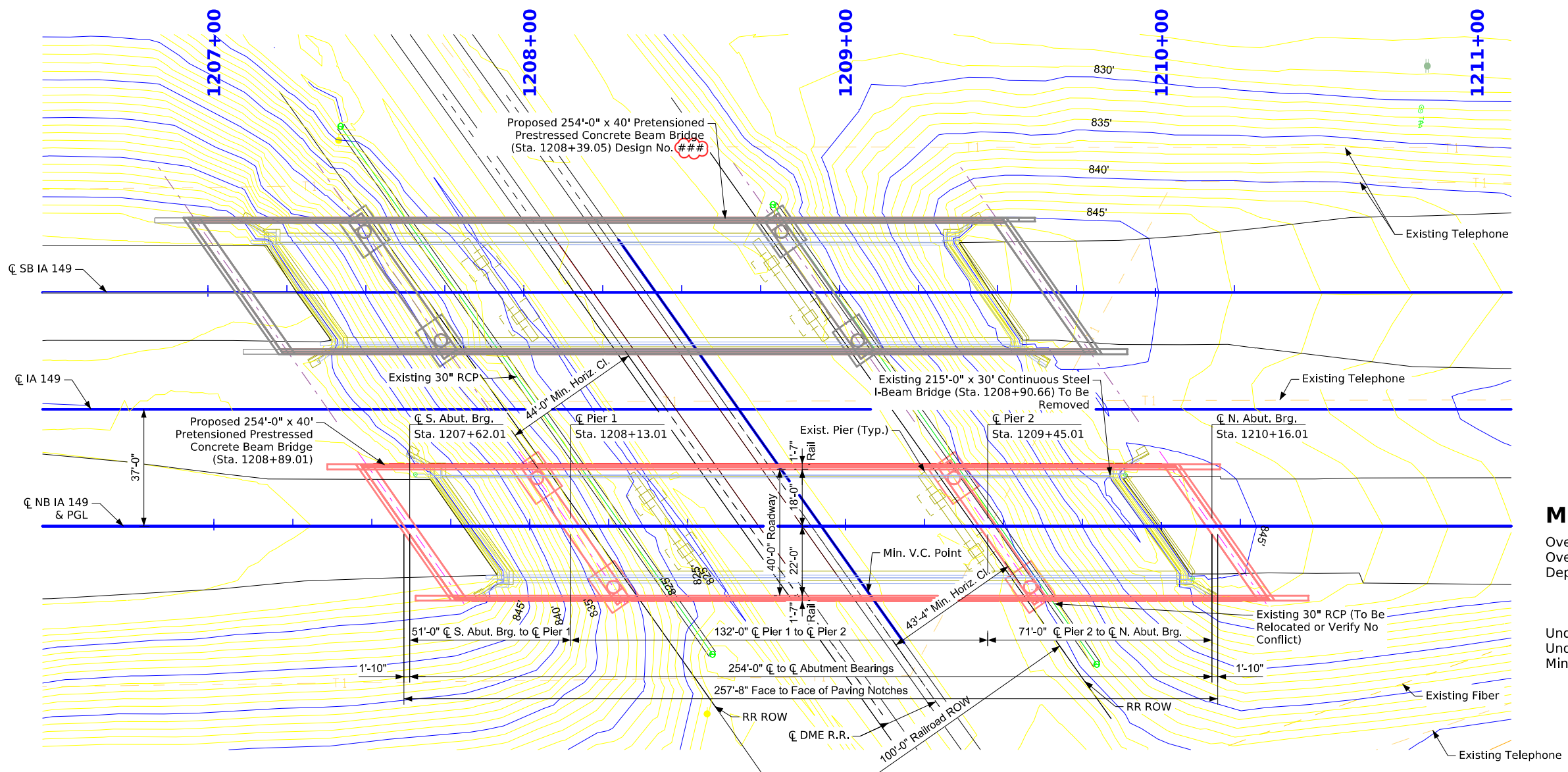
Refer to Revised V.1

Design For 35° Skew RA
254'-0" x 40'-0" Pretensioned Prestressed Concrete Beam Bridge
 51'-0" & 71'-0" End Spans BTD Beams 132'-0" Interior Span
Situation Plan
 STA. 1208+39.05 (SB IA 149) August, 2024
Wapello County
 IOWA DEPARTMENT OF TRANSPORTATION
 Design No. 45 Design Sheet No. 1 of 1 FHWA No. 50680

g₁ = 1.80 % g₂ = -6.10 %

VPI Sta. = 1208+37.83
 VPI Elev. = 859.95
 VC = 595.84'

Proposed Profile
 Grade \bar{C} NB IA 149



Minimum Vertical Clearance

Overhead Station = 1209+06.58, Offset 20.08' Rt.
 Overhead Elevation = 851.87
 Depth of Superstructure = 64.5 in.
 Deck Thickness = 8.5 in.
 Estimated Haunch = 2 in.
 Beam Depth = 54 in.
 Underpass Station = 4+53.05, Offset 0
 Underpass Elevation = 822.72
 Minimum Vertical Clearance = 23.78'

General Notes:
 --This design is for the replacement of the existing 215'-0" x 30'-0" Continuous I-Beam Bridge, Wapello County Design 260, FHWA No. 50670, Maint. No. 9003.9R149.

Design Notes:
 --TL-5 Bridge Railing Proposed
 --Pier Type - Frame (4' Assumed Column Diameter)

Plan Notes:
 --Top of bridge deck at centerline roadway is 0.03' below the profile grade to account for deck cross slope and parabolic crown.
 --Proposed bridge will not effect flow in existing railroad ditches or drainage structures.

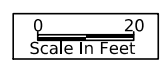
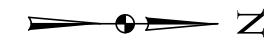
Utilities Note:
 Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

General Utility Symbols:
 T - Telephone Line FO - Fiber Optic Line
 ● - Power Poles

Situation Plan

Location

NB IA 149 over DME RR
 T-72N R-13 & 14W
 Section 1 & 6
 Center Township
 Wapello County
 City of Ottumwa
 FHWA No. 50670 (Existing)
 Bridge Maint. No. 9003.9R149
 Latitude 41.066988°
 Longitude -92.409861°



Traffic Data

2022 AADT 3,500 V.P.D.
 TRUCKS 6 %

Refer to Revised V.2

Preliminary

Design For 35 Skew RA

254'-0" x 40'-0" Prestressed Concrete Beam Bridge

51'-0" & 71'-0" End Spans BTD Beams 132'-0" Interior Span

Situation Plan

STA. 1208+89.01 (NB IA 149) August 2024

Wapello County

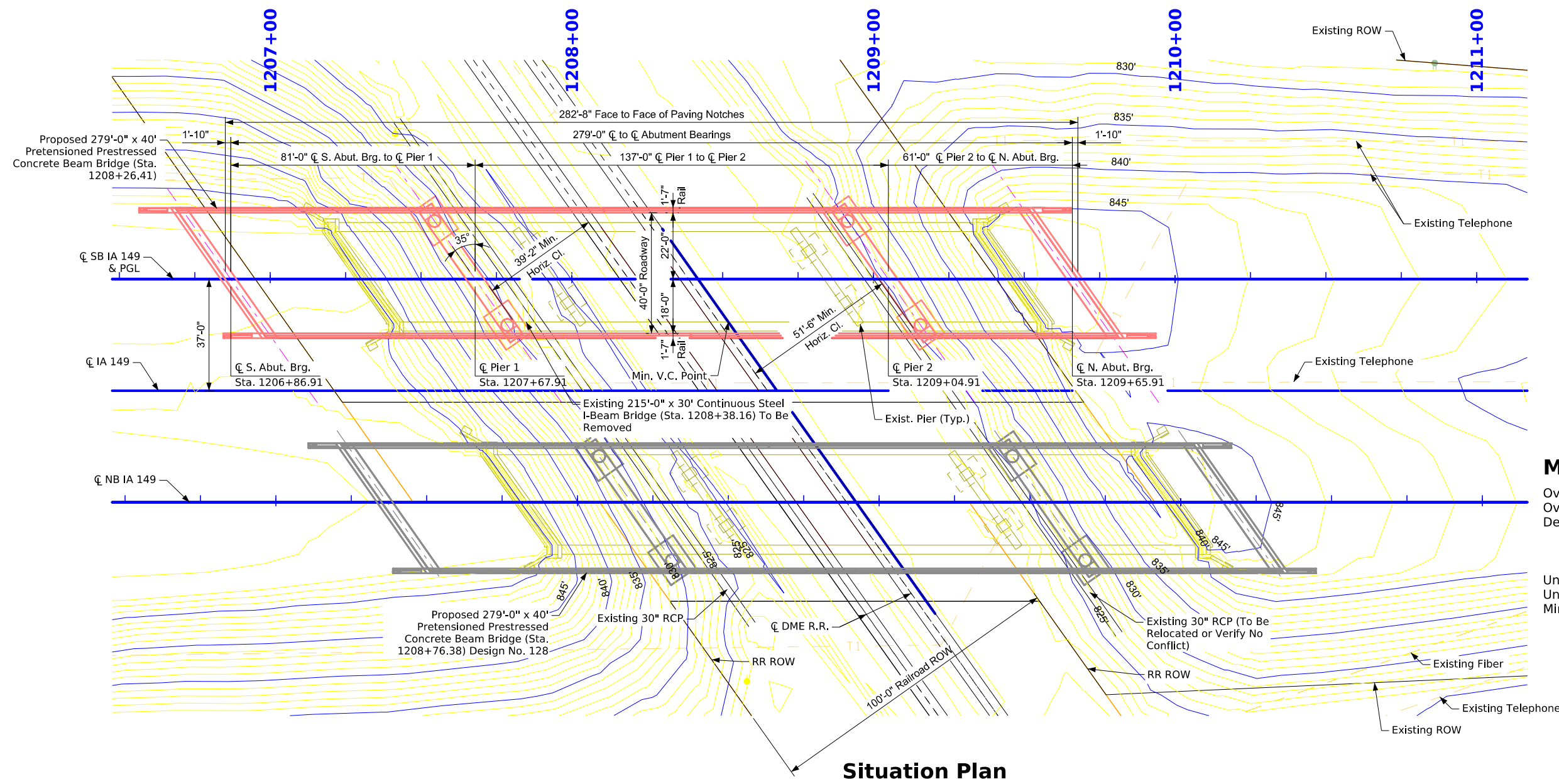
IOWA DEPARTMENT OF TRANSPORTATION

Design No. 46 Design Sheet No. 1 of 1 FHWA No. 50670

g₁ = 2.97 % g₂ = -5.01 %

VPI Sta. = 1207+45.00
 VPI Elev. = 861.220
 VC = 700.00'

Proposed Profile
 Grade \bar{C} SB IA 149



Minimum Vertical Clearance

Overhead Station = 1208+53.29, Offset 16.08' Rt.
 Overhead Elevation = 852.14
 Depth of Superstructure = 64.5 in.
 Deck Thickness = 8.5 in.
 Estimated Haunch = 2 in.
 Beam Depth = 54 in.
 Underpass Station = 3+57.41, Offset 0
 Underpass Elevation = 823.29
 Minimum Vertical Clearance = 23.48'

Situation Plan

General Notes:
 --This design is for the replacement of the existing 215'-0" x 30'-0" Continuous I-Beam Bridge, Wapello County Design 260, FHWA No. 50680, Maint. No. 9003.9L149.

Design Notes:
 --TL-5 Bridge Railing Proposed
 --Pier Type - Frame (4' Assumed Column Diameter)
 --Two conduits should be provided in the outside rail for future use

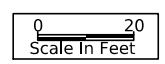
Plan Notes:
 --Top of bridge deck at centerline roadway is 0.03' below the profile grade to account for deck cross slope and parabolic crown.
 --Proposed bridge will not effect flow in existing railroad ditches or drainage structures.

Utilities Note:
 Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

General Utility Symbols:
 T - Telephone Line FO - Fiber Optic Line
 ● - Power Poles

Location

SB IA 149 over DME RR
 T-72N R-13 & 14W
 Section 1 & 6
 Center Township
 Wapello County
 City of Ottumwa
 FHWA No. 50680 (Existing)
 FHWA No. 50681 (Proposed)
 Bridge Maint. No. 9003.9L149
 Latitude 41.067479°
 Longitude -92.410135°
 FRA No. 375788T



Traffic Data

2022 AADT 3,500 V.P.D.
 TRUCKS 6 %

Preliminary

Design For 35° Skew RA
**279'-0" x 40'-0" Prestensioned
 Prestressed Concrete Beam Bridge**
 81'-0" & 61'-0" End Spans BTB Beams 137'-0" Interior Span

Situation Plan

STA. 1208+26.41 (SB IA 149) August, 2024

Wapello County
 IOWA DEPARTMENT OF TRANSPORTATION

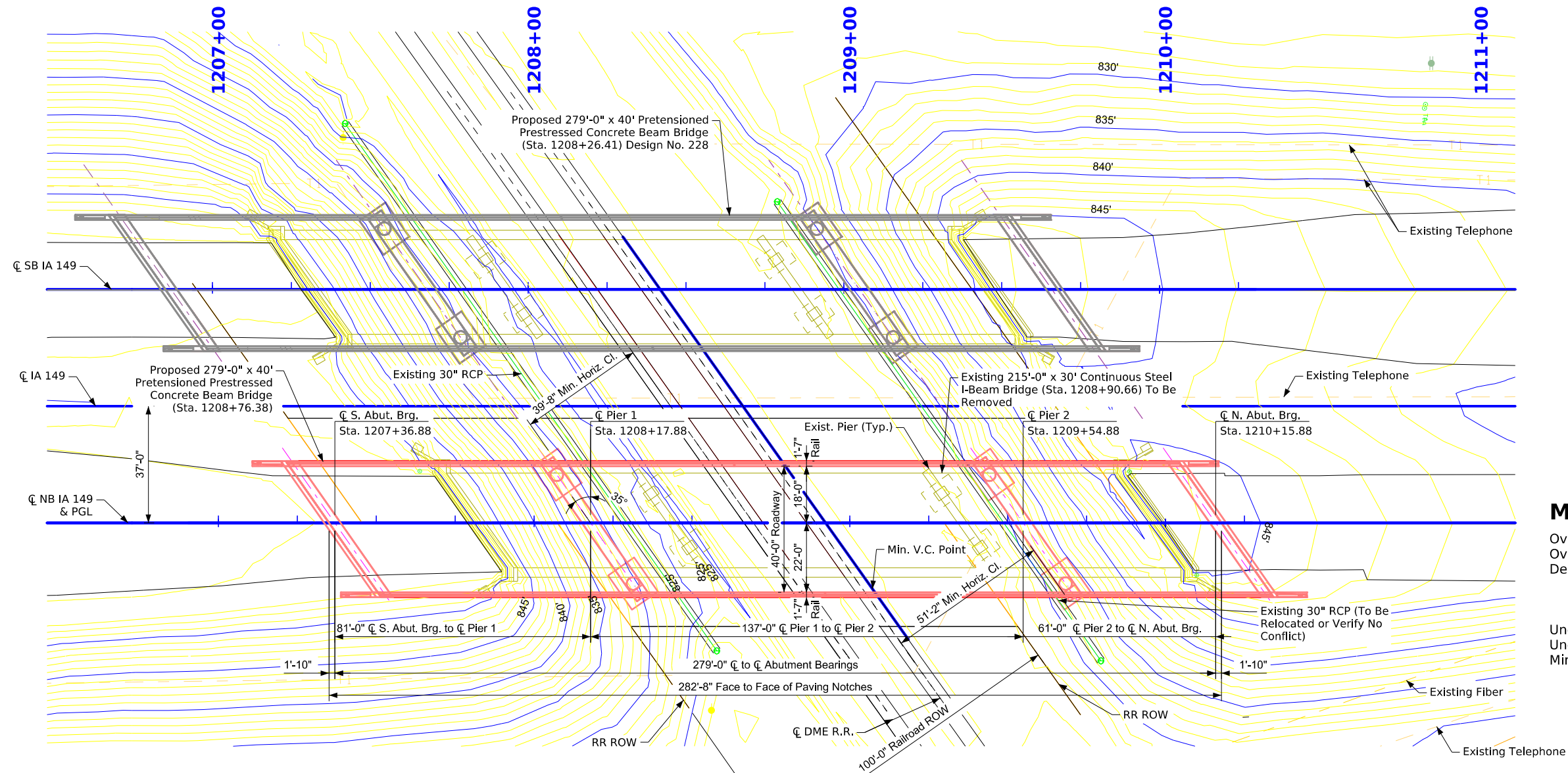
Design No. 228 Design Sheet No. 1 of 1 FHWA No. 50681

Revised

g₁ = 1.92 % g₂ = -5.00 %

VPI Sta. = 1208+00.00
 VPI Elev. = 859.798
 VC = 600.00'

Proposed Profile
 Grade \bar{C} NB IA 149



Minimum Vertical Clearance

Overhead Station = 1209+06.58, Offset 20.08' Rt.
 Overhead Elevation = 851.91
 Depth of Superstructure = 64.5 in.
 Deck Thickness = 8.5 in.
 Estimated Haunch = 2 in.
 Beam Depth = 54 in.
 Underpass Station = 4+53.05, Offset 0
 Underpass Elevation = 822.80
 Minimum Vertical Clearance = 23.73'

General Notes:
 --This design is for the replacement of the existing 215'-0" x 30'-0" Continuous I-Beam Bridge, Wapello County Design 260, FHWA No. 50670, Maint. No. 9003.9R149.

Design Notes:
 --TL-5 Bridge Railing Proposed
 --Pier Type - Frame (4' Assumed Column Diameter)
 --Two conduits should be provided in the outside rail for future use

Plan Notes:
 --Top of bridge deck at centerline roadway is 0.03' below the profile grade to account for deck cross slope and parabolic crown.
 --Proposed bridge will not effect flow in existing railroad ditches or drainage structures.

Situation Plan

Location

NB IA 149 over DME RR
 T-72N R-13 & 14W
 Section 1 & 6
 Center Township
 Wapello County
 City of Ottumwa
 FHWA No. 50670 (Existing)
 FHWA No. 50671 (Proposed)
 Bridge Maint. No. 9003.9R149
 Latitude 41.066988°
 Longitude -92.409861°
 FRA No. 375788T



0 20
 Scale In Feet

Traffic Data

2022 AADT 3,500 V.P.D.
 TRUCKS 6 %

Utilities Note:

Utilities shown on this sheet are for information only. See Road Design sheets for utility information.

General Utility Symbols:

T - Telephone Line FO - Fiber Optic Line
 ● - Power Poles

Preliminary

Design For 35 Skew RA
279'-0" x 40'-0" Prestressed Concrete Beam Bridge
 81'-0" & 61'-0" End Spans BTD Beams 137'-0" Interior Span
Situation Plan
 STA. 1208+76.38 (NB IA 149) August 2024
Wapello County
 IOWA DEPARTMENT OF TRANSPORTATION
 Design No. 128 Design Sheet No. 1 of 1 FHWA No. 50671

Revised

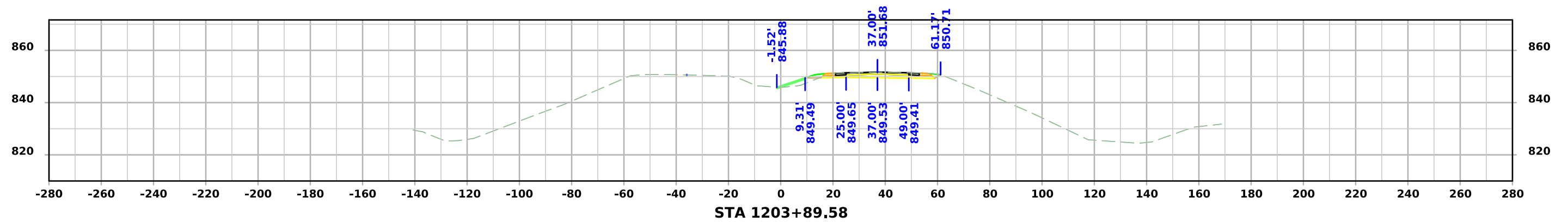
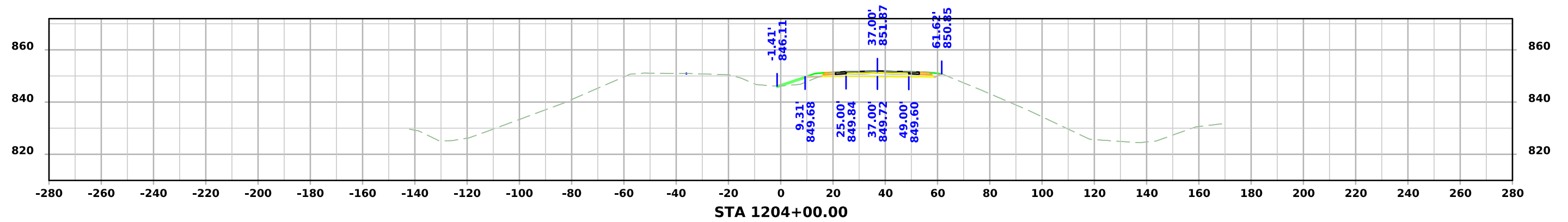
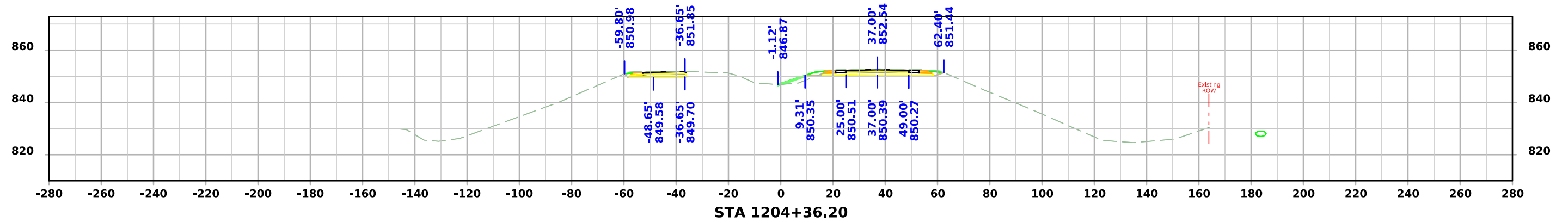
CROSS SECTION VIEW COLOR LEGEND

Design Color No.	Feature	Design Color No.	Feature
Aggregate			
(64)	Choke Stone	(112)	Noise Wall
(42)	Engineering Fabric	(112)	Noise Wall Footing
(8)	Flooded Backfill	(112)	Retaining Wall Back
(92)	Macadam Stone	(112)	Retaining Wall Back Excavate
(20)	Modified	(112)	Retaining Wall Face
(12)	Plowing Shaping	(112)	Retaining Wall Front Excavate
(14)	Porous Backfill	(112)	Retaining Wall Front Footing
(8)	Revetment Class A	(112)	Retaining Wall MSE Gutter
(6)	Revetment Class B	(112)	Retaining Wall Reinforced Earth
(62)	Revetment Class C	Grading	
(188)	Revetment Class D	(8)	Behind Curb Cut
(28)	Revetment Class E	(6)	Granular
(12)	Shoulder Special Backfill	(13)	Granular Back Fill
(12)	Special Backfill	(48)	Rock Undercut
(20)	Subbase	(8)	Shoulder Earth Fill
(20)	Subbase Lower	(2)	Side Slopes
(20)	Subbase Upper	(226)	Side Slopes Dressing
(118)	Subgrade Treatment	Substrata	
Asphalt			
(207)	HMA Base Course	(128)	Boulder Substrata
(207)	HMA Interim Course	(48)	Broken Weathered Substrata
(207)	HMA Surface Course	(3)	Core Out Substrata
Concrete			
(0)	Barrier Concrete	(203)	Existing Pavement Substrata
(0)	Barrier Concrete Footing	(6)	Loam Substrata
(0)	Curb Gutter	(80)	Rock Substrata
(48)	Flowable Mortar	(4)	Select Sand Substrata
(0)	Median Concrete	(3)	Shale Substrata
(0)	PCC Pavement	(10)	Topsoil Substrata
(0)	Sidewalk	Unsuitable / Waste	
Shoulder			
(209)	Shoulder HMA	(3)	Unsuitable Type A
(0)	Shoulder PCC	(13)	Unsuitable Type B
(6)	Shoulder Granular	(11)	Unsuitable Type C
(3)		(3)	Waste
Existing			
(0)	Existing Pavement		

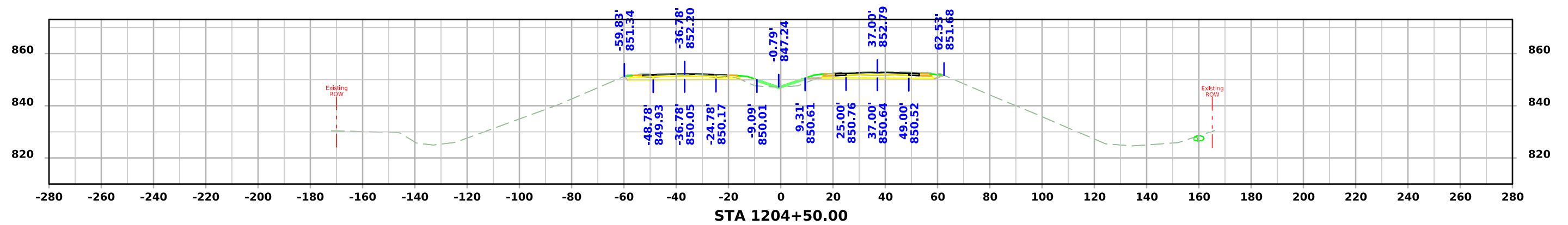
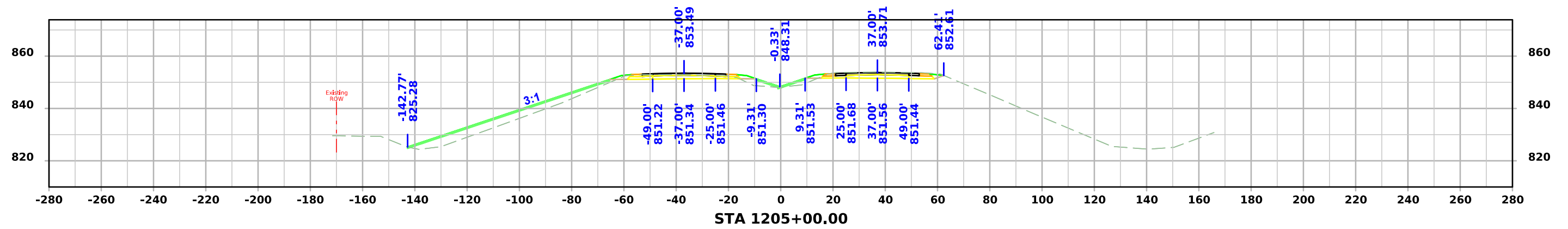
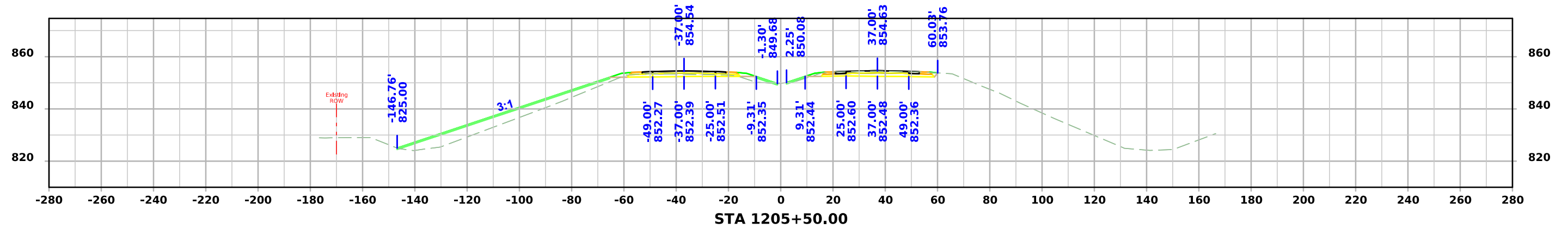
CROSS SECTIONS LEGEND AND INFORMATION SHEET

(COVERS SHEET SERIES W, X, Y, & Z)

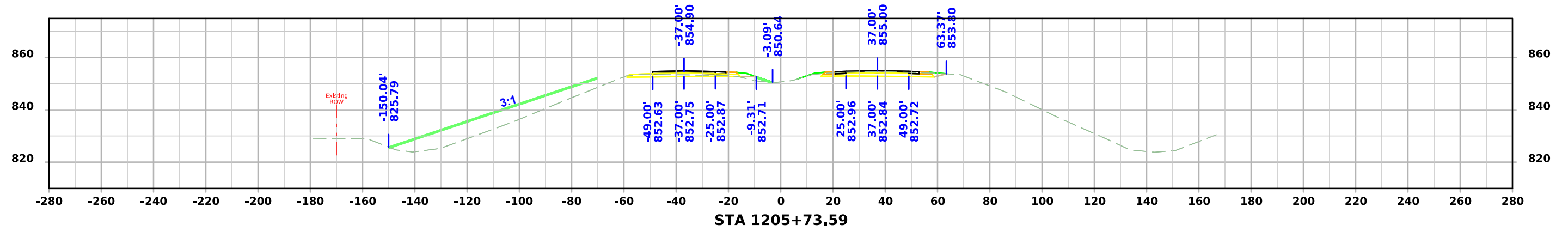
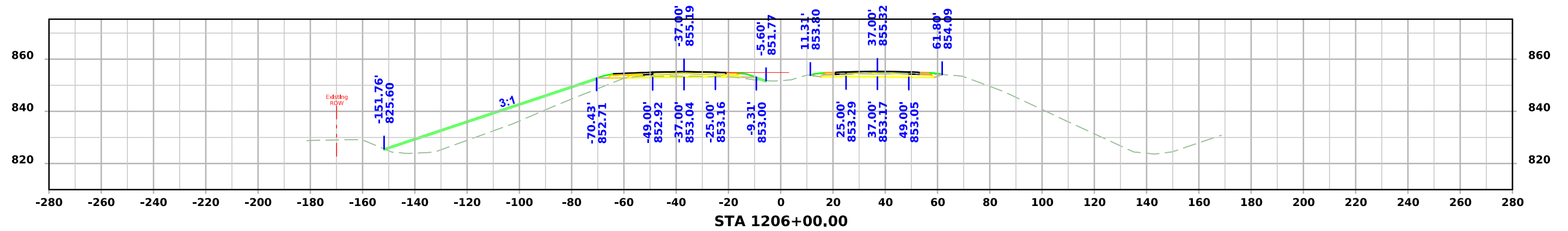
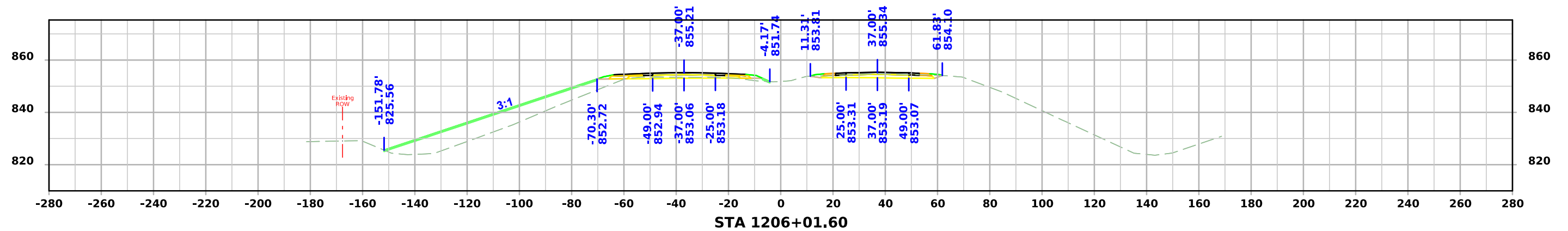
IA HWY 149



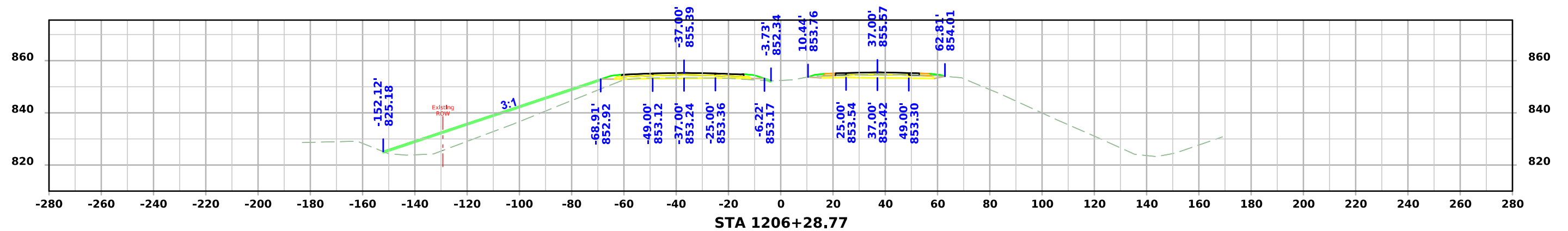
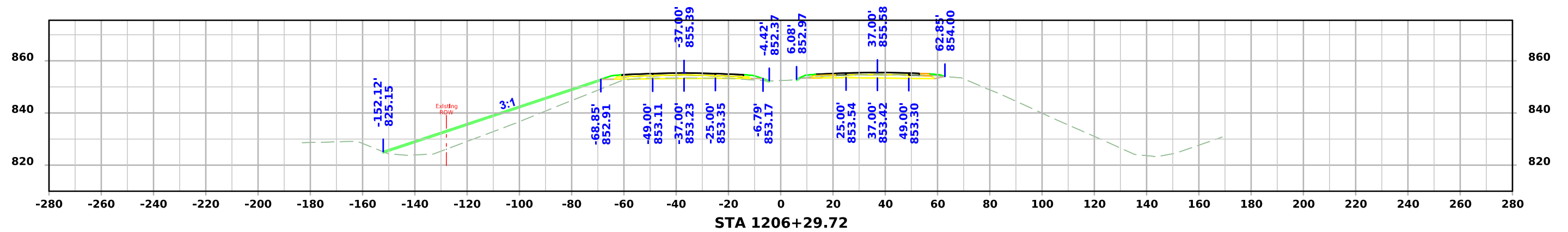
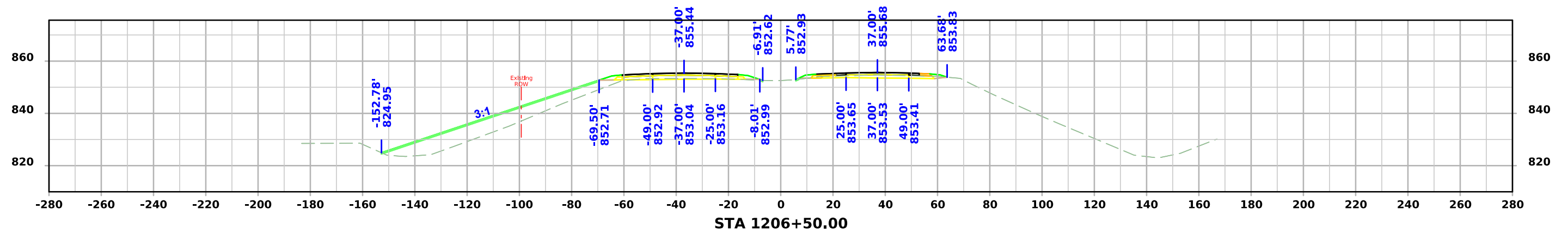
IA HWY 149



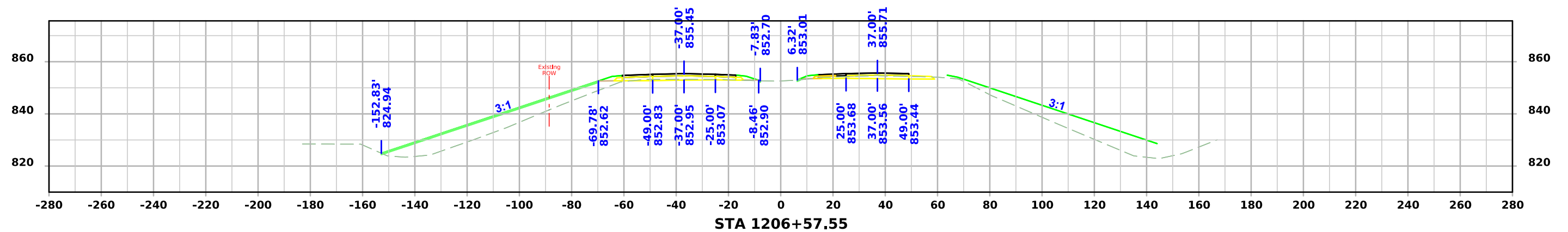
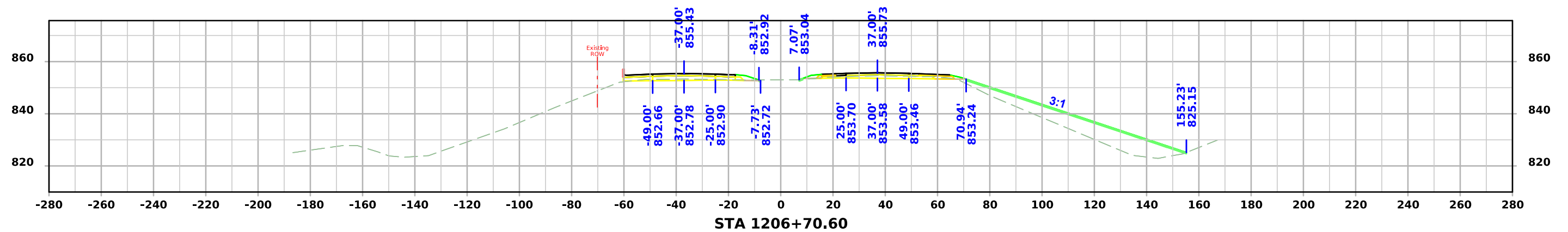
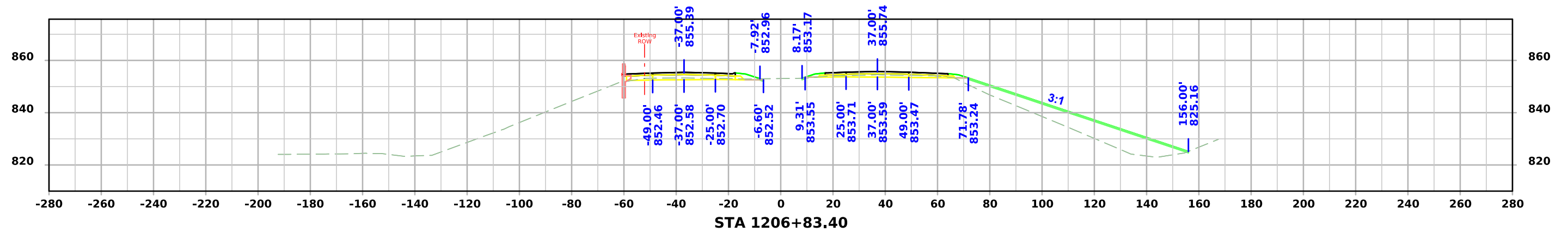
IA HWY 149



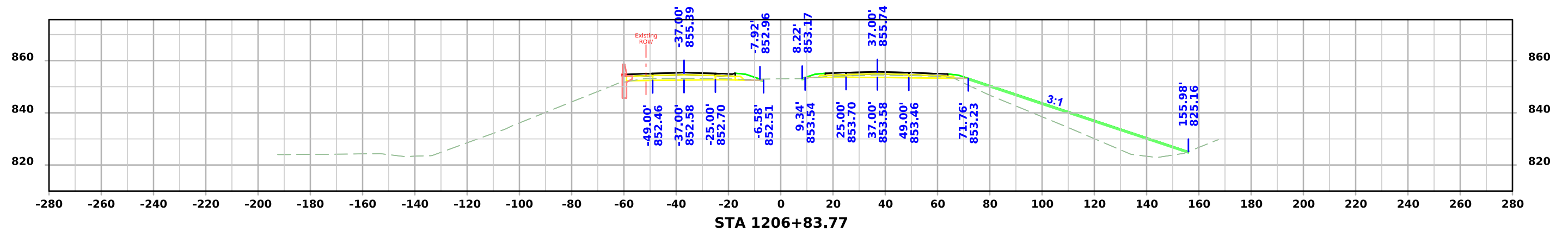
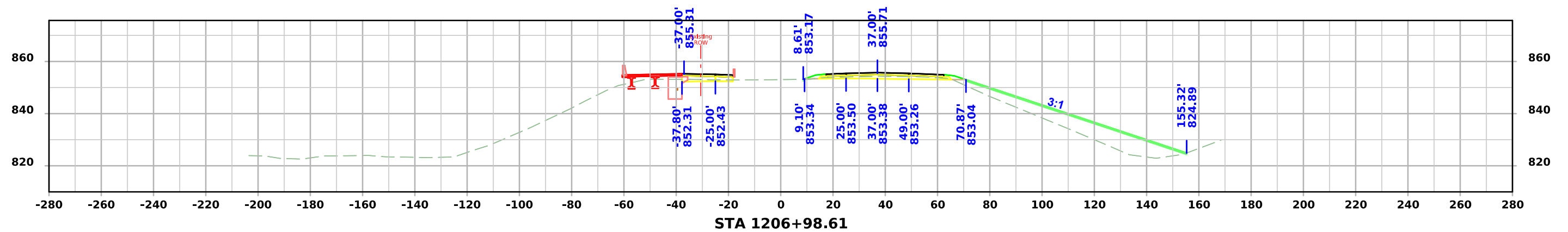
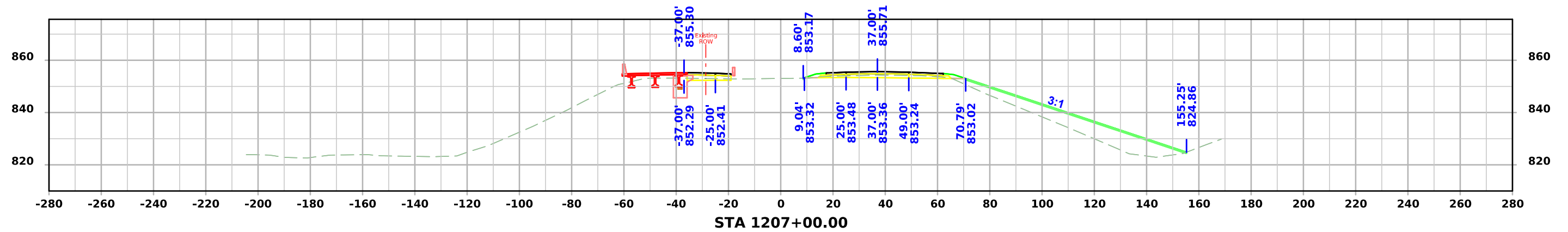
IA HWY 149



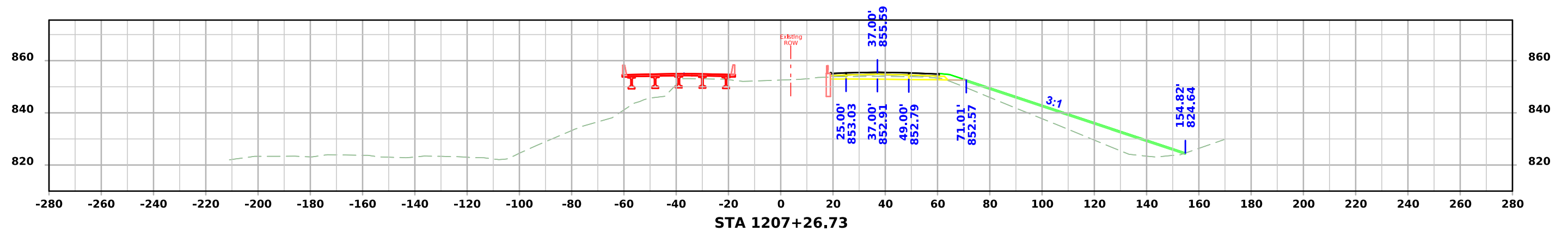
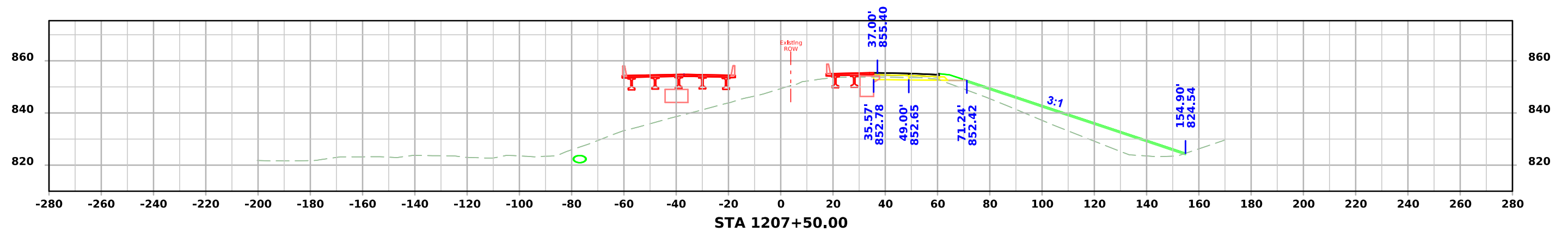
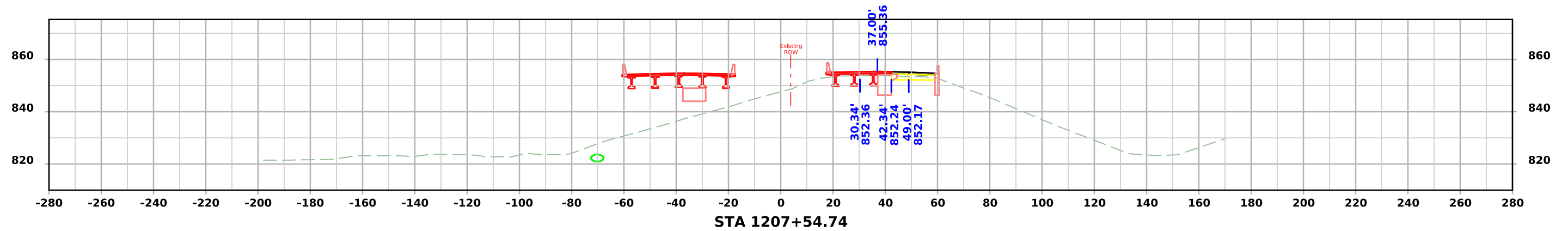
IA HWY 149



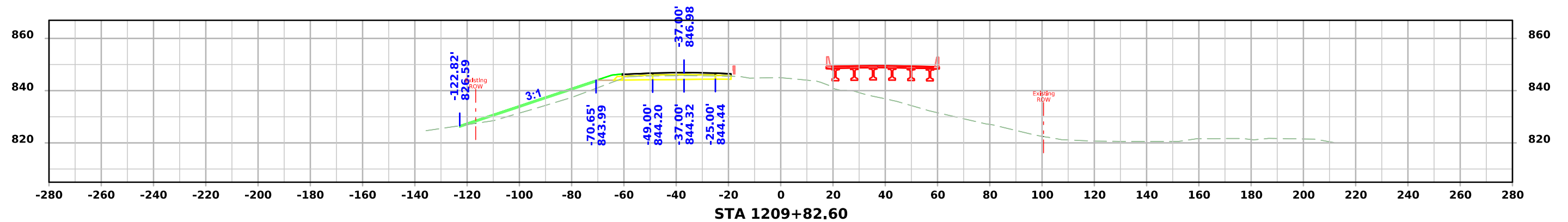
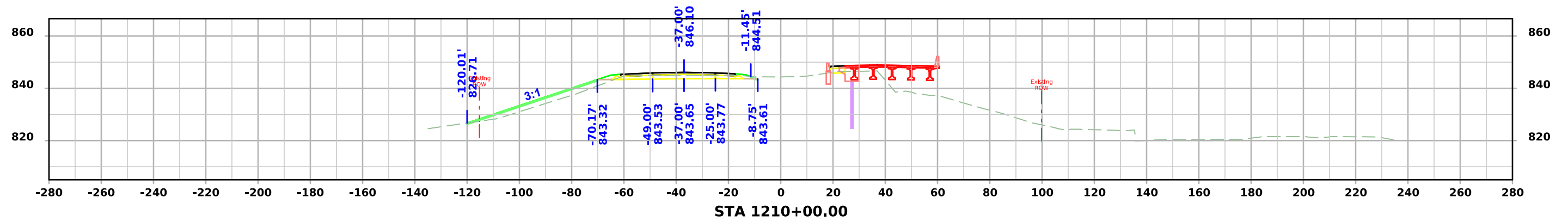
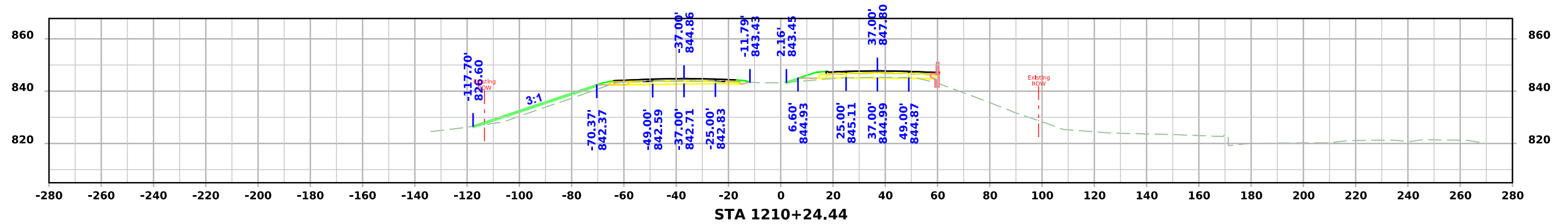
IA HWY 149



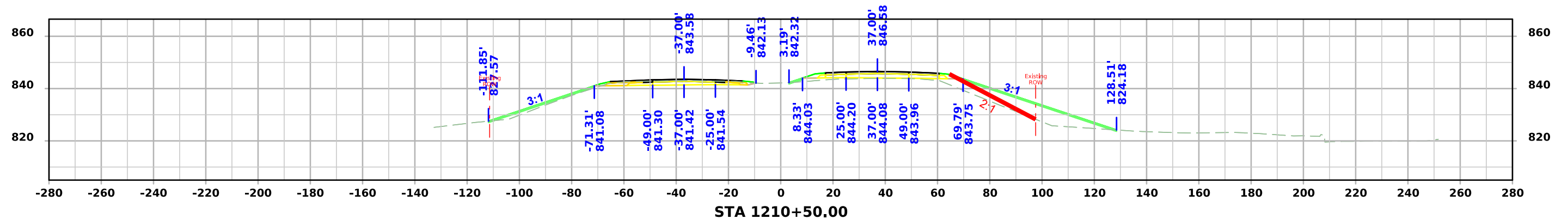
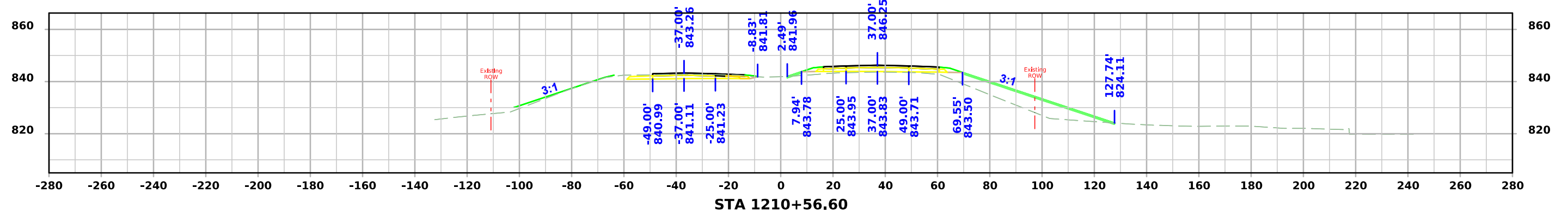
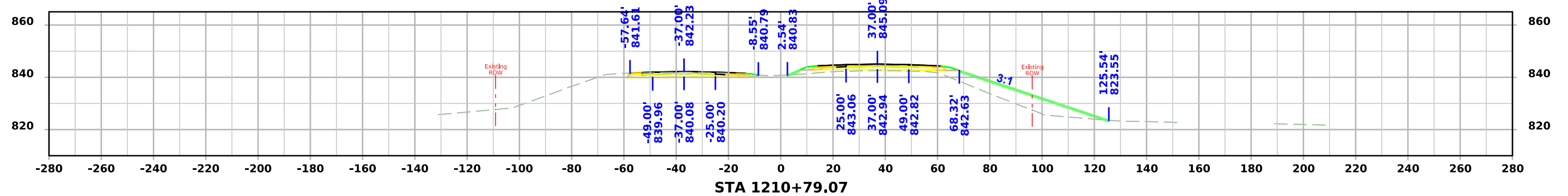
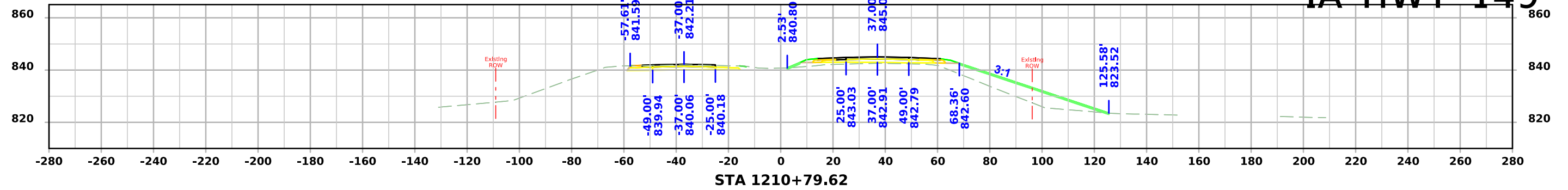
IA HWY 149



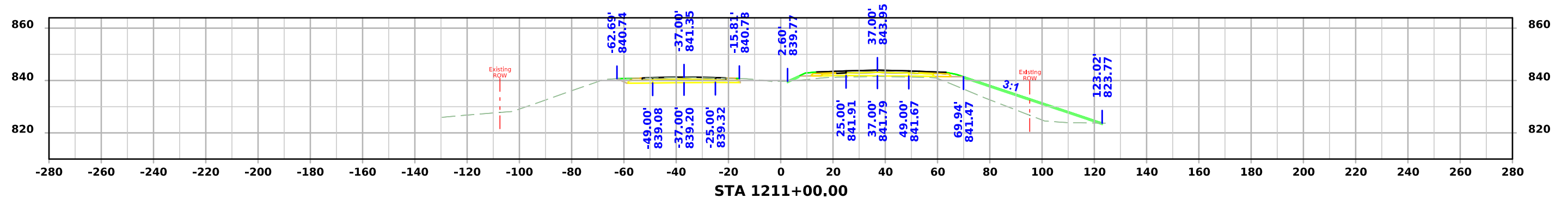
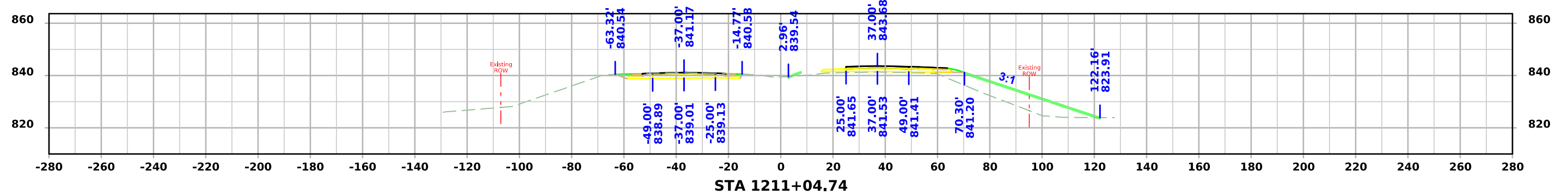
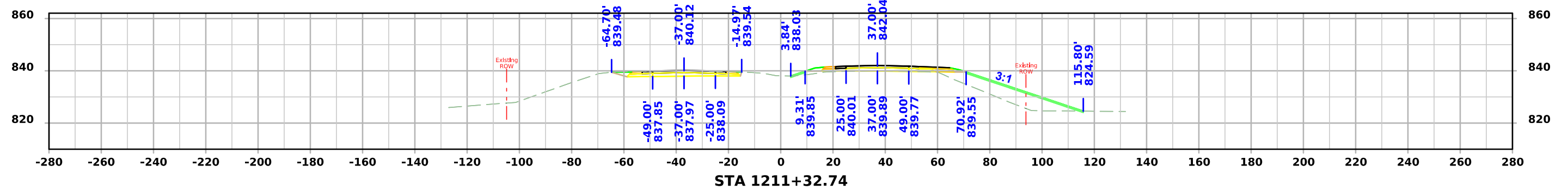
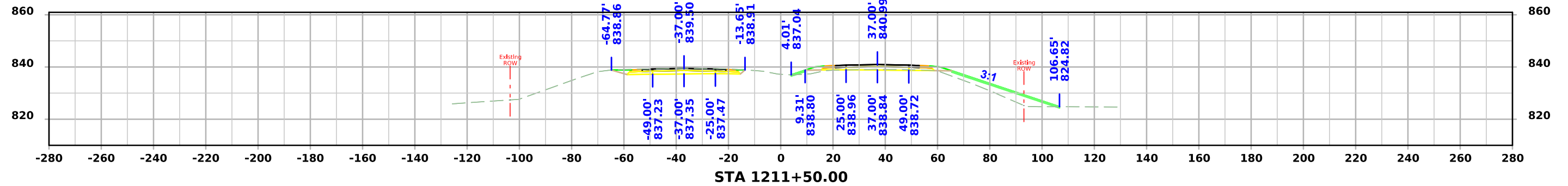
IA HWY 149



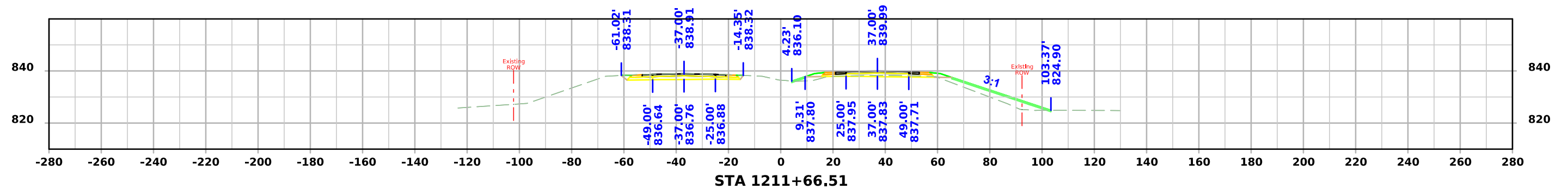
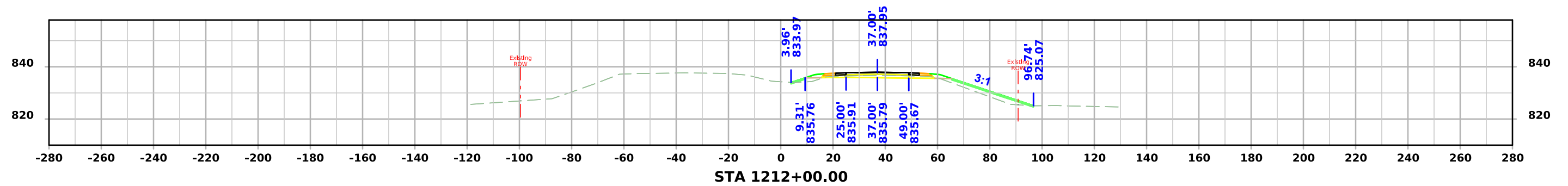
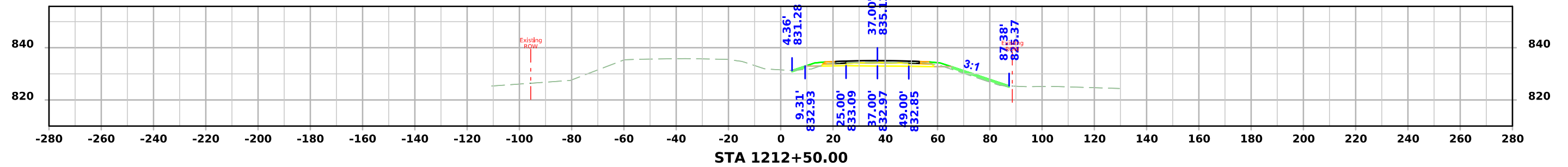
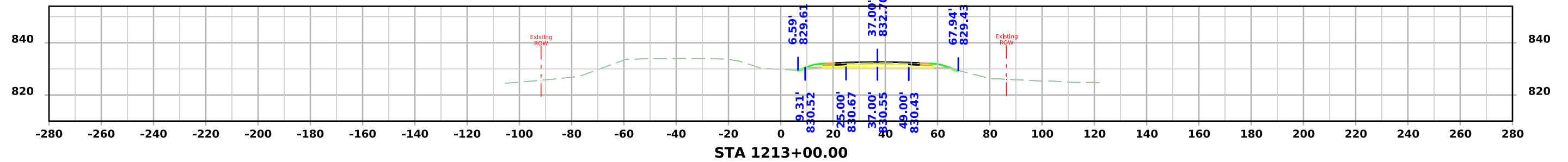
IA HWY 149



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