

PLANS OF PROPOSED IMPROVEMENTS ON THE

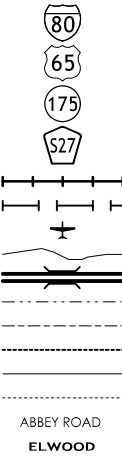
# PRIMARY ROAD SYSTEM SCOTT COUNTY

BRIDGE AND APPROACHES - PPCB  
**IA 22 OVER DONALDSON CREEK**  
**2.8 MILES W.OF I-280**

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2012, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

**LEGEND**

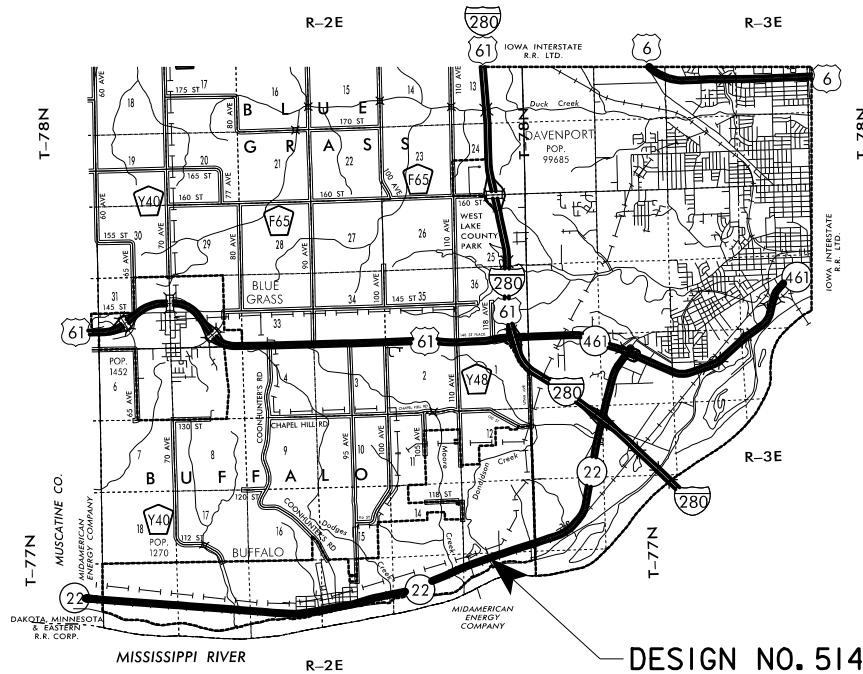
- INTERSTATE HIGHWAY
- PRIMARY HIGHWAY-DIVIDED
- PRIMARY HIGHWAY
- PORTLAND CEMENT CONCRETE ROAD
- ASPHALT ROAD
- BITUMINOUS ROAD
- GRAVEL ROAD
- EARTHEN ROAD
  
- INTERSTATE HIGHWAY
- UNITED STATES HIGHWAY
- STATE HIGHWAY
- COUNTY HIGHWAY
- RAILROAD
- PIPELINE
- AIRPORT
- HYDROLOGY
- BRIDGE
- STATE BOUNDARY
- COUNTY BOUNDARY
- CORPORATE BOUNDARY
- TOWNSHIP LINE
- SECTION LINE
- ROAD NAMES
- UNINCORPORATED PLACE



BRIDGE AND APPROACHES - PPCB  
LETTING DATE 3-18-14  
BRFN-022-5(29)--39-82

SCOTT COUNTY

SCOTT COUNTY - DESIGN NO. 514



STATE OF ILLINOIS

LOCATION MAP

PROJECT DIRECTORY NAME: 8202203010

REVISIONS
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STANDARD ROAD PLANS	
STANDARD ROAD PLANS ARE LISTED ON SHEET NUMBER	C.6

DESIGN DATA RURAL			
2013 AADT	4900	V.P.D.	
2033 AADT	6100	V.P.D.	
2033 DHV	630	V.P.H.	
TRUCKS	21	%	
Total Design ESALS			

TOTAL SHEETS	152
PROJECT NUMBER	BRFN-022-5(29)--39-82
R.O.W. PROJECT NUMBER	
PROJECT IDENTIFICATION NUMBER	10-82-022-030

INDEX OF SHEETS	
NO.	DESCRIPTION
1	TITLE SHEET
2	ESTIMATE SHEET - DESIGN 514
2-30	DESIGN 514
SPS.1-SPS.4	SOIL PROFILE SHEETS
C.1, C.2	ESTIMATE SHEET FOR ROADWAY
A.1 - W.57	ROADWAY SHEETS

INDEX OF SEALS		
SHEET NO.	NAME	TYPE
I	WILLIAM D. TUCKER	STRUCTURAL DESIGN
I	PATRICIA G. SCHWARZ	HYDRAULIC DESIGN
SPS.I, CS.I	ROBERT L. STANLEY	GEOTECHNICAL DESIGN
A.I	JASON M. HOLST	ROADWAY DESIGN

HYDRAULIC DESIGN	
	I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.
	<i>Patricia G. Schwarz</i> 11-26-2013 Signature Date Patricia G. Schwarz Printed or Typed Name
	My license renewal date is December 31, 2014
	Pages or sheets covered by this seal: SHEETS 4 THRU 7 OF 152

STRUCTURAL DESIGN	
	I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.
	<i>William D. Tucker</i> 11-26-2013 Signature Date William D. Tucker Printed or Typed Name
	My license renewal date is December 31, 2015
	Pages or sheets covered by this seal: SHEETS 1 THRU 30 OF 152

**ESTIMATED BRIDGE QUANTITIES**

ITEM NO.	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUAN.
1	2104-2710020	EXCAVATION, CLASS 10, CHANNEL	CY	88.5	
2	2401-6745625	REMOVAL OF EXISTING BRIDGE	LS	1.00	
3	2402-2720000	EXCAVATION, CLASS 20	CY	197	
4	2403-0100010	STRUCTURAL CONCRETE (BRIDGE)	CY	76.9	
5	2403-7000210	HIGH PERFORMANCE STRUCTURAL CONCRETE	CY	251.0	
6	2403-7000220	TRIAL BATCH HIGH PERFORMANCE STRUCTURAL CONCRETE	LS	1.00	
7	2404-7775000	REINFORCING STEEL	LB	246	
8	2404-7775005	REINFORCING STEEL, EPOXY COATED	LB	68,403	
9	2407-0562900	BEAMS, PRETENSIONED PRESTRESSED CONCRETE, BTB100	EACH	9	
10	2408-7800000	STRUCTURAL STEEL	LB	3,484	
11	2414-6424110	CONCRETE BARRIER RAILING	LF	244.0	
12	2501-0201057	PILES, STEEL, HP 10 X 57	LF	610	
13	2501-6335010	PREBORED HOLES	LF	165	
14	2507-0050005	EARTH SCREW ANCHOR	EACH	21	
15	2507-3250005	ENGINEERING FABRIC	SY	1,394.0	
16	2507-6800061	REVTMENT, CLASS E	TON	529.8	
17	2507-8029000	EROSION STONE	TON	10.2	
18	2507-8500060	FABRIC FORMED CONCRETE REVTMENT, ARTICULATING BLOCK MAT, 6 INCH	SY	941.3	
19	2507-8501100	CONCRETE GROUT FOR FABRIC FORMED CONCRETE REVTMENT	CY	169.4	
20	2533-4980005	MOBILIZATION	LS	1.00	
21	2599-9999018	HIGH FRICTION SURFACE TREATMENT	SY	1,526.0	
22	2601-2638650	BRIDGE WING ARMORING - EROSION STONE	SY	18.3	

**ESTIMATE REFERENCE INFORMATION**

ITEM NO.	ITEM CODE	DESCRIPTION
11	2414-6424110	<b>CONCRETE BARRIER RAILING</b> INCLUDES MATERIAL AND LABOR ASSOCIATED WITH PROVIDING AND INSTALLING THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS.  INCLUDES 130 LF OF 2" DIAMETER RIGID STEEL CONDUIT.  IF PLACEMENT OF CONCRETE IS DONE BY THE SLIPFORMING METHOD, CLASS BR CONCRETE IS REQUIRED. CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. PRICE BID FOR THIS ITEM SHALL INCLUDE THE COST OF CAST-IN-PLACE FORMS IF REQUIRED FOR PLACEMENT OF THE CONCRETE.
12	2501-0201057	<b>PILES, STEEL, HP 10 X 57</b> --
13	2501-6335010	<b>PREBORED HOLES</b> ONLY REQUIRED AT WEST ABUTMENT. HOLES MUST BE BREBORED THROUGH ROCK TO ELEVATION 547.63. PREBORING WILL REQUIRE DRILLING THROUGH SOIL, BROKEN AND WEATHERED LIMESTONE, AND MEDIUM HARD TO HARD LIMESTONE.
14	2507-0050005	<b>EARTH SCREW ANCHOR</b> EARTH SCREW ANCHORS SHALL BE FURNISHED AND PLACED IN CONFORMANCE WITH THE PLAN. MEASUREMENT WILL BE PER EACH. FOR CONSTRUCTION OF EARTH SCREW ANCHORS THE CONTRACTOR WILL BE PAID THE CONTRACT PRICE PER EACH FOR THE QUANTITY OF ANCHORS FURNISHED AND PLACED.  EARTH SCREW ANCHORS SHALL BE STEEL HELICAL SCREW ANCHORS, 48" OVERALL LENGTH, 5/8" ROD DIA., 6" HELIX DIAMETER. ANCHORS SHALL BE PAINTED OR GALVANIZED.  ANCHORS SHALL BE INSTALLED SO THAT THE EYE IS EMBEDDED IN THE CENTER OF THE FINISHED ABM BLOCK. THE EYE SHALL CONNECT TO AT LEAST ONE OF THE REINFORCING CABLES BY EITHER 1) HOOKING AROUND A CABLE OR 2) USE OF A SHORT LOOP OF CABLE, SPLICED PER THE SPECIFICATIONS. INSTALLATION OF THE ANCHOR MAY BE MADE BY SLITTING OF THE FABRIC AND SEALING OF THE SLIT WITH TAPE.
15	2507-3250005	<b>ENGINEERING FABRIC</b> ENGINEERING FABRIC SHALL BE MATERIAL AS SPECIFIED FOR EMBANKMENT EROSION CONTROL IN ACCORDANCE WITH ARTICLE 4196.01,B,3, OF THE STANDARD SPECIFICATIONS.
16	2507-6800061	<b>REVTMENT, CLASS E</b> ESTIMATED AT 1.6 TON/CY.
17	2507-8029000	<b>EROSION STONE</b> ESTIMATED AT 1.6 TON/CY.
18	2507-8500060	<b>FABRIC FORMED CONCRETE REVTMENT, ARTICULATING BLOCK MAT, 6 INCH</b> FABRIC FORMED REVTMENT ARTICULATING MAT DIMENSIONS SHOWN ON THE DRAWINGS REFLECT THE FINISHED DIMENSION ALONG GRADE FOLLOWING FILLING WITH CONCRETE GROUT. THE MAT QUANTITY IS BASED ON THE FINISHED MAT DIMENSIONS.  THE FABRIC FORMED REVTMENT QUANTITY INCLUDES THE FORM AREA IN ANCHOR, TERMINAL OR TOE TRENCHES SHOWN ON THE DRAWINGS AND ALL FORM LAYERS IN OVERLAPS SHOWN ON THE DRAWINGS. LABOR, EQUIPMENT AND MATERIALS REQUIRED TO ANCHOR FABRIC FORMS DURING PLACEMENT AND CURING OF GROUT SHALL BE CONSIDERED INCIDENTAL TO THIS WORK.  ARTICULATING MAT DIMENSIONS SHOWN ON THE DRAWINGS ARE IN INCREMENTS OF THE SPECIFIED FINISHED BLOCK DIMENSIONS, 20 INCH IN THE MILL WIDTH (MW) DIRECTION X 20 INCH PERPENDICULAR TO THE MILL WIDTH DIRECTION.  ANY CABLE PROTRUDING FROM MAT THAT FORMS A CLOSED LOOP SHALL BE CUT FOLLOWING PLACEMENT AND INITIAL SET OF GROUT TO PREVENT SNAGGING OF FLOOD DEBRIS BY THE CABLE LOOP. THE RESULTANT CABLE ENDS PROTRUDING FROM THE MAT SHALL NOT EXCEED 18 INCH IN LENGTH.  GROUT SHALL NOT BE DISCHARGED ON THE TOP OF THE MAT THAT WILL FILL THE SEAM 'VALLEY' BETWEEN BLOCKS. A WEAKENED PLANE ALONG THE SEAM LINES IS REQUIRED TO FACILITATE CRACKING AND ARTICULATION.  COMPENSATION FOR EXCAVATION WILL BE MADE UNDER THE EXCAVATION, CLASS 10 CHANNEL BID ITEM.
19	2507-8501100	<b>CONCRETE GROUT FOR FABRIC FORMED CONCRETE REVTMENT</b> --
20	2533-4980005	<b>MOBILIZATION</b> --
21	2599-9999018	<b>HIGH FRICTION SURFACE TREATMENT</b> CONCRETE SHALL HAVE A MINIMUM AGE OF 28 DAYS BEFORE BEGINNING SURFACE PREPARATION. THE ENTIRE BRIDGE DECK WILL RECIEVE TREATMENT, BUT ONLY THE 48 FOOT WIDTH OF TRAFFIC LANES ON APPROACHES WILL RECIEVE TREATMENT. INCLUDES 779 SY FOR THE BRIDGE AND 747 SY FOR THE APPROACHES. INCLUDES 70 FEET OF APPROACH LENGTH AT EACH END OF BRIDGE.
22	2601-2638650	<b>BRIDGE WING ARMORING - EROSION STONE</b> INCLUDES FURNISHING AND PLACING ENGINEERING FABRIC, EROSION STONE, AND ALL REQUIRED EXCAVATING, SHAPING AND COMPACTING FOR WING ARMORING.

**ESTIMATE REFERENCE INFORMATION**

ITEM NO.	ITEM CODE	DESCRIPTION
1	2104-2710020	<b>EXCAVATION, CLASS 10, CHANNEL</b> INCLUDES EXCAVATION NECESSARY FOR PLACEMENT OF ALL REVTMENT.
2	2401-6745625	<b>REMOVAL OF EXISTING BRIDGE</b> --
3	2402-2720000	<b>EXCAVATION, CLASS 20</b> --
4	2403-0100010	<b>STRUCTURAL CONCRETE (BRIDGE)</b> INCLUDES FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), FLOODABLE BACKFILL, POROUS BACKFILL, GEOTEXTILE FABRIC, WATER FLOODING, AND SUBDRAIN OUTLET AND PIPE CLIP AT ABUTMENTS.  INCLUDES FURNISHING AND PLACING 3 INCH DIAMETER PVC PLASTIC PIPE AND EXPANDING FOAM IN THE ABUTMENT WINGS.  INCLUDES 2.2 CY OF CONCRETE TO BE PLACED IN BOTTOM OF PREBORED HOLES AT WEST ABUTMENT. QUANTITY IS BASED ON A 3 FOOT DEPTH OF CONCRETE IN 11-18 INCH DIAMETER HOLES.
5	2403-7000210	<b>HIGH PERFORMANCE STRUCTURAL CONCRETE</b> THIS BID ITEM INCLUDES THE CONCRETE FOR THE SLAB AND ABUTMENT DIAPHRAGMS. REFER TO THE DEVELOPMENTAL SPECIFICATION FOR "HIGH PERFORMANCE CONCRETE FOR STRUCTURES" FOR ADDITIONAL INFORMATION.
6	2403-7000220	<b>TRIAL BATCH HIGH PERFORMANCE STRUCTURAL CONCRETE</b> --
7	2404-7775000	<b>REINFORCING STEEL</b> --
8	2404-7775005	<b>REINFORCING STEEL, EPOXY COATED</b> INCLUDES MECHANICAL SPLICERS IN THE ABUTMENT FOOTING.
9	2407-0562900	<b>BEAMS, PRETENSIONED PRESTRESSED CONCRETE, BTB100</b> COARSE AGGREGATES FOR PRESTRESSED CONCRETE BRIDGE UNITS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 4115 CLASS III DURABILITY. GRADATION OF THE COARSE AGGREGATE SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF ARTICLE 2407.02, A, OF THE STANDARD SPECIFICATIONS.  INCLUDES ABUTMENT BEARING MATERIAL.
10	2408-7800000	<b>STRUCTURAL STEEL</b> INCLUDES 6 DRAINS AT 92 LB EACH.

NOTE:  
ROADWAY QUANTITIES SHOWN  
ELSEWHERE IN THESE PLANS.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED  
CONCRETE BEAM BRIDGE**  
  
**QUANTITIES**  
STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 1 OF 29 FILE NO. 30687 DESIGN NO. 514

**GENERAL NOTES:**

THIS DESIGN IS FOR REPLACEMENT OF THE EXISTING 30' x 83'-5 CONCRETE ARCH BRIDGE, DESIGN NO. 4124 & 370 SCOTT COUNTY, WITH A 100'-0 x 68' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE. PLANS OF THE EXISTING BRIDGE WILL BE MADE AVAILABLE TO THE CONTRACTOR. CONTACT THE OFFICE OF CONTRACTS - HIGHWAY DIVISION - IOWA D.O.T. - AMES.

THE LUMP SUM BID FOR "REMOVAL OF EXISTING BRIDGE" SHALL INCLUDE REMOVAL OF THE EXISTING 30' x 83'-5 CONCRETE ARCH BRIDGE. STAGE 1 CONSTRUCTION OF NEW BRIDGE WILL BE COMPLETED BEFORE STARTING REMOVAL OF EXISTING ARCH BRIDGE. PORTIONS OF THE SOUTH FLARED WINGWALLS AND ARCH FOOTINGS SHALL REMAIN IN PLACE. PORTIONS OF THE 2 ABANDONED WATER LINES IN THE STREAM BED SHALL BE REMOVED AS NECESSARY TO COMPLETE CONSTRUCTION.

REMOVALS SHALL BE IN ACCORDANCE WITH SECTION 2401 OF THE STANDARD SPECIFICATIONS. PARTIAL REMOVALS OF CONCRETE SHALL BE INITIATED WITH A 3/4 INCH SAW CUT.

ANY DAMAGE TO ANY STEEL OR CONCRETE NOT TO BE REMOVED SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND REPAIRED AT NO EXTRA COST TO THE STATE.

CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD WILL REQUIRE THE USE OF A CLASS BR CONCRETE IN ACCORDANCE WITH ARTICLE 2513.03, A, 2 OF THE STANDARD SPECIFICATIONS. CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. CLASS D CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS (CAST-IN-PLACE OR SLIPFORMED METHOD).

THESE BRIDGE PLANS LABEL ALL REINFORCING STEEL WITH ENGLISH NOTATION (5g1 IS 5/8 INCH DIAMETER BAR). ENGLISH REINFORCING STEEL RECEIVED IN THE FIELD MAY DISPLAY THE FOLLOWING "BAR DESIGNATION". THE "BAR DESIGNATION" IS THE STAMPED IMPRESSION ON THE REINFORCING BARS, AND IS EQUIVALENT TO THE BAR DIAMETER IN MILLIMETERS.

ENGLISH SIZE	3	4	5	6	7	8	9	10	11
BAR DESIGNATION	10	13	16	19	22	25	29	32	36

THIS BRIDGE IS DESIGNED FOR HL-93 LOADING, PLUS 20 LBS. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.

IT SHALL BE THE BRIDGE CONTRACTOR'S RESPONSIBILITY TO PROVIDE SITES FOR EXCESS EXCAVATED MATERIAL. NO PAYMENT FOR OVERHAUL WILL BE ALLOWED FOR MATERIAL HAULED TO THESE SITES.

**SPECIFICATIONS:**

DESIGN: AASHTO LRFD 6th Ed, SERIES OF 2012.

CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2012, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT INCLUDING SPECIAL PROVISIONS FOR:  
 -WORK ON RR ROW (D, M, & E I.e. CP)  
 -HIGH FRICTION SURFACE TREATMENT  
 AND DEVELOPMENTAL SPECIFICATIONS FOR:  
 -FABRIC FORMED CONCRETE STRUCTURE REVETMENT  
 -HIGH PERFORMANCE CONCRETE FOR STRUCTURES  
 -FLOATING SILT CURTAIN  
 -COMPACTION WITH MOISTURE CONTROL

**DESIGN STRESSES:**

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 6th Ed, SERIES OF 2012. REINFORCING STEEL IN ACCORDANCE WITH LRFD AASHTO SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH LRFD AASHTO SECTION 5, f'c = 4.0 KSI, EXCEPT PRESTRESSED BEAM AND BRIDGE DECK CONCRETE AS NOTED. PRESTRESSED CONCRETE BEAMS, SEE DESIGN SHEETS 21. BRIDGE DECK = f'c = 5.0 KSI. STRUCTURAL STEEL IN ACCORDANCE WITH LRFD AASHTO SECTION 6. ASTM A709 GRADE 36, (AASHTO M270 GRADE 36).

FAINT LINES ON PLANS INDICATE THE EXISTING STRUCTURE.

UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE BRIDGE CONTRACTOR OF THE STARTING DATE.

KEYWAY DIMENSIONS SHOWN ON THE PLANS ARE BASED ON NOMINAL DIMENSIONS UNLESS STATED OTHERWISE. IN ADDITION, THE BEVEL USED ON THE KEYWAY SHALL BE LIMITED TO A MAXIMUM OF 10 DEGREES FROM VERTICAL.

THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING (SHEET PILE OR OTHER) TO PREVENT THE EARTH UNDER THE TRAFFIC LANE, FROM SLOUGHING IN DURING CONSTRUCTION. ALL COST OF SHORING, WILL BE CONSIDERED INCIDENTAL TO CONSTRUCTION AND NO DIRECT PAYMENT WILL BE MADE. ALL MATERIAL USED FOR SHORING SHALL REMAIN THE PROPERTY OF THE CONTRACTOR, SHORING IS TO BE REMOVED ONLY AFTER BACKFILLING HAS BEEN COMPLETED. THE CONTRACTOR SHALL SUBMIT SHORING PLANS FOR REVIEW. IN ADDITION TO THE REQUIREMENTS NOTED ABOVE, ARTICLE 1107.07 OF THE STANDARD SPECIFICATIONS, STILL APPLIES.

FLOOD PLAIN DEVELOPMENT PERMIT NUMBER FP 2012-109 SHALL APPLY TO WORK ON THIS PROJECT.

THE CONTRACTOR SHALL NOTE THE STANDARD ABUTMENT DETAILS FOR THE EAST ABUTMENT HAVE BEEN MODIFIED TO OFFSET THE ABUTMENT FOOTING FROM THE WINGWALL TO AID IN TYING THE REINFORCING STEEL BETWEEN THE FOOTING TO WINGWALL AND THE FOOTING TO BACKWALL.

CONCRETE FORMS ARE REQUIRED TO REMAIN IN PLACE 5 DAYS OR LONGER IN ACCORDANCE WITH ARTICLE 2403.03, M, 2, OF THE STANDARD SPECIFICATIONS, EXCEPT THE MINIMUM CONCRETE FLEXURAL STRENGTH REQUIRED BEFORE REMOVAL OF FORMS SHALL BE 575 PSI.

THE BRIDGE CONTRACTOR IS TO CLEAR AND/OR SHAPE THE CHANNEL WITHIN THE LIMITS AS SHOWN ON THE "SITUATION PLAN" AND "LONGITUDINAL SECTION ALONG CENTERLINE APPROACH ROADWAY".

THIS PROJECT USES THE LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHODOLOGY FOR DETERMINING PILE CONTRACT LENGTH AND NOMINAL AXIAL BEARING RESISTANCE. NOMINAL AXIAL BEARING RESISTANCES WILL BE LARGER THAN BEARING VALUES IN THE PAST, BUT CONSTRUCTION CONTROL BLOW COUNTS WILL BE APPROXIMATELY THE SAME. A WEAP ANALYSIS AND BEARING GRAPH WILL BE PREPARED BY THE OFFICE OF CONSTRUCTION THAT GIVES THE RELATIONSHIP BETWEEN REQUIRED NOMINAL AXIAL BEARING RESISTANCE AND BLOW COUNT.

FOR THE CONTRACTOR'S BIDDING PURPOSES, PARTICULARLY FOR THE SIZING OF THE PILE DRIVING HAMMER, THE APPROXIMATE PREVIOUS DESIGN METHODOLOGY BEARING VALUES AT END OF DRIVE (EOD) ARE GIVEN BELOW. THESE VALUES SHALL NOT BE USED FOR CONSTRUCTION CONTROL AND ARE GIVEN ONLY FOR COMPARATIVE PURPOSES.

THE PREVIOUS DESIGN BEARING FOR THE WEST ABUTMENT PILES WOULD HAVE BEEN ABOUT 63 TONS. THE PREVIOUS DESIGN BEARING FOR THE EAST ABUTMENT PILES WOULD HAVE BEEN ABOUT 62 TONS.

THE NEW CONCRETE FOR THE BRIDGE DECK AND APPROACHES WILL NOT REQUIRE LONGITUDINAL GROOVING. THE FINAL FINISH SHALL BE A BROOM FINISH IN ACCORDANCE WITH ARTICLE 2301.03, H, 2 OF THE STANDARD SPECIFICATIONS.

**BRIDGE DECK DIMENSIONS TABLE**

NO.	ITEM	UNIT	QUANTITY
1	DECK LENGTH	L.F.	103.0
2	MINIMUM DECK WIDTH	L.F.	71.2
3	MAXIMUM DECK WIDTH	L.F.	71.2
4	DECK AREA	S.F.	7334

1. DECK LENGTH IS MEASURED FROM FACE-TO-FACE OF PAVING NOTCHES ALONG THE CENTERLINE OF THE ROADWAY.
2. 3. DECK WIDTHS ARE MEASURED FROM OUT-TO-OUT OF DECK PERPENDICULAR TO THE CENTERLINE OF ROADWAY.
4. DECK AREA IS TO BE BASED ON THE FACE-TO-FACE PAVING NOTCH DISTANCE AND OUT-TO-OUT DECK DIMENSIONS.

**MECHANICAL SPLICE NOTES:**

THE 8a1 and 8a2 BARS IN THE ABUTMENT FOOTING SHALL BE SPLICED AT THE LOCATIONS SHOWN USING MECHANICAL SPLICE ASSEMBLIES. MECHANICAL SPLICE ASSEMBLIES CONSIST OF MECHANICAL SPLICERS AND REINFORCING SPLICE BARS AS REQUIRED TO FACILITATE THE USE OF THE MECHANICAL SPLICER. THE MECHANICAL SPLICE ASSEMBLY USED SHALL MEET THE REQUIREMENTS OF MATERIALS IM 451 APPENDIX E. REINFORCING SPLICE BARS SHALL BE A MINIMUM OF 1 INCH DIA. FOR THE 8a1 AND 8a2 BARS. ALL MECHANICAL SPLICE ASSEMBLIES TO BE USED IN SPLICING THE 8a1 AND 8a2 BARS IN THE ABUTMENT FOOTINGS SHALL BE EPOXY COATED.

THE COST OF ALL SPLICE ASSEMBLIES IS TO BE INCLUDED IN THE PRICE BID FOR "REINFORCING STEEL EPOXY COATED" AND NO SEPARATE PAYMENT WILL BE MADE. THE WEIGHT OF MECHANICAL SPLICE ASSEMBLIES IS NOT INCLUDED IN THE QUANTITY SHOWN FOR "REINFORCING STEEL EPOXY COATED". A TOTAL OF 18 EPOXY COATED SPLICE ASSEMBLIES WILL BE REQUIRED.

**DESIGN HISTORY AT THIS SITE**

DES. NO.	TYPE OF WORK
4124	ORIGINAL ARCH BRIDGE
370	ARCH BRIDGE EXTENSION
514	REPLACEMENT PPCB BRIDGE

**TRAFFIC CONTROL PLAN**

NOTE: THE ROADWAY WILL BE OPEN TO THRU TRAFFIC. REFER TO THE TRAFFIC CONTROL PLAN ON THE ROAD PLAN IN THESE PLANS.

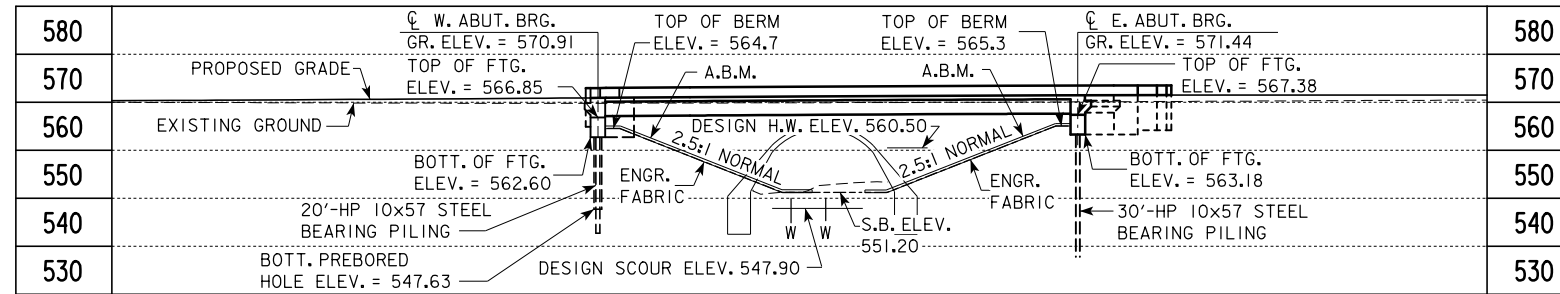
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**

**GENERAL NOTES**

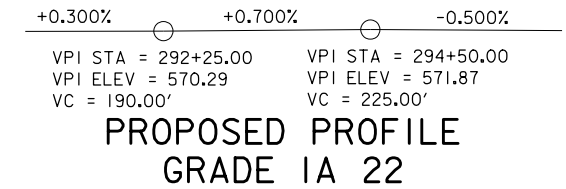
STATION: 293+68.30 DECEMBER, 2012

**SCOTT COUNTY**

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 2 OF 29 FILE NO. 30687 DESIGN NO. 514



LONGITUDINAL SECTION ALONG CL APPROACH ROADWAY



**HYDRAULIC DATA**

DRAINAGE AREA = 3.3 SQ. MI.  
 STREAM SLOPE = 47.6 FT./MI.

Q2 = 390 CFS  
 STAGE = 555.7  
 CHANNEL VELOCITY = 4.2 FPS

Q50 = 1920 CFS  
 STAGE = 560.5  
 BACKWATER = 0.0 FT.  
 AVG. BRIDGE VELOCITY = 5.2 FPS

Q100 = 2320 CFS  
 STAGE = 561.3  
 BACKWATER = 0.0 FT.  
 AVG. BRIDGE VELOCITY = 5.5 FPS  
 CALCULATED DESIGN SCOUR = 547.9\*

Q500 = 3110 CFS  
 STAGE = 562.9  
 AVG. BRIDGE VELOCITY = 5.8 FPS  
 CALCULATED CHECK SCOUR = 547.9\*

ROADWAY OVERTOP 570.0  
 STA. 291+30

EXTREME HW STAGE = NOT AVAILABLE  
 AVG. LOW WATER STAGE = 551.4

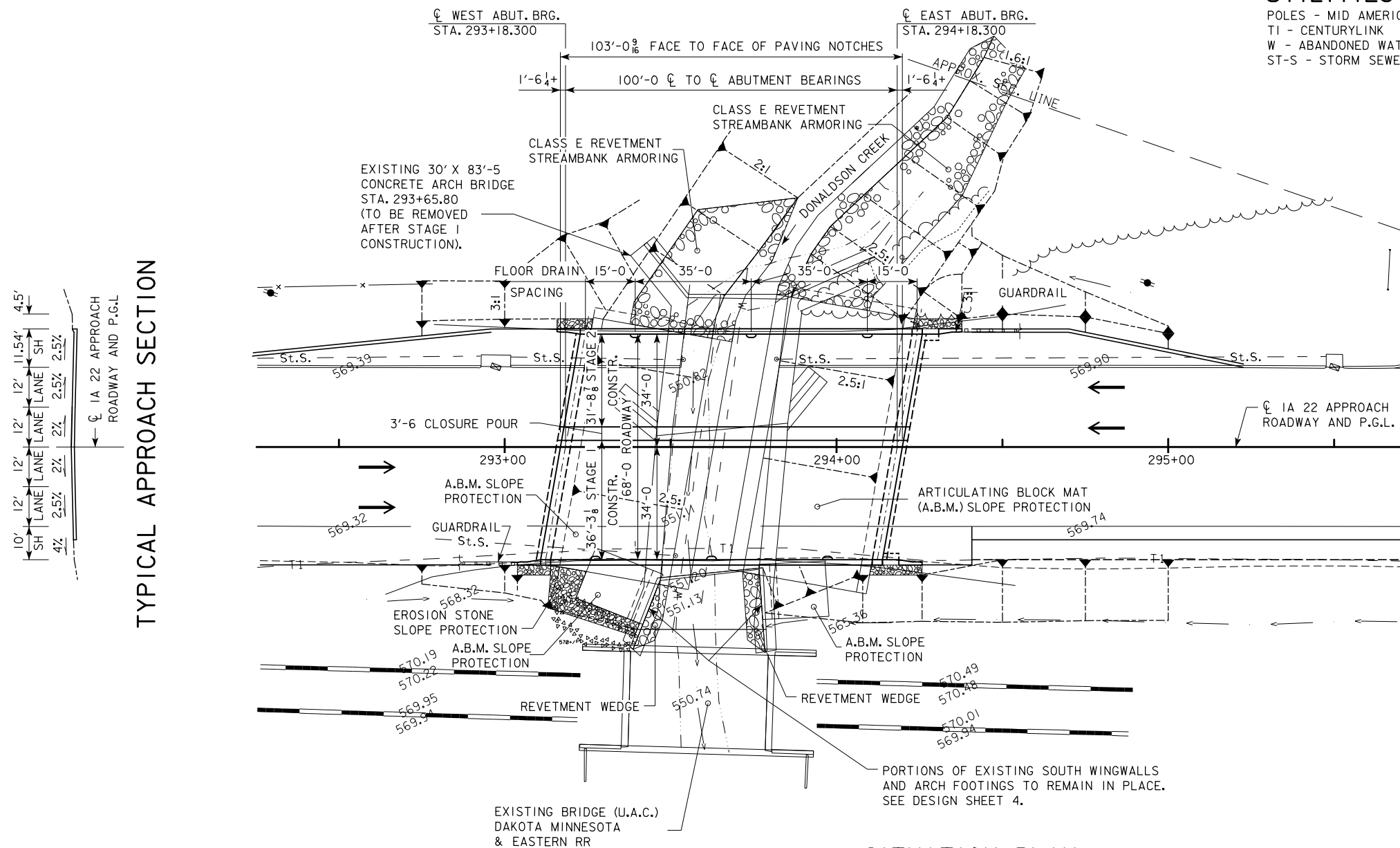
\* ACTUAL SCOUR WILL BE DEPENDENT UPON SOILS REVIEW OF THE ELEVATION TO COMPETENT ROCK.

**NOTES:**

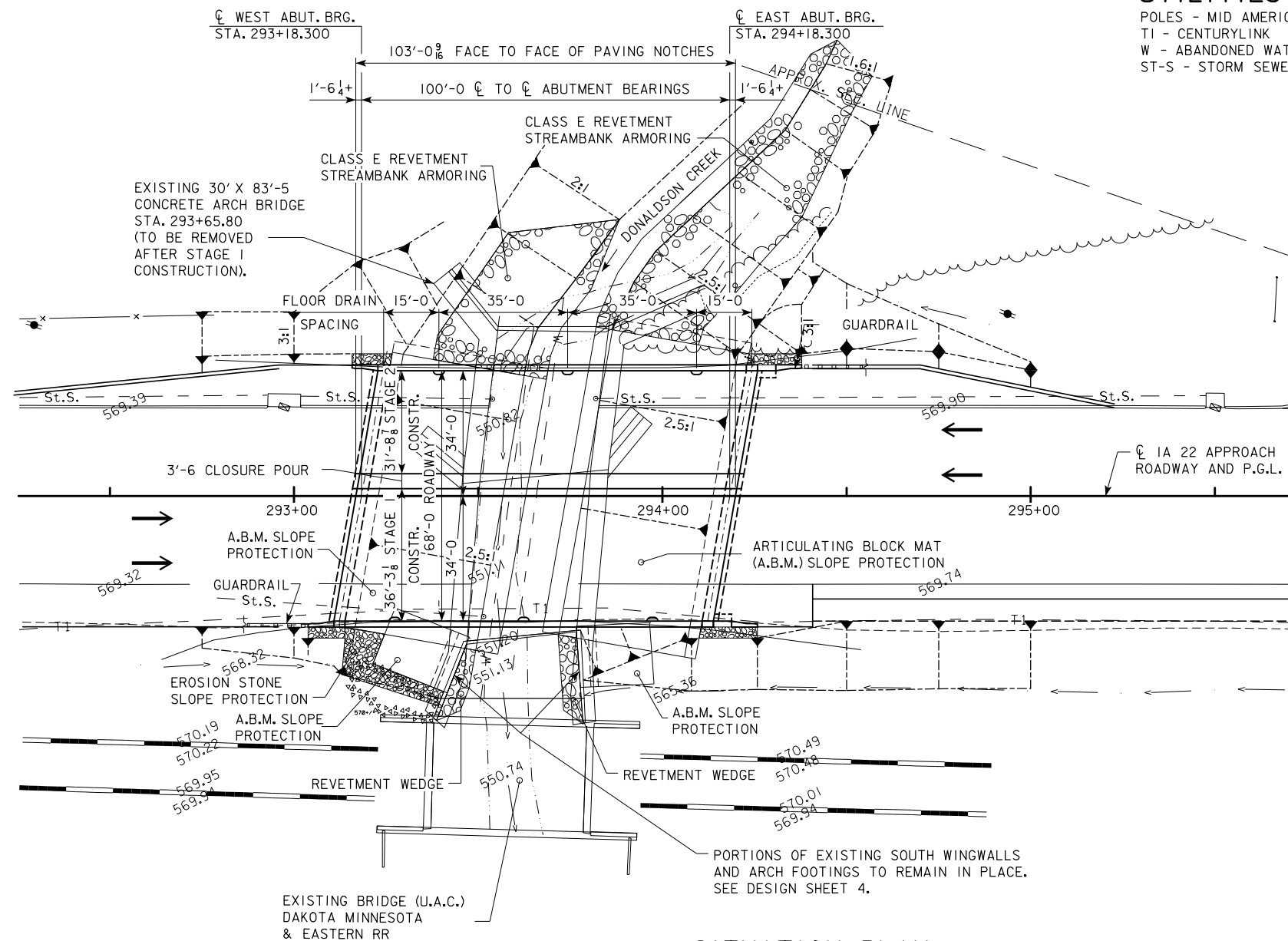
TOP OF BRIDGE DECK CROWN 0.03' BELOW PROFILE GRADE.

**UTILITIES LEGEND:**

POLES - MID AMERICAN ENERGY  
 TI - CENTURYLINK  
 W - ABANDONED WATER LINE LINWOOD MINING  
 ST-S - STORM SEWER CITY OF BUFFALO



TYPICAL APPROACH SECTION



SITUATION PLAN

**TRAFFIC ESTIMATE**

2013 AADT	4900	V.P.D.
2033 AADT	6100	V.P.D.
2033 DHV	630	V.P.H.
TRUCKS	21	%
TOTAL DESIGN ESALs		

**LOCATION**

IA 22 OVER DONALDSON CREEK  
 T-77N R-2E  
 SECTION 24  
 BUFFALO TOWNSHIP  
 SCOTT COUNTY  
 FHWA NO. 047181  
 EXISTING BRIDGE MAINT. NO. 8292.8S022  
 LATITUDE 41.464382°  
 LONGITUDE -90.681313°

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**

**SITUATION PLAN**  
 STATION: 293+68.30    DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 3 OF 29    FILE NO. 30687    DESIGN NO. 514

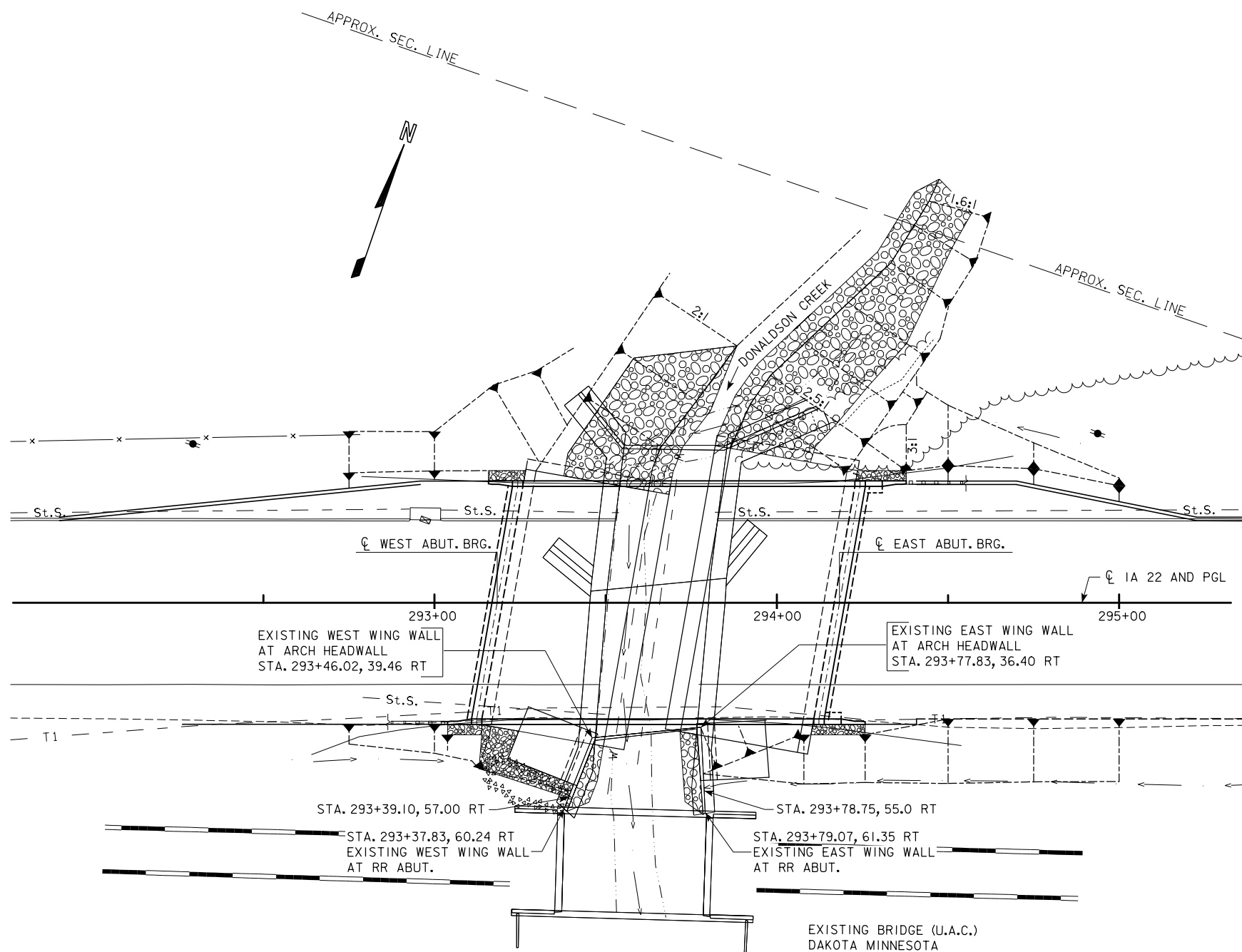
580	STA. 293+37.83, 60.24 RT AT R.R. ABUT. ELEV. 561.0 (U.A.C.)	STA. 293+39.10, 57.00 RT EXISTING ELEV. 561.9± (U.A.C.)	580
570	PROPOSED TOP OF WALL	STA. 293+46.02, 39.46 RT ARCH BRIDGE HDWL PROPOSED ELEV. 552.7	570
560	EXISTING TOP OF WALL		560
550	EXISTING GROUND CREEK SIDE OF WALL (FRONT FACE)		550
540			540
530	BOTT. FTG. (U.A.C.)	CL STREAMBED ELEV. 551.2	530

SECTION ALONG EXISTING WEST WING WALL

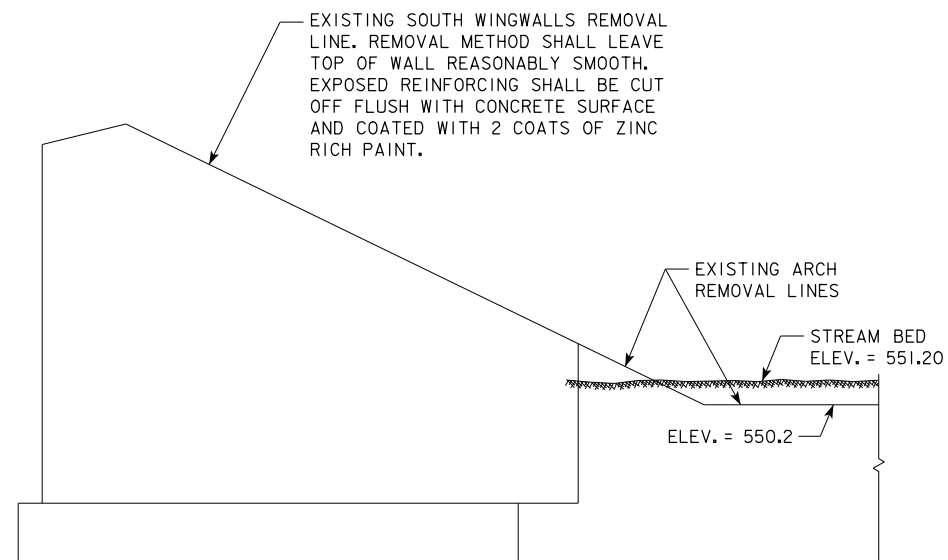
580	STA. 293+78.75, 55.0 RT EXISTING ELEV. 562.4± U.A.C.	BENCH MARK NO. 603 STA. 293+83.546, 45.303 LT. FD IHC BM ON INHDWL OF ARCH BRG, ELEV. 566.498	580
570	STA. 293+77.83, 36.40 RT AT ARCH BRIDGE HEADWALL PROPOSED ELEV. 553.5	STA. 293+79.07, 61.35 RT EXISTING EAST WING WALL AT RR ABUT. ELEV. 561.07 (U.A.C.)	570
560	PROPOSED TOP OF WALL	EXISTING TOP OF WALL	560
550	EXISTING GROUND CREEK SIDE OF WALL (FRONT FACE)		550
540			540
530	BOTT. FTG. U.A.C.	CL STREAMBED ELEV. 551.2	530

SECTION ALONG EXISTING EAST WING WALL

NOTES:  
THE DOWNSTREAM WING WALLS WILL REMAIN IN PLACE, WITH PROPOSED MODIFICATION TO THE TOP OF WALL PROFILES AS SHOWN IN THE SECTIONS.

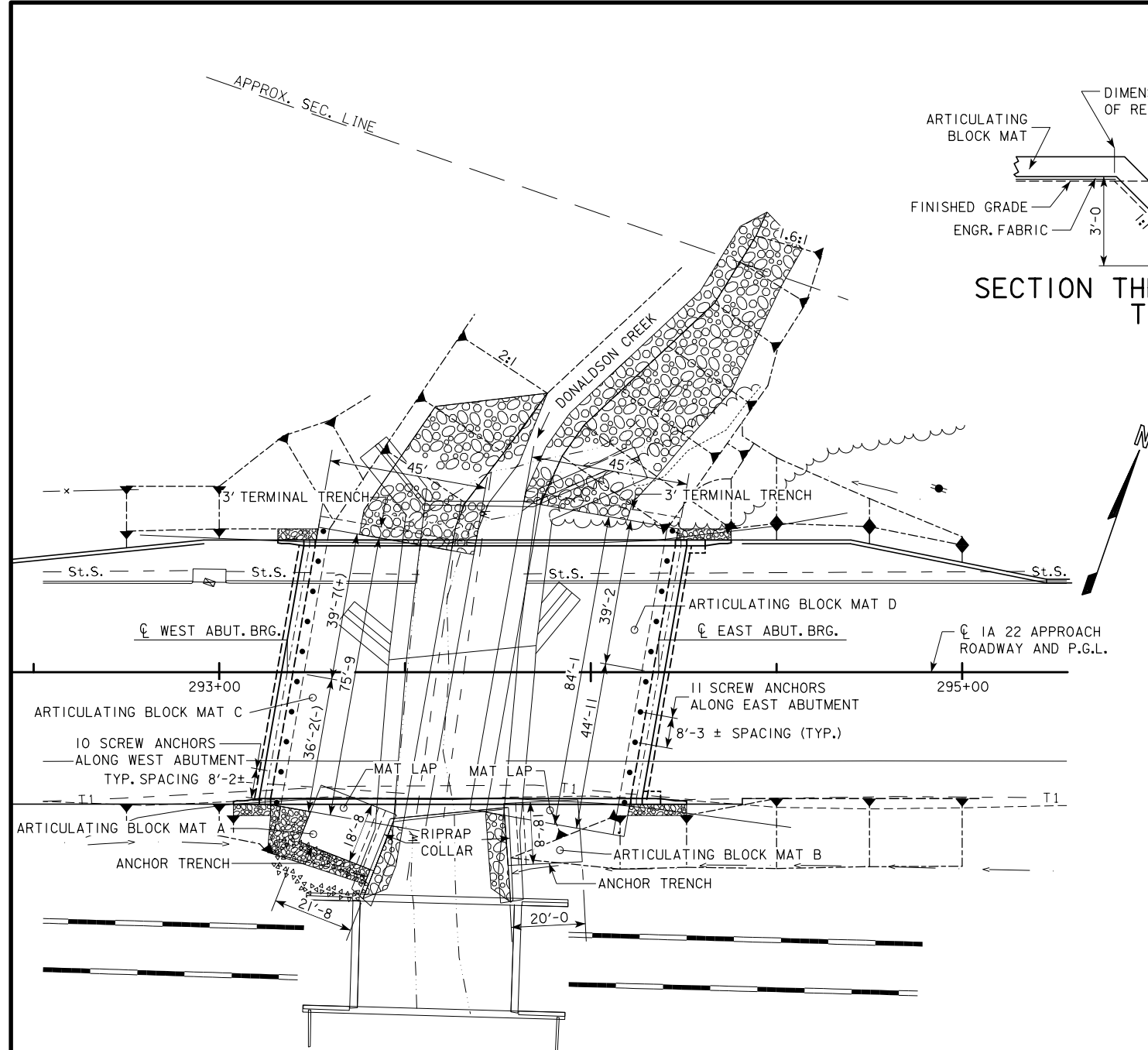


SITUATION PLAN

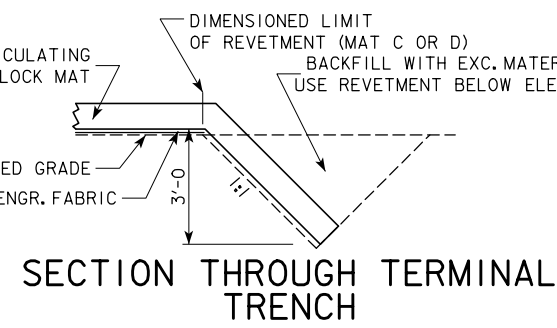


EXISTING ARCH REMOVAL LIMITS

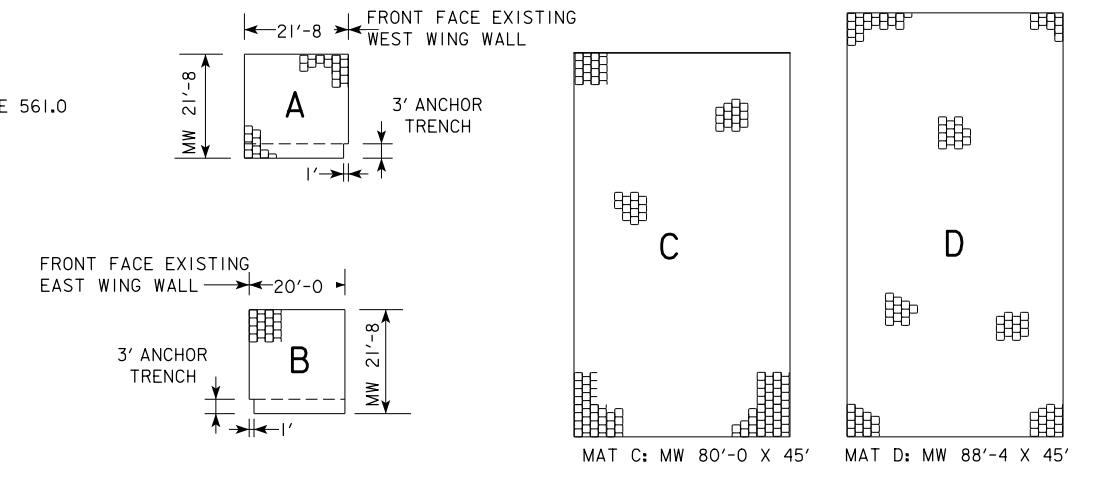
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 SITUATION PLAN  
 STATION: 293+68.30 DECEMBER, 2012  
 SCOTT COUNTY  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 4 OF 29 FILE NO. 30687 DESIGN NO. 514



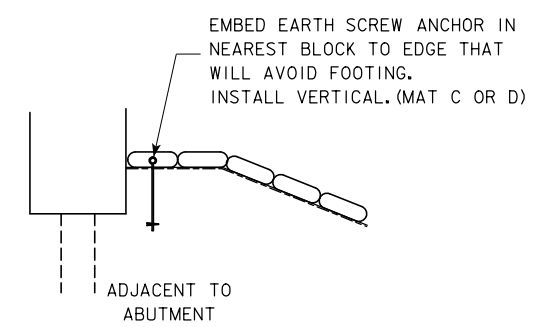
- NOTES:
1. ARTICULATING BLOCK MAT (A.B.M.) DIMENSIONS ARE ALONG GRADE.
  2. A.B.M. SHALL BE UNDERLAIN WITH ENGINEERING FABRIC TO THE DIMENSIONED LIMITS.
  3. POSITION THE EDGE OF MATS A AND B FLUSH WITH THE FRONT FACE OF EXISTING WING WALLS. MATS A AND B REQUIRE A 1' X 3' NOTCH OUT DUE TO THE ANCHOR TRENCH TERMINATING AT THE BACK FACE OF THE WING WALL.
  4. POSITION THE EDGE OF MATS C AND D FLUSH ALONG THE FRONT FACE OF ABUTMENTS. MATS C AND D OVERLAP MATS A AND B.
  5. SCREW ANCHORS SHALL BE INSTALLED ALONG THE FRONT FACE OF EACH ABUTMENT AT THE NUMBER AND SPACING SHOWN. SEE DETAIL.



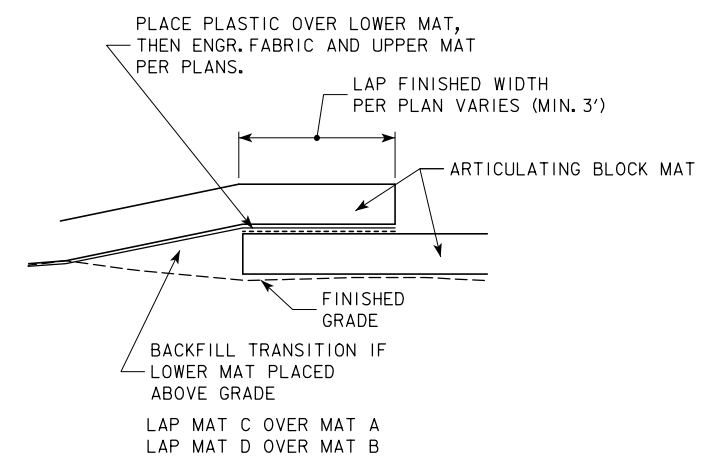
SECTION THROUGH TERMINAL TRENCH



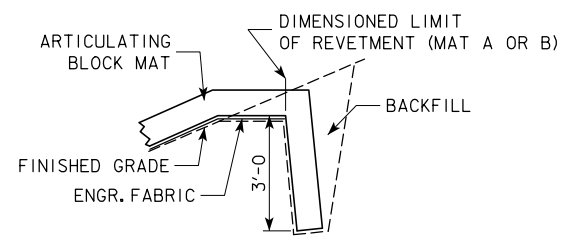
MAT SIZE DETAILS



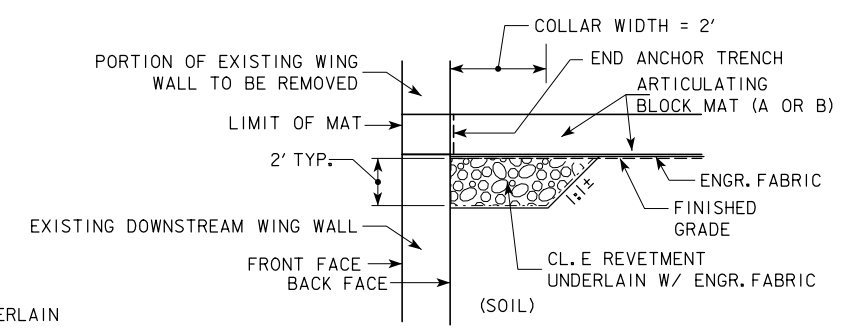
EARTH SCREW ANCHOR DETAIL



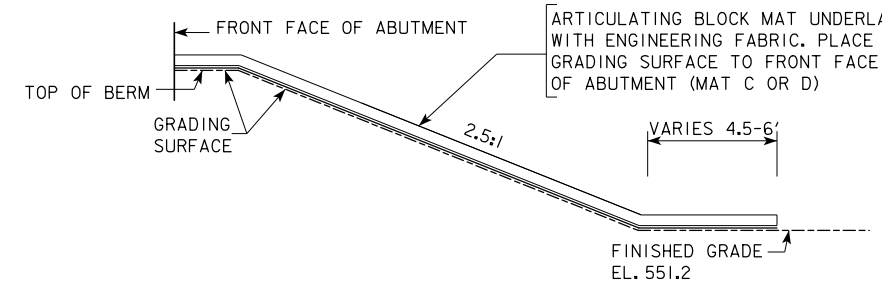
MAT LAP DETAIL



SECTION THROUGH ANCHOR TRENCH



EDGE DETAIL AND RIPRAP COLLAR



SECTION THRU NON-EMBEDDED REVETMENT BERM

SITUATION PLAN

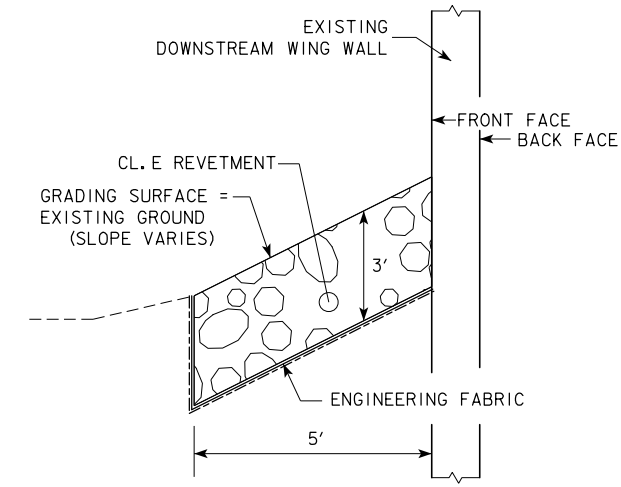
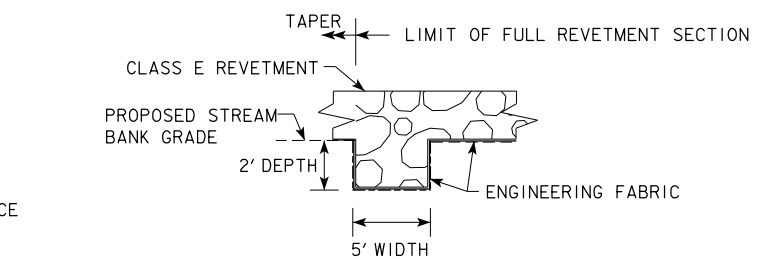
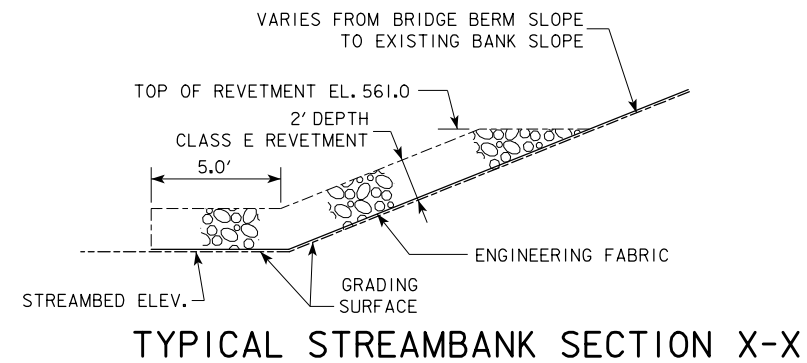
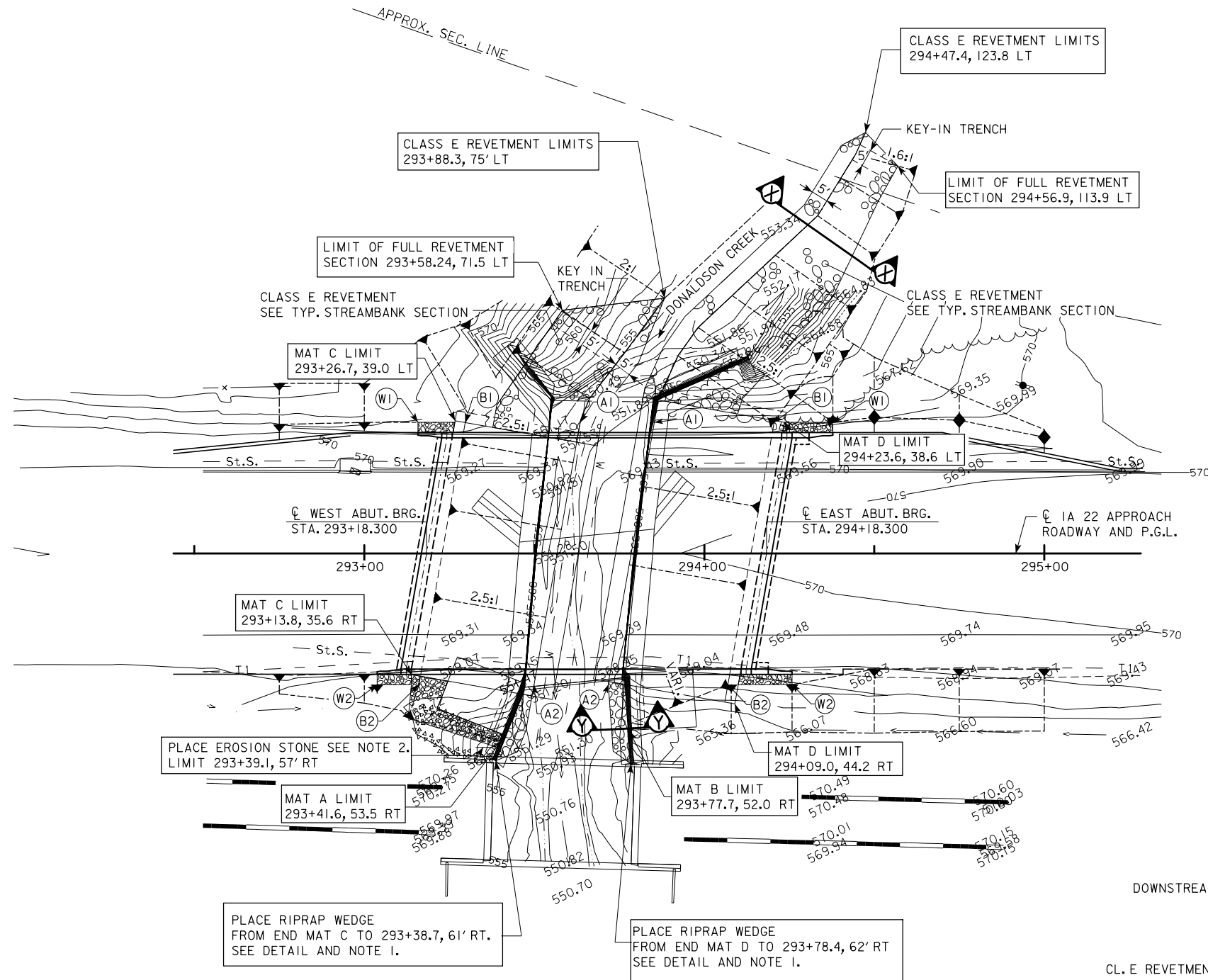
DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 SITUATION PLAN  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 5 OF 29 FILE NO. 30687 DESIGN NO. 514

NOTES:

- FOR RIPRAP WEDGE, CORE OUT 3' DEPTH OF SOIL ON CREEK SIDE (FRONT FACE) OF EXISTING WING WALLS. PLACE 3' DEPTH CLASS E REVETMENT UNDERLAIN WITH ENGINEERING FABRIC. MATCH TOP OF REVETMENT TO ADJACENT BANK SLOPES.
- CORE OUT 9 INCH DEPTH OF SOIL AT SOUTHWEST CORNER DITCH AREA AS SHOWN. PLACE 9 INCH DEPTH OF EROSION STONE UNDERLAIN WITH ENGINEERING FABRIC. MATCH TOP OF EROSION STONE TO ADJACENT GRADES AND TOP OF MAT.

BERM SLOPE LOCATION TABLE						
POINTS	WEST ABUTMENT			EAST ABUTMENT		
	STATION	OFFSET	ELEV.	STATION	OFFSET	ELEV.
A1	293+63.94	38.58 LT	551.20	293+84.74	38.58 LT	551.20
A2	293+50.34	38.58 RT	551.20	293+71.14	38.58 RT	551.20
B1	293+29.67	38.58 LT	564.70	294+20.53	38.58 LT	565.30
B2	293+16.07	38.58 RT	564.70	294+06.93	38.58 RT	565.30
W1	293+15.77	38.58 LT	570.14	294+37.80	38.58 LT	570.73
W2	293+03.78	38.58 RT	570.05	294+25.80	38.58 RT	570.70

BERM SLOPE ELEVATIONS REFLECT THE GRADING SURFACE



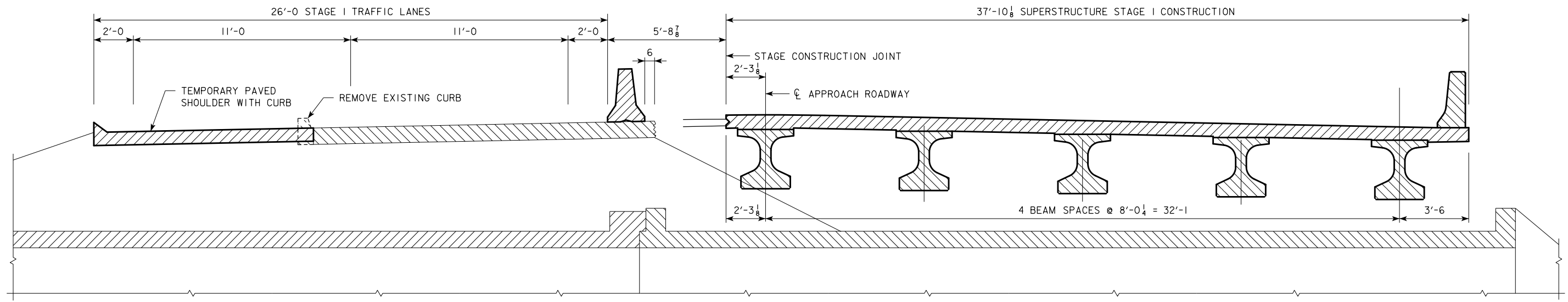
TYPICAL RIPRAP WEDGE SECTION Y-Y

ESTIMATED ARMORING QUANTITIES							
LOCATION	REVETMENT CL. E (TON)	ENGINEERING FABRIC (SY)	EXCAVATION (CY)	6 IN. A.B.M. (SY)	EROSION STONE (T)	CONC. GROUT (CY)	SCREW ANCH. (EA.)
BANK LINING - WEST STREAMBANK	172.0	158.9	9.1	0	0	0	0
BANK LINING - EAST STREAMBANK	305.7	284.6	6.2	0	0	0	0
BERM LINING - WEST ABUTMENT	0	378.8	15.0	400.0	0	72.0	10
BERM LINING - EAST ABUTMENT	0	420.5	15.0	441.7	0	79.5	11
BANK LINING AND COLLAR - WEST WING	23.6	88.1	23.3	51.8	10.2	9.3	0
BANK LINING AND COLLAR - EAST WING	28.5	63.1	19.9	47.8	0	8.6	0
TOTALS	529.8	1394.0	88.5	941.3	10.2	169.4	21

EXCAVATION QUANTITY CALCULATED FROM GRADING SURFACE.

SITE PLAN

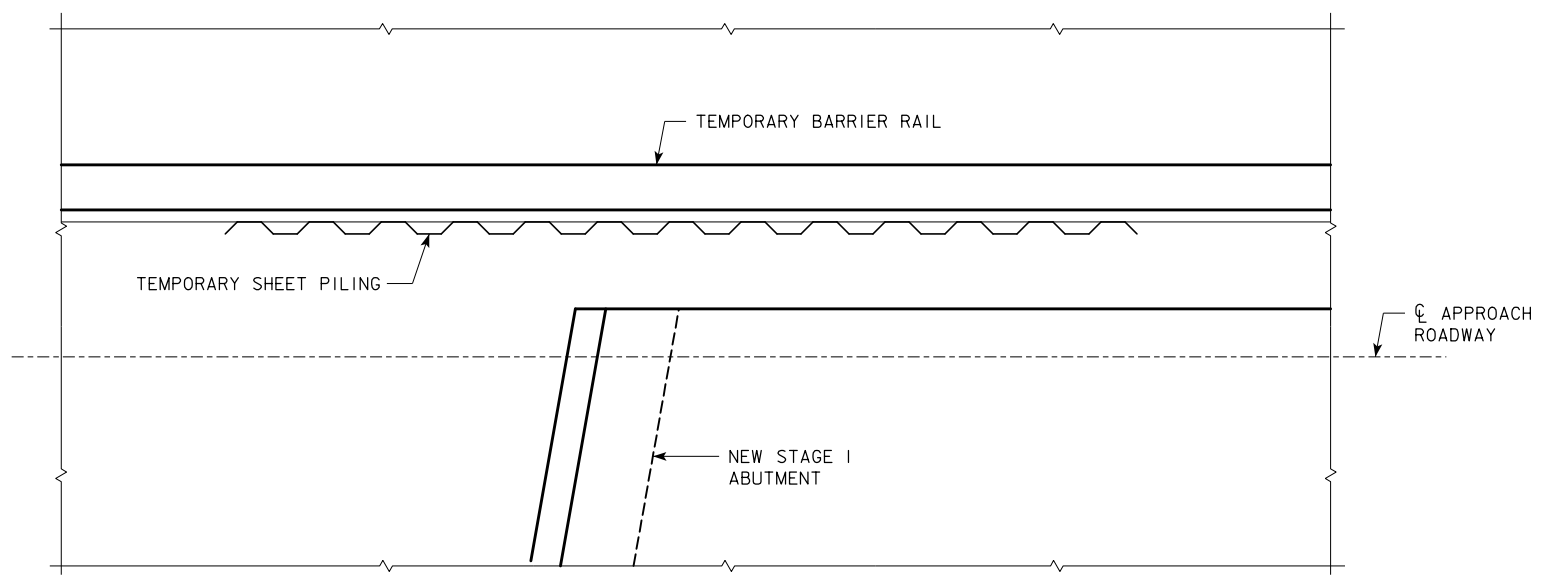
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 SITUATION PLAN  
 STATION: 293+68.30 DECEMBER, 2012  
 SCOTT COUNTY  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 6 OF 29 FILE NO. 30687 DESIGN NO. 514



**STAGE I ELEVATION**  
(LOOKING EAST)

**STAGE I NOTES (ROADWAY PLAN STAGES I AND II):**

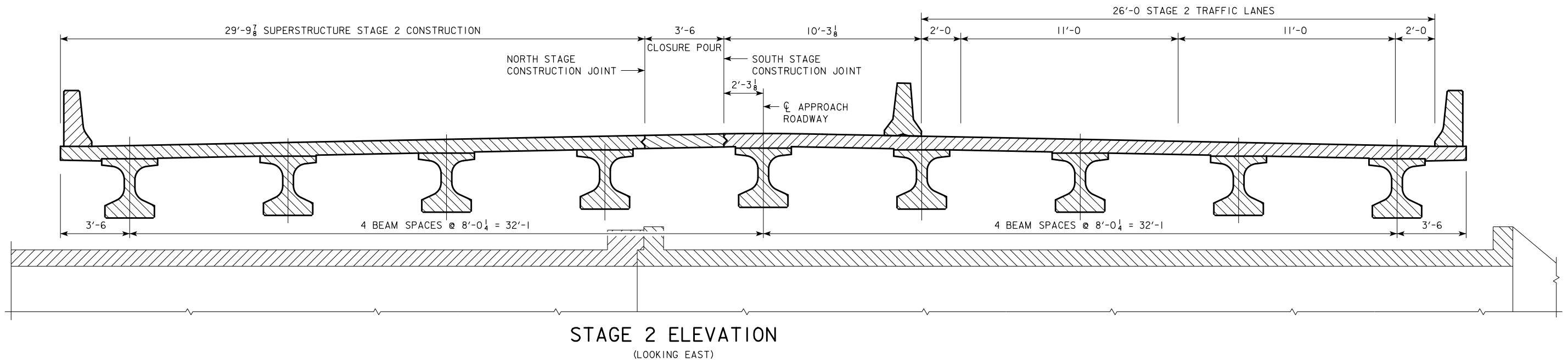
1. REMOVE EXISTING CURB AND PLACE TEMPORARY SHOULDER PAVEMENT IN W.B. LANES. SEE ROAD SHEETS FOR DETAILS AND TRAFFIC CONTROL.
2. PLACE TBR AND OTHER TRAFFIC CONTROL MEASURES ON THE EXISTING WB LANES AND SHIFT ALL TRAFFIC TO THE WB LANES IN ACCORDANCE WITH THE TRAFFIC CONTROL PLAN NOTE ON THE ROAD SHEETS.
3. REMOVE THE EXISTING EB LANES AND APPROACH PAVEMENT.
4. PLACE SHEET PILING NEAR AND PARALLEL TO THE SLAB BREAKLINE NEAR THE PROPOSED ABUTMENT TO SUPPORT STAGE I TRAFFIC LANES.
5. CONSTRUCT STAGE I OF NEW ABUTMENTS.
6. CONSTRUCT STAGE I OF THE SUPERSTRUCTURE.
7. PLACE SHORING BETWEEN THE STAGE I ABUTMENT AND SHEET PILING DURING BACK FILLING TO CONTAIN SOIL.



**PART PLAN VIEW**  
(WEST ABUTMENT ORIENTATION SHOWN,  
EAST ABUTMENT DETAILS ARE SIMILAR.)

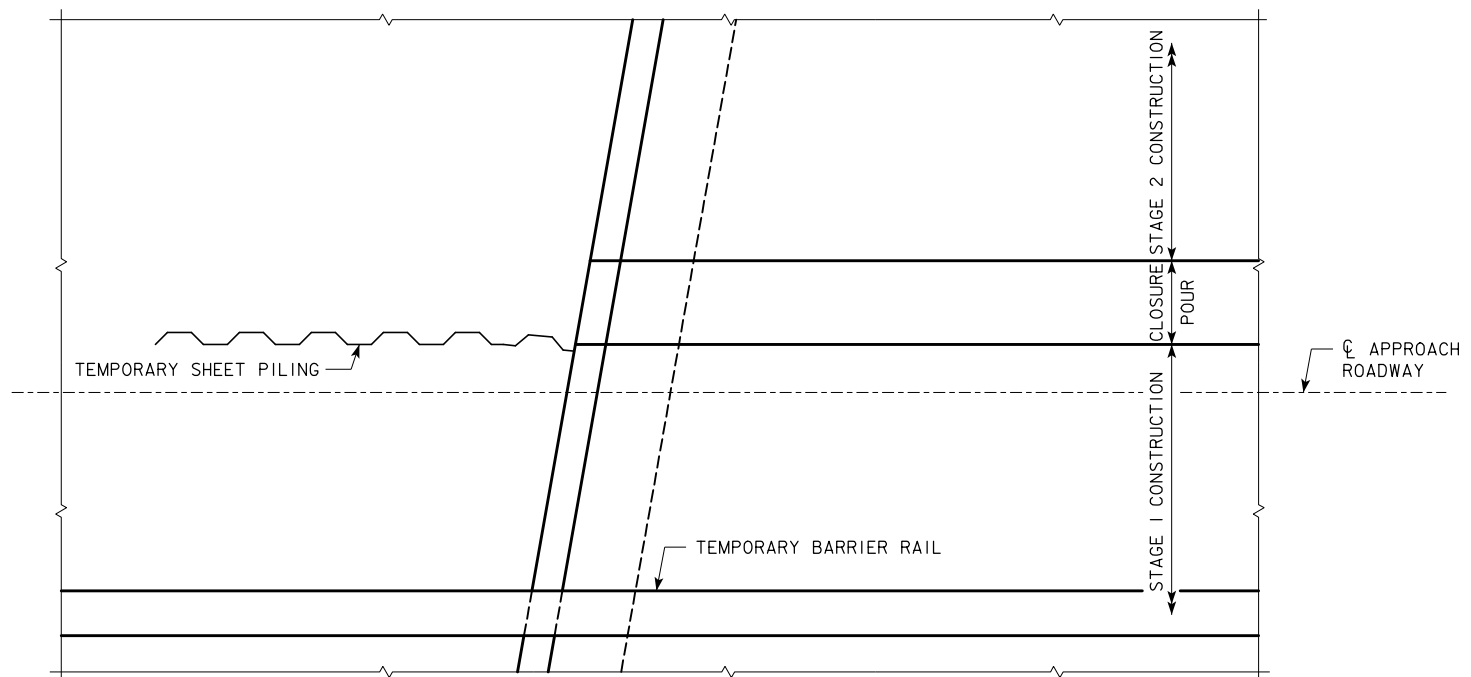
DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**  
**STAGE I DETAILS**  
 STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 7 OF 29    FILE NO. 30687    DESIGN NO. 514





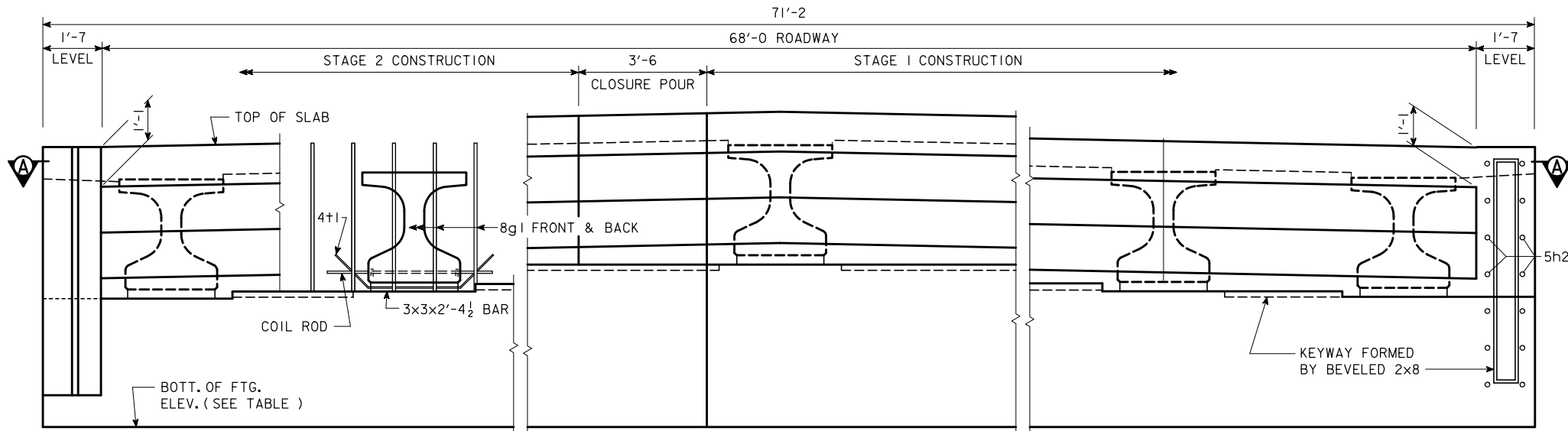
**STAGE 2 NOTES (ROADWAY PLAN STAGE III):**

1. PLACE TBR AND OTHER TRAFFIC CONTROL MEASURES ON NEW E.B. LANES AND SHIFT ALL TRAFFIC TO THE E.B. LANES IN ACCORDANCE WITH THE TRAFFIC CONTROL PLAN NOTE ON THE ROAD SHEETS.
2. REMOVE EXISTING W.B. LANES, APPROACH PAVEMENT, AND EXISTING ARCH STRUCTURE. PORTIONS OF EXISTING SOUTH WINGWALLS AND ARCH FOOTINGS SHALL REMAIN IN PLACE AS DETAILED.
3. PLACE SHEET PILING NEAR AND PARALLEL TO THE STAGE 1 CONSTRUCTION JOINT OF THE NEW ABUTMENT TO SUPPORT STAGE 2 TRAFFIC LANES.
4. CONSTRUCT STAGE 2 OF THE SUPERSTRUCTURE. PLACE CLOSURE POUR.
5. REMOVE SHEET PILING AFTER BACKFILLING ABUTMENTS.
6. SHIFT W.B. TRAFFIC TO THE NEW W.B. TRAFFIC LANES AND REMOVE THE TBR.

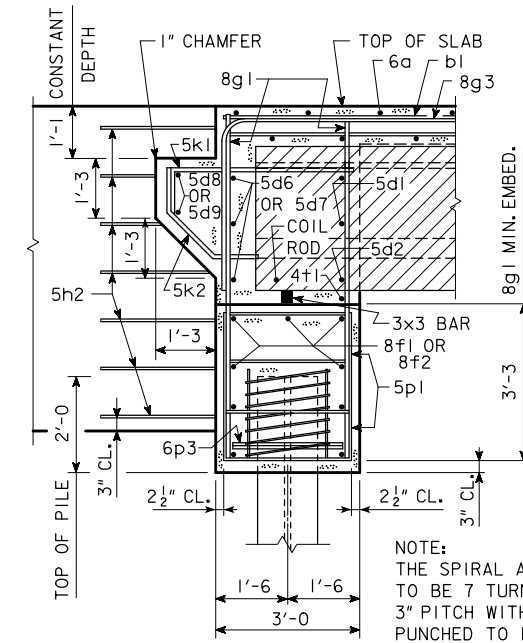


**PART PLAN VIEW**  
(WEST ABUTMENT ORIENTATION SHOWN,  
EAST ABUTMENT DETAILS ARE SIMILAR.)

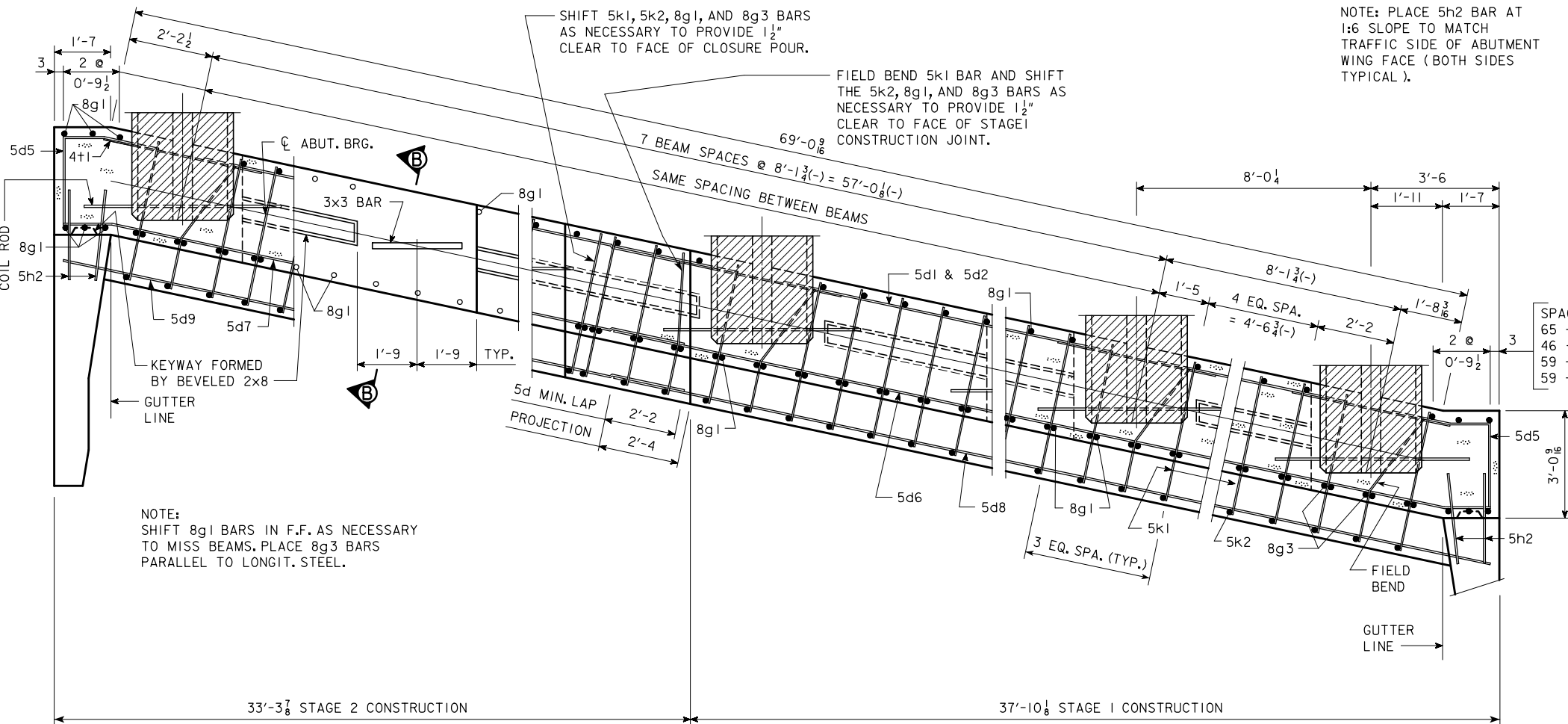
DESIGN FOR 10° SKEW (L.A.)	
<b>100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE</b>	
<b>STAGE 2 DETAILS</b>	
STATION: 293+68.30	DECEMBER, 2012
<b>SCOTT COUNTY</b>	
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION	
DESIGN SHEET NO. <u>8</u> OF <u>29</u>	FILE NO. <u>30687</u> DESIGN NO. <u>514</u>



PART REAR ELEVATION AT ABUTMENT



PART SECTION B-B



PART SECTION A-A

**WEST ABUTMENT PILING NOTES:**  
 THE CONTRACTOR SHALL PLACE HP 10X57 PILES IN 15 FOOT DEEP PREBORED HOLES. MINIMUM ALLOWABLE PREBORED HOLE DIAMETER IS 18 INCHES. HOLES MUST BE PREBORED THROUGH ROCK TO ELEVATION 547.66. PREBORING WILL REQUIRE DRILLING THROUGH SOIL, BROKEN AND WEATHERED LIMESTONE, AND MEDIUM HARD TO HARD LIMESTONE. REFER TO SOIL PROFILE SHEETS. THE PREBORED HOLES ARE TO BE MAINTAINED OPEN DURING DRIVING OF THE PILES TO THE EXTENT THAT CASING MAY BE REQUIRED FOR COLLAPSING SOILS. DRIVE PILES TO PRACTICAL REFUSAL WITH AN APPROVED HAMMER TO ASSURE UNIFORM CONTACT WITH SOUND ROCK THAT IS SUITABLE FOR END BEARING. BOTTOM OF HOLES SHALL BE FILLED WITH 3 FEET OF CONCRETE BEFORE OR AFTER PILES ARE DRIVEN TO PRACTICAL REFUSAL TO LOCK PILES IN PLACE. RETARDER MAY BE REQUIRED TO KEEP THE CONCRETE PLASTIC IF IT IS PLACED BEFORE THE PILES ARE DRIVEN TO PRACTICAL REFUSAL. THE REMAINING 12 FOOT DEPTH OF PREBORED HOLES SHALL BE FILLED WITH LOOSE DRY SAND. A REASONABLE EFFORT SHALL BE EXPENDED TO CLEAN BOTTOM OF HOLE BEFORE INSERTING PILE OR PLACING CONCRETE. PREBORING THROUGH ROCK LAYERS MAY BE ACCOMPLISHED BY ROCK CORING TECHNIQUES, AUGERING, OR OTHER PROCEDURES APPROVED BY THE ENGINEER. NO ADDITIONAL COMPENSATION WILL BE MADE FOR DIFFICULTIES ASSOCIATED WITH PREBORING TO THE SPECIFIED ELEVATION. TOTAL FACTORED AXIAL LOAD PER PILE (PU) IS 183 KIPS USING A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.7 FOR ROCK END BEARING. THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE FOR WEST ABUTMENT PILES IS 131 TONS AT END OF DRIVE. CONSTRUCTION CONTROL REQUIRES A WEAP ANALYSIS WITH BEARING GRAPH.

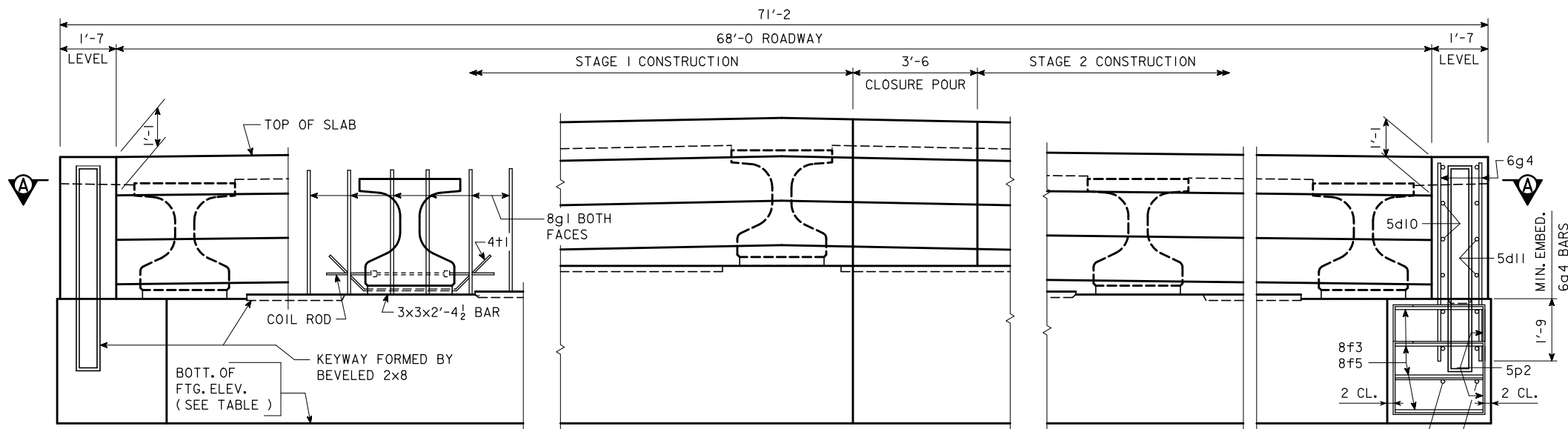
**ABUTMENT NOTES:**

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.  
 IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.

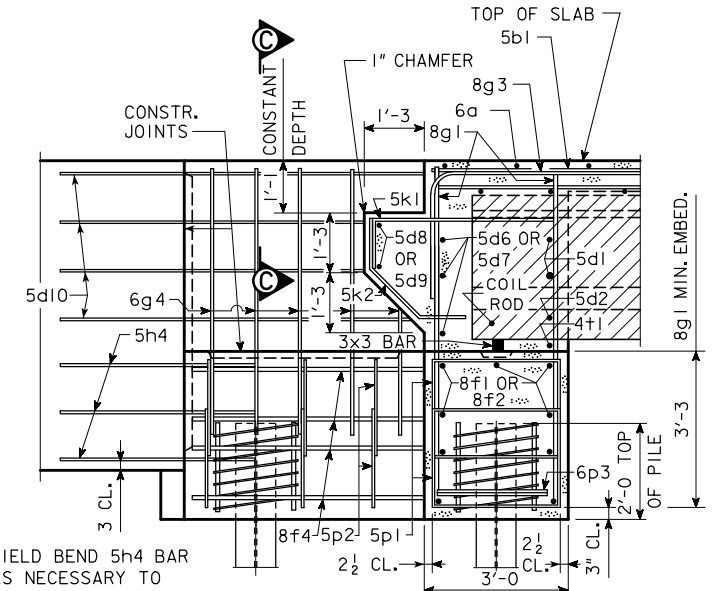
NOTE: BARRIER RAIL NOT SHOWN IN DETAILS.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**WEST ABUTMENT DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 9 OF 29 FILE NO. 30687 DESIGN NO. 514

REVISION 07-08 - NUMBER OF SPIRAL SPACES CHANGED TO 3.  
 ENGLISHBTB INTEGRAL BRIDGES.DGN - 2080-BTB - THIS SHEET ISSUED 02-08.



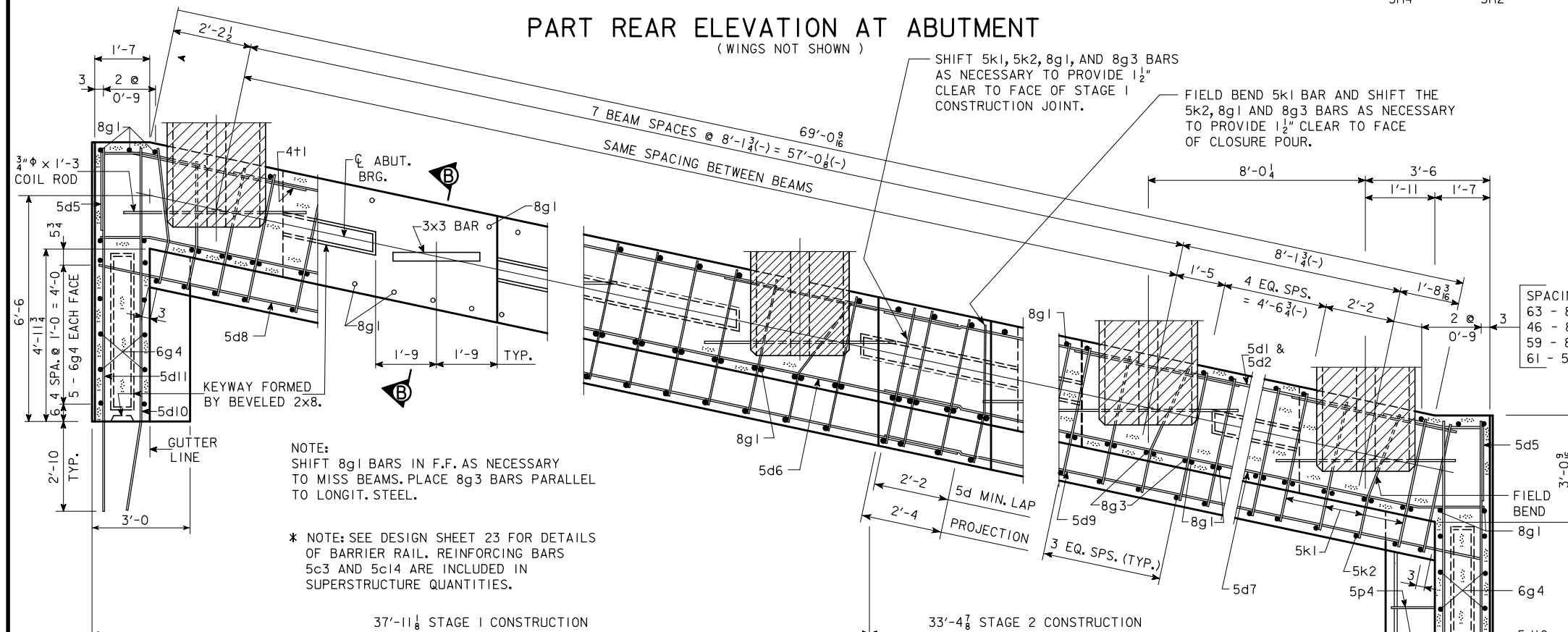
**PART REAR ELEVATION AT ABUTMENT**  
(WINGS NOT SHOWN)



**PART SECTION B-B**

FIELD BEND 5h4 BAR AS NECESSARY TO PROVIDE 2" CLEAR AND AVOID PILE IN ABUTMENT WING.

NOTE:  
THE SPIRAL AT THE TOP OF EACH PILE TO BE 7 TURNS OF No. 2 BAR, 21" DIAMETER, 3" PITCH WITH 3 - L<sub>2</sub> x 7/8 x 1/8 SPACERS PUNCHED TO HOLD SPIRAL.



**PART SECTION A - A**

**ABUTMENT NOTES:**

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.  
IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.

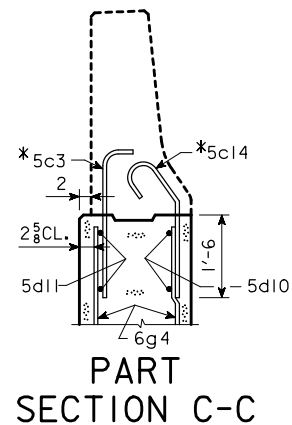
SPACING FOR:  
63 - 8g1 BACK FACE  
46 - 8g1 FRONT FACE  
59 - 8g3 BACK FACE  
61 - 5k1 & 5k2 BACK FACE

**PILE NOTES:**

THE CONTRACT LENGTH OF 30 FEET FOR THE EAST ABUTMENT PILES IS BASED ON A COHESIVE SOIL CLASSIFICATION, A TOTAL FACTORED AXIAL LOAD PER PILE (PU) OF 181 KIPS, AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.65 FOR SOIL AND 0.7 FOR ROCK.

THE NOMINAL AXIAL BEARING RESISTANCE FOR CONSTRUCTION CONTROL WAS DETERMINED FROM A COHESIVE SOIL CLASSIFICATION AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.65 FOR SOIL AND 0.7 FOR ROCK END BEARING.

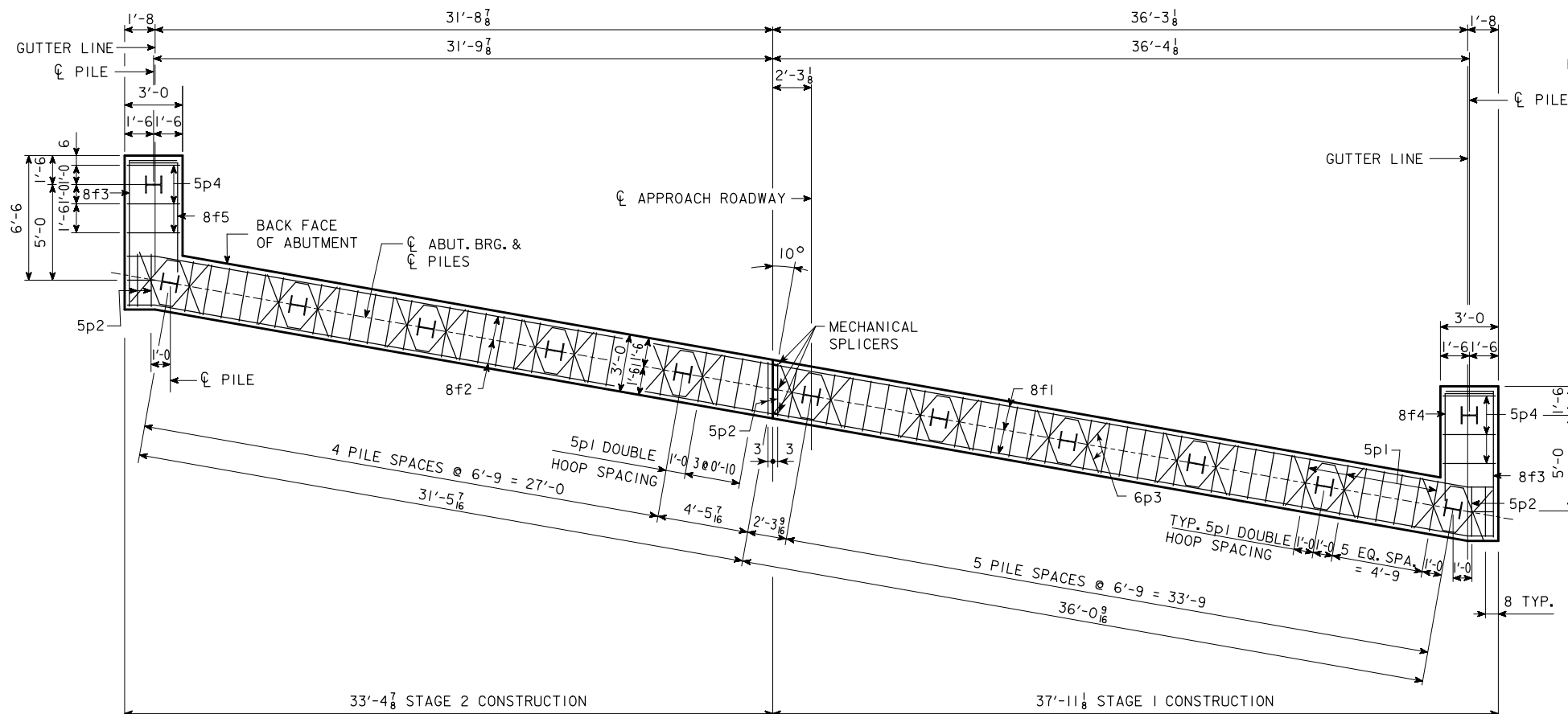
THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE FOR EAST ABUTMENT PILES IS 131 TONS AT END OF DRIVE. THE PILE CONTRACT LENGTH SHALL BE DRIVEN AS PER PLAN UNLESS PILES REACH REFUSAL. CONSTRUCTION CONTROL REQUIRES A WEAP ANALYSIS WITH BEARING GRAPH.



**PART SECTION C-C**

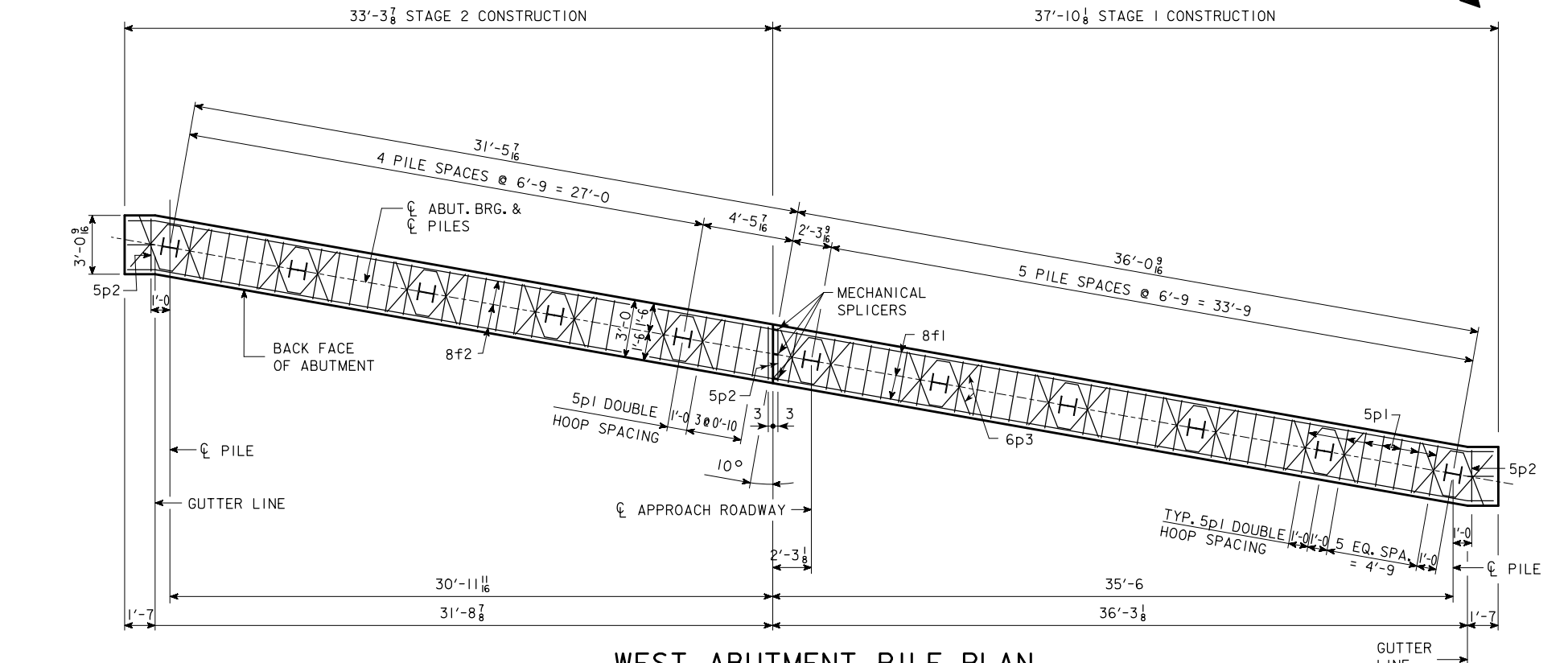
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**EAST ABUTMENT DETAILS**  
STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 10 OF 29 FILE NO. 30687 DESIGN NO. 514

BENCH MARK NO. 603 STA. 293+83.546, 45.303 LT. FD IHC BM ON INHDL OF ARCH BRG, ELEV. 566.498.



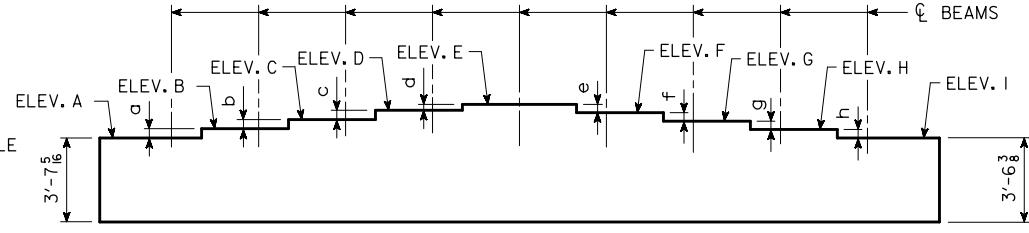
**EAST ABUTMENT PILE PLAN**

13 - HPI0x57 STEEL BEARING PILING REQUIRED AT EAST ABUTMENT.



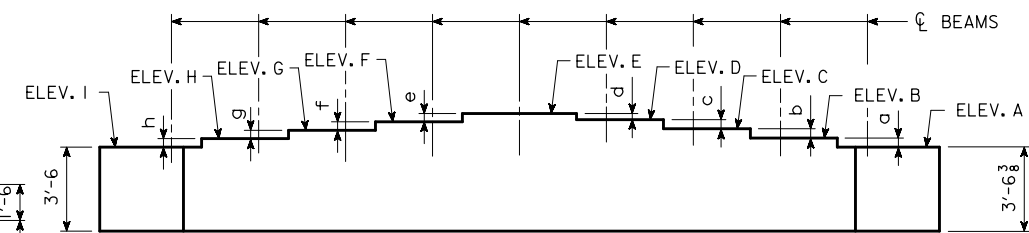
**WEST ABUTMENT PILE PLAN**

11 - HPI0x57 STEEL BEARING PILING REQUIRED AT WEST ABUTMENT.



**WEST ABUTMENT STEP DIAGRAM**

(REAR ELEVATION)



**EAST ABUTMENT STEP DIAGRAM**

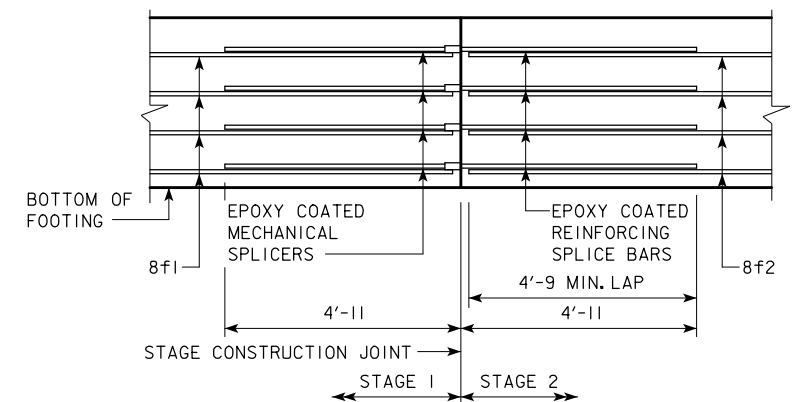
(REAR ELEVATION)

**TABLE OF ABUTMENT ELEVATIONS**

POINT	WEST ABUT.	EAST ABUT.
ELEV. A	566.21	566.71
ELEV. B	566.40	566.91
ELEV. C	566.59	567.11
ELEV. D	566.76	567.29
ELEV. E	566.85	567.38
ELEV. F	566.74	567.28
ELEV. G	566.55	567.09
ELEV. H	566.34	566.89
ELEV. I	566.13	566.68
BOTT. FTG. ELEV.	562.60	563.18

**TABLE OF ABUTMENT STEPS**

STEP	WEST ABUT.	EAST ABUT.
a	2 1/4	2 3/8
b	2 1/4	2 3/8
c	2 1/16	2 3/16
d	1 1/16	1 1/16
e	1 5/16	1 3/16
f	2 1/4	2 1/4
g	2 1/2	2 3/8
h	2 1/2	2 1/2



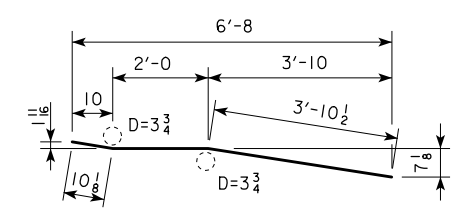
**PART ELEVATION AT STAGE CONSTRUCTION JOINT**

(SEE DESIGN SHEET 2 FOR MECHANICAL SPLICE NOTES)

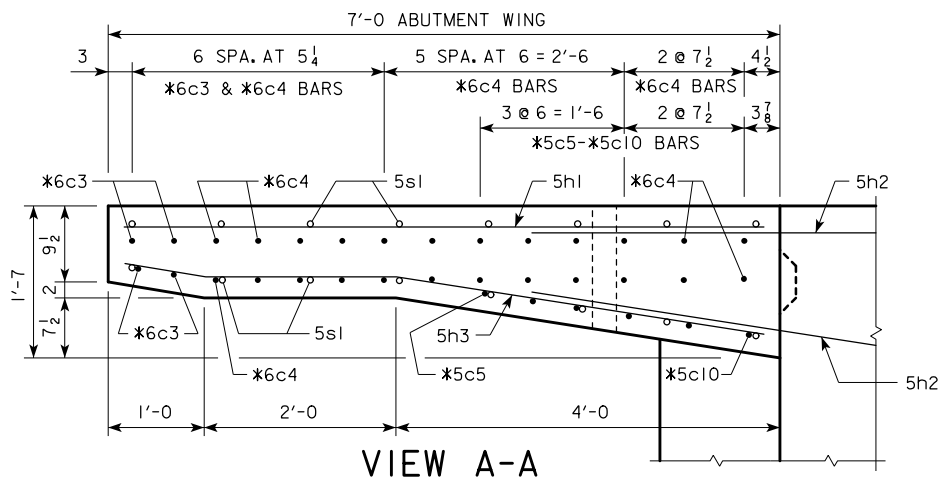
DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**ABUTMENT DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 11 OF 29 FILE NO. 30687 DESIGN NO. 514

**REINFORCING BAR LIST - ONE W. ABUT. WING**

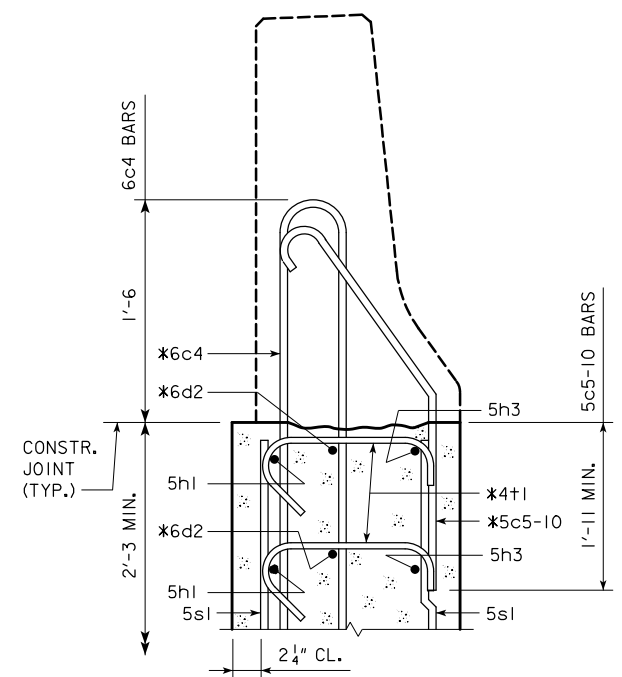
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
5h1	HORIZONTAL BACK FACE	—	7	6'-8"	49
5h3	HORIZONTAL TRAFFIC FACE	—	7	6'-9"	49
5s1	VERTICAL BOTH FACES	—	16	6'-2"	103
TOTAL (LBS.)					201



**5h3**  
NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.  
**BENT BAR DETAILS**



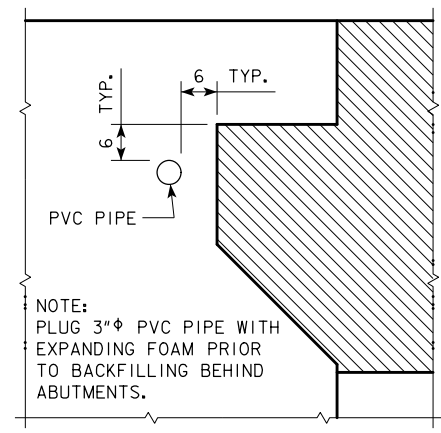
NOTE:  
PLUG 3"φ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



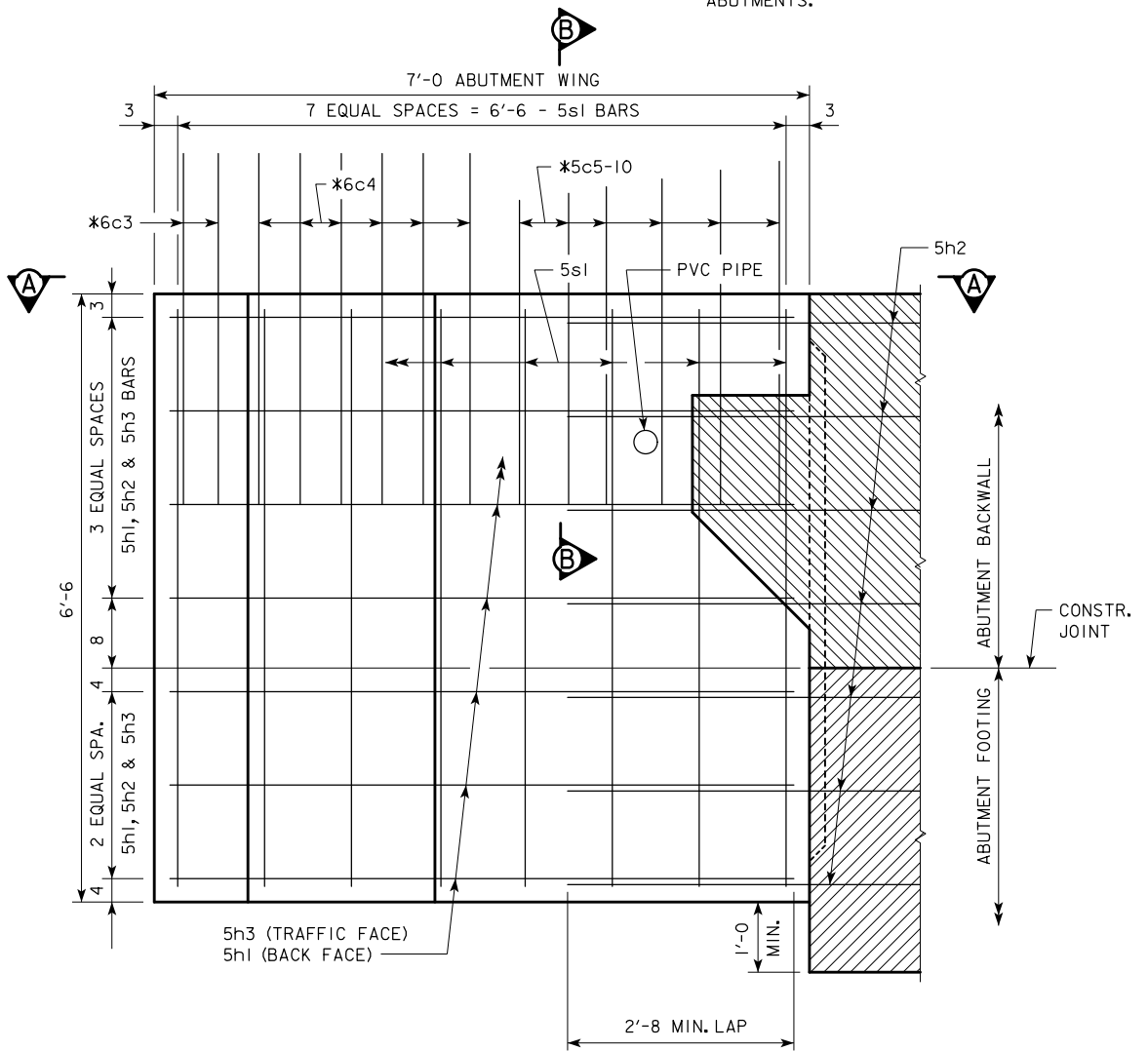
**SECTION B-B**

\* BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.

SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4+1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.



**PVC PIPE LOCATION**



**ABUTMENT WING - ELEVATION VIEW**

**CONCRETE PLACEMENT SUMMARY**

CONCRETE	TOTAL
ONE ABUTMENT WING	1.9
TOTAL (CU. YDS.)	1.9

**NOTE:**

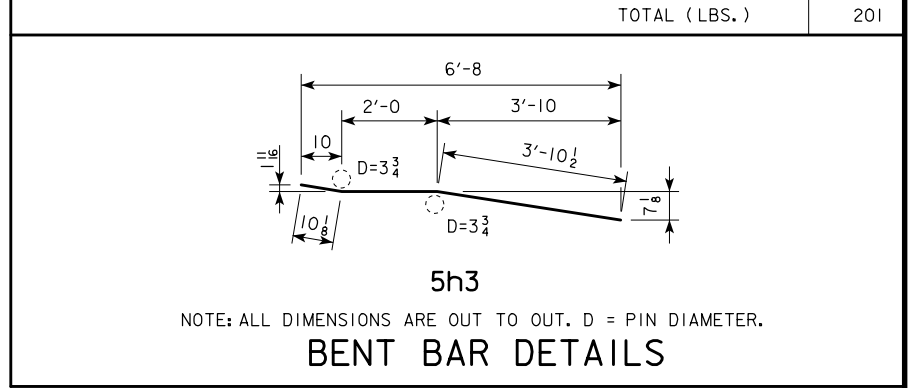
REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**WEST ABUTMENT WING DETAILS**  
STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 12 OF 29      FILE NO. 30687      DESIGN NO. 514

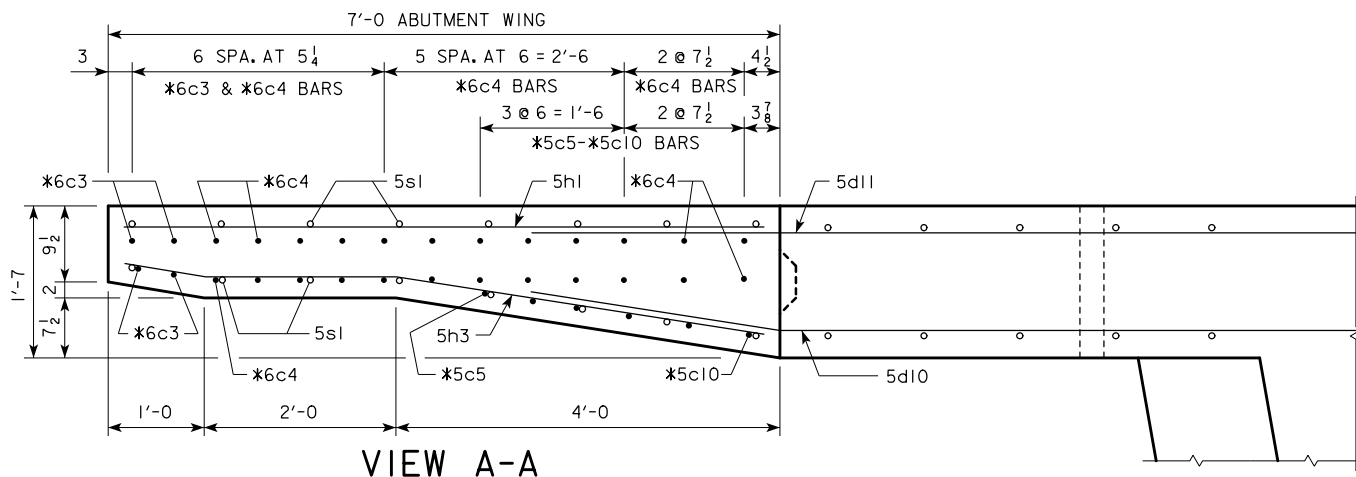
ENGLISHMISCELLANEOUSBRIDGES.DGN - 2111 - THIS SHEET ISSUED 02-08.

**REINFORCING BAR LIST - ONE E. ABUT. WING**

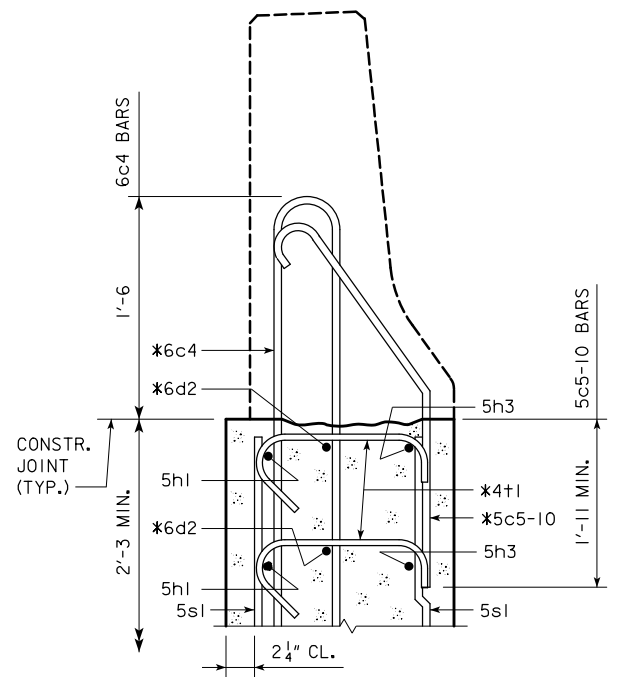
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
5h1	HORIZONTAL BACK FACE	—	7	6'-8	49
5h3	HORIZONTAL TRAFFIC FACE	—	7	6'-9	49
5s1	VERTICAL BOTH FACES	—	16	6'-2	103
TOTAL (LBS.)					201



**5h3**  
NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.  
**BENT BAR DETAILS**

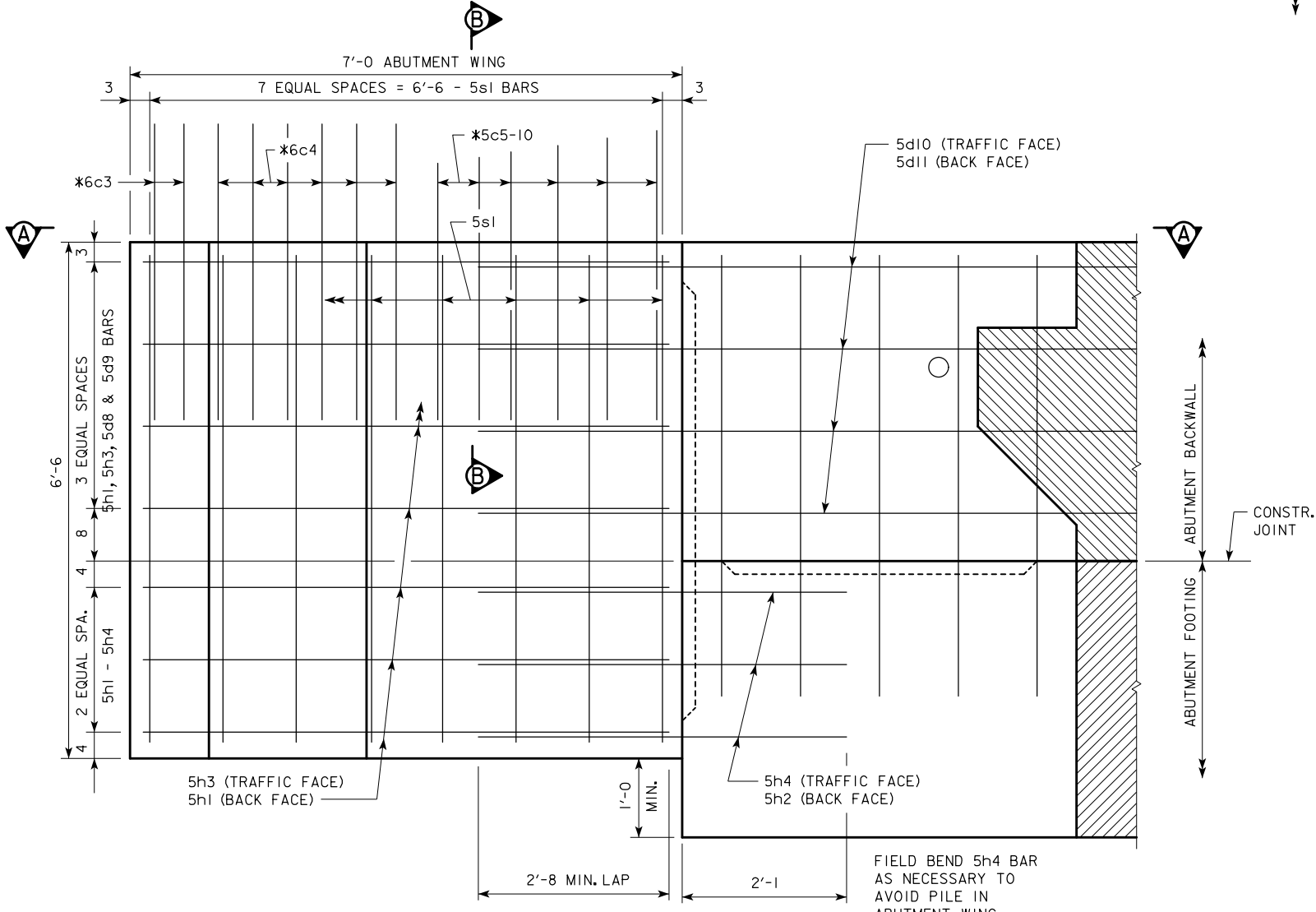


**VIEW A-A**



**SECTION B-B**

\* BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.  
SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4+1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

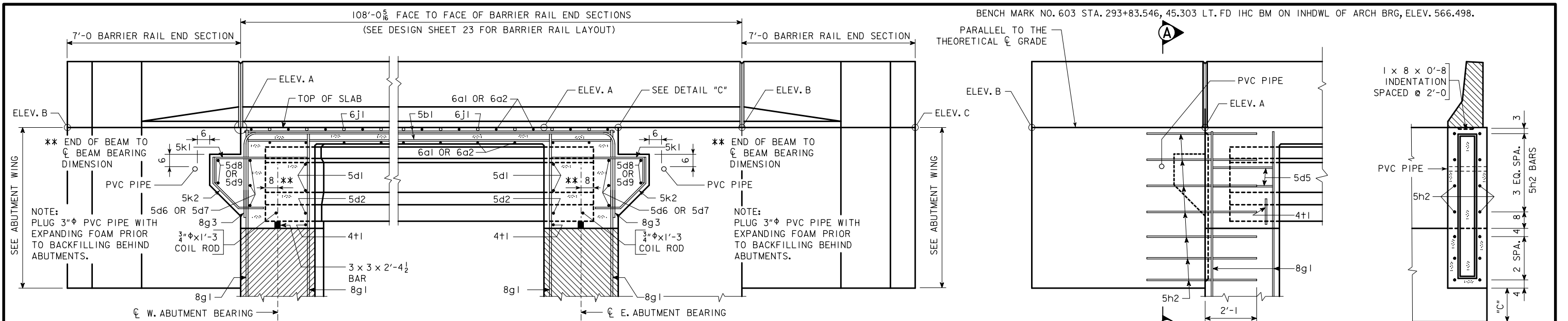


**ABUTMENT WING - ELEVATION VIEW**

CONCRETE PLACEMENT SUMMARY	
CONCRETE	TOTAL
ONE ABUTMENT WING	1.9
TOTAL (CU. YDS.)	1.9

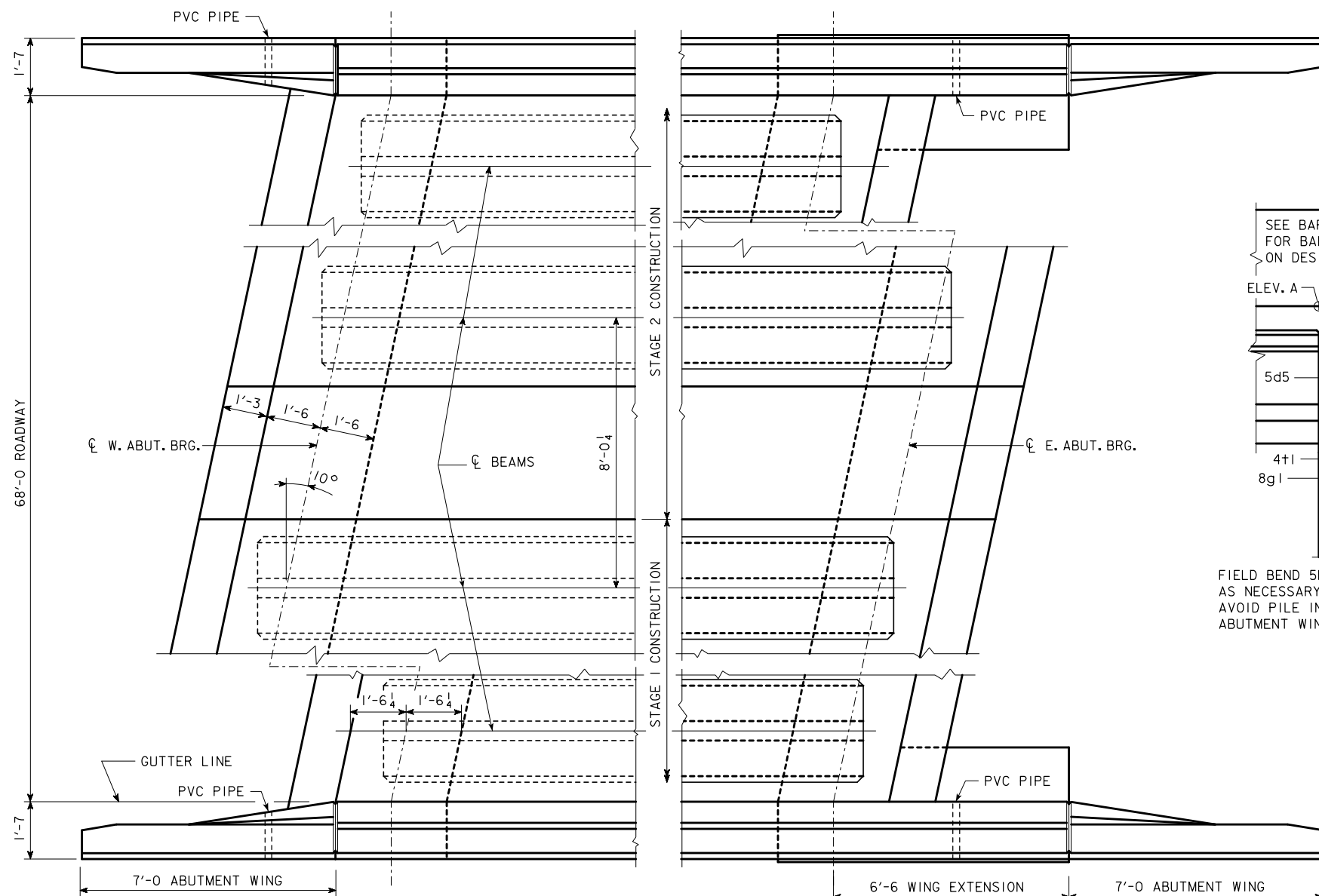
**NOTE:**  
REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**EAST ABUTMENT WING DETAILS**  
STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 13 OF 29 FILE NO. 30687 DESIGN NO. 514

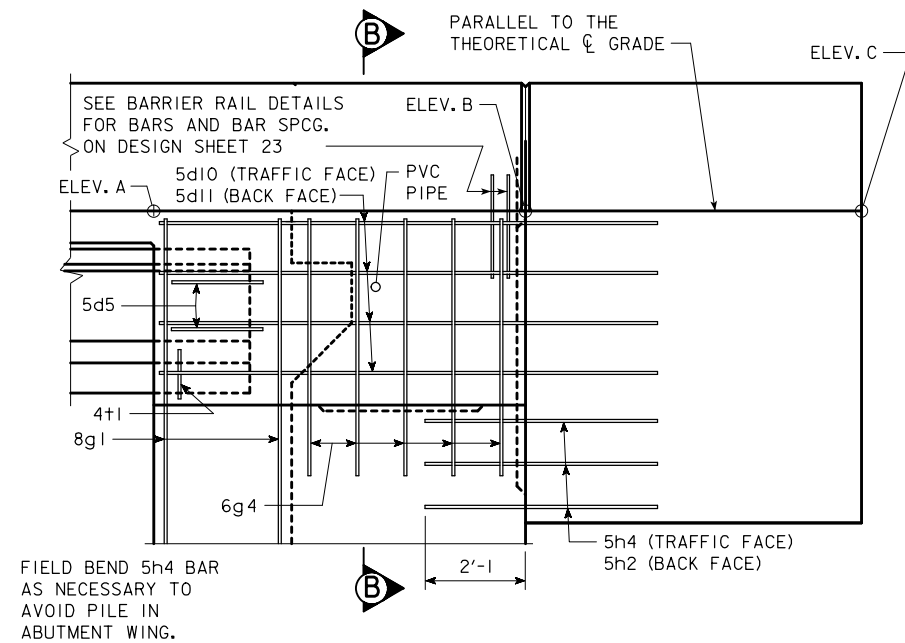


**PART LONGITUDINAL SECTION NEAR GUTTER**  
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE DESIGN SHEET 20)

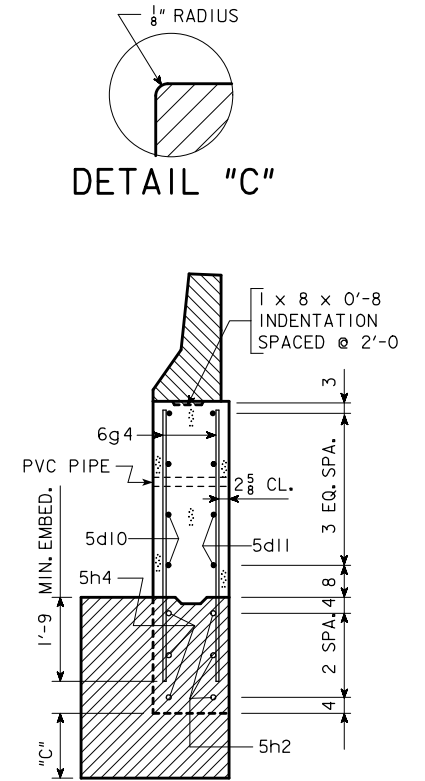
**PART END VIEW AT W. ABUTMENT** SECTION A-A



**PART PLAN**



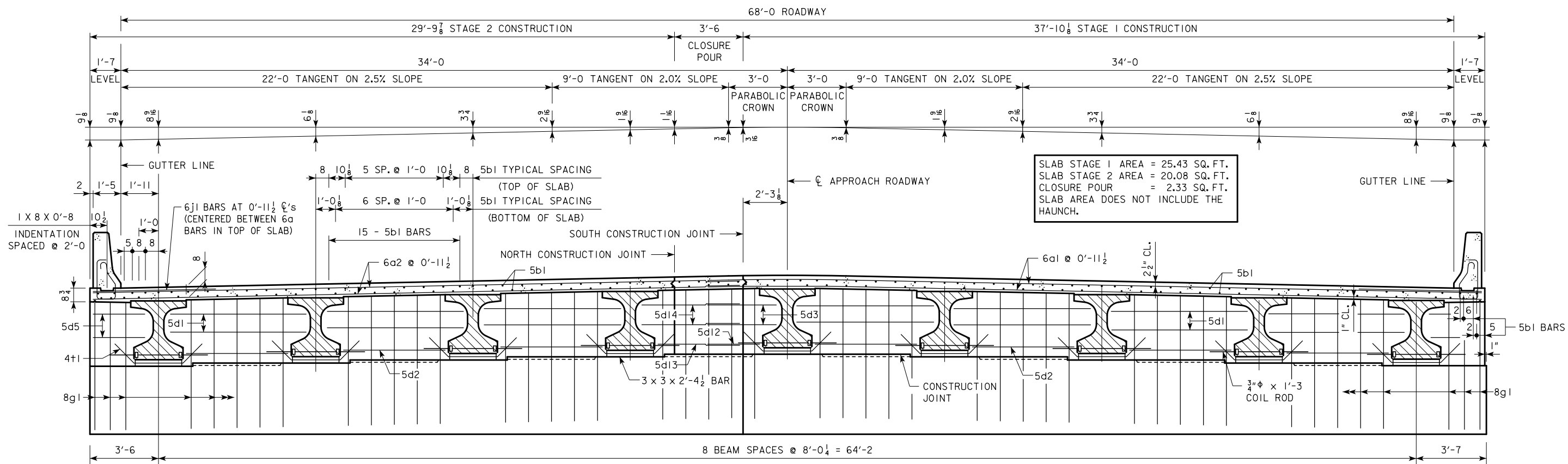
**PART END VIEW AT E. ABUTMENT**



**SECTION B-B**

TABLE OF WINGWALL ELEVATIONS				
LOCATION	DIM "C"	ELEV. A	ELEV. B	ELEV. C
S.W. CORNER	1'-0"	570.10	570.05	—
N.W. CORNER	1'-0 <sup>15</sup> / <sub>16</sub> "	570.18	570.14	—
S.E. CORNER	1'-0 <sup>3</sup> / <sub>8</sub> "	570.66	570.69	570.70
N.E. CORNER	1'-0 <sup>3</sup> / <sub>8</sub> "	570.70	570.71	570.73

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**SUPERSTRUCTURE DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 14 OF 29 FILE NO. 30687 DESIGN NO. 514

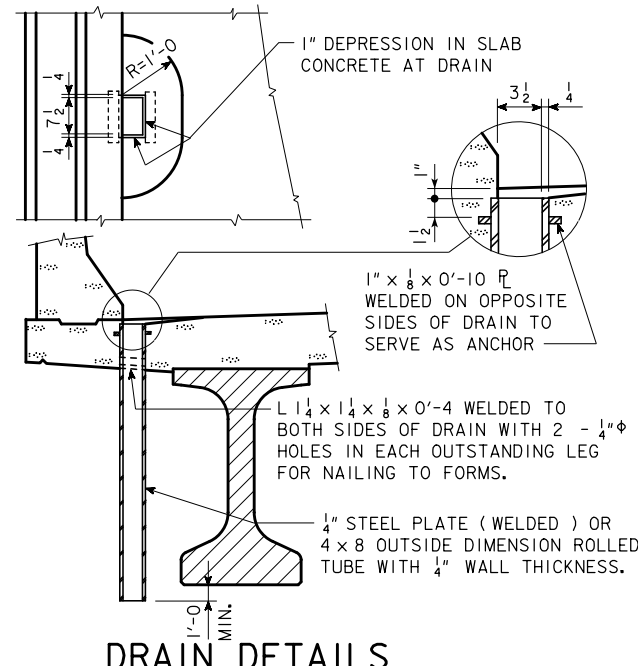
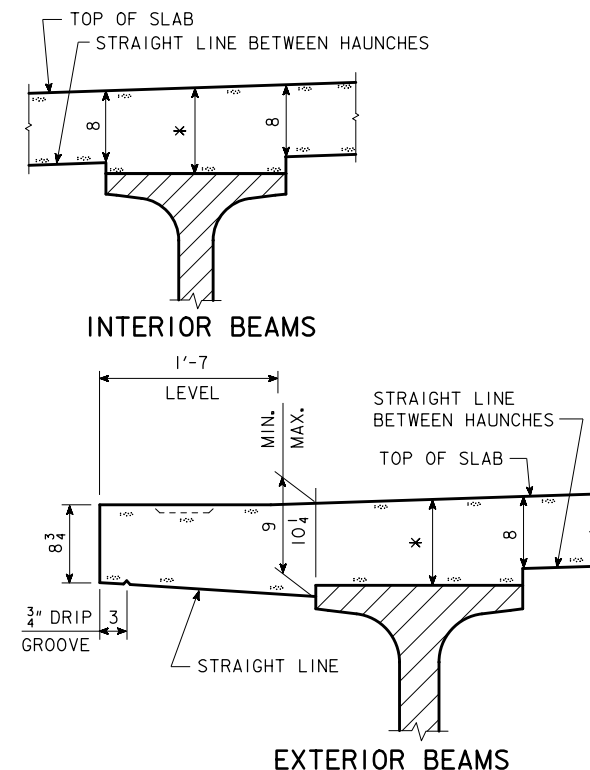


SLAB STAGE 1 AREA = 25.43 SQ. FT.  
 SLAB STAGE 2 AREA = 20.08 SQ. FT.  
 CLOSURE POUR = 2.33 SQ. FT.  
 SLAB AREA DOES NOT INCLUDE THE HAUNCH.

HALF SECTION NEAR WEST ABUTMENT

HALF SECTION NEAR EAST ABUTMENT

(LOOKING EAST)



NOTE: FOR DETAILS OF INTERMEDIATE DIAPHRAGMS SEE DESIGN SHEET 20.

**SUPERSTRUCTURE NOTES:**

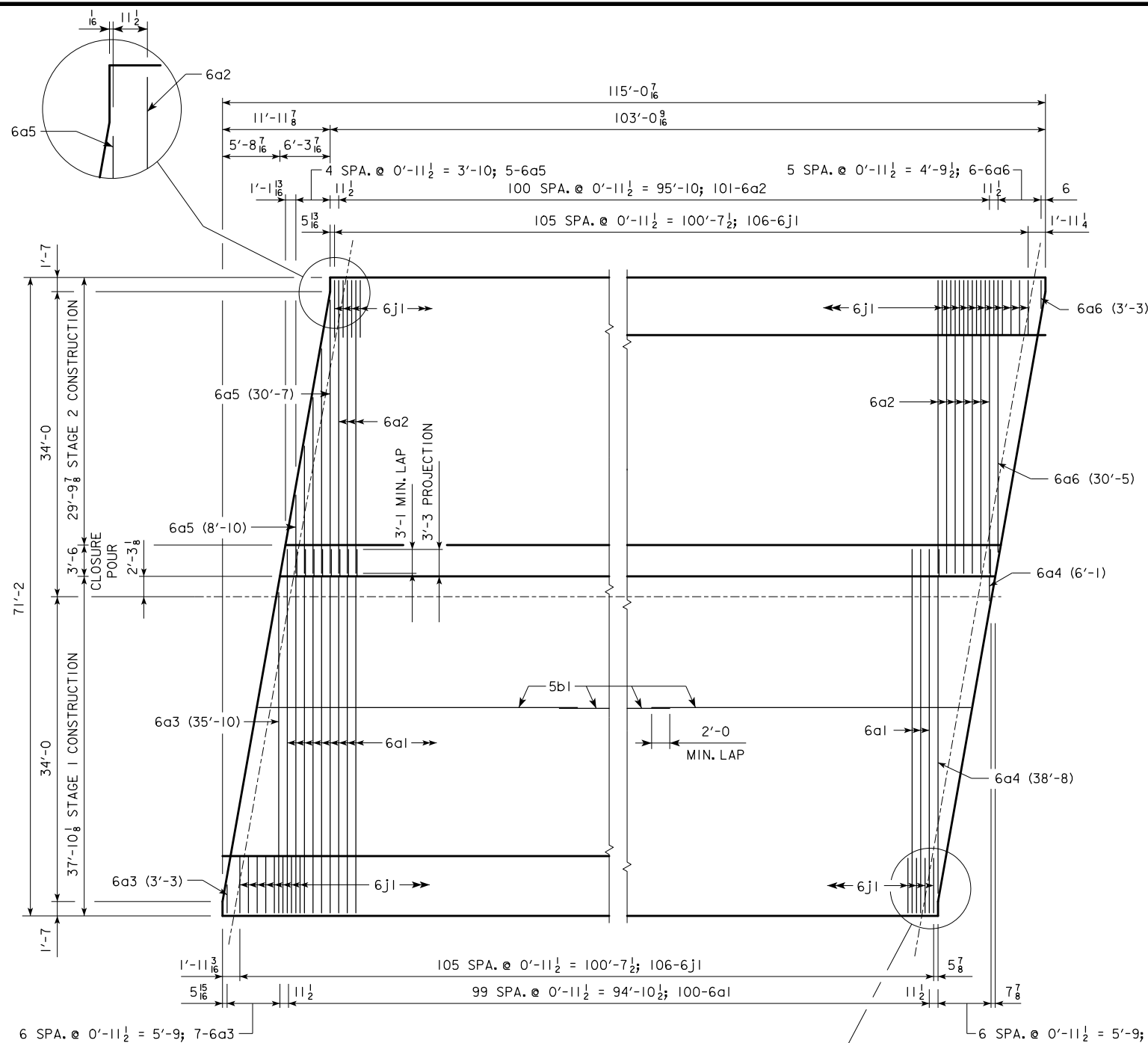
THE FLOOR SLAB AS SHOWN INCLUDES 1/2" INTEGRAL WEARING SURFACE.  
 THE ABUTMENT DIAPHRAGM CONCRETE IS TO BE PLACED MONOLITHICALLY WITH THE FLOOR SLAB.  
 ALL BEAMS ARE TO BE SET VERTICAL.  
 FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE PRESTRESSED CONCRETE BEAMS.  
 CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.  
 ALL SLAB AND DIAPHRAGM REINFORCING IS TO BE WIRED IN PLACE AND ADEQUATELY SUPPORTED BEFORE CONCRETE IS PLACED.  
 TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 2 1/2" CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 1" CLEAR ABOVE BOTTOM OF SLAB.  
 TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-0" CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4'-0" APART. I.M. 451.01 REQUIREMENTS SHALL APPLY FOR BAR CHAIRS, BAR HIGH CHAIRS, AND SLAB BOLSTERS.  
 COST OF BEARING MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "PRETENSIONED PRESTRESSED CONCRETE BEAMS".  
 LONGITUDINAL REINFORCING NEAR CLOSURE POUR CONSTRUCTION JOINT MAY BE SHIFTED SLIGHTLY TO PROVIDE 2" CLEARANCE.

NOTE: DRAINS ARE TO BE GALVANIZED. 6 DRAINS REQUIRED. SEE "SITUATION PLAN" FOR LOCATION. WEIGHT OF DRAINS IS INCLUDED IN THE QUANTITY FOR "STRUCTURAL STEEL". WEIGHT IS BASED ON ROLLED TUBE.

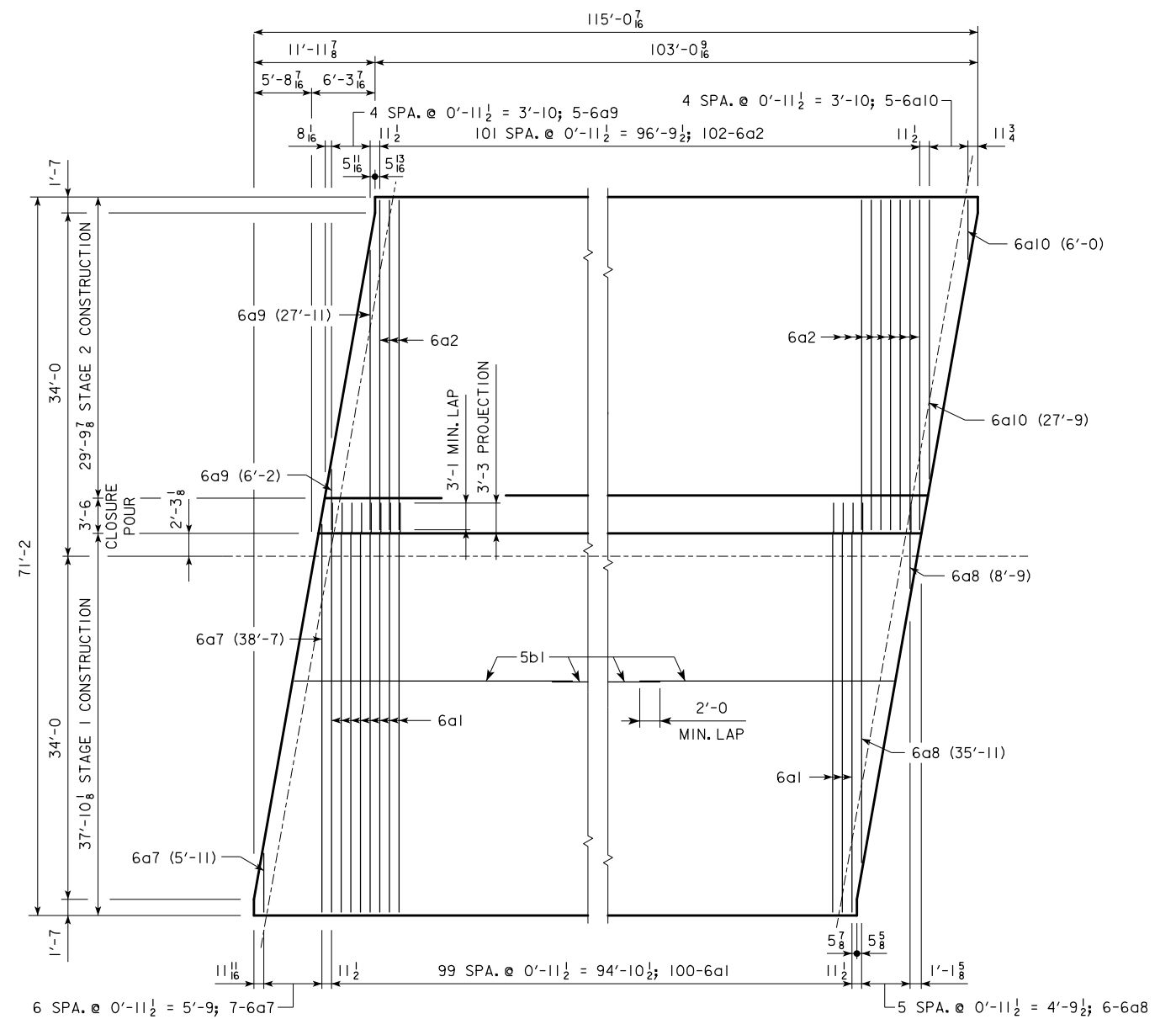
DATA FOR ONE DRAIN	
BEAM SIZE	BTB
DRAIN WEIGHT (LBS.)	92
DRAIN LENGTH (FT.)	4'-8 3/4"

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**SUPERSTRUCTURE DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 15 OF 29 FILE NO. 30687 DESIGN NO. 514

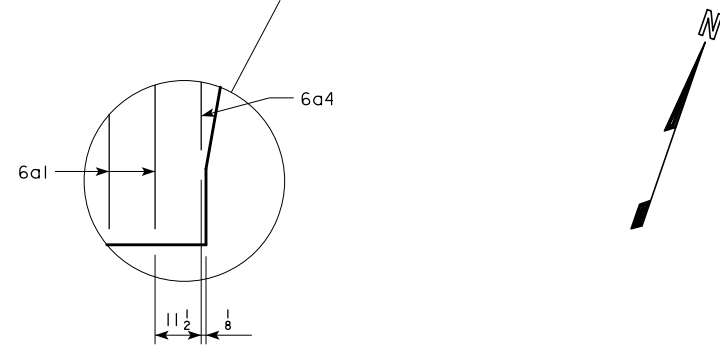




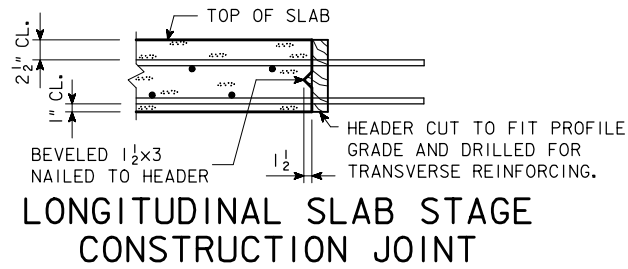
TOP OF SLAB TRANSVERSE AND LONGITUDINAL REINFORCING LAYOUT



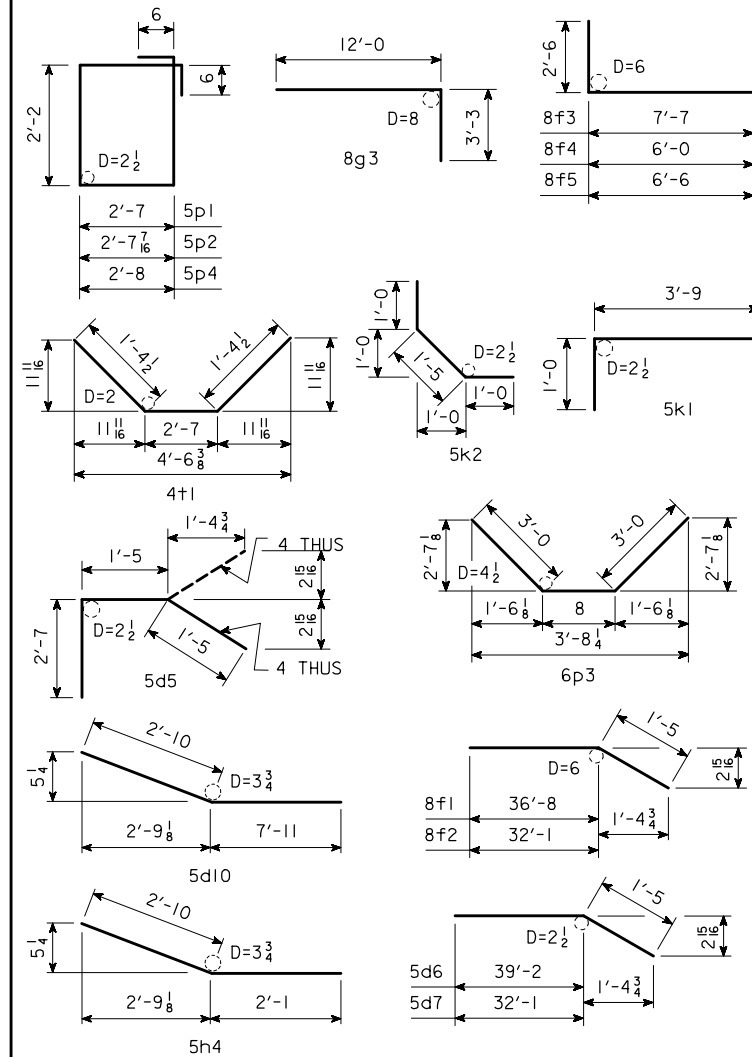
BOTTOM OF SLAB TRANSVERSE AND LONGITUDINAL REINFORCING LAYOUT



DESIGN FOR 10° SKEW (L.A.)  
100'-0 x 68'-0 PRETENSIONED PRESTRESSED  
CONCRETE BEAM BRIDGE  
SUPERSTRUCTURE DETAILS  
STATION: 293+68.30 DECEMBER, 2012  
SCOTT COUNTY  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 16 OF 29 FILE NO. 30687 DESIGN NO. 514



### BENT BAR DETAILS



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D= PIN DIAMETER.

### CONC. PLACEMENT QUANTITIES ONE SUPER. & TWO ABUTS.

LOCATION	STAGE 1	STAGE 2	QUANTITY
HIGH PERFORMANCE STRUCTURAL CONCRETE			
SLAB & ABUT. DIAPHS.	133.4	105.4	238.8
CLOSURE POUR & ABUT. DIAPHS.			12.2
HIGH PERFORMANCE STRUCTURAL CONCRETE - TOTAL (CU. YDS.)			
			251.0
STRUCTURAL CONCRETE (BRIDGE)			
WEST ABUTMENT FOOTINGS	16.9	14.9	31.8
EAST ABUTMENT FOOTINGS	18.7	16.6	35.3
ABUTMENT WINGS 4 AT 1.9			7.6
PREBORED HOLES W. ABUT. ONLY			2.2
STRUCTURAL CONCRETE (BRIDGE) - TOTAL (CU. YDS.)			
			76.9

### ESTIMATED QUANTITIES ONE SUPER. & TWO ABUTS.

ITEM	UNIT	QUANTITY
HIGH PERFORMANCE STRUCTURAL CONCRETE	CU. YD.	251.0
STRUCTURAL CONCRETE (BRIDGE)	CU. YD.	76.9
STRUCTURAL STEEL	LBS.	3484
REINFORCING STEEL EPOXY COATED	LBS.	68,403
REINFORCING STEEL	LBS.	246
PRETENSIONED PRESTRESSED CONCRETE BEAMS	EACH	9
CLASS 20 EXCAVATION	CU. YD.	197
PILES HP 10x57 11 @ 20' W.A.; 13 @ 30' E.A.	L.F.	610
PREBORED HOLES 11 @ 15' W.A. ONLY	L.F.	165

### REINF. BAR LIST-ONE SUPER. & TWO ABUTS.

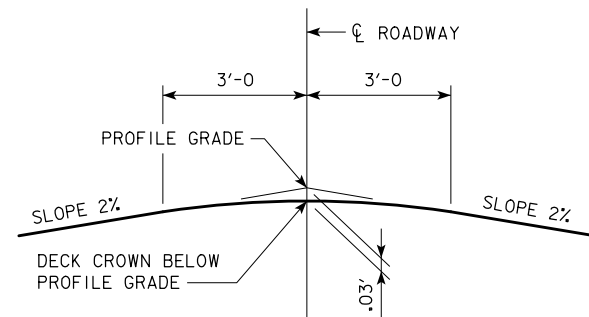
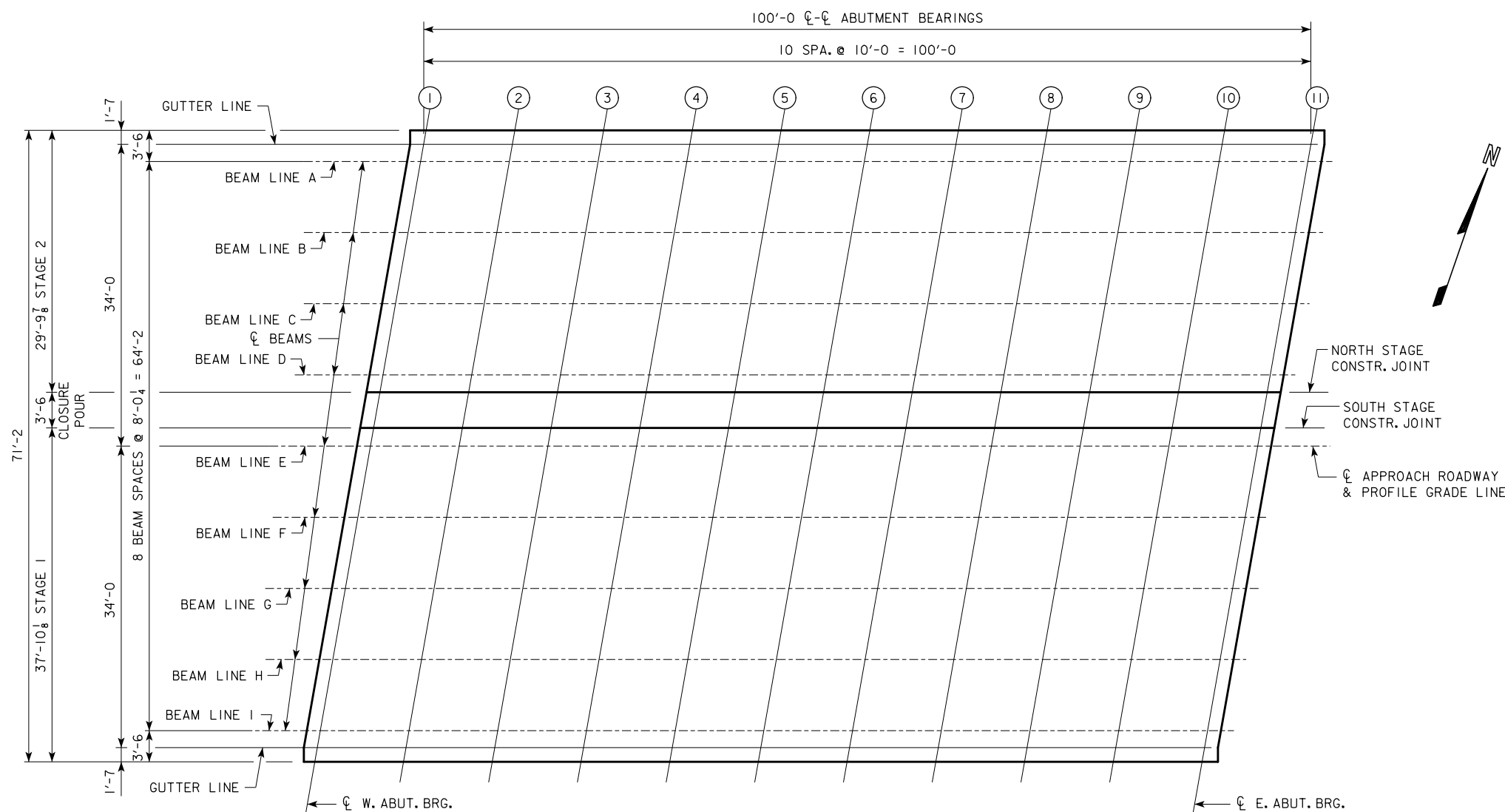
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
6a1	SLAB TRANSV. TOP & BOTT., STAGE 1	—	200	41'-0	12316
6a2	SLAB TRANSV. TOP & BOTT., STAGE 2	—	203	33'-0	10062
6a3	SLAB TRANSV. TOP W. END, STAGE 1	—	7	VARIES	205
6a4	SLAB TRANSV. TOP E. END, STAGE 1	—	7	VARIES	235
6a5	SLAB TRANSV. TOP W. END, STAGE 2	—	5	VARIES	148
6a6	SLAB TRANSV. TOP E. END, STAGE 2	—	6	VARIES	152
6a7	SLAB TRANSV. BOTT. W. END, STAGE 1	—	7	VARIES	234
6a8	SLAB TRANSV. BOTT. E. END, STAGE 1	—	6	VARIES	201
6a9	SLAB TRANSV. BOTT. W. END, STAGE 2	—	5	VARIES	128
6a10	SLAB TRANSV. BOTT. E. END, STAGE 2	—	5	VARIES	127
5b1	SLAB LONGIT. TOP & BOTT.	—	435	35'-9	16220
5d1	ABUT. DIAPH. LONGIT.	—	28	7'-3	212
5d2	ABUT. DIAPH. BOTT., LONGIT.	—	14	5'-4	78
5d3	ABUT. DIAPH. LONGIT., STAGE 1	—	4	4'-3	18
5d4	ABUT. DIAPH. LONGIT., STAGE 2	—	4	5'-3	22
5d5	ABUT. DIAPH. ENDS	□	8	5'-5	45
5d6	ABUT. DIAPH. LONGIT. B.F., STAGE 1	—	6	40'-7	254
5d7	ABUT. DIAPH. LONGIT. B.F., STAGE 2	—	6	33'-6	210
5d8	PAVING NOTCH LONGIT., STAGE 1	—	4	40'-8	170
5d9	PAVING NOTCH LONGIT., STAGE 2	—	4	33'-7	140
5d10	E. ABUT. DIAPH. WING EXT. LONGIT.	—	8	10'-9	90
5d11	E. ABUT. DIAPH. WING EXT. LONGIT.	—	8	10'-8	89
5d12	ABUT. DIAPH., BOTT., LONGIT., STAGE 1	—	2	3'-3	7
5d13	ABUT. DIAPH., BOTT., LONGIT., STAGE 2	—	2	4'-3	9
8f1	ABUT. FOOTING LONGIT. BOTH F., STAGE 1	—	18	38'-1	1830
8f2	ABUT. FOOTING LONGIT. BOTH F., STAGE 2	—	18	33'-6	1610
8f3	E. ABUT. FOOTING. EXT.	—	8	10'-1	215
8f4	E. ABUT. FOOTING. EXT., STAGE 1	—	4	8'-6	91
8f5	E. ABUT. FOOTING. EXT., STAGE 2	—	4	9'-0	96
8g1	ABUT. VERT. BOTH F.	—	220	7'-0	4112
8g3	ABUT. DIAPH. VERT. B.F.	—	118	15'-3	4805
6g4	E. ABUT. DIAPH. WING EXT. VERT.	—	20	5'-8	170
5h2	ABUT. TO WING ANCHOR	—	34	4'-11	174
5h4	E. ABUT. TO WING ANCHOR	—	6	4'-11	31
6j1	TOP OF SLAB TRANSV. (AT RAIL)	—	212	6'-3	1990
5k1	PAVING NOTCH	—	120	4'-9	595
5k2	PAVING NOTCH	—	120	3'-5	428
5p1	ABUT. HOOPS	□	236	10'-6	2585
5p2	ABUT. HOOPS AT ENDS & STAGE CONSTR. JT.	□	20	10'-7	221
6p3	ABUT. BOTT. AT PILES	—	44	6'-8	441
5p4	E. ABUT. EXTENSION HOOPS	□	12	10'-8	134
4+1	UNDER BEAMS AT ABUTMENTS	—	18	5'-4	64
BARRIER RAIL - SEE DESIGN SHT. NO. 23					6635
ABUTMENT WING - SEE DESIGN SHT. NO. 12 & 13					804
REINFORCING STEEL EPOXY COATED - TOTAL (LBS.)					68403
#2	PILE SPIRAL	—	24	38'-6	154
	SPIRAL SPACERS, L 7/8 x 7/8 x 1/8 x 0.70	—	72	1'-10	92
REINFORCING STEEL - TOTAL (LBS.)					246

### NON-COATED

DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**SUPERSTRUCTURE DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 17 OF 29 FILE NO. 30687 DESIGN NO. 514

CORRECTION 02-10 - 4+1 BAR LENGTH CHANGED. ENGLISH INTEGRAL BRIDGES.DGN - 4516-BTB - THIS SHEET ISSUED 02-08.

TOP OF SLAB ELEVATIONS											
LOCATION	℄ W. ABUT. BRG.										℄ E. ABUT. BRG.
	LINE 1	LINE 2	LINE 3	LINE 4	LINE 5	LINE 6	LINE 7	LINE 8	LINE 9	LINE 10	LINE 11
GUTTER LINE	570.20	570.27	570.34	570.40	570.46	570.51	570.56	570.60	570.64	570.67	570.70
BEAM LINE A	570.24	570.31	570.38	570.45	570.51	570.56	570.61	570.65	570.69	570.72	570.74
BEAM LINE B	570.43	570.50	570.58	570.64	570.70	570.76	570.80	570.85	570.88	570.92	570.94
BEAM LINE C	570.62	570.69	570.76	570.83	570.89	570.94	570.99	571.04	571.08	571.11	571.14
BEAM LINE D	570.79	570.86	570.94	571.00	571.06	571.12	571.17	571.22	571.25	571.29	571.32
STAGE CONSTRUCTION JOINT	570.83	570.90	570.98	571.04	571.10	571.16	571.21	571.25	571.29	571.33	571.36
STAGE CONSTRUCTION JOINT	570.90	570.97	571.04	571.11	571.17	571.23	571.28	571.32	571.37	571.40	571.43
℄ APPROACH ROADWAY & BEAM LINE E	570.91	570.98	571.06	571.12	571.19	571.24	571.29	571.34	571.38	571.41	571.44
BEAM LINE F	570.77	570.84	570.91	570.99	571.05	571.11	571.16	571.20	571.24	571.28	571.31
BEAM LINE G	570.58	570.65	570.72	570.79	570.86	570.91	570.97	571.01	571.06	571.09	571.12
BEAM LINE H	570.37	570.44	570.51	570.59	570.65	570.71	570.76	570.81	570.85	570.89	570.92
BEAM LINE I	570.16	570.23	570.30	570.37	570.44	570.50	570.55	570.60	570.64	570.68	570.71
GUTTER LINE	570.11	570.18	570.25	570.33	570.39	570.45	570.50	570.55	570.60	570.63	570.67



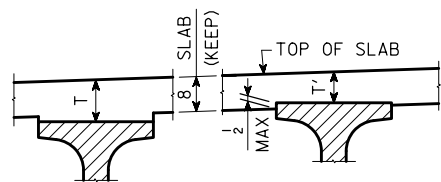
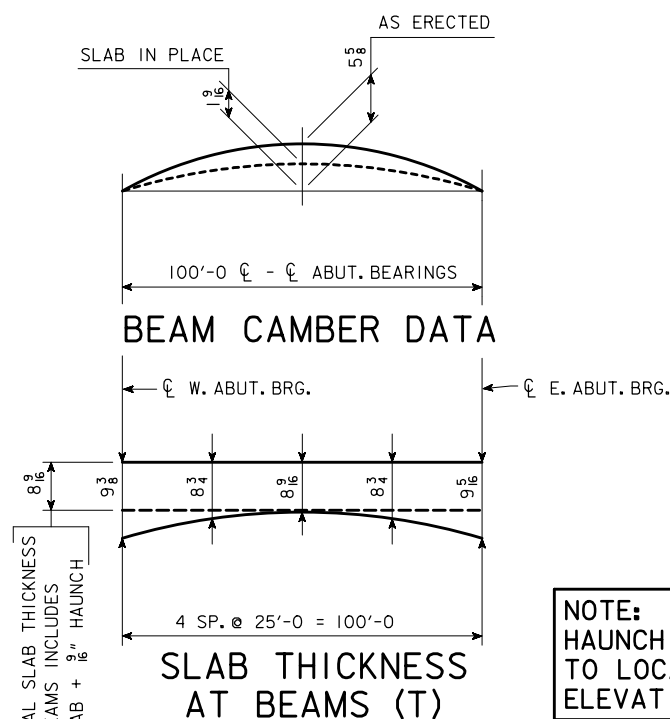
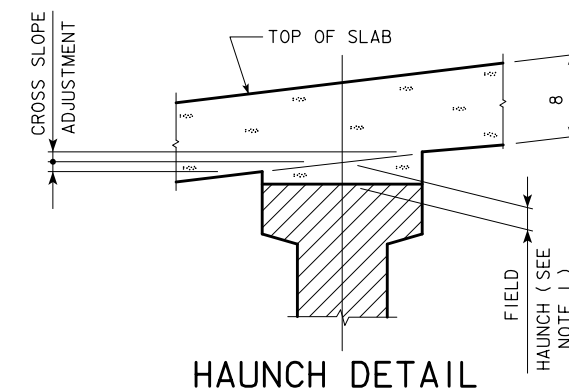
**CROWN TEMPLATE**  
TOP OF BRIDGE DECK CROWN .03' BELOW PROFILE GRADE.

TOP OF SLAB ELEVATION LOCATIONS

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 TOP OF SLAB ELEVATIONS  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 18 OF 29 FILE NO. 30687 DESIGN NO. 514

TABLE OF BEAM LINE HAUNCH ELEVATIONS											
LOCATION	C.L. W. ABUT. BRG.										C.L. E. ABUT. BRG.
	LINE 1	LINE 2	LINE 3	LINE 4	LINE 5	LINE 6	LINE 7	LINE 8	LINE 9	LINE 10	LINE 11
BEAM LINE A	569.57	569.76	569.93	570.06	570.16	570.23	570.26	570.27	570.24	570.17	570.08
BEAM LINE B	569.77	569.96	570.13	570.26	570.36	570.43	570.46	570.46	570.43	570.37	570.28
BEAM LINE C	569.95	570.14	570.31	570.45	570.55	570.62	570.65	570.66	570.63	570.57	570.47
BEAM LINE D	570.13	570.32	570.49	570.62	570.72	570.79	570.83	570.83	570.80	570.74	570.65
BEAM LINE E	570.25	570.44	570.61	570.74	570.84	570.91	570.95	570.96	570.93	570.87	570.78
BEAM LINE F	570.11	570.30	570.46	570.60	570.71	570.78	570.81	570.82	570.79	570.73	570.64
BEAM LINE G	569.91	570.11	570.27	570.41	570.51	570.59	570.62	570.63	570.61	570.55	570.46
BEAM LINE H	569.71	569.90	570.06	570.20	570.31	570.38	570.42	570.43	570.40	570.35	570.26
BEAM LINE I	569.49	569.68	569.85	569.99	570.10	570.17	570.21	570.22	570.19	570.14	570.05

MISCELLANEOUS DATA TABLE													
	BEAM LINE	C.L. W. ABUT. BRG.										C.L. E. ABUT. BRG.	
		LINE 1	LINE 2	LINE 3	LINE 4	LINE 5	LINE 6	LINE 7	LINE 8	LINE 9	LINE 10	LINE 11	
ANTICIPATED DEFLECTION DUE TO SLAB (in.)	ALL	0	1 <sup>7</sup> / <sub>16</sub>	2 <sup>5</sup> / <sub>8</sub>	3 <sup>7</sup> / <sub>16</sub>	3 <sup>7</sup> / <sub>8</sub>	4 <sup>1</sup> / <sub>16</sub>	3 <sup>7</sup> / <sub>8</sub>	3 <sup>7</sup> / <sub>16</sub>	2 <sup>5</sup> / <sub>8</sub>	1 <sup>7</sup> / <sub>16</sub>	0	
CROSS SLOPE ADJUSTMENTS IN.	A, B, C, G, H, I											7 <sup>7</sup> / <sub>16</sub>	
	D, F											5 <sup>5</sup> / <sub>16</sub>	
	E											1 <sup>1</sup> / <sub>16</sub>	
ALLOWABLE FIELD HAUNCH IN. & (FT.)	MAX.	ALL											2 <sup>1</sup> / <sub>2</sub> (0.21)
	MIN.	A, B, C, G, H, I											-1 <sup>1</sup> / <sub>16</sub> (-0.01)
		D, F											-3 <sup>3</sup> / <sub>16</sub> (-0.02)
		E											-7 <sup>7</sup> / <sub>16</sub> (-0.04)



**SLAB THICKNESS DETAILS**

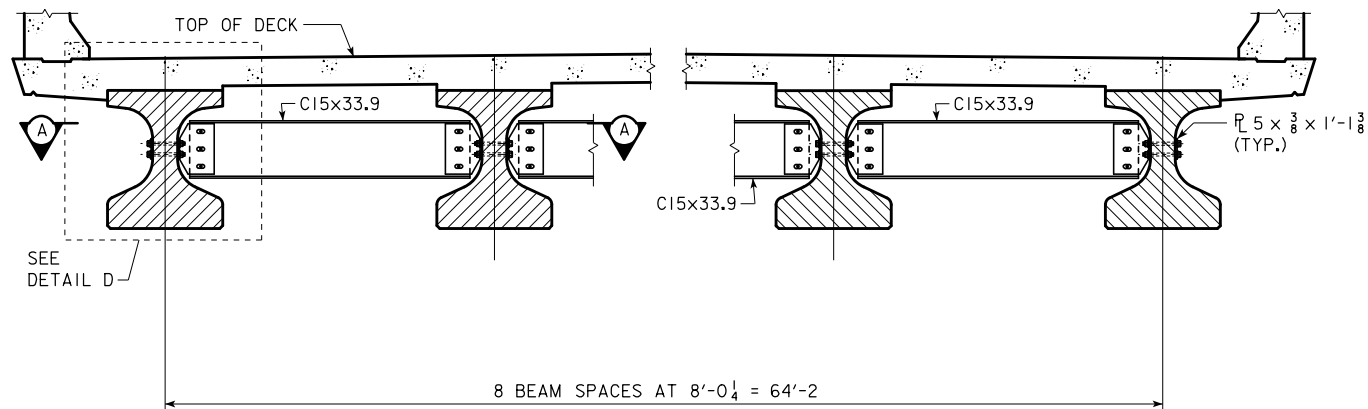
NOTE: THE SLAB THICKNESS (T) AT BEAMS IS BASED ON THE ANTICIPATED BEAM CAMBER AND DEFLECTIONS. THESE VALUES ARE USED BY THE DESIGNER TO SET BEAM ELEVATIONS AND ESTIMATE CONCRETE QUANTITIES. REFER TO THE HAUNCH DATA DETAILS SHEET FOR ADDITIONAL INFORMATION TO AID THE CONTRACTOR IN SETTING THE FIELD HAUNCHES REQUIRED FOR CONSTRUCTION.

**NOTE:**  
HAUNCH ELEVATIONS IN TABLE CORRESPOND TO LOCATIONS AS SHOWN ON "TOP OF SLAB ELEVATION LAYOUT" ON DESIGN SHEET 16.

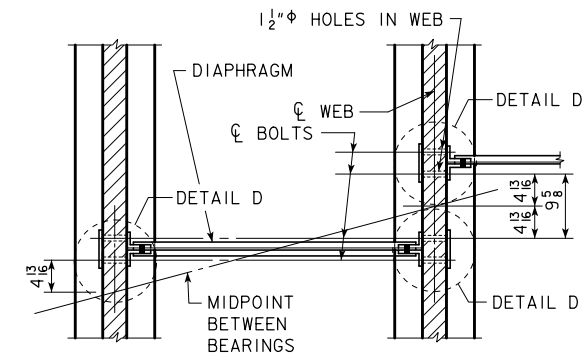
**NOTE 1:**  
TO CALCULATE FIELD HAUNCH REQUIRED AT EACH LOCATION, SURVEY THE BEAM TOPS CONSISTENT WITH THE SPACINGS SHOWN ON THE "TOP OF SLAB ELEVATIONS" LAYOUT. SUBTRACT THE SURVEYED BEAM SHOT FROM THE "BEAM LINE HAUNCH ELEVATION". THIS VALUE WILL BE THE HAUNCH NEEDED (SEE "FIELD HAUNCH" IN HAUNCH DETAIL). THE "BEAM LINE HAUNCH ELEVATION" INCLUDES ADJUSTMENTS FOR SLAB THICKNESSES AND ANTICIPATED DEFLECTIONS. NO ADDITIONAL CALCULATIONS ARE REQUIRED. IF THE FIELD HAUNCH EXCEEDS THE MAXIMUMS AND MINIMUMS, SHOWN IN INCHES AND DECIMALS FEET, ARE GIVEN IN THE MISCELLANEOUS DATA TABLE, ADJUSTMENTS TO THE GRADE OR ADDITIONAL HAUNCH REINFORCEMENT WILL BE REQUIRED.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**SUPERSTRUCTURE DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 19 OF 29 FILE NO. 30687 DESIGN NO. 514

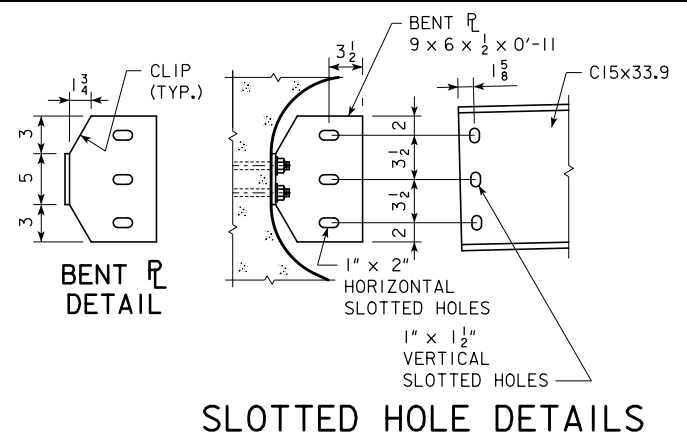
REVISION 04-13 - CORRECTED THE 'D' DIMENSIONS LOCATING THE HOLES IN THE BEAM WEB IN PART SECTION A-A. ENGLISHBEAMS.DGN - 1036-BTBW - THIS SHEET ISSUED 02-08.



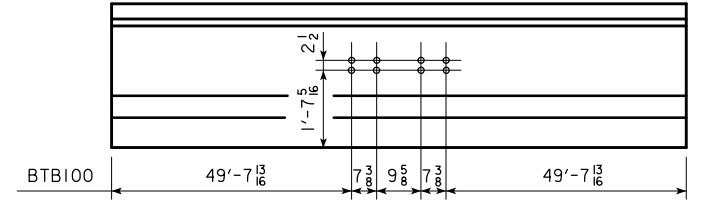
SECTION SHOWING INTERMEDIATE DIAPHRAGM



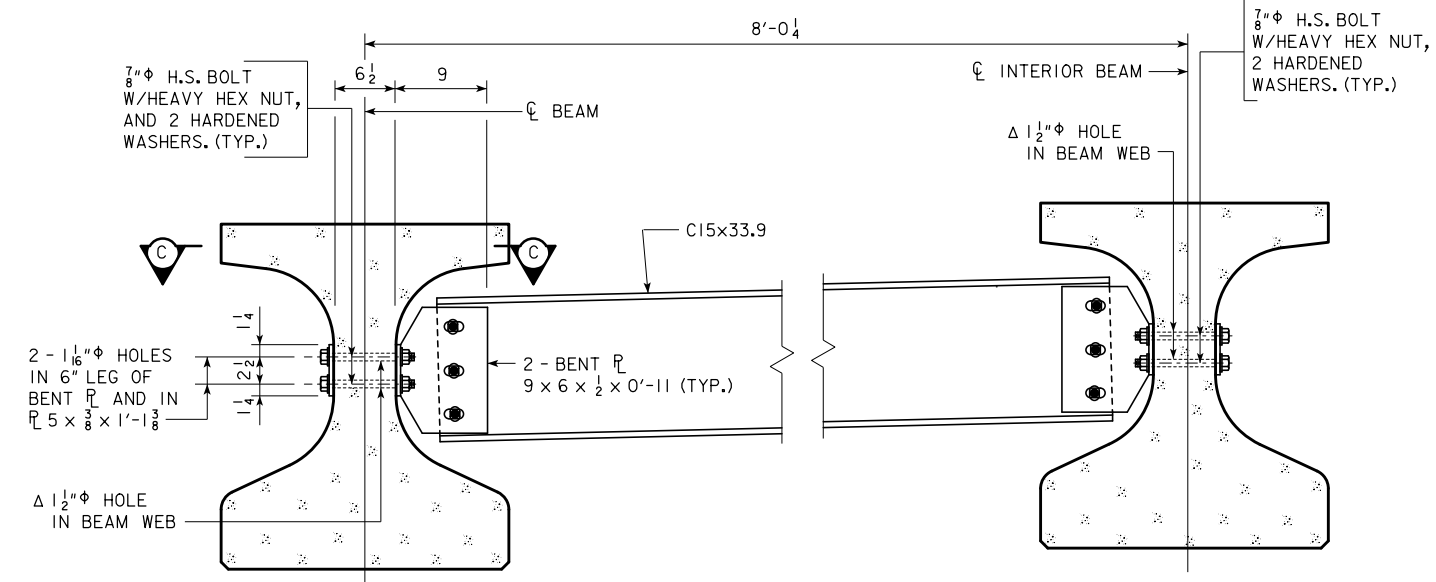
PART SECTION A-A  
FOR BRIDGES SKEWED GREATER THAN 7°30'



SLOTTED HOLE DETAILS

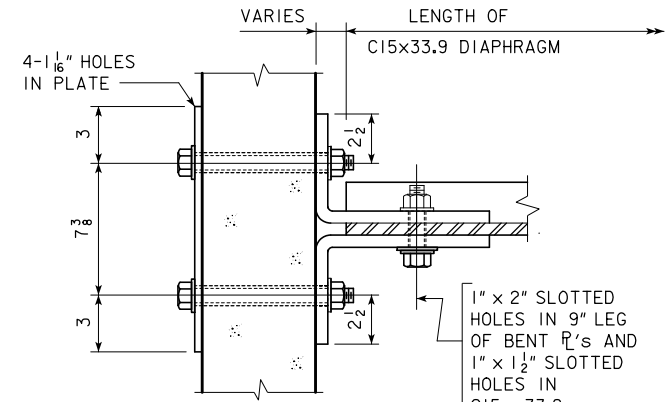


(7°31' TO 30° SKEW)  
INTERMEDIATE DIAPHRAGM  
BOLT HOLE LOCATIONS



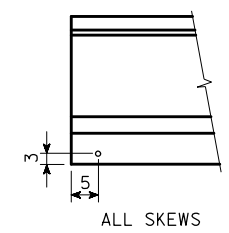
DETAIL D

Δ BOLT HOLES SHALL BE SPACED SO AS TO MISS PRESTRESSED STRANDS IN CONCRETE BEAMS.



SECTION C-C

**NOTES:**  
ALL DIAPHRAGM MATERIALS, INCLUDING BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED.  
SHOP DRAWINGS OF THE STEEL DIAPHRAGMS SHOWING LAYOUT AND DETAILS OF THE DIAPHRAGMS SHALL BE SUBMITTED FOR APPROVAL.  
ALL COSTS FOR FURNISHING AND INSTALLING STEEL INTERMEDIATE DIAPHRAGMS SHALL BE INCLUDED IN THE PRICE BID FOR STRUCTURAL STEEL.  
THE 1 1/2" HOLES FOR THE 7/8" H.S. BOLTS SHALL BE CAST INTO THE WEB. DRILLING IS NOT ALLOWED.  
THE 7/8" H.S. BOLTS THROUGH THE WEB SHALL HAVE A THREAD LENGTH OF 3" MIN. AND 4" MAX. AND SHALL MEET THE REQUIREMENTS OF ASTM A449.  
ALL BOLTS ARE TO BE TIGHTENED PRIOR TO PLACING BRIDGE FLOOR CONCRETE WITH THE FOLLOWING EXCEPTION: BOLTS IN DIAPHRAGMS LOCATED UNDER LONGITUDINAL BRIDGE FLOOR CONSTRUCTION JOINTS SHALL NOT BE TIGHTENED UNTIL STAGE TWO OF THE BRIDGE FLOOR HAS BEEN PLACED.

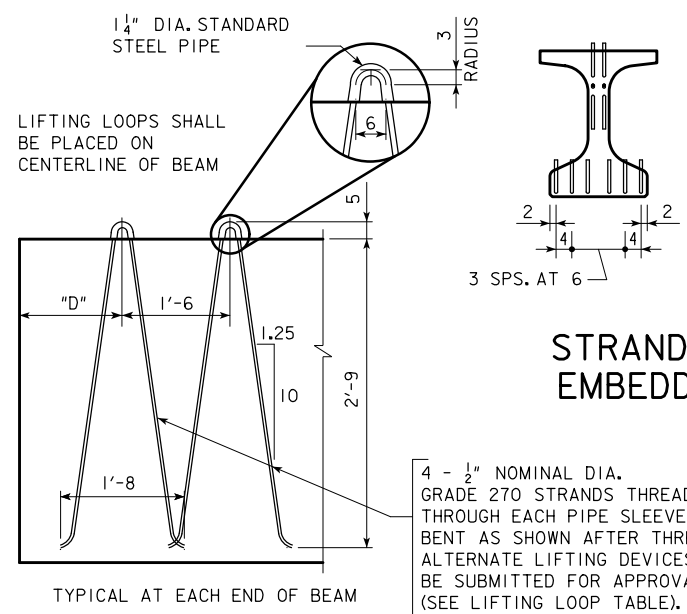


ALL SKEWS  
INTEGRAL ABUT.  
BEAM COIL TIE LOCATIONS

BULB TEE "B" BEAM INTERMEDIATE DIAPHRAGM STRUCTURAL STEEL			
ONE BEAM CONNECTION (DETAIL "D")		WEIGHT	
	NO. OF BEAM CONNECTIONS		
4 - 7/8" φ x 9 1/4" H.S. BOLTS WITH NUTS & WASHERS = 9.6 LBS.	16		154
<b>ONE DETAIL "D"</b>			
1 - BACKING 5 x 3/8 x 1'-1 3/8" = 7.1 LBS.	16		114
2 - BENT PL 9 x 6 x 1/2 x 0'-11" = 46.8 LBS.	16		749
ONE DIAPHRAGM		NUMBER OF DIAPHRAGMS	
6 - 7/8" φ x 3" H.S. BOLTS WITH NUTS & WASHERS = 7.8 LBS.	8		62
<b>LENGTH OF MEMBER</b>			
1 - C15 x 33.9 = 33.9 LBS./FT.	6'-10"	8	1853
INTERMEDIATE DIAPHRAGM STRUCTURAL STEEL - TOTAL (LBS.)			2932

DESIGN FOR 10° SKEW (L.A.)  
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**INTERMEDIATE DIAPHRAGM DETAILS**  
 STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 20 OF 29      FILE NO. 30687      DESIGN NO. 514

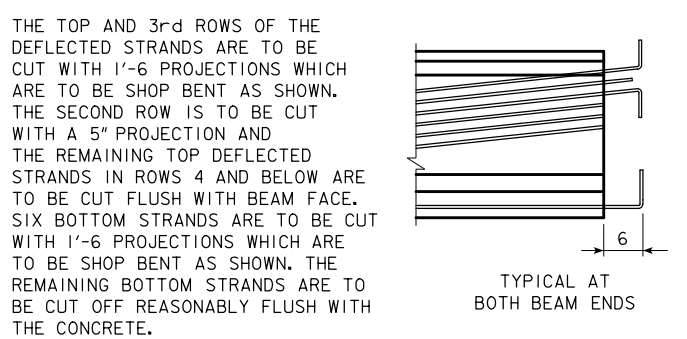
REVISION 08-12 - I.M. REFERENCE NOTE FOR SEALING BEAM ENDS DISTINGUISHES BETWEEN THE FABRICATOR AND CONTRACTOR. DECK PANEL OPTION NOTE WAS DELETED. ENGLISHBEAMS.DGN - 4765 - THIS SHEET ISSUED 02-08.



**LIFTING LOOP DETAIL**

LIFTING LOOP AND OVERHANG TABLE				
BEAMS	LIFTING LOOPS EACH END	# OF STRANDS PER LOOP	D	BEAM OVERHANG (FT)
BTB100	2	4	6'-3	12

\*\* IN ACCORDANCE WITH STANDARD SPECIFICATION 2407.13, LIFTING LOOPS SHALL CARRY LOADS EQUALLY.



**STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS**

**DESIGN STRESSES:**

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007. REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5. PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 270.

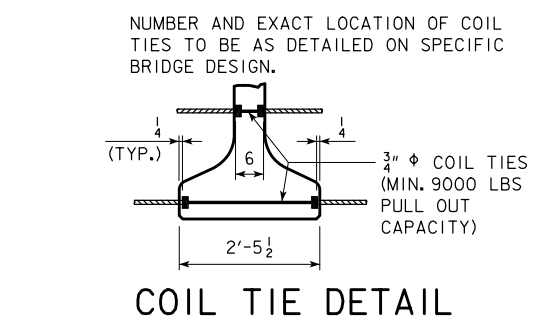
**SPECIFICATIONS:**

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.

DESIGN: AASHTO LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

**ALTERNATE BAR NOTES:**

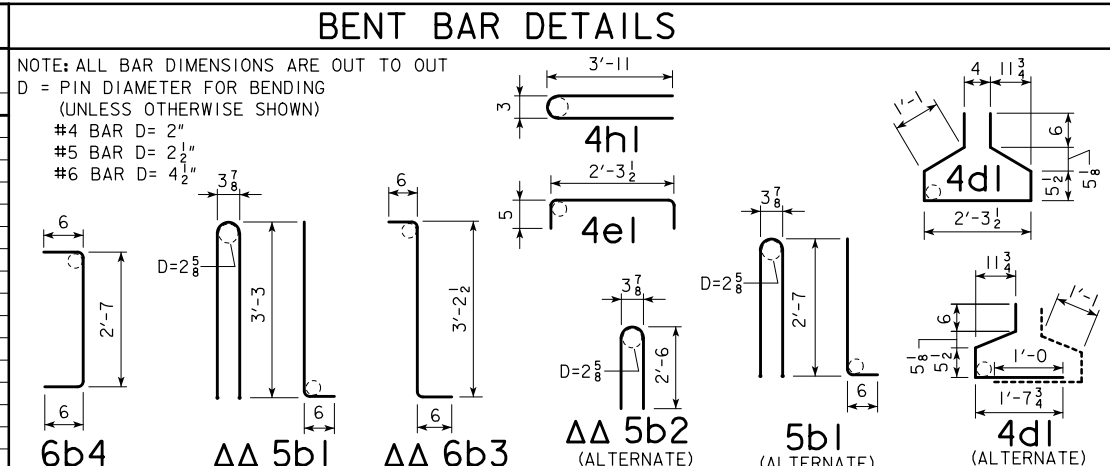
ALTERNATE BARS SHOWN IN BENT BAR DETAILS MAY BE USED IN LIEU OF REINFORCING BARS SHOWN IN BAR LIST. NO ADDITIONAL PAYMENT SHALL BE MADE FOR USE OF ALTERNATE BARS.



**COIL TIE DETAIL**

ΔΔ 5b1 AND 6b3 BARS TO BE EPOXY COATED  
\* 6b3 AND 6b4 BARS TO BE USED IN PAIRS

REINFORCING BAR LIST				
BEAM	BAR	SHAPE	NO.	LENGTH
BTB100	5a1	—	12	32'-9
	5a2	—	6	40'-0
ΔΔ	5b1	—	79	7'-8
ΔΔ *	6b3	—	32	4'-3
	6b4	—	16	3'-7
	4c1	—	121	2'-7
	4d1	—	101	6'-5
	4e1	—	26	3'-2
	4h1	—	4	8'-0



**BTB BEAM DATA**

BTB BEAM	SPAN LENGTH @ BEARING	OVERALL BEAM LENGTH (L)	CONCRETE STRENGTH		STRAND SIZE DIA. (in)	NO. OF STRAND		TOTAL INITIAL PRESTRESS kips	HOLD DOWN FORCE-kips	CAMBER (in)		DEFLECTION (in) Δ <sub>D</sub>		PERMISSIBLE MAXIMUM SPACING	WEIGHT (TONS)	CONCRETE (CU YD.)	REINFORCING STEEL (WEIGHT-LBS)
			f'ci (ksi)	f'c (ksi)		STRAIGHT	DEFLECTED			AT RELEASE	AFTER LOSSES	IMMEDIATE (ELASTIC) Δ <sub>E</sub>	TIME (PLASTIC) Δ <sub>T</sub>				
			HL-93 LOADING	STEEL DIAPHRAGM		STEEL DIAPHRAGM											
④ BTB100	100'-0	101'-4	8.0	9.0	0.60	32	12	1871	20.9	3.19	5.63	3.44	0.86	8'-6	33.3	16.5	2300

- ① DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM. THE DEFLECTIONS SHOWN ARE FOR A SLAB (8 in) AND HAUNCH (1.5 in) WEIGHT OF:  
0.93 kips/ft FOR 8'-6 BEAM SPACING  
0.80 kips/ft FOR 7'-4 BEAM SPACING  
AND ONE STEEL DIAPHRAGM (0.500 kips) AT C<sub>L</sub> OF SPAN. FOR DIFFERENT SLAB AND DIAPHRAGM WEIGHTS, DEFLECTIONS WILL BE DIRECTLY PROPORTIONAL.
- ② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.  
TOTAL BEAM DEFLECTIONS AT C<sub>L</sub> OF SPAN, Δ<sub>D</sub>, DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE:  
(A) Δ<sub>D</sub> = Δ<sub>E</sub> + Δ<sub>T</sub> FOR SIMPLE SPAN.  
(B) Δ<sub>D</sub> = Δ<sub>E</sub> + 3/4 Δ<sub>T</sub> FOR END SPANS OF CONTINUOUS BRIDGE.  
(C) Δ<sub>D</sub> = Δ<sub>E</sub> + 1/2 Δ<sub>T</sub> FOR INTERIOR SPANS OF CONTINUOUS BRIDGE.
- ③ TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f's, f's = 270 ksi. AND A<sub>s</sub> = 0.217 in<sup>2</sup>.
- ④ REQUIRES 4000 psi COMPRESSIVE STRENGTH FOR CAST-IN-PLACE SLAB CONCRETE.

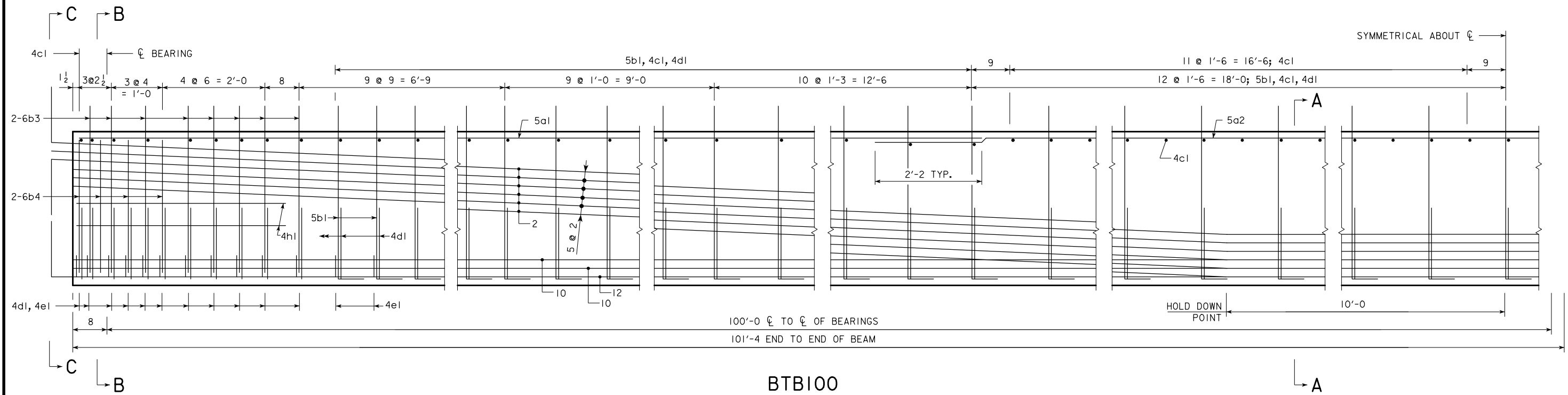
CALCULATED DESIGN CAMBERS HAVE BEEN REDUCED FROM THEIR THEORETICAL VALUES BY 15% TO AID CONSTRUCTABILITY.

**BEAM NOTES:**

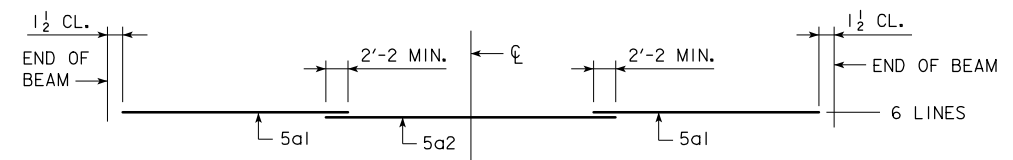
THESE BEAMS ARE DESIGNED FOR AASHTO LIVE LOADS AS INDICATED IN ABOVE TABLE WITH AN ALLOWANCE OF 20 LBS PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE. ALL PPC BEAMS SHALL USE HIGH PERFORMANCE CONCRETE (HPC) IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION. ALL PRESTRESSING STRANDS EXCEPT LIFTING LOOP STRANDS SHALL BE 0.60 IN. NOMINAL DIAMETER (NOMINAL STEEL AREA = 0.217 in<sup>2</sup>) AND CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS. MINIMUM STRAND BREAKING STRENGTH SHALL BE 58.6 kips. TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINISHED AS PER MATERIALS 1M570. BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS. BEAMS TO BE USED IN BRIDGES MADE CONTINUOUS BY THE POURED IN PLACE FLOOR, ARE TO BE AT LEAST 28 DAYS OLD BEFORE THE FLOOR IS PLACED UNLESS A SHORTER CURING TIME IS APPROVED BY THE BRIDGE ENGINEER. THE PORTIONS OF THE PRESTRESSED BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT AND PIER DIAPHRAGMS SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS. ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE. FOR TRANSPORTING, THE ALLOWABLE OVERHANG IS SHOWN IN THE LIFTING LOOP AND OVERHANG TABLE. THE CONTRACTOR SHALL ASSURE THE LATERAL STABILITY OF THE BEAMS DURING HANDLING, TRANSPORTING AND ERECTION BY PROVIDING TEMPORARY BRACING AS NEEDED. HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET. IF SOLE PLATE IS REQUIRED FOR BEARING, SOLE PLATE IS TO BE SET IN FORMS WHEN BEAM IS CAST AND FORMED OUT BELOW TO EXCLUDE CONCRETE AS DETAILED ON THE BEARING SHEET. IF STUB ABUTMENTS ARE USED, ALL STRANDS AT THE ENDS OF BEAMS AT STUB ABUTMENTS SHALL BE CUT OFF REASONABLY FLUSH WITH THE CONCRETE. MINIMUM CONCRETE f'c (AT 28 DAYS) AND MINIMUM f'ci AT RELEASE ARE LOCATED IN THE BTB BEAM DATA TABLE ABOVE. FOUR 0.60 IN. DIAMETER STRANDS STRESSED TO NOT MORE THAN 5000 lbs EACH MAY BE USED IN LIEU OF BARS 5a1 AND 5a2 IN THE TOP FLANGE. WHEN EXPANSION JOINTS ARE USED, CONCRETE SEALER SHALL BE APPLIED TO THE PRESTRESSED BEAM END SECTIONS. THE SEALING SHALL BE IN ACCORDANCE WITH MATERIALS I.M. 570 (FABRICATOR APPLICATION) AND I.M. 491.12 (CONTRACTOR APPLICATION).

DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**BTB100 BEAM DETAILS**  
 STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 21 OF 29    FILE NO. 30687    DESIGN NO. 514

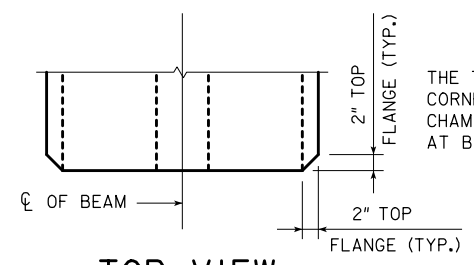
ENGLISHBEAMS.DGN - 4766 - THIS SHEET ISSUED 02-08.



BTB100

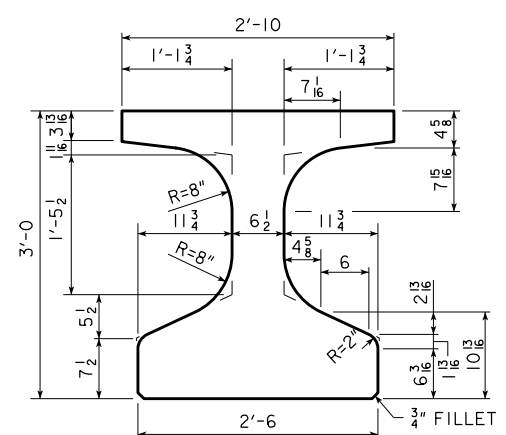


TOP FLANGE LONGITUDINAL BAR LAYOUT



TOP VIEW

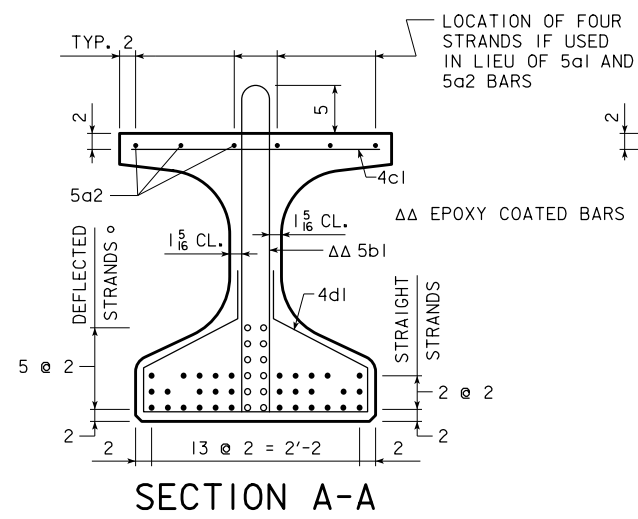
THE TOP FLANGE BEAM CORNERS ARE TO BE CHAMFERED 2" AS SHOWN AT BOTH ENDS OF THE BEAM.



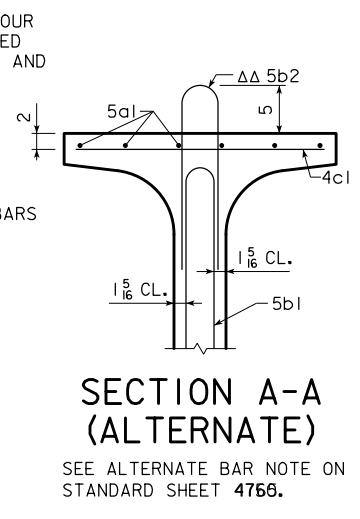
BEAM SECTION PROPERTIES

AREA = 631.7 in<sup>2</sup>  
 $\bar{y}_b = 17.14$  in.  
 $I = 99,980$  in<sup>4</sup>

BTB BEAM CROSS SECTION

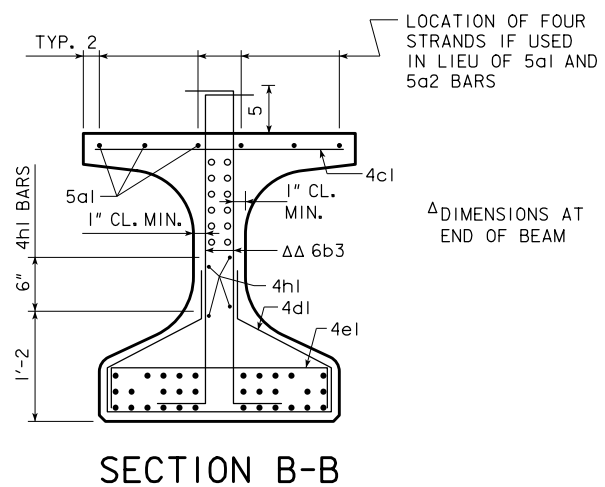


SECTION A-A

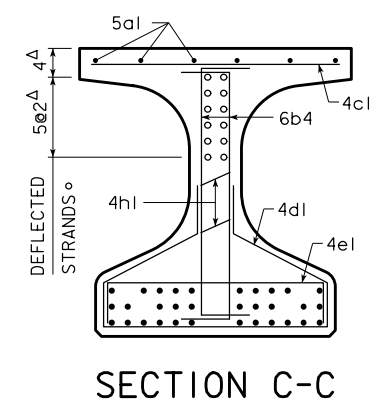


SECTION A-A (ALTERNATE)

SEE ALTERNATE BAR NOTE ON STANDARD SHEET 4766.

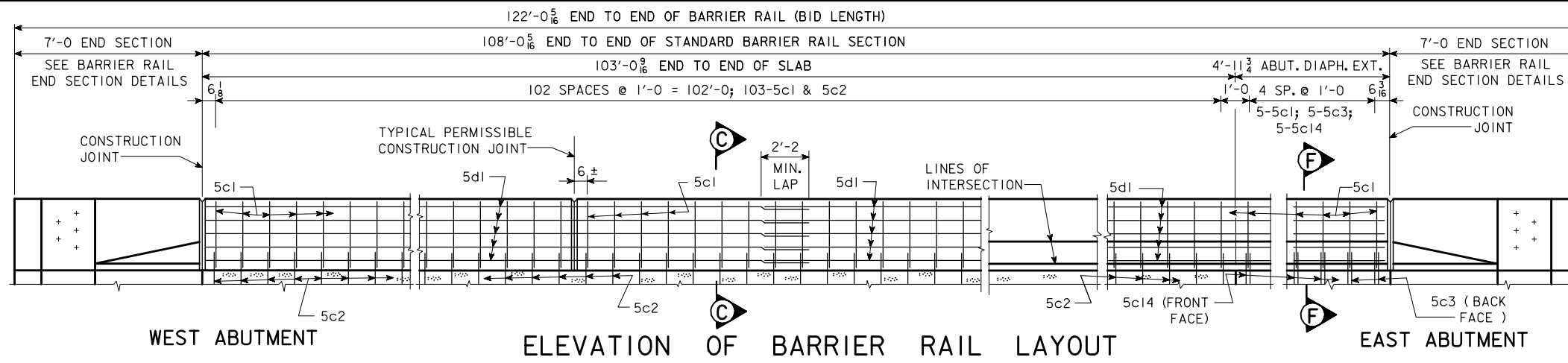


SECTION B-B

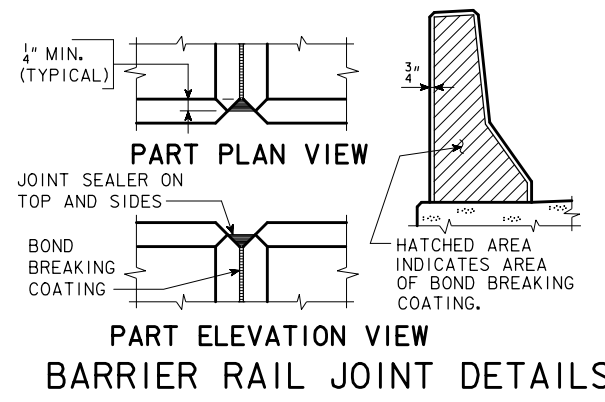


SECTION C-C

DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**BTB100 BEAM DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 22 OF 29 FILE NO. 30687 DESIGN NO. 514



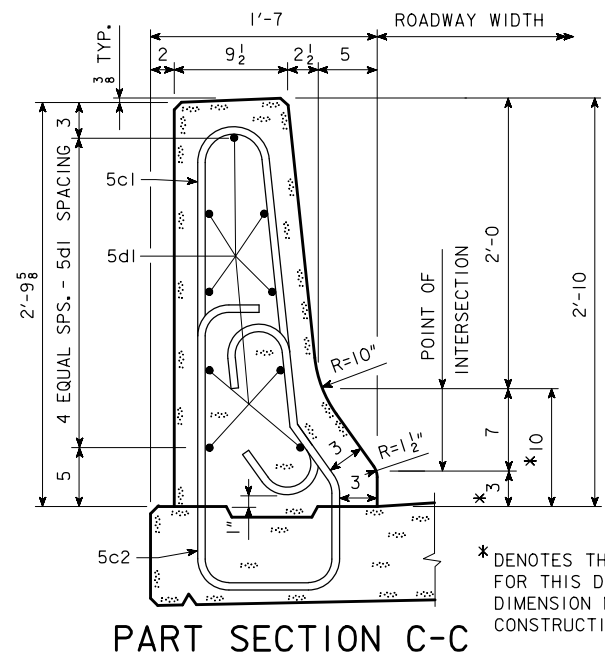
ELEVATION OF BARRIER RAIL LAYOUT



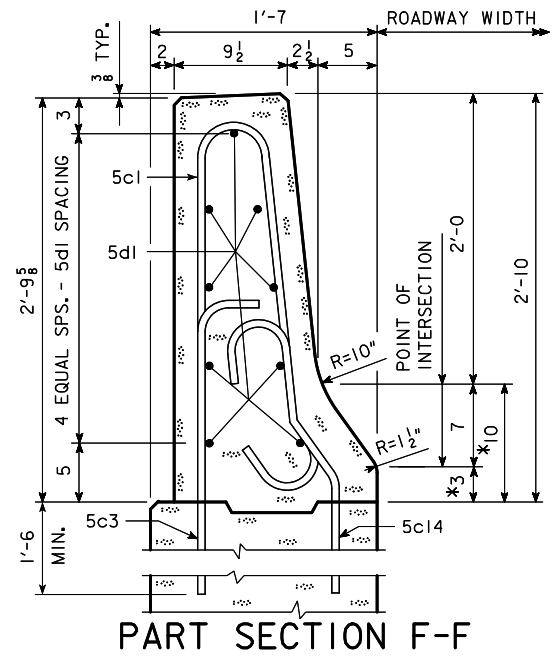
BARRIER RAIL JOINT DETAILS

**BARRIER RAIL NOTES:**

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.  
 THE PERMISSIBLE CONSTRUCTION JOINTS ARE TO BE PLACED BETWEEN VERTICAL BARS AT A MINIMUM SPACING OF 20 FEET. CONSTRUCTION JOINT CONTACT SURFACES ARE TO BE COATED WITH AN APPROVED BOND BREAKER. COST OF THE JOINT SEALER AND BOND BREAKER SHALL BE CONSIDERED INCIDENTAL TO OTHER CONSTRUCTION.  
 ALL BARRIER RAIL REINFORCING STEEL IS TO BE EPOXY COATED.  
 THE CONCRETE BARRIER RAIL IS TO BE BID ON A LINEAL FOOT BASIS. THE NUMBER OF LINEAL FEET OF BARRIER RAIL INSTALLED WILL BE PAID FOR AT THE CONTRACT PRICE PER LINEAL FOOT BASED ON PLAN QUANTITIES. PRICE BID FOR CONCRETE BARRIER RAILING SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIAL, EXCLUDING REINFORCING STEEL, AND ALL OF THE EQUIPMENT AND LABOR REQUIRED TO ERECT THE RAIL IN ACCORDANCE WITH THESE PLANS AND CURRENT SPECIFICATIONS. IF CONDUIT IS REQUIRED IN THIS PLAN THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS CONSIDERED INCIDENTAL TO THE COST OF THE RAILING.  
 ALL BARRIER RAIL REINFORCING STEEL IS TO BE INCLUDED WITH THE SUPERSTRUCTURE REINFORCING STEEL.  
 THE JOINT SEALER SHALL BE LIGHT GRAY NONSAG LATEX CAULKING SEALER MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED.  
 TOP OF THE BARRIER RAIL IS TO BE PARALLEL TO THE THEORETICAL CL GRADE.  
 CROSS SECTIONAL AREA OF THE STANDARD SECTION OF THE BARRIER RAIL = 2.84 SQUARE FEET.



PART SECTION C-C

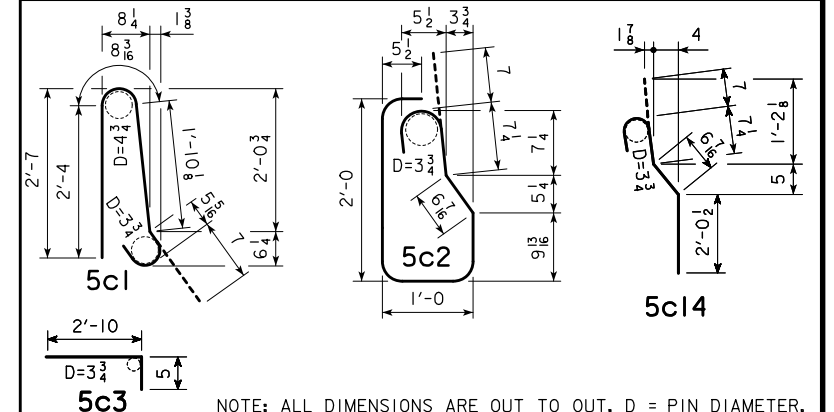


PART SECTION F-F

**EPOXY REINF. STEEL-TWO BARRIER RAILS**

SECTION	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
STANDARD SECTION	5c1	VERTICAL	U	216	5'-11"	1333
	5c2	VERTICAL	U	206	6'-0"	1289
	5c3	VERTICAL	U	10	3'-3"	34
	5c14	VERTICAL	U	10	3'-10"	40
	5d1	LONGITUDINAL	—	54	37'-5"	2107
BARRIER RAIL END SECTION				4 AT 458 LBS.		1832
(INCLUDE WITH SUPERSTRUCTURE REINFORCING)				TOTAL (LBS.)		6635

**BENT BAR DETAILS**



**CONCRETE PLACEMENT SUMMARY**

SECTION	TOTAL
STANDARD SECTION	22.7
BARRIER RAIL END SECTION	2.6
TOTAL (CU. YD.)	25.3

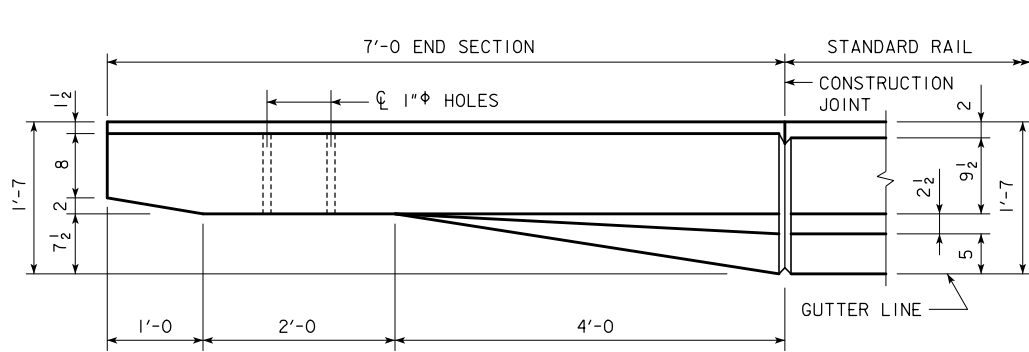
**CONCRETE BARRIER RAIL QUANTITIES**

ITEM	UNIT	QUANTITY
CONCRETE BARRIER RAILING	L.F.	244.0

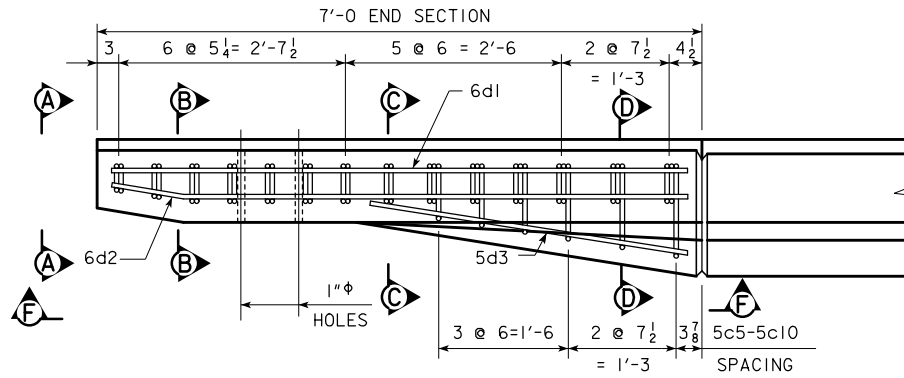
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**CONCRETE BARRIER RAIL DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 23 OF 29 FILE NO. 30687 DESIGN NO. 514



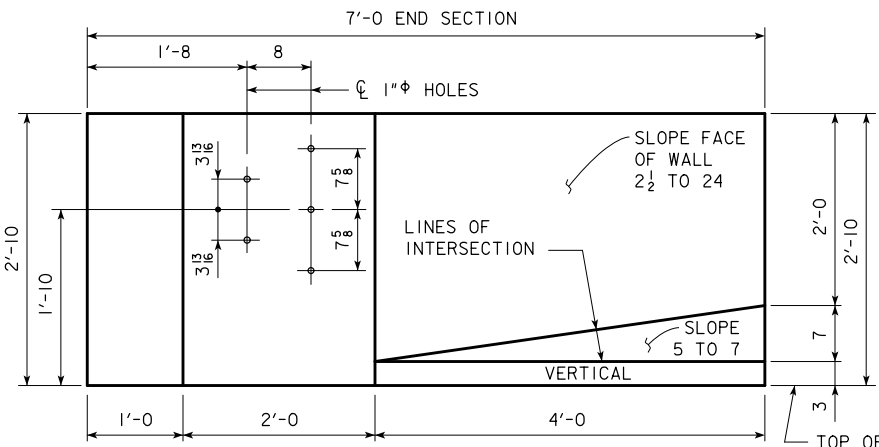
REVISION 02-08 - REINFORCING STEEL PATTERN & SIZE CHANGED AND WEIGHT ADJUSTED. CONCRETE THICKNESS WAS INCREASED 1/2" AND QUANTITY ADJUSTED. ENGLISH@RAILBRIDGES.DGN 10/17 - THIS SHEET ISSUED 09-01



PART PLAN VIEW

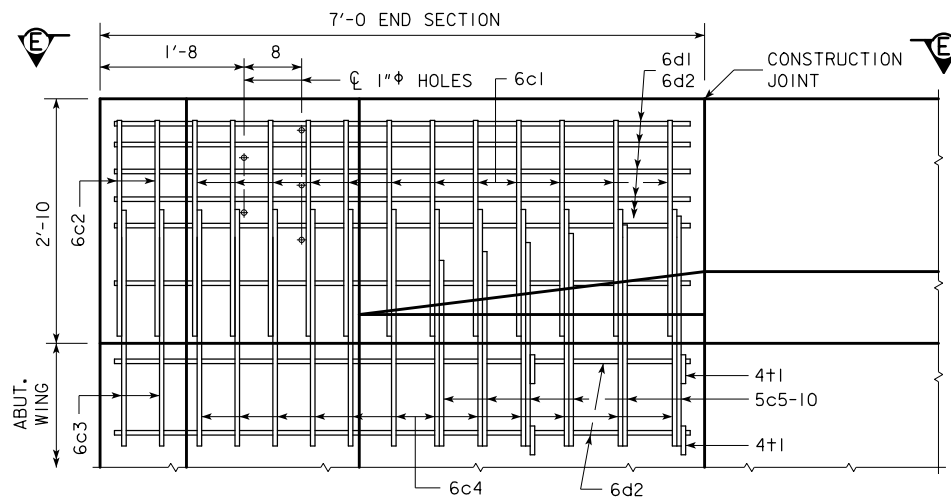


PART VIEW E-E

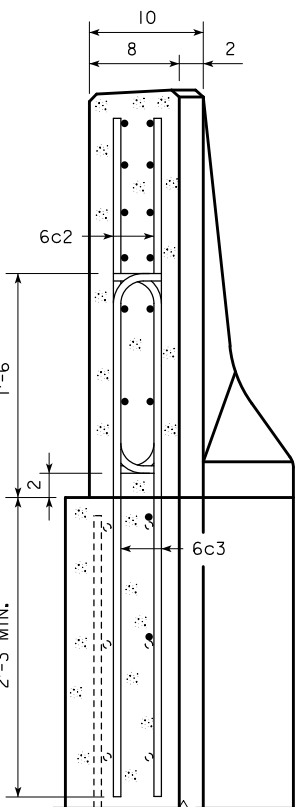


PART ELEVATION VIEW

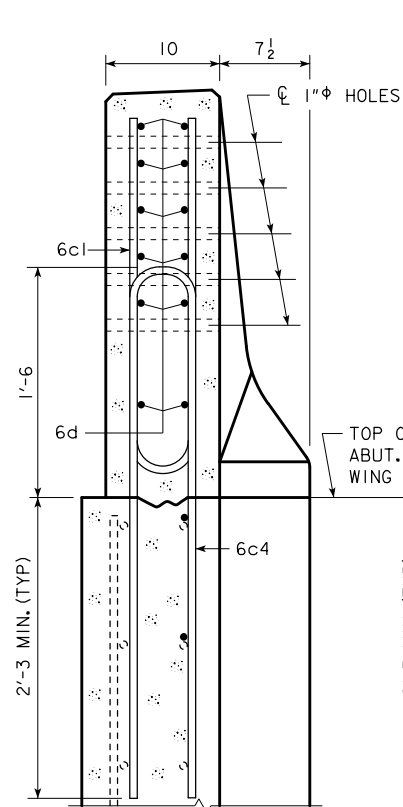
PROVIDE 5 HOLES FORMED WITH 1" PLASTIC CONDUIT. COST TO BE INCLUDED IN PRICE BID FOR CONCRETE BARRIER RAILING.



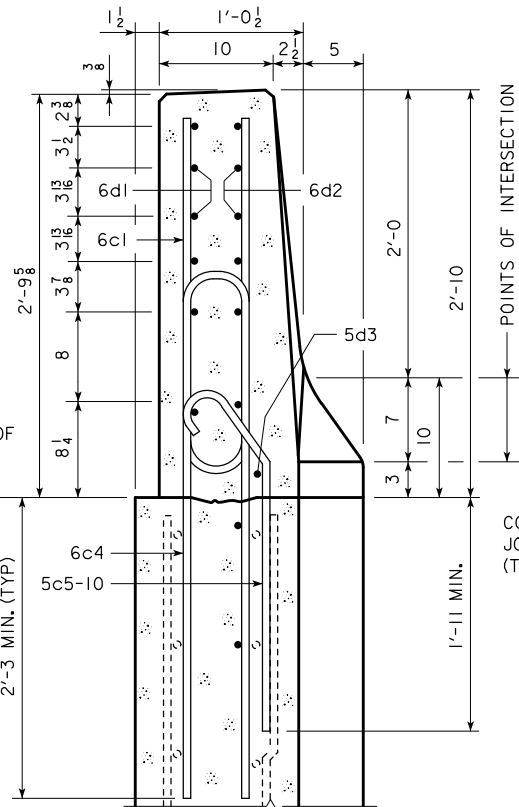
PART VIEW F-F



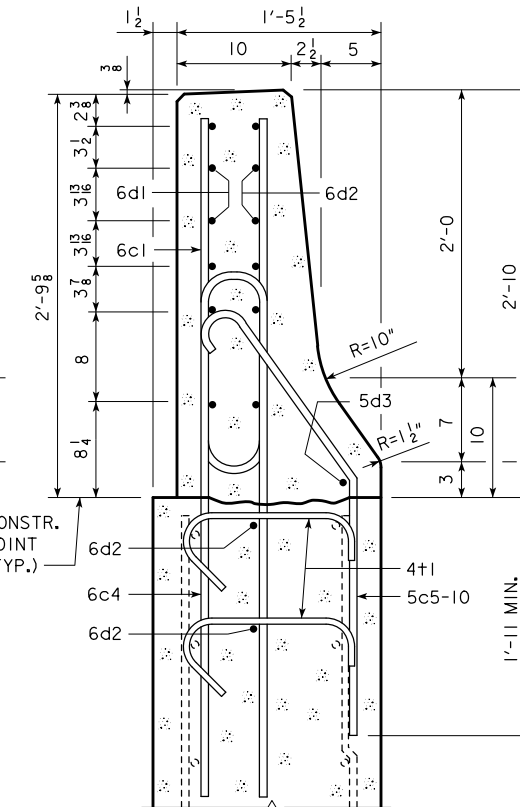
VIEW A-A



SECTION B-B



SECTION C-C



SECTION D-D

NOTE:  
4+1 PLACEMENT- 2 BARS EACH LEVEL OF 6d2 IN WING FOOTING.

NOTE:  
CONSTRUCTION JOINT BETWEEN TOP OF WING AND BARRIER RAIL IS ROUGHENED CONCRETE.

NOTE:  
THE 10" RADIUS AND 1 1/2" RADIUS ARE TYPICAL AND SHALL BE USED WHEN CONSTRUCTING THE CORNERS FOR VIEW A-A, SECTION B-B, SECTION C-C AND SECTION D-D.

NOTE:  
THE 6c4, 6c3, 5c5-10, 2 - 6d2 AND 4+1 BARS ARE TO BE PLACED WITH THE ABUTMENT WING. THE DETAILS FOR PLACEMENT ARE SHOWN ON THE WING ABUTMENT SHEET.

NOTE:  
DASHED LINES BELOW THE TOP OF WING ARE THE ABUTMENT WING REINFORCING STEEL. SEE WING ABUTMENT SHEET FOR PLACEMENT.

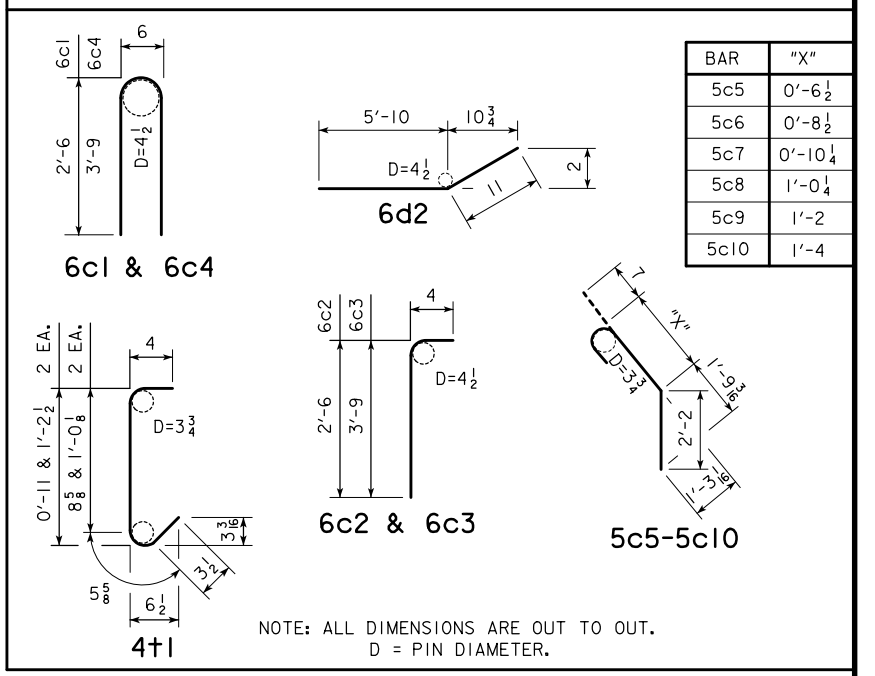
EPOXY REINFORCING STEEL - ONE END SECTION

BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT	
6c1	VERTICAL	U	12	5'-6"	99	
6c2	VERTICAL	U	4	2'-10"	17	
6c3	VERTICAL	U	4	4'-1"	25	
6c4	VERTICAL	U	12	8'-0"	144	
5c5-10	VERTICAL	U	6	VARIES	23	
6d1	HORIZONTAL	—	6	6'-8"	60	
6d2	HORIZONTAL	—	8	6'-9"	81	
5d3	HORIZONTAL	—	1	3'-9"	4	
4+1	ABUTMENT WING TIE BARS	—	4	VARIES	5	
(INCLUDE WITH BARRIER RAIL REINFORCING)					TOTAL WEIGHT (LBS.)	458

CONCRETE PLACEMENT SUMMARY

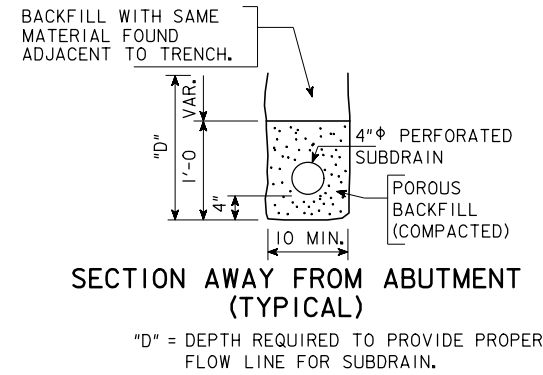
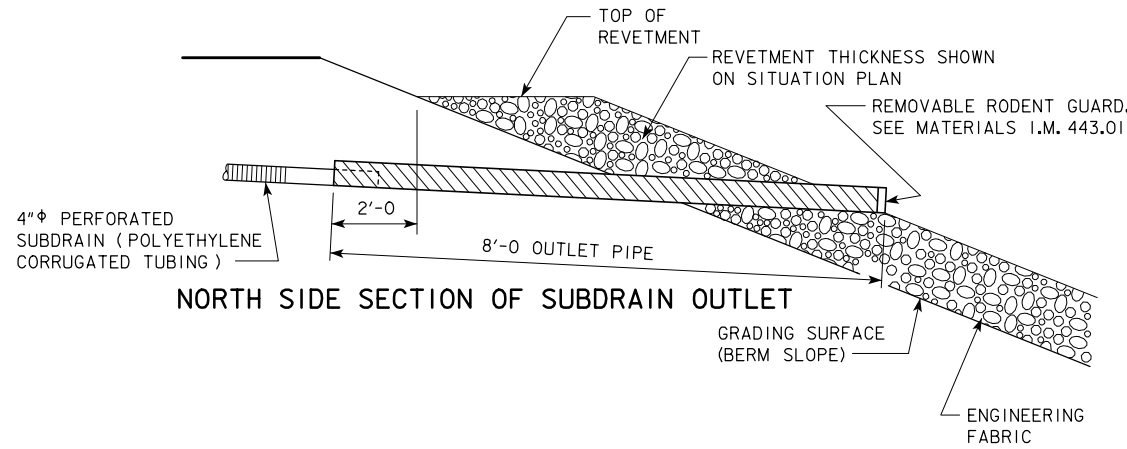
SECTION	TOTAL
BARRIER RAIL ONE END SECTION	0.65 CU. YD.

BENT BAR DETAILS



DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**BARRIER RAIL END SECTION DETAILS**  
 STATION: 293+68.30  
 SCOTT COUNTY  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 24 OF 29 FILE NO. 30687 DESIGN NO. 514  
 DECEMBER, 2012

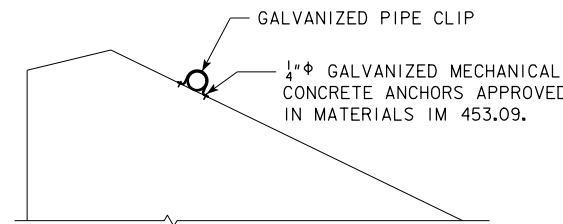
BENCH MARK NO. 603 STA. 293+83.546, 45.303 LT. FD IHC BM ON INHDL OF ARCH BRG, ELEV. 566.498.



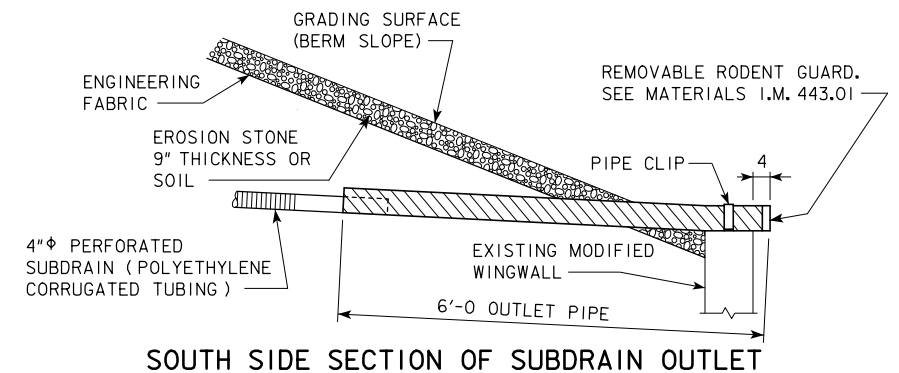
**SUBDRAIN NOTES :**

THIS PLAN SHEET SHOWS DETAILS FOR PLACING ALL SUBDRAINS AND SUBDRAIN OUTLETS REQUIRED FOR THIS STRUCTURE.  
 THE SUBDRAINS SHALL BE 4" IN DIAMETER AND SHALL BE IN ACCORDANCE WITH ARTICLE 4143.01, B, OF THE STANDARD SPECIFICATIONS.  
 THE SUBDRAIN OUTLET SHALL CONSIST OF A LENGTH OF CMP PIPE WITH A REMOVABLE RODENT GUARD AS DETAILED ON THIS SHEET. THE LENGTH OF THE OUTLET PIPE ON THE NORTH SIDE SHALL BE DETERMINED BY THE REVETMENT AND ITS PLACEMENT LOCATION. THE CONTRACTOR IS TO INSURE THE OUTLET PIPE IS ADEQUATELY STRONG ENOUGH AND WILL NOT BE DAMAGED WHEN REVETMENT IS PLACED. A CHECK WILL BE MADE AT THE SUBDRAIN OUTLET TO INSURE THAT THE SUBDRAIN IS NOT DAMAGED AND IS DRAINING PROPERLY DURING THE BACKFILL FLOODING PROCESS. THE GALVANIZED CORRUGATED METAL OUTLET PIPE USED, SHALL BE 6 INCHES IN DIAMETER AND COUPLED TO THE 4 INCH DIAMETER SUBDRAIN IN ONE OF THE TWO FOLLOWING WAYS.  
 1. USE AN INSIDE FIT REDUCER COUPLER (COUPLER MUST BE INSERTED A MINIMUM OF 1'-0 INTO THE METAL OUTLET PIPE).  
 2. INSERT 1'-0 OF THE 4" SUBDRAIN INTO THE 6" METAL OUTLET PIPE, THEN FULLY SEAL THE ENTIRE OPENING WITH GROUT.  
 THE COST OF FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), GRANULAR BACKFILL, POROUS BACKFILL, AND SUBDRAIN OUTLET AND PIPE CLIP IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)". NO EXTRA PAYMENT WILL BE MADE.  
 THE DIMENSIONS SHOWN FOR THE PROPOSED SUBDRAINS ARE BASED ON THE PROPOSED GRADING LAYOUT OF BRIDGE BERMS. THE DIMENSIONS SHOWN ARE FOR ESTIMATING ONLY. REQUIRED LENGTHS AND GENERAL LOCATIONS OF SUBDRAINS ARE SUBJECT TO CHANGE DUE TO FIELD ADJUSTMENTS OF THE GRADING LAYOUT.

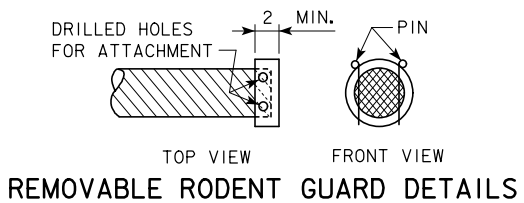
SUBDRAIN OUTLET ELEVATIONS	
LOCATION	ELEVATION
WEST ABUTMENT (NORTH SIDE)	562.5
WEST ABUTMENT (SOUTH SIDE)	561.0
EAST ABUTMENT (NORTH SIDE)	563.0
EAST ABUTMENT (SOUTH SIDE)	562.0



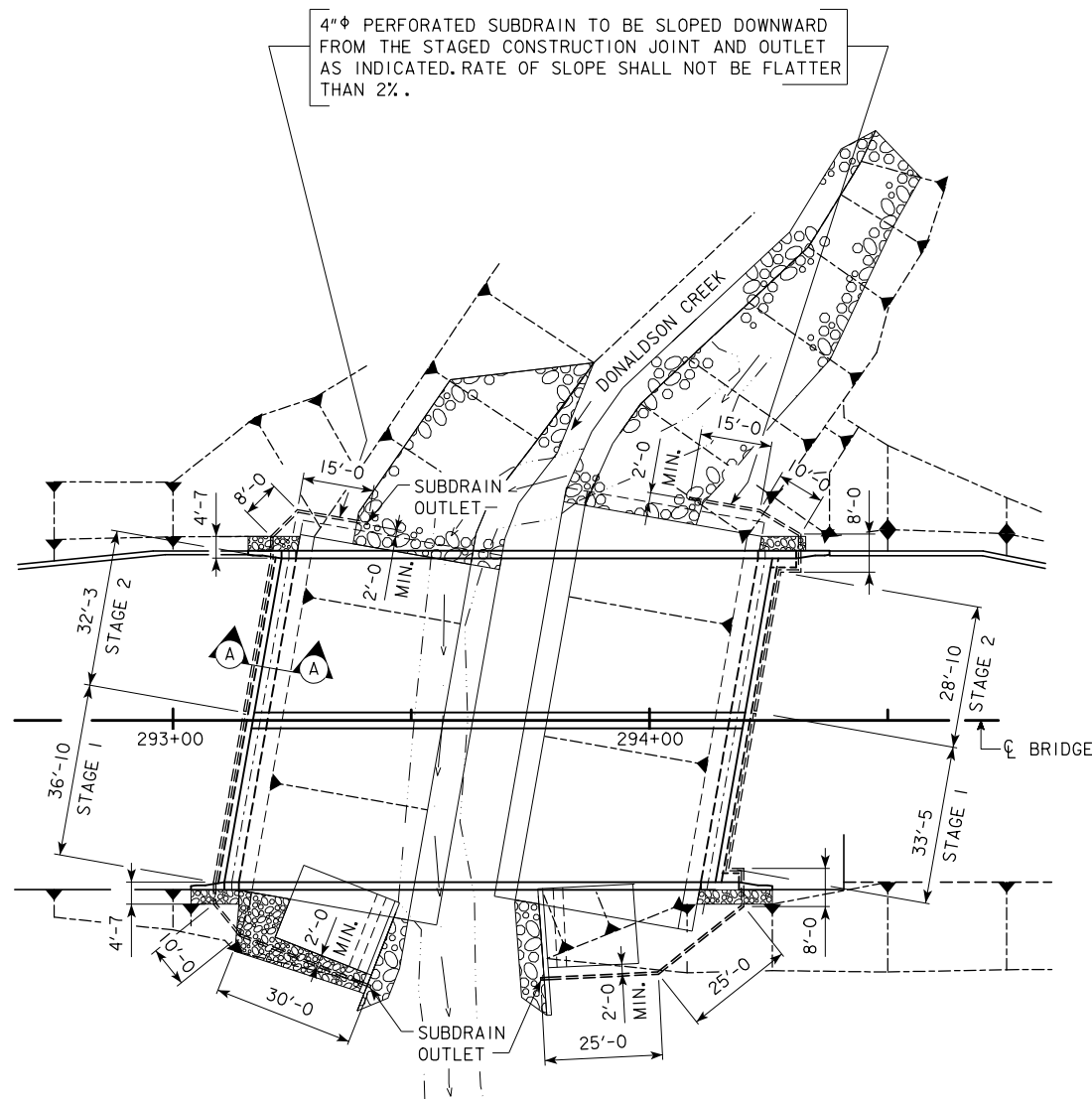
**SUBDRAIN OUTLET CONNECTION TO TOP OF EXISTING MODIFIED WINGWALLS.**



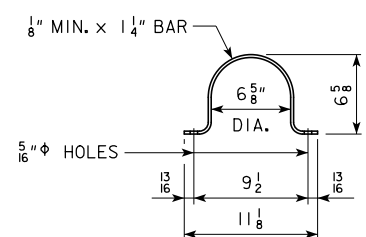
**SOUTH SIDE SECTION OF SUBDRAIN OUTLET**



**REMOVABLE RODENT GUARD DETAILS**



**SITUATION PLAN**  
SHOWING SUBDRAIN LOCATIONS



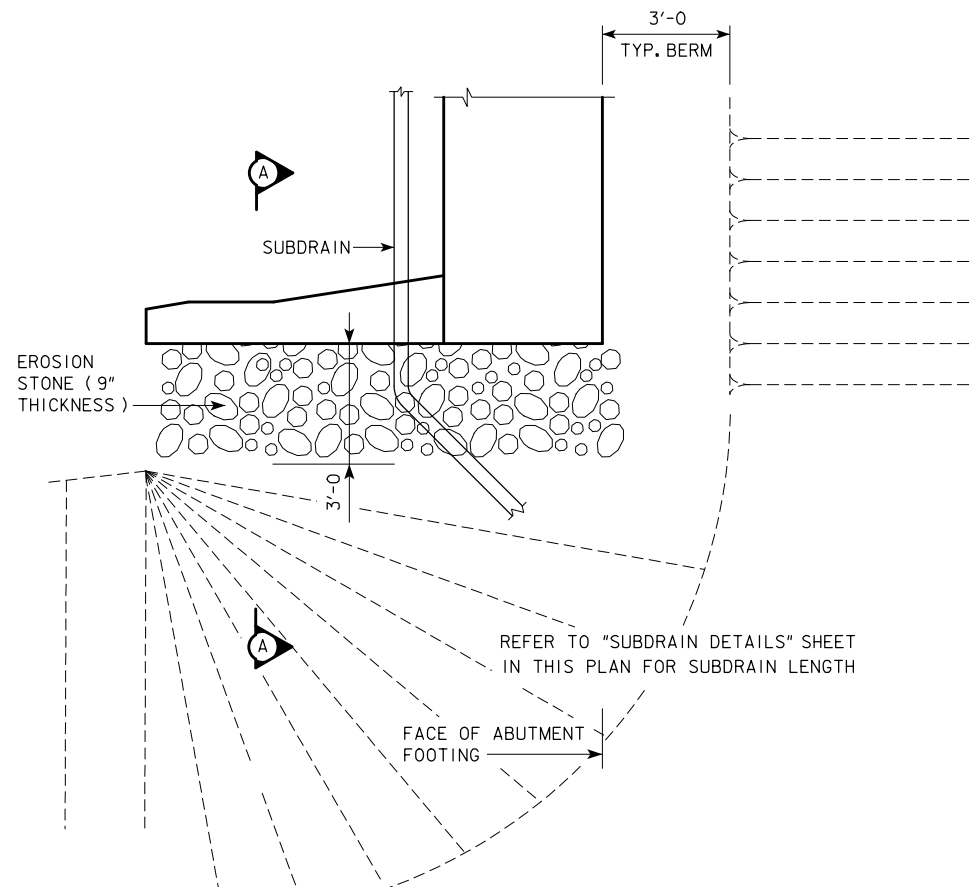
**PIPE CLIP DETAIL**  
ANY COMMERCIALY AVAILABLE GALVANIZED CARBON STEEL PIPE CLIP FOR 6" DIAMETER PIPES OF SIMILAR DESIGN MAY BE USED.

NOTE:  
SECTION A - A IS SHOWN ON ABUTMENT BACKFILL DETAILS SHEET.

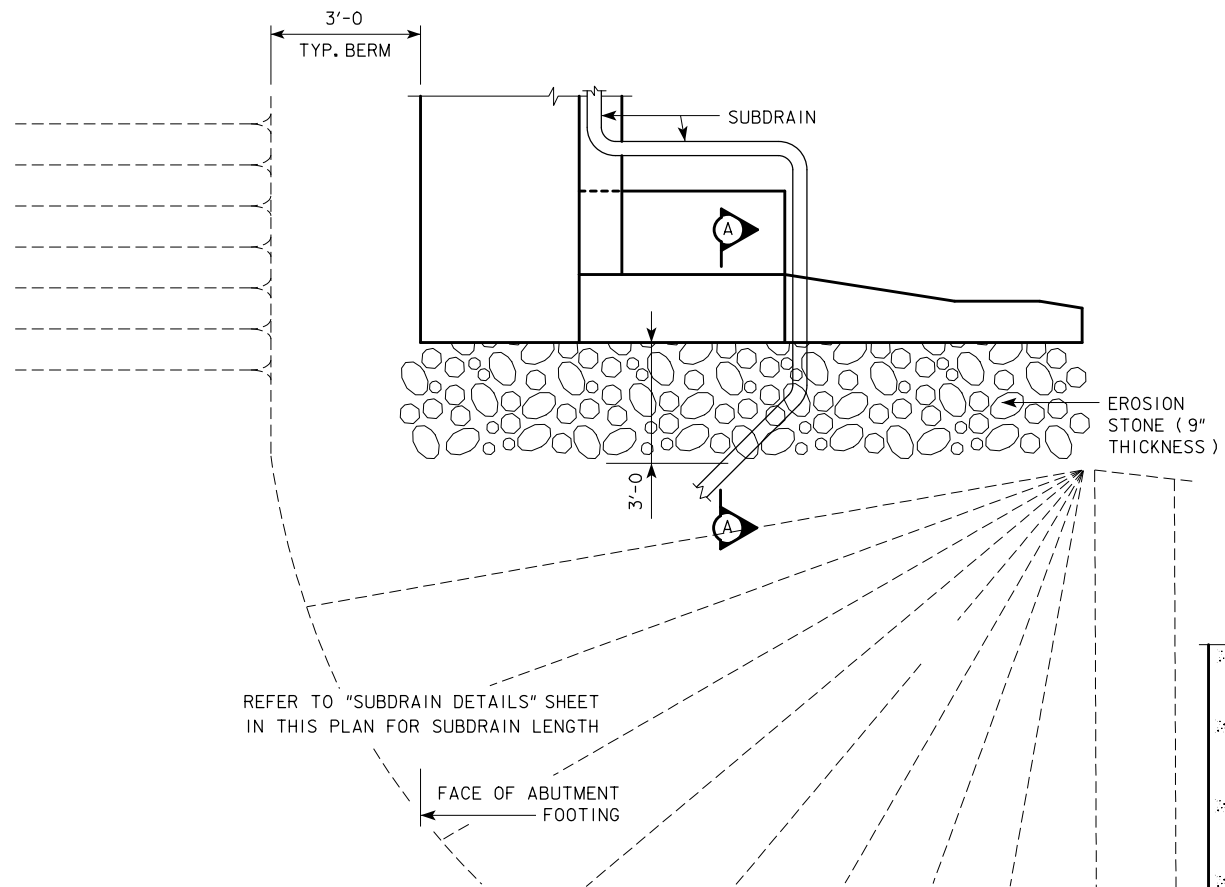
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 x 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**SUBDRAIN DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 25 OF 29 FILE NO. 30687 DESIGN NO. 514

REVISED 02-12 - THE 3'-0 TOP OF THE BERM SLOPE PROTECTION WAS CHANGED TO EROSION STONE FOR ALL SLOPE PROTECTION CONDITIONS. ENGLISH FORESLOPE PROTECTION BRIDGES.DGN 1007C - THIS SHEET ISSUED 06-02 FOR WATER CROSSINGS.

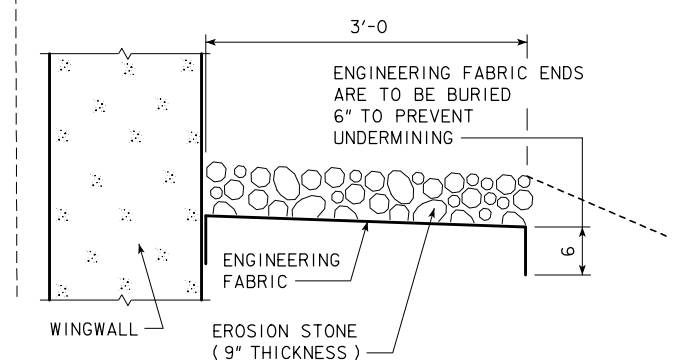
REVISED 07-11 - THE LABELING OF THE BERM SLOPE IS IDENTIFIED AS 'GRADING SURFACE'. MACADAM STONE WAS CHANGED TO EROSION STONE. ENGLISHFORPROTECTIONBRIDGES.DGN 1005A - THIS SHEET ISSUED 06-02.



TOP VIEW OF WING ARMORING AT WEST ABUTMENT



TOP VIEW OF WING ARMORING AT EAST ABUTMENT



SECTION A-A

A CHECK SHALL BE MADE AT THE SUBDRAIN OUTLET TO INSURE THAT IT IS DRAINING PROPERLY DURING THE BACKFILL FLOODING PROCESS.

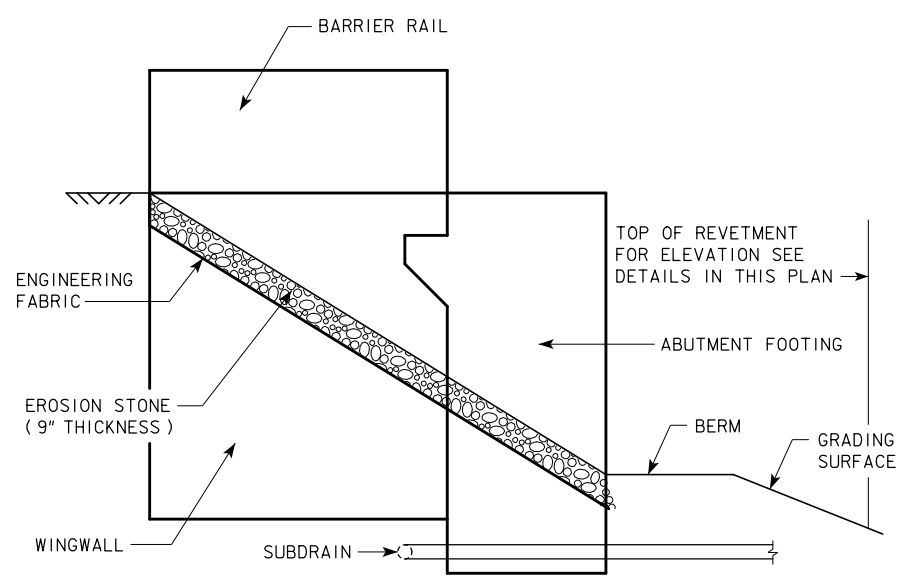
**GENERAL NOTES:**

EROSION STONE SHALL BE PLACED ALONG THE SIDES OF THE WINGS AND ABUTMENT FOOTING AS SHOWN IN SECTION A-A. THIS IS TYPICAL AT EACH CORNER OF THE BRIDGE UNLESS OTHERWISE NOTED IN THE PLANS. THE EROSION STONE AT THESE LOCATIONS SHALL BE UNDERLAYED WITH ENGINEERING FABRIC IN ACCORDANCE WITH ARTICLE 4196.01, B, 3, OF THE STANDARD SPECIFICATIONS.

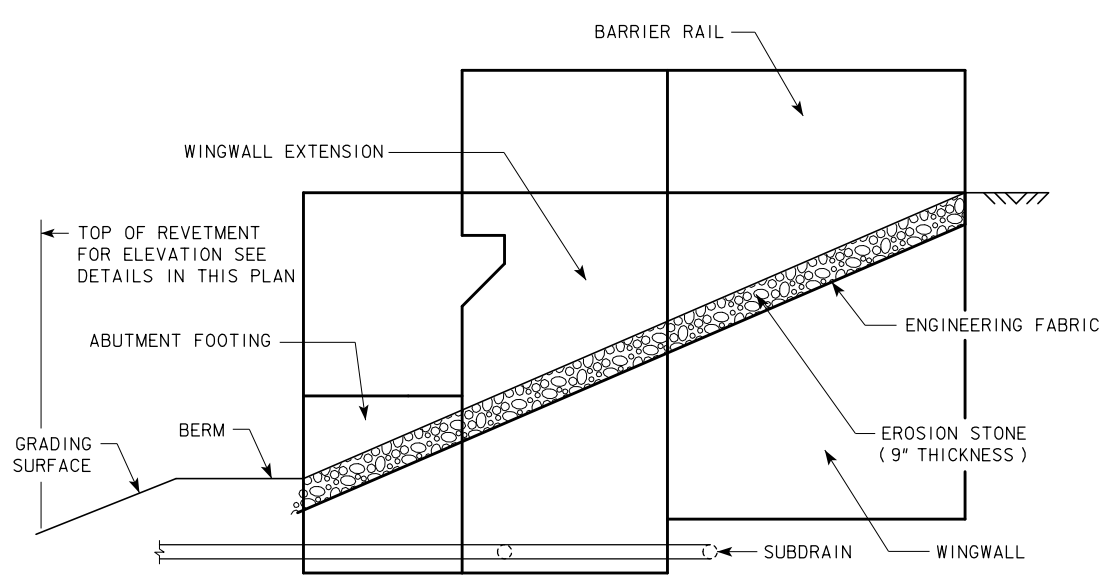
THE EROSION STONE SHALL BE IN ACCORDANCE WITH SECTION 4130, OF THE STANDARD SPECIFICATIONS. MATERIAL PASSING THE 3 INCH SCREEN BUT 100% RETAINED ON A 1 INCH SCREEN MAY BE USED AS CHOKE STONE.

THE EROSION STONE SHALL BE DEPOSITED, SPREAD, CONSOLIDATED AND SHAPED BY MECHANICAL OR HAND METHODS THAT WILL PROVIDE UNIFORM 9" DEPTH AND DENSITY AND PROVIDE UNIFORM SURFACE APPEARANCE.

PAYMENT FOR THE BRIDGE WING ARMORING WILL BE BID PER SQUARE YARD. COST WILL INCLUDE ENGINEERING FABRIC, EROSION STONE, EXCAVATION, SHAPING, AND COMPACTION TO DIMENSIONS SHOWN IN THESE PLANS. BID ITEM SHALL BE "BRIDGE WING ARMORING - EROSION STONE".



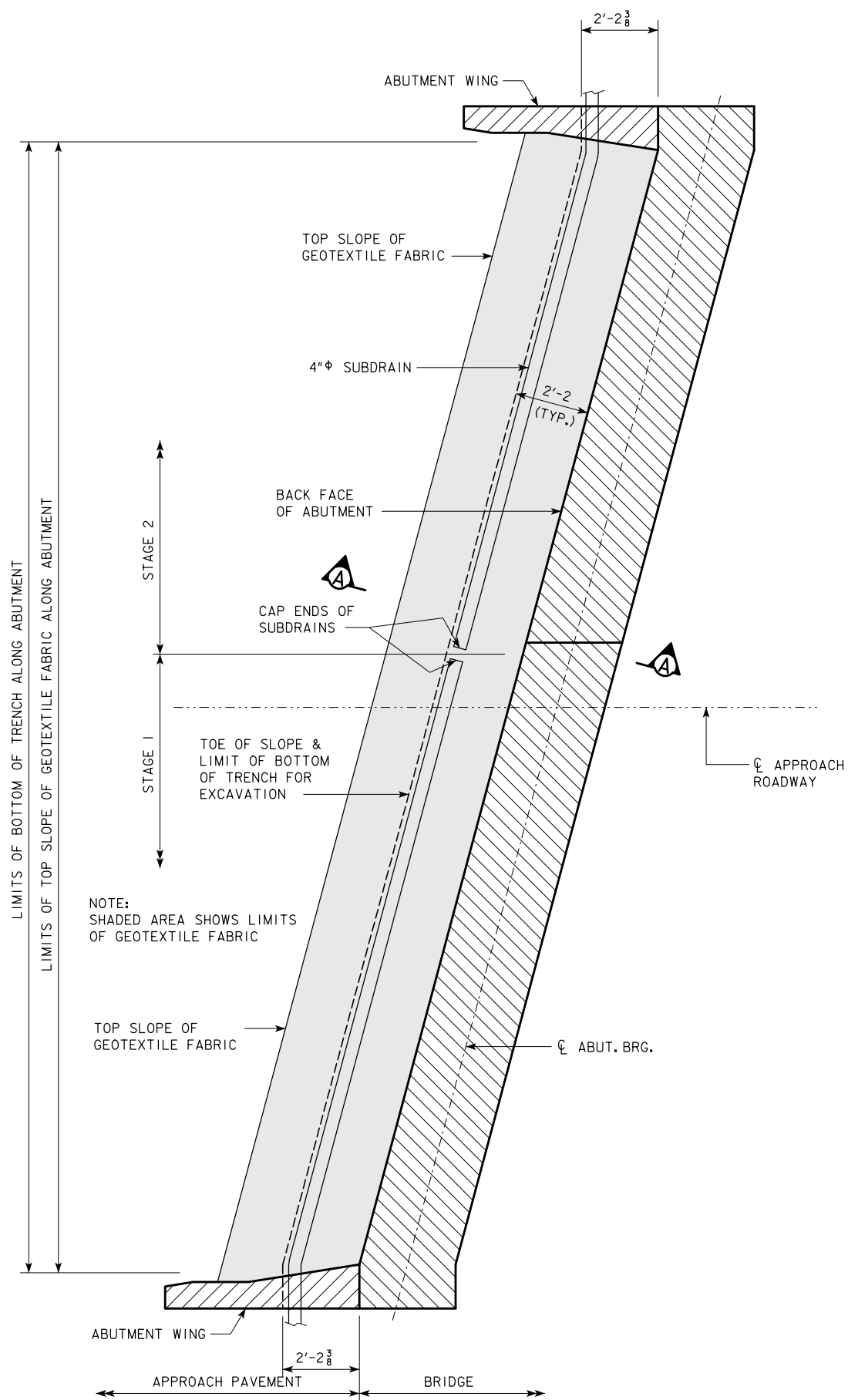
PROFILE VIEW OF WING ARMORING  
(SHOWN FOR WEST ABUTMENT)



PROFILE VIEW OF WING ARMORING  
(SHOWN FOR EAST ABUTMENT)

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**  
**BRIDGE WING ARMORING**  
 STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 26 OF 29    FILE NO. 30687    DESIGN NO. 514

REVISED 04-12 - EXCAVATION LIMIT ON THE FRONT FACE OF THE ABUTMENT WAS CHANGED TO A 3'-0" LIMIT. THE APPROACH FILL WAS IDENTIFIED AS THE GRADING SURFACE. ENGLISH\FOR\PROTECTION\BRIDGES.DGN - 1007D - THIS SHEET ISSUED 08-07.



**WEST ABUTMENT PLAN WITHOUT WING EXTENSIONS**

**TECHNICAL DATA INFORMATION - GEOTEXTILE FABRIC**

MECHANICAL PROPERTIES	TEST METHOD	UNIT	MINIMUM AVERAGE ROLL VALUE	
			MD	CD
TENSILE STRENGTH (AT 5% STRAIN)	ASTM D 4595	kN/m (LBS/FT)	19.8 (1356)	19.8 (1356)
APPARENT OPENING SIZE (AOS)	ASTM D 4751	mm (U.S. SIEVE)	0.43 MAX (#40)	
FLOW RATE	ASTM D 4491	L/MIN/m <sup>2</sup> (GAL/MIN/FT <sup>2</sup> )	733 (18)	
UV RESISTANCE (AT 500 HOURS)	ASTM D 4355	% STRENGTH RETAINED	70	

**ABUTMENT BACKFILL PROCESS:**

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

AFTER THE SUBGRADE HAS BEEN SHAPED, THE GEOTEXTILE FABRIC SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS SHOWN. THE FABRIC IS INTENDED TO BE INSTALLED IN THE BASE OF THE EXCAVATION AND EXTENDED VERTICALLY UP THE ABUTMENT BACKWALL, ABUTMENT WING WALLS, AND EXCAVATION FACE TO A HEIGHT THAT WILL BE APPROXIMATELY 1 TO 2 FOOT HIGHER THAN THE HEIGHT OF THE POROUS BACKFILL PLACEMENT AS SHOWN IN THE "BACKFILL DETAILS" ON THIS SHEET. THE STRIPS OF THE FABRIC PLACED SHALL OVERLAP APPROXIMATELY 1 FOOT AND SHALL BE PINNED IN PLACE. THE FABRIC SHALL BE ATTACHED TO THE ABUTMENT BY USING LATH FOLDED IN THE FABRIC AND SECURED TO THE CONCRETE WITH SHALLOW CONCRETE NAILS. THE FABRIC PLACED AGAINST THE EXCAVATION FACE SHALL BE PINNED.

WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN SHALL BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO BE CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH FLOODABLE BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE FLOODABLE BACKFILL MATERIAL SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. THE FLOODABLE BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

START SURFACE FLOODING FOR EACH FLOODABLE BACKFILL LIFT AT THE HIGH POINT OF THE SUBDRAIN AND PROGRESS TO THE LOW POINT WHERE THE SUBDRAIN EXITS THE FABRIC. TO ENSURE UNIFORM SURFACE FLOODING, WATER RUNNING FULL IN A 2-INCH DIAMETER HOSE SHOULD BE SPRAYED IN SUCCESSIVE 6-FOOT TO 8-FOOT INCREMENTS FOR 3 MINUTES WITHIN EACH INCREMENT.

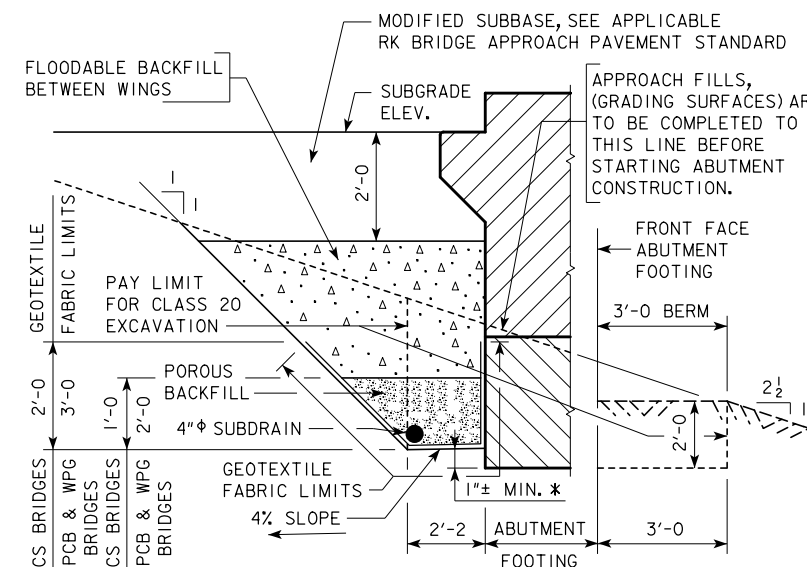
FLOODABLE BACKFILL LIFT PLACEMENT, FLOODING, AND COMPACTION SHALL PROGRESS UNTIL THE REQUIRED FULL THICKNESS OF THE ABUTMENT BACKFILL HAS BEEN COMPLETED.

WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, FLOODABLE BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

THE COST OF WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, FLOODABLE BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR STRUCTURAL CONCRETE.

NOTE:

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM STAGED CONSTRUCTION JOINT WHEN OUTLETTING BOTH SIDES OF THE ABUTMENT.



**SECTION A-A  
BACKFILL DETAILS**

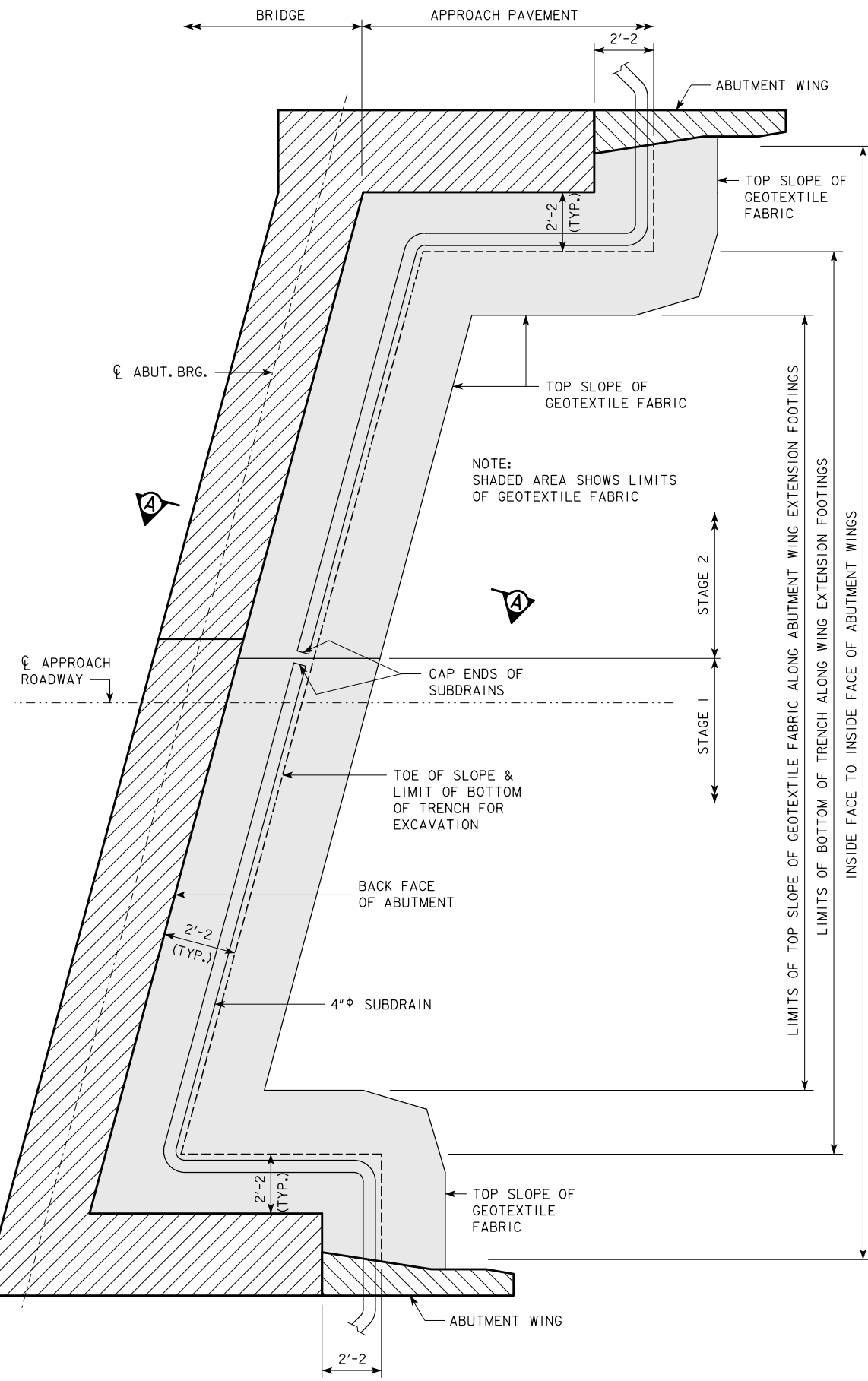
NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

\* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

NOTE:  
SEE SUBDRAIN DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**WEST ABUTMENT BACKFILL DETAILS**  
 STATION: 293+68.30 DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 27 OF 29 FILE NO. 30687 DESIGN NO. 514

REVISED 04-12 - EXCAVATION LIMIT ON THE FRONT FACE OF THE ABUTMENT WAS CHANGED TO A 3'-0" LIMIT. THE APPROACH FILL WAS IDENTIFIED AS THE GRADING SURFACE. ENGLISH\FOR\PROTECTION\BRIDGES.DGN - 1007E - THIS SHEET ISSUED 08-07.



**EAST ABUTMENT PLAN WITH WING EXTENSIONS**

**TECHNICAL DATA INFORMATION - GEOTEXTILE FABRIC**

MECHANICAL PROPERTIES	TEST METHOD	UNIT	MINIMUM AVERAGE ROLL VALUE	
			MD	CD
TENSILE STRENGTH (AT 5% STRAIN)	ASTM D 4595	kN/m (LBS/FT)	19.8 (1356)	19.8 (1356)
APPARENT OPENING SIZE (AOS)	ASTM D 4751	mm (U.S. SIEVE)	0.43 MAX (#40)	
FLOW RATE	ASTM D 4491	L/MIN/m <sup>2</sup> (GAL/MIN/FT <sup>2</sup> )	733 (18)	
UV RESISTANCE (AT 500 HOURS)	ASTM D 4355	% STRENGTH RETAINED	70	

**ABUTMENT BACKFILL PROCESS:**

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WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN SHALL BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO BE CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH FLOODABLE BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE FLOODABLE BACKFILL MATERIAL SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. THE FLOODABLE BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

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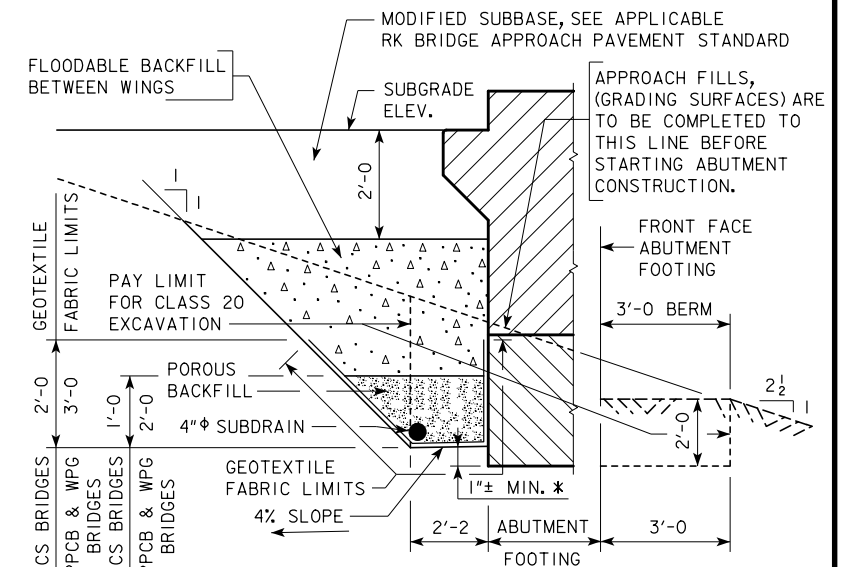
FLOODABLE BACKFILL LIFT PLACEMENT, FLOODING, AND COMPACTION SHALL PROGRESS UNTIL THE REQUIRED FULL THICKNESS OF THE ABUTMENT BACKFILL HAS BEEN COMPLETED.

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THE COST OF WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, FLOODABLE BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR STRUCTURAL CONCRETE.

NOTE:

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM STAGED CONSTRUCTION JOINT WHEN OUTLETTING BOTH SIDES OF THE ABUTMENT.



**SECTION A-A  
BACKFILL DETAILS**

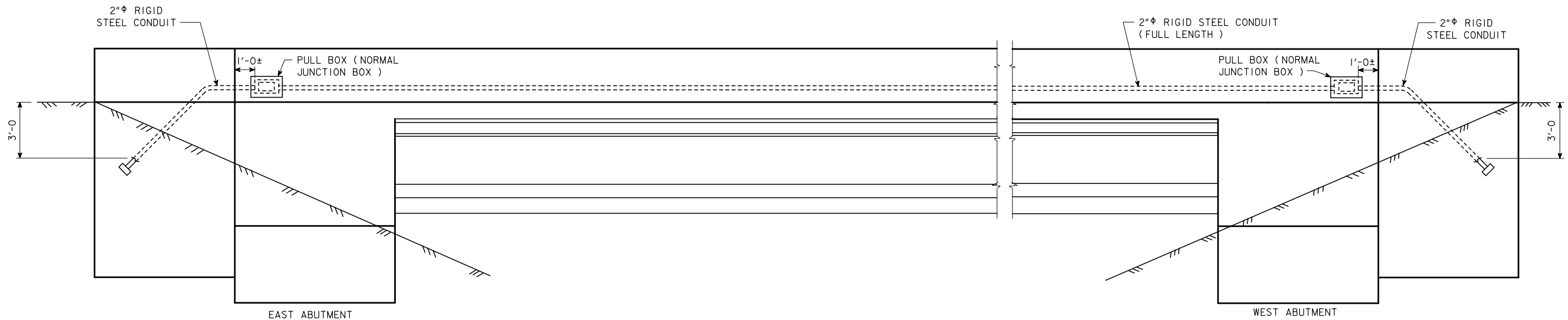
NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

\* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

NOTE:  
SEE SUBDRAIN DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**EAST ABUTMENT BACKFILL DETAILS**  
 STATION: 293+68.30  
 SCOTT COUNTY  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 28 OF 29 FILE NO. 30687 DESIGN NO. 514

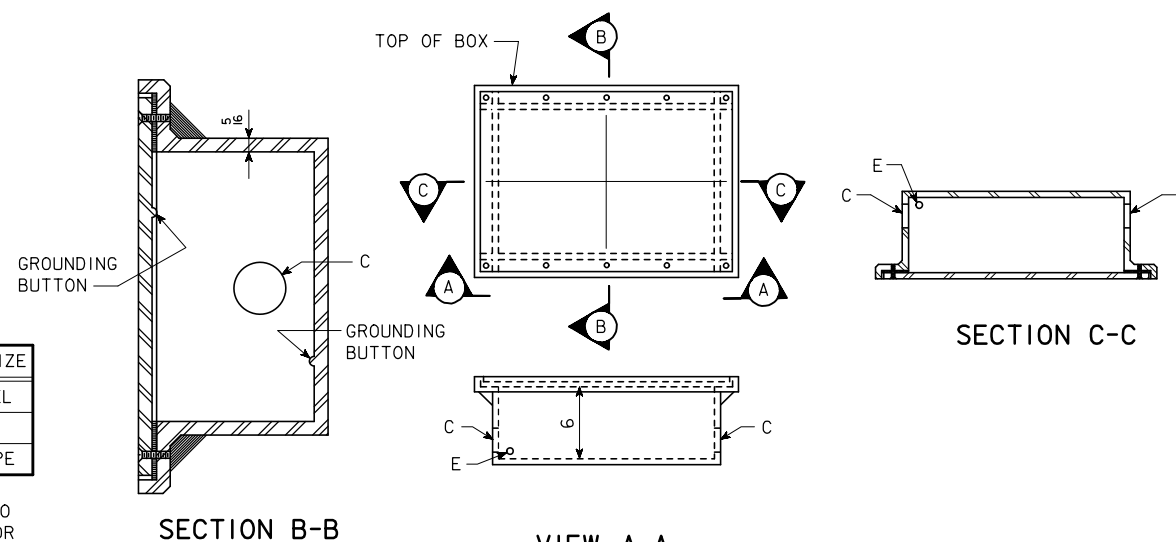
REVISION 05-11 - ADDED THE WORD 'MINIMUM' TO THE 3 1/2 INCH DIMENSION FOR THE LOCATION OF THE 2 INCH CONDUIT IN THE BARRIER RAIL. ENGLISHDECKRAILBRIDGES.DGN 1030AS2 - THIS SHEET ISSUED 09-03.



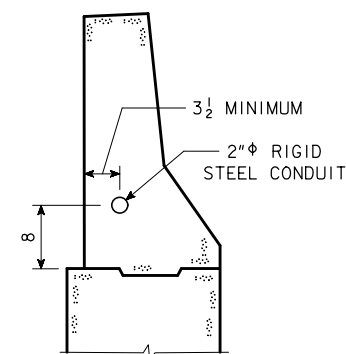
EXTERIOR ELEVATION - NORTH BARRIER RAIL - LOOKING SOUTH

BOSS FOR	HOLE	FOR CONDUIT SIZE
5 THREADS	C	2" $\phi$ RIGID STEEL
NONE	E	1/2" $\phi$ COPPER PIPE

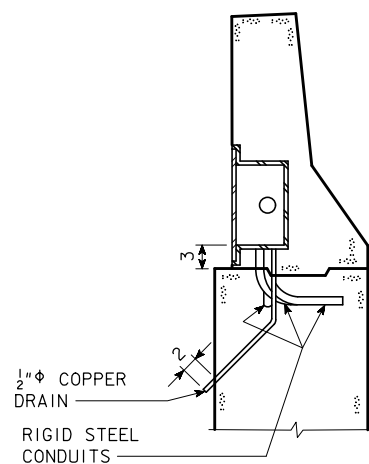
NOTE:  
THE GROUNDING BUTTONS ARE TO BE BLIND DRILLED AND TAPPED FOR 3/8"  $\phi$  x 0'-0 3/4" BOLTS.



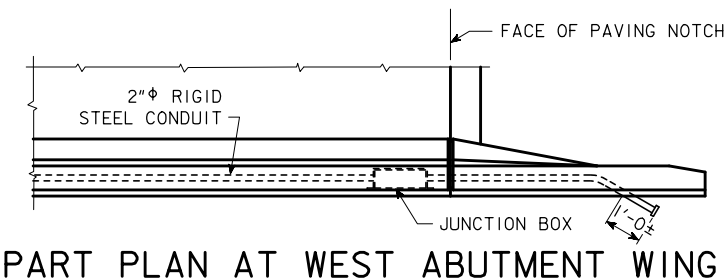
RM-37, TYPE I JUNCTION BOX  
WATERTIGHT, CAST IRON - FLUSH MOUNT



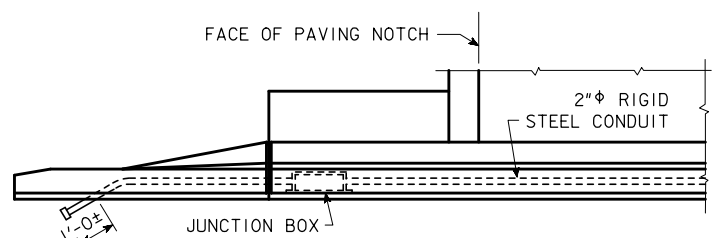
SECTION THRU BARRIER RAIL



SECTION THRU JUNCTION BOX



PART PLAN AT WEST ABUTMENT WING



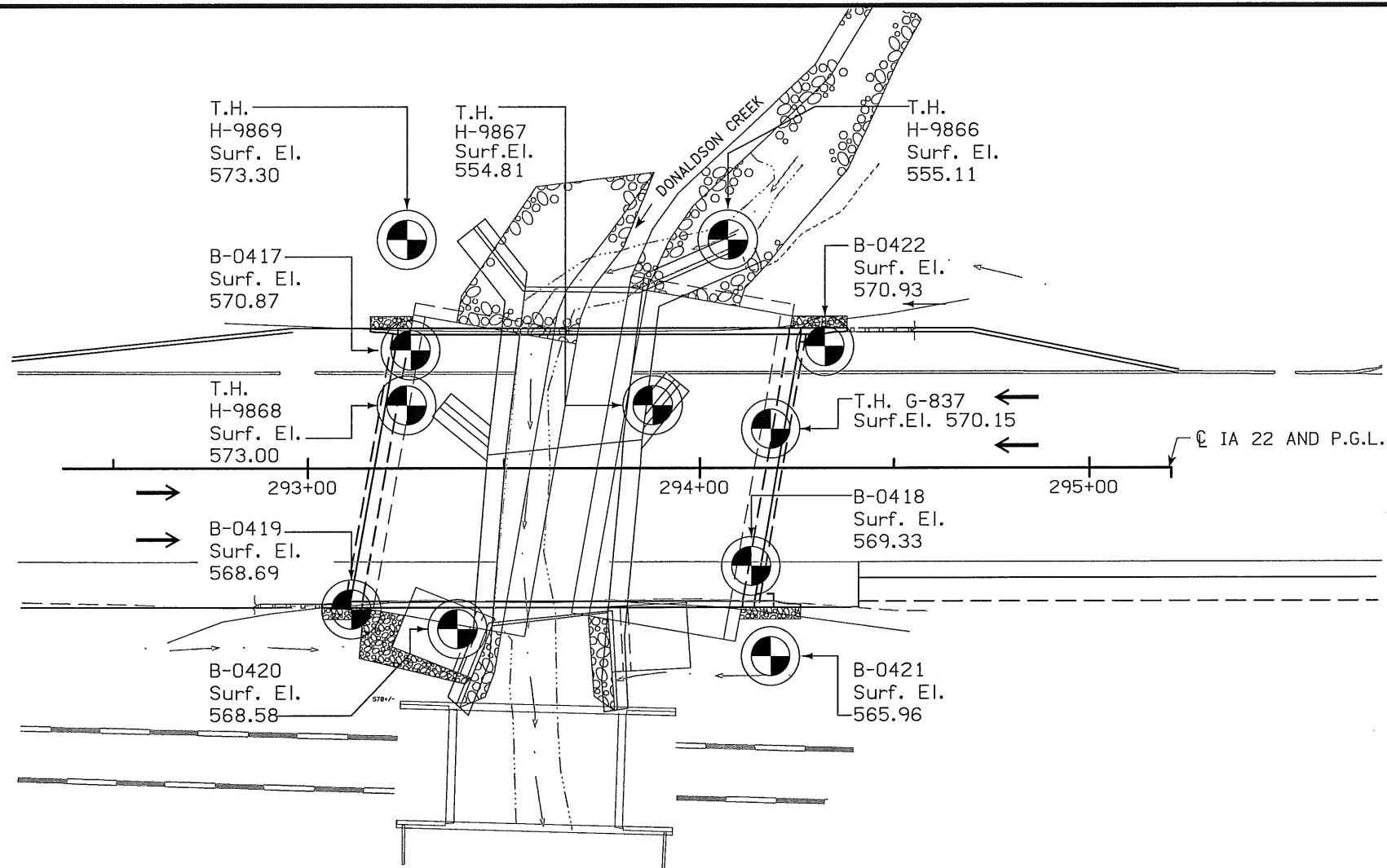
PART PLAN AT EAST ABUTMENT WING

DESIGN FOR 10° SKEW (L.A.)  
**100'-0" x 68'-0" PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
**LIGHTING DETAILS**  
 STATION: 293+68.30      DECEMBER, 2012  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 29 OF 29      FILE NO. 30687      DESIGN NO. 514

THIS SHEET IS INCLUDED TO SHOW SOIL INFORMATION. DETAILS AND NOTES SHOWN ELSEWHERE IN THESE PLANS SHALL BE USED FOR STRUCTURE CONSTRUCTION.


**LOCATION**

IA 22 OVER DONALDSON CREEK  
 T-77N R-2E  
 SECTION 24  
 BUFFALO TOWNSHIP  
 SCOTT COUNTY  
 BRIDGE MAINT. NO. 8292.8S022  
 LATITUDE 41.464382°  
 LONGITUDE -90.681313°



**GEOTECHNICAL DESIGN**

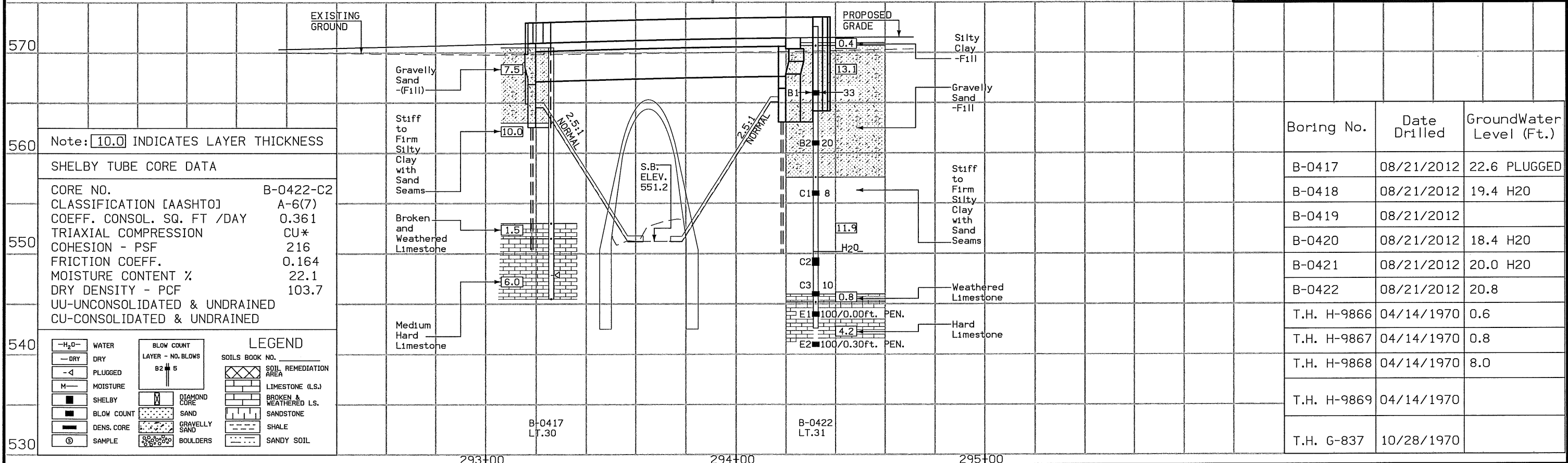
I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

  
 Signature: *Robert Stanley* 2-13-13  
 Printed or Typed Name: **Robert L. Stanley**  
 My license renewal date is December 31, 2014.

Pages or sheets covered by this seal: SPS.1, SPS.2, SPS.3, and SPS.4

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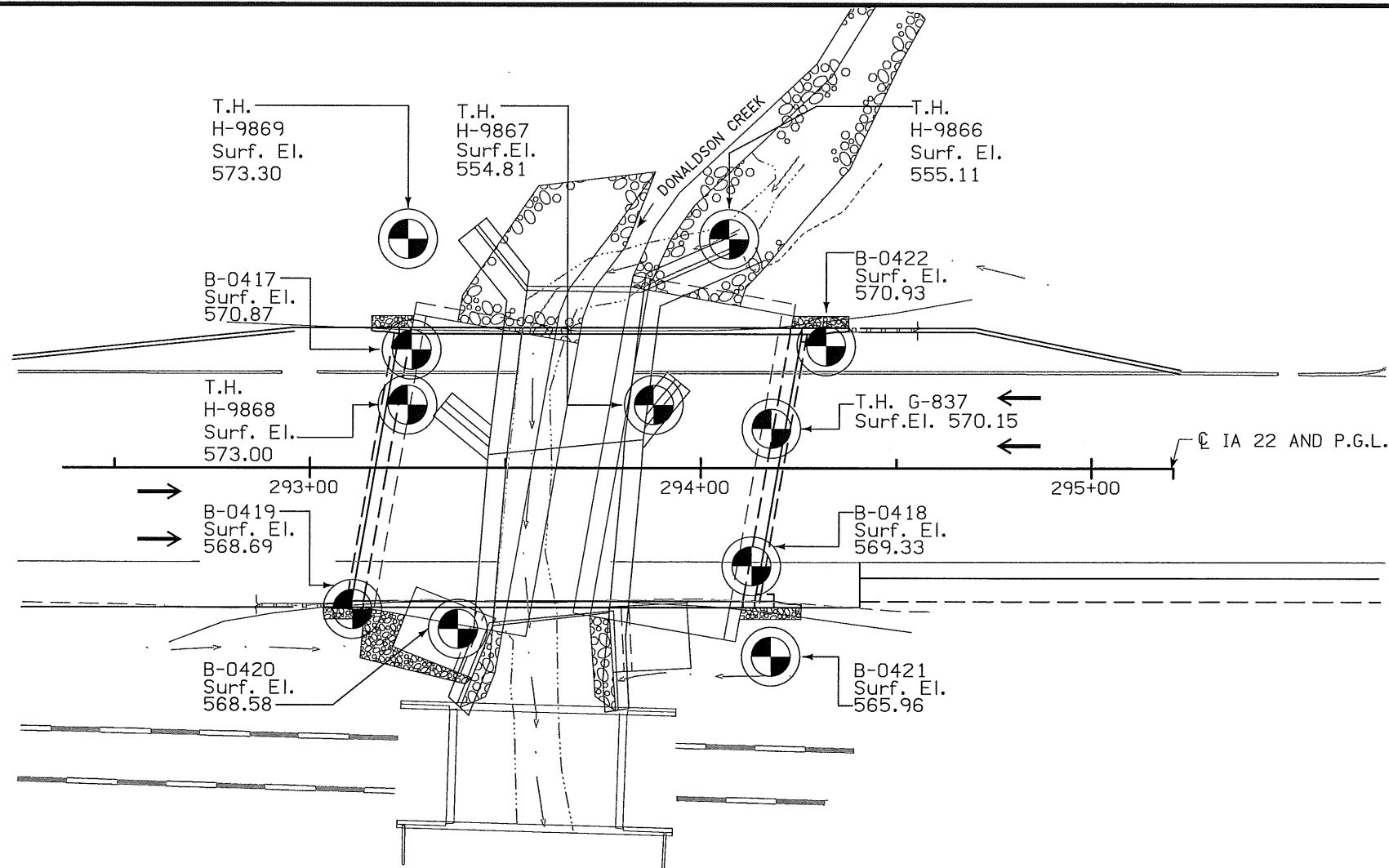
DESIGN FOR 10° SKEW (L.A.)  
**100'-0 X 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0 SPAN (BTB BEAMS)  
**SOIL PROFILE SHEET**  
 STATION 293+68.30 IA 22  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 1 OF 4 FILE NO. 30687 DESIGN NO. 514



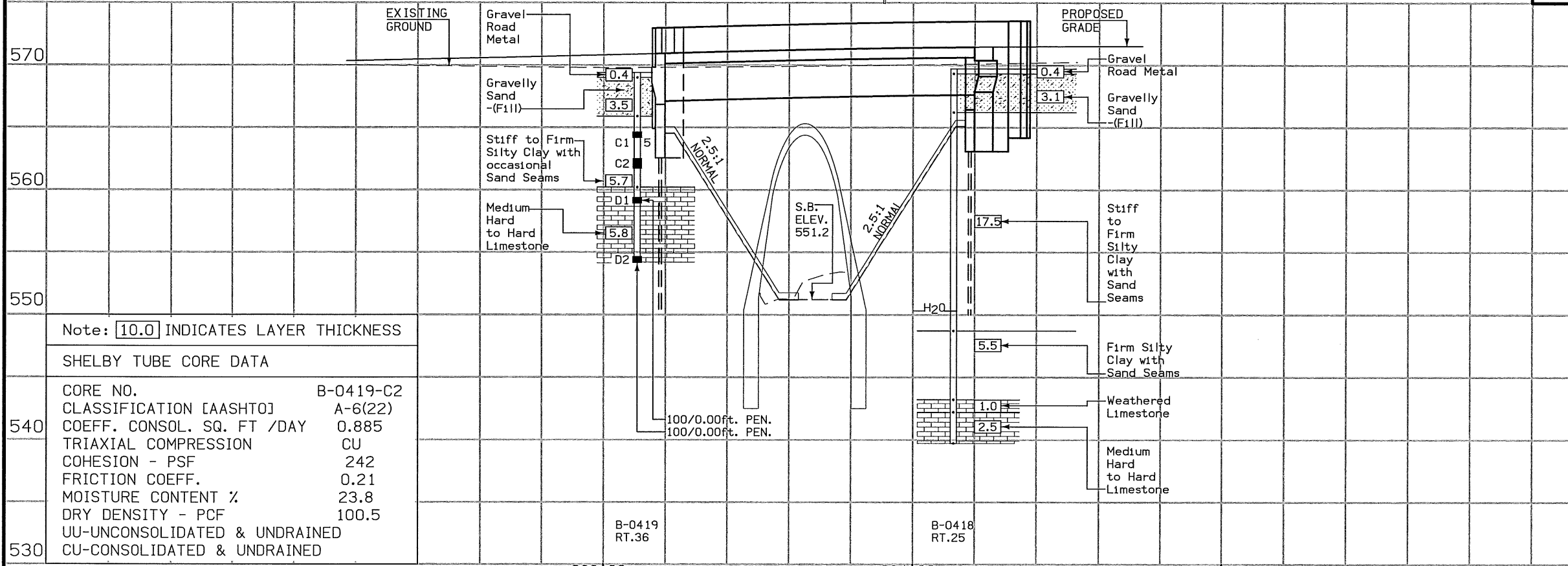
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**LOCATION**

IA 22 OVER DONALDSON CREEK  
T-77N R-2E  
SECTION 24  
BUFFALO TOWNSHIP  
SCOTT COUNTY  
BRIDGE MAINT. NO. 8292.8S022  
LATITUDE 41.464382°  
LONGITUDE -90.681313°



DESIGN FOR 10° SKEW (L.A.)  
**100'-0 X 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
(BTB BEAMS)  
**SOIL PROFILE SHEET**  
STATION 293+68.30 IA 22  
**SCOTT COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO. 2 OF 4 FILE NO. 30687 DESIGN NO. 514



Boring No.	Date Drilled	GroundWater Level (Ft.)
B-0417	08/21/2012	22.6 PLUGGED
B-0418	08/21/2012	19.4 H2O
B-0419	08/21/2012	
B-0420	08/21/2012	18.4 H2O
B-0421	08/21/2012	20.0 H2O
B-0422	08/21/2012	20.8
T.H. H-9866	04/14/1970	0.6
T.H. H-9867	04/14/1970	0.8
T.H. H-9868	04/14/1970	8.0
T.H. H-9869	04/14/1970	
T.H. G-837	10/28/1970	

Note: [10.0] INDICATES LAYER THICKNESS

**SHELBY TUBE CORE DATA**

CORE NO.	B-0419-C2
CLASSIFICATION [AASHTO]	A-6(22)
COEFF. CONSOL. SQ. FT /DAY	0.885
TRIAxIAL COMPRESSION	CU
COHESION - PSF	242
FRICTION COEFF.	0.21
MOISTURE CONTENT %	23.8
DRY DENSITY - PCF	100.5
UU-UNCONSOLIDATED & UNDRAINED	
CU-CONSOLIDATED & UNDRAINED	

**LEGEND**

SOILS BOOK NO. \_\_\_\_\_

SOIL REMEDIATION AREA

LIMESTONE (L.S.)

BROKEN & WEATHERED L.S.

SANDSTONE

SHALE

SANDY SOIL

WATER

DRY

PLUGGED

MOISTURE

SHELBY

BLOW COUNT

DENS. CORE

SAMPLE

BLOW COUNT

LAYER - NO. BLOWS

DIAMOND CORE

SAND

GRAVELLY SAND

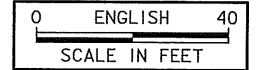
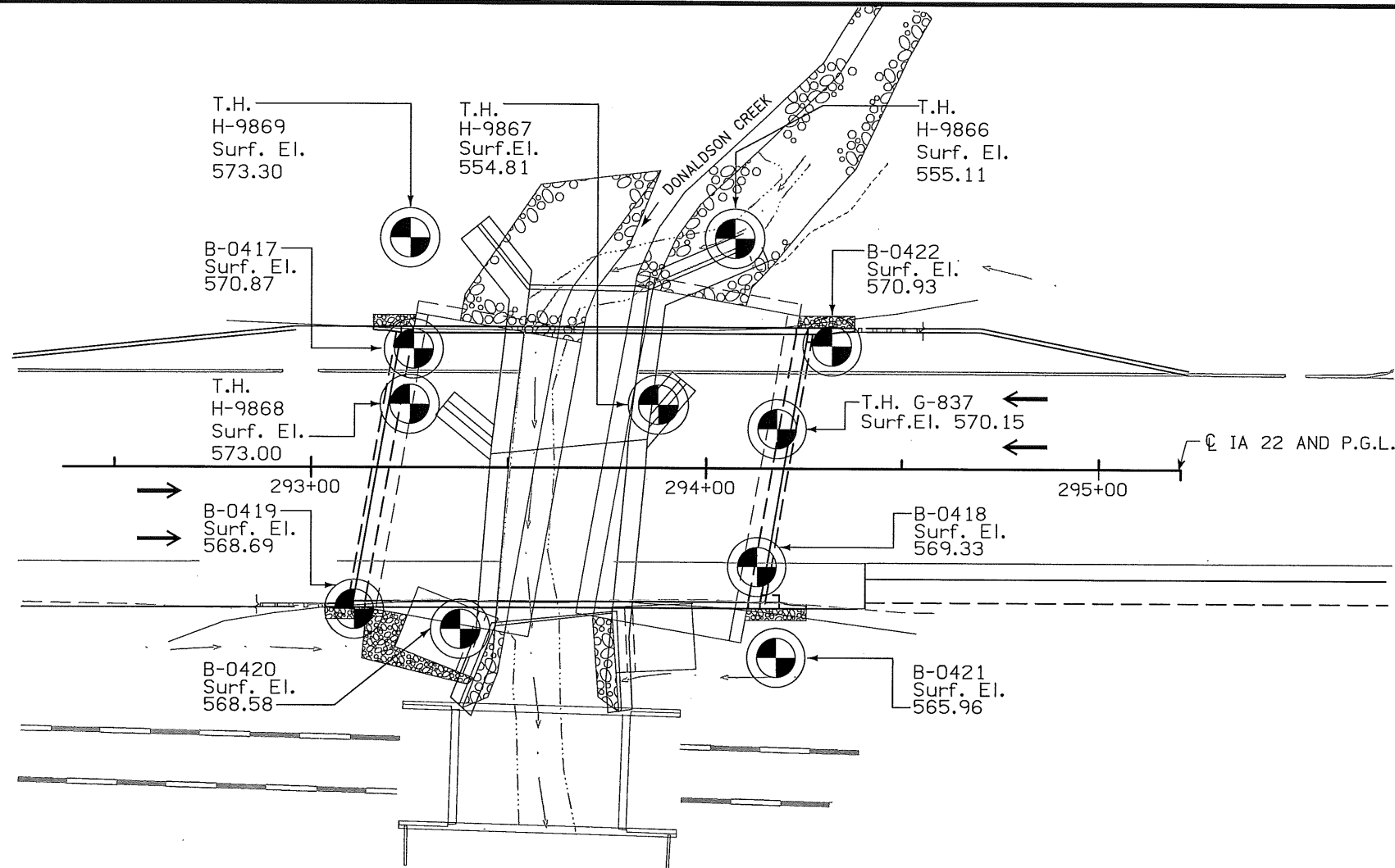
BOULDERS



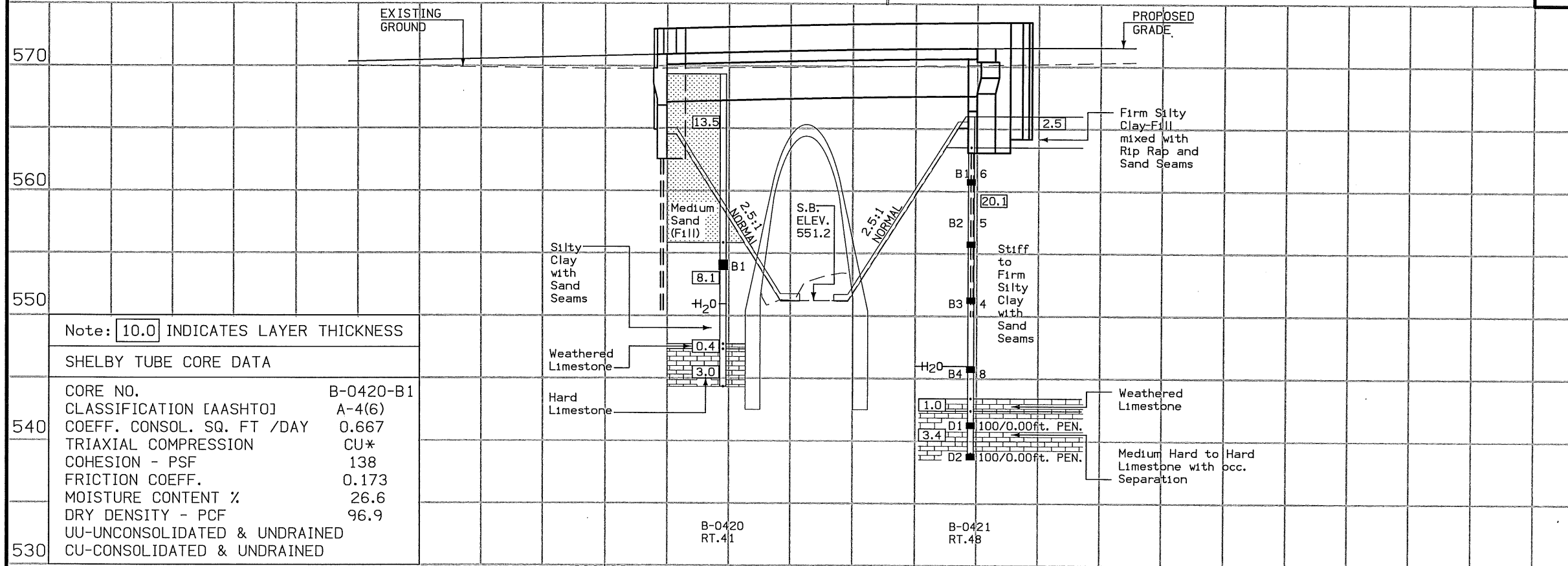
THIS SHEET IS INCLUDED TO SHOW SOIL INFORMATION. DETAILS AND NOTES SHOWN ELSEWHERE IN THESE PLANS SHALL BE USED FOR STRUCTURE CONSTRUCTION.

**LOCATION**

IA 22 OVER DONALDSON CREEK  
 T-77N R-2E  
 SECTION 24  
 BUFFALO TOWNSHIP  
 SCOTT COUNTY  
 BRIDGE MAINT. NO. 8292.85022  
 LATITUDE 41.464382°  
 LONGITUDE -90.681313°



DESIGN FOR 10° SKEW (L.A.)  
**100'-0 X 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 (BTB BEAMS)  
**SOIL PROFILE SHEET**  
 STATION 293+68.30 IA 22  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 3 OF 4 FILE NO. 30687 DESIGN NO. 514



Boring No.	Date Drilled	GroundWater Level (Ft.)
B-0417	08/21/2012	22.6 PLUGGED
B-0418	08/21/2012	19.4 H2O
B-0419	08/21/2012	
B-0420	08/21/2012	18.4 H2O
B-0421	08/21/2012	20.0 H2O
B-0422	08/21/2012	20.8
T.H. H-9866	04/14/1970	0.6
T.H. H-9867	04/14/1970	0.8
T.H. H-9868	04/14/1970	8.0
T.H. H-9869	04/14/1970	
T.H. G-837	10/28/1970	

Note: 10.0 INDICATES LAYER THICKNESS

SHELBY TUBE CORE DATA	
CORE NO.	B-0420-B1
CLASSIFICATION [AASHTO]	A-4(6)
COEFF. CONSOL. SQ. FT /DAY	0.667
TRIAXIAL COMPRESSION	CU*
COHESION - PSF	138
FRICTION COEFF.	0.173
MOISTURE CONTENT %	26.6
DRY DENSITY - PCF	96.9
UU-UNCONSOLIDATED & UNDRAINED	
CU-CONSOLIDATED & UNDRAINED	

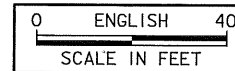
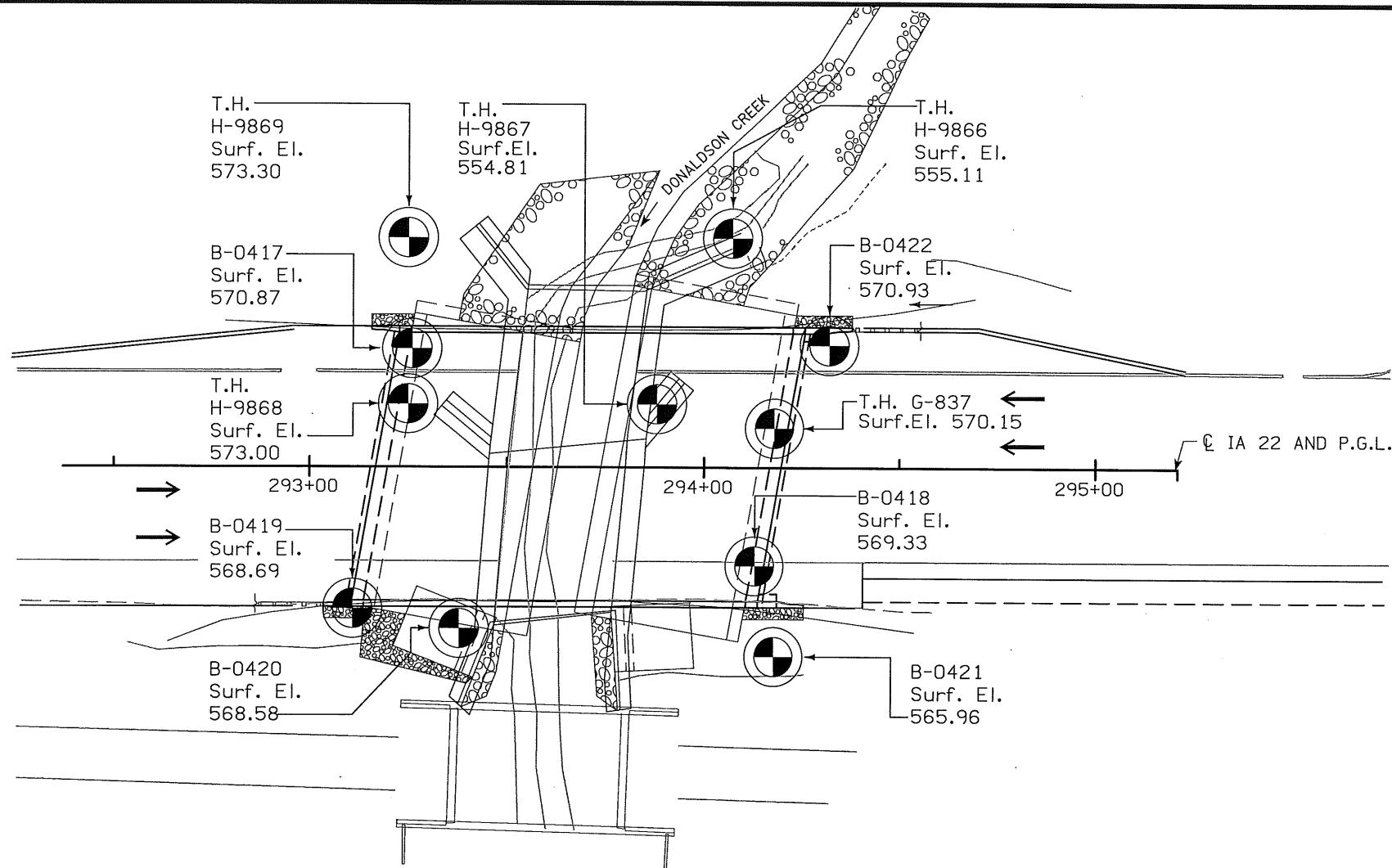
**LEGEND**

WATER	BLOW COUNT	
DRY	PLUGGED	
MOISTURE	SHELBY	
BLOW COUNT	DENS. CORE	
SAMPLE	DIAMOND CORE	
	SAND	
	GRAVELLY SAND	
	BOULDERS	

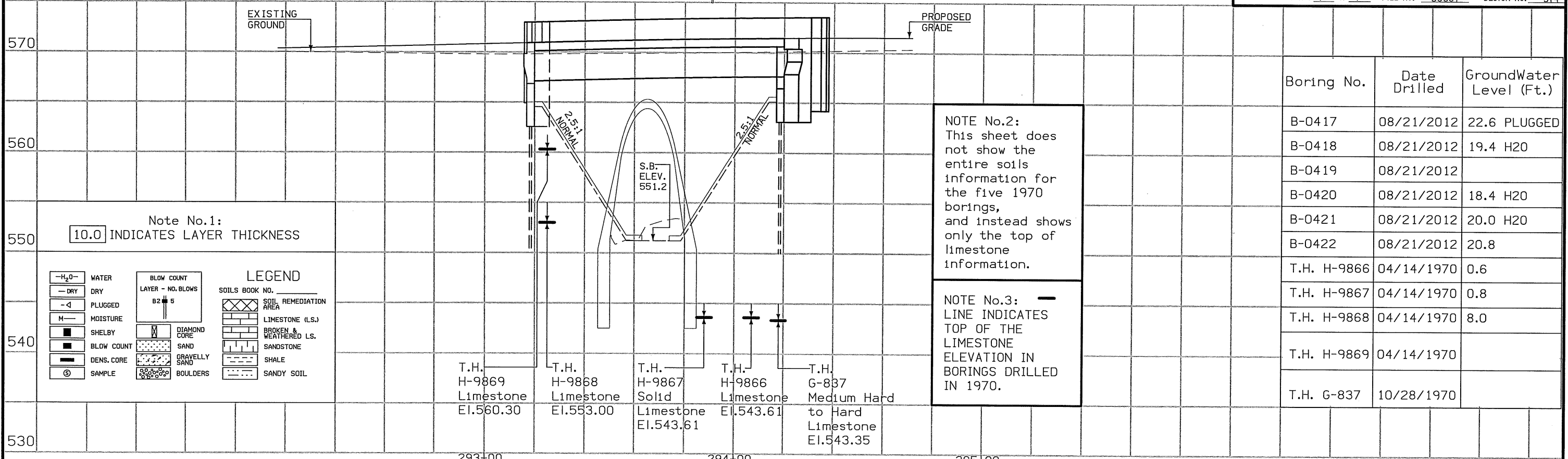
THIS SHEET IS INCLUDED TO SHOW SOIL INFORMATION. DETAILS AND NOTES SHOWN ELSEWHERE IN THESE PLANS SHALL BE USED FOR STRUCTURE CONSTRUCTION.

**LOCATION**

IA 22 OVER DONALDSON CREEK  
 T-77N R-2E  
 SECTION 24  
 BUFFALO TOWNSHIP  
 SCOTT COUNTY  
 BRIDGE MAINT. NO. 8292.85022  
 LATITUDE 41.464382°  
 LONGITUDE -90.681313°



DESIGN FOR 10° SKEW (L.A.)  
**100'-0 X 68'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0 SPAN (BTB BEAMS)  
**SOIL PROFILE SHEET**  
 STATION 293+68.30 IA 22  
**SCOTT COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
 DESIGN SHEET NO. 4 OF 4 FILE NO. 30687 DESIGN NO. 514



Note No.1: 10.0 INDICATES LAYER THICKNESS

**LEGEND**

WATER	BLOW COUNT LAYER - NO. BLOWS	SOIL REMEDIATION AREA
DRY	DIAMOND CORE	LIMESTONE (L.S.)
PLUGGED	SAND	BROKEN & WEATHERED L.S.
MOISTURE	GRAVELLY SAND	SANDSTONE
SHELBY BLOW COUNT	BOULDERS	SHALE
DENS. CORE		SANDY SOIL
SAMPLE		

NOTE No.2: This sheet does not show the entire soils information for the five 1970 borings, and instead shows only the top of limestone information.

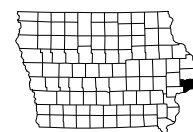
NOTE No.3: LINE INDICATES TOP OF THE LIMESTONE ELEVATION IN BORINGS DRILLED IN 1970.

Boring No.	Date Drilled	GroundWater Level (Ft.)
B-0417	08/21/2012	22.6 PLUGGED
B-0418	08/21/2012	19.4 H2O
B-0419	08/21/2012	
B-0420	08/21/2012	18.4 H2O
B-0421	08/21/2012	20.0 H2O
B-0422	08/21/2012	20.8
T.H. H-9866	04/14/1970	0.6
T.H. H-9867	04/14/1970	0.8
T.H. H-9868	04/14/1970	8.0
T.H. H-9869	04/14/1970	
T.H. G-837	10/28/1970	

T.H. H-9869 Limestone El.560.30  
 T.H. H-9868 Limestone El.553.00  
 T.H. H-9867 Solid Limestone El.543.61  
 T.H. H-9866 Limestone El.543.61  
 T.H. G-837 Medium Hard to Hard Limestone El.543.35

## INDEX OF SHEETS

No.	DESCRIPTION
<b>A Sheets</b>	<b>Title Sheets</b>
A.1	Title Sheet
<b>B Sheets</b>	<b>Typical Cross Sections and Details</b>
B.1 - 8	Typical Cross Sections and Details
<b>C Sheets</b>	<b>Quantities and General Information</b>
C.1	Project Description
C.1 - 2	Estimated Roadway Quantities
C.3 - 5	Estimate Reference Information
C.6	Standard Road Plans
C.6	Index of Tabulations
C.6	General Notes
C.7 - 13	Tabulations (beg. with tab. of incidentals if needed)
<b>CS Sheets</b>	<b>Mainline Plan and Profile Sheets</b>
CS.1	Soils Design Tabulations
<b>D Sheets</b>	<b>Mainline Plan and Profile Sheets</b>
* D.1	Plan & Profile Legend & Symbol Information Sheet
* D.2 - 3	IA 22
<b>F Sheets</b>	<b>Detour or Temporary Pavement Sheets</b>
* F.1 - 3	Detour Plan and Profile Sheets - Westbound side
* F.4	Detour Plan and Profile Sheets - Eastbound side
<b>G Sheets</b>	<b>Survey Sheets</b>
G.1	Survey Information
G.2	Reference Ties and Bench Marks
G.3	Horizontal Control Tab. & Curve Data
<b>H Sheets</b>	<b>Right-of-Way Sheets</b>
* H.1 - 2	IA 22
<b>J Sheets</b>	<b>Traffic Control and Staging Sheets</b>
* J.1	Traffic Control Plan
* J.1	Staging Notes Stage
* J.1	Tabulation of Special Events & Coordinated Operations
* J.2	Traffic Control & Staging Legend & Symbol Info. Sheet
* J.3 - 10	Staging and Traffic Control Sheets
<b>M Sheets</b>	<b>Storm Sewer Sheets</b>
M.1	Storm Sewer Tabulations
M.2	Storm Sewer Legend & Symbol Information Sheet
M.3 - 13	Storm Sewer Plan and Profile Sheets IA 22
<b>T Sheets</b>	<b>Earthwork Quantity Sheets</b>
T.1 - 2	Earthwork Quantity Sheets
<b>U Sheets</b>	<b>500 Series, Mod.Stds. and Detail Sheets</b>
U.1	Beam Guardrail End Anchorage Detail
<b>W Sheets</b>	<b>Mainline Cross Sections</b>
W.1	Cross Sections Legend & Symbol Information Sheet
W.2 - 18	IA 22 Cross Sections - Stage 1
W.19 - 36	IA 22 Cross Sections - Stage 2
W.37 - 53	IA 22 Cross Sections - Stage 3
W.54 - 57	IA 22 Cross Sections - Stage 4
	* Color Plan Sheets



DESIGN DATA RURAL			
2013 AADT	4900		V.P.D.
2033 AADT	6100		V.P.D.
2033 DHV	630		V.P.H.
TRUCKS	21		%
Total			
Design ESALs	--		

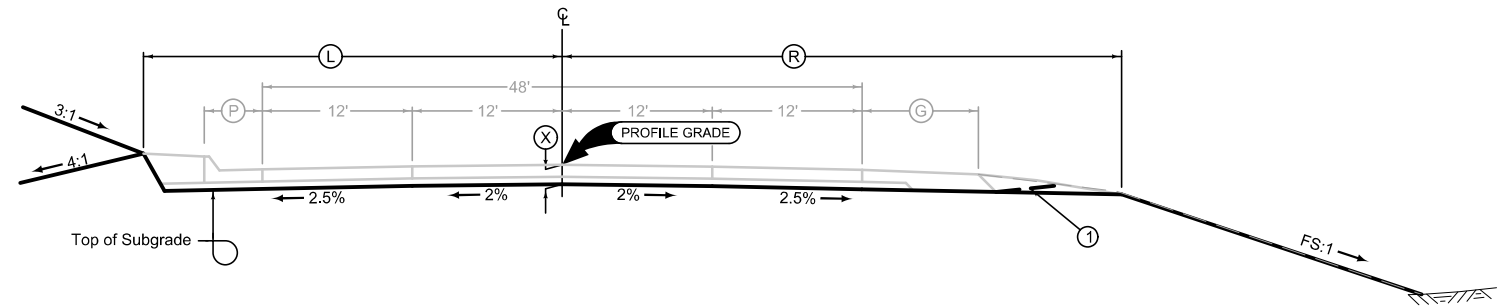
Design No. 514  
File No. 30687

ROADWAY DESIGN	
	<p>I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.</p> <p style="text-align: right;"><i>Jason M. Holst</i> 12-03-2013 Signature Date</p> <p>Jason M. Holst Printed or Typed Name</p> <p>My license renewal date is December 31, 2013</p>
<p>Pages or sheets covered by this seal: <u>A.1, B.1-B.8, C.1-C.13, D.1-D.3, F.1-F.4, G.1-G.3, H.1-H.2, J.1-J.10, M.1-M.13, T.1-T.2, W.1-W.57</u></p>	

LOCATION		DIMENSIONS				
ROAD IDENTIFICATION	STATION TO STATION	Ⓛ Feet	Ⓡ Feet	ⓧ Inches	FS	
IA 22	291+01.00	291+54.00	29.5	35.7	22	4
IA 22	291+54.00	291+64.71	29.5	35.7-39.4	22	4
IA 22	291+64.71	291+90.38	29.5	39.4-40.6	22	4
IA 22	291+90.38	292+26.33	29.5-33.1	40.6	22	4
IA 22	292+26.33	292+95.78	33.1-40.0	40.6-44	22	4
IA 22	292+95.78	293+15.77	40.0	44	22	4
IA 22	294+25.80	294+70.00	40.0	44.2	22	4
IA 22	294+70.00	295+22.70	40.0-29.5	44.2	22	4
IA 22	295+22.70	298+15.00	29.5	44.2	22	4

Normal section shown may be modified appropriately in areas of super-elevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and back-slopes.



**4 LANE GRADING**

① In areas where proposed shoulder elevation closely matches existing shoulder elevation, grade only as required to place proposed shoulder. Refer to cross sections for more information.

2\_Grade  
Modified

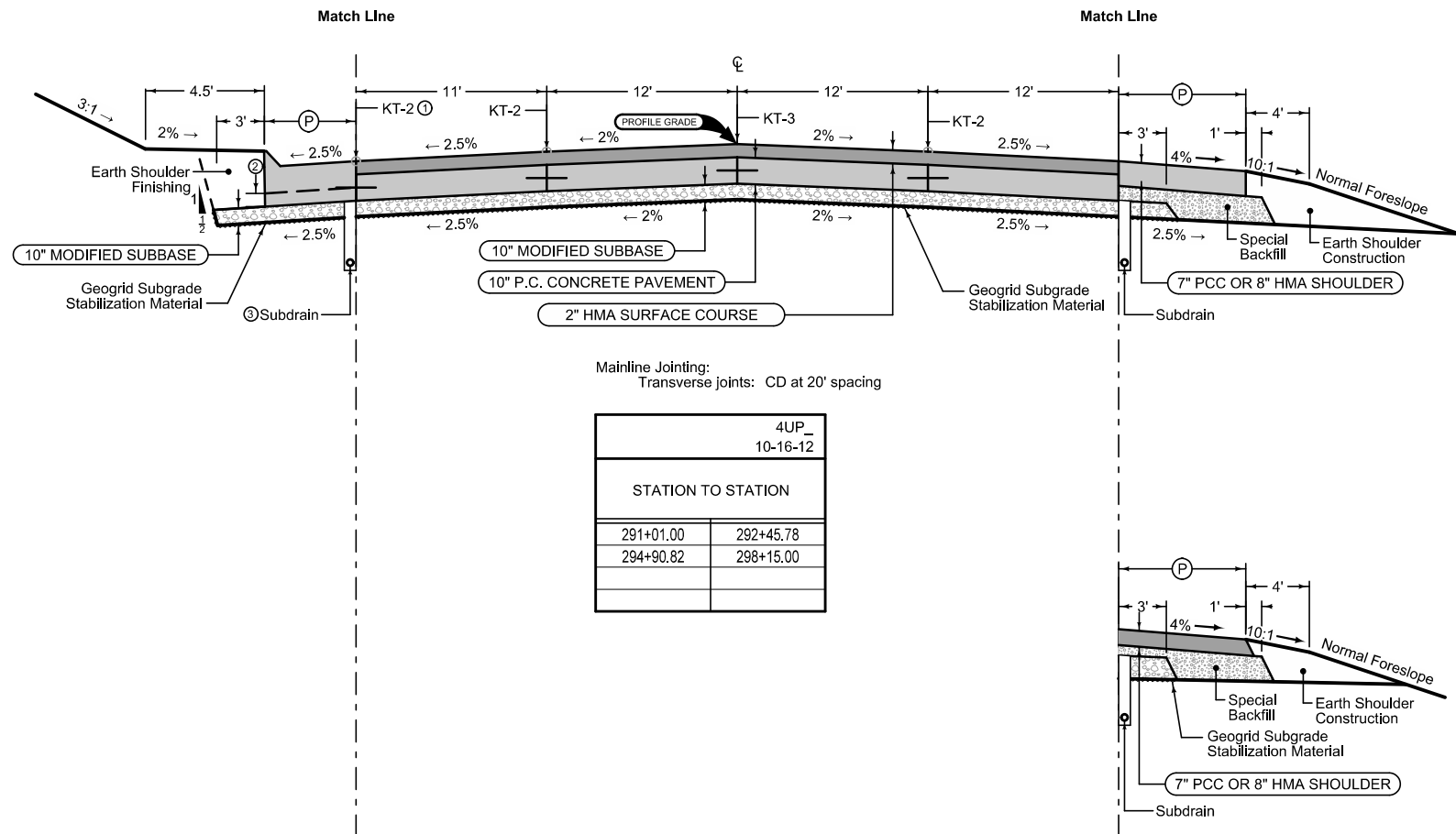
**Curbed Shoulder**

① Shoulder Jointing:  
Longitudinal joint is optional when distance from back of curb to nearest joint is less than 15':

Single pour: L-2  
Staged: KT-2  
Transverse: C at 20' spacing

STATION TO STATION		Ⓟ Feet	Curb Type See PV-102
291+06.00	291+90.38	2	6" Sloped
291+90.38	292+95.78	2.0-11.54	6" Sloped
292+95.78	293+15.77	11.54	See RK-20
294+37.80	294+40.82	11.54	See RK-20
294+40.82	294+70.00	11.54	4" Sloped
294+70.00	295+22.70	11.54-2.0	4" Sloped
295+22.70	295+53.20	2	4" Sloped
295+53.20	296+48.20	2	1.5" Drop
296+48.20	298+15.00	2	6" Sloped

- ② An additional 2" of Curb and Gutter will be paved to tie into the mainline pavement.
- ③ Refer to the Standard Road Plan RF-19C, Type 12 installation for details.



Mainline Jointing:  
Transverse joints: CD at 20' spacing

STATION TO STATION	
291+01.00	292+45.78
294+90.82	298+15.00

**Paved Shoulder Alternates**

PCC Shoulder Jointing:  
Longitudinal joint: BT-1 or BT-3  
Transverse joints: C at 20' spacing  
HMA Shoulder Jointing:  
Longitudinal joint: B

4_P_ALT_10-19-10			
Direction of Travel	BEGIN STATION	END STATION	Ⓟ Feet
EB	291+06.59	291+30.00	0-10
EB	291+30.00	291+54.00	10
EB	294+40.82	298+15.00	10

**Paved Shoulder at Guardrail**

Shoulder Jointing:  
Longitudinal joint: B

\*\* See 7156 for shoulder width and additional breaks.

4_P_Guard_04-16-13			
Direction of Travel	BEGIN STATION	END STATION	Ⓟ Feet
EB	291+54.00	292+55.78	**

Design No. 514  
File No. 30687

See Tab 100-24 for pavement quantities.

See Tab 112-9 for shoulder quantities.

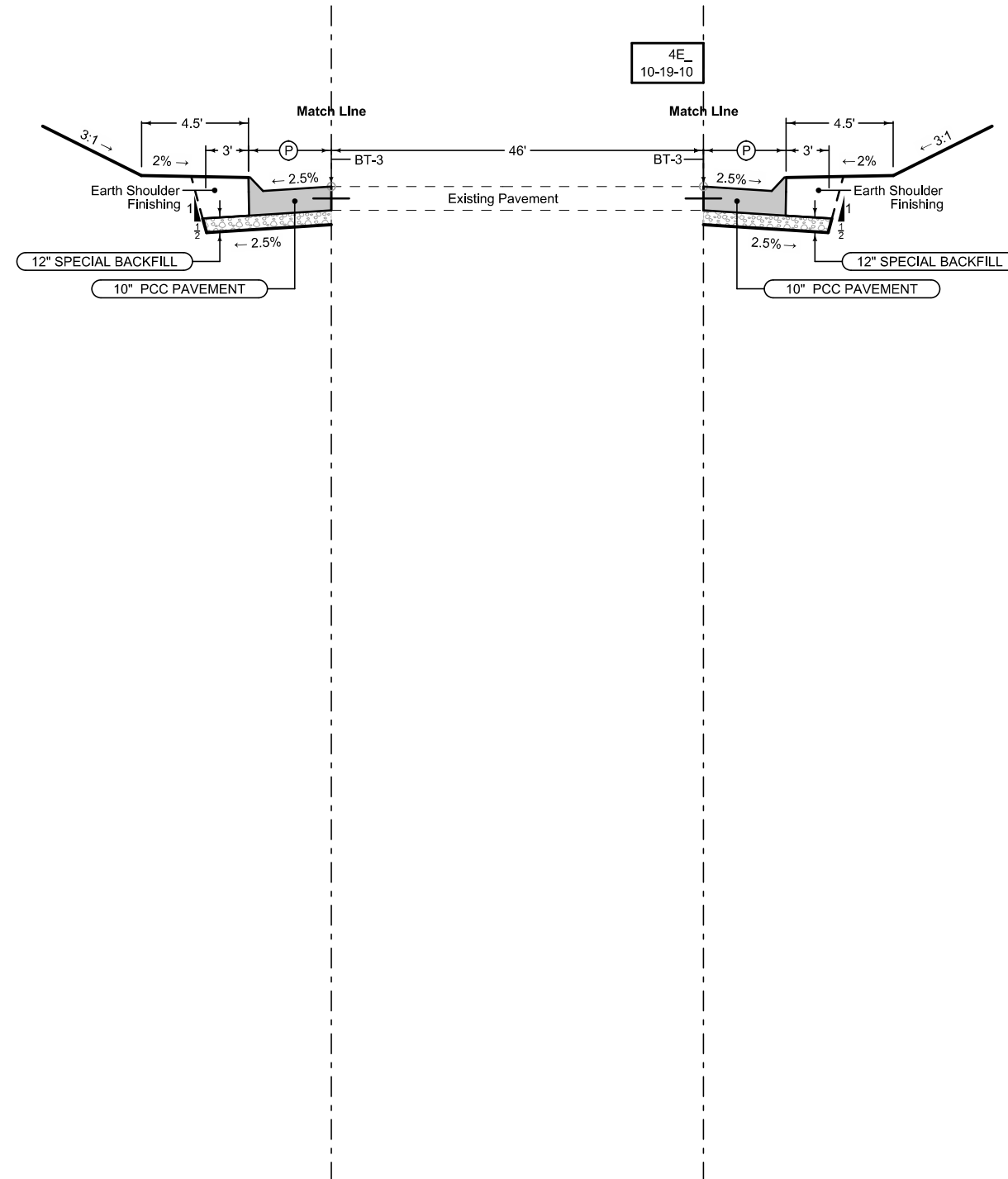
**IA 22**

### Curbed Shoulder

Shoulder Jointing:  
Transverse: C matches existing joint spacing

STATION TO STATION		(P) Feet	Curb Type See PV-102
287+60.00		1.5-2	(1)
287+65.00	290+47.00	2	6" Sloped
298+15.00	300+36.50	2	6" Sloped
300+36.50	300+41.50	2-1.5	(1)

(1) 5' Curb Transition from 6" Sloped to 6" Standard



### Curbed Shoulder

Shoulder Jointing:  
Transverse: C matches existing joint spacing

STATION TO STATION		(P) Feet	Curb Type See PV-102
287+57.00		1.5-2	(2)
287+75.00	290+25.40	2	6" Sloped
290+25.40	290+52.00	(2)	6" Sloped

- (2) Refer to Typical SW\_RETURN on Sheet B.8.
- (3) 5' Curb Transition from 6" Sloped to 6" Standard

See Tab 100-24 for pavement quantities.  
See Tab 112-9 for shoulder quantities.  
**IA 22 - Existing Pavement  
Proposed Curb & Granular Shoulder**

Design No. 514  
File No. 30687

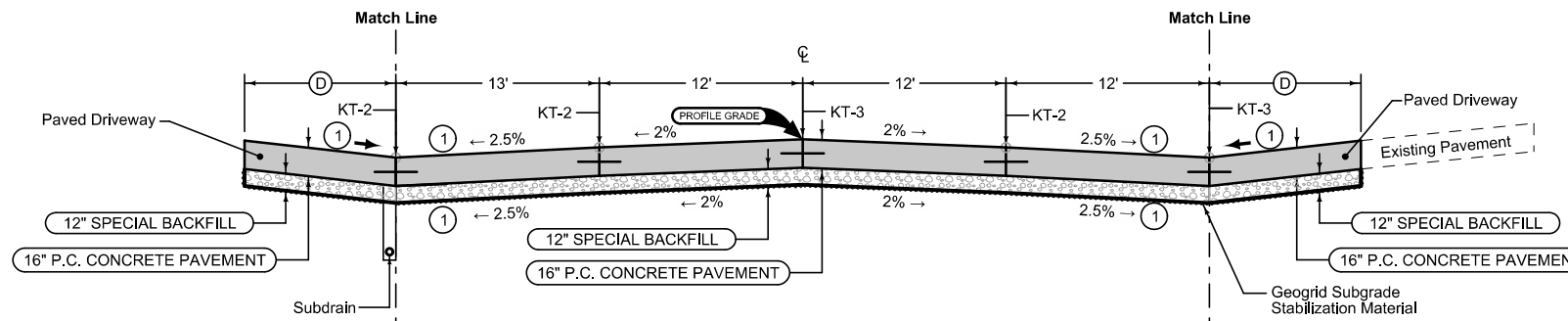
### Mine Entrance at Crossing

Shoulder Jointing:  
Refer to the Typical on Sheet B.6 for details.

Mine Entrance		
STATION TO STATION	(D)	Feet
290+52.00	291+01.00	10

Normal section shown may be modified appropriately in areas of superelevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and backslopes.



Mainline Jointing:  
Transverse joints: CD at 20' spacing

4UP_	
10-16-12	
STATION TO STATION	
290+52.00	291+01.00

### Mine Entrance at Crossing

Shoulder Jointing:  
Refer to the Typical on Sheet B.6 for details.

Mine Entrance		
STATION TO STATION	(D)	Feet
290+52.00	291+01.00	10

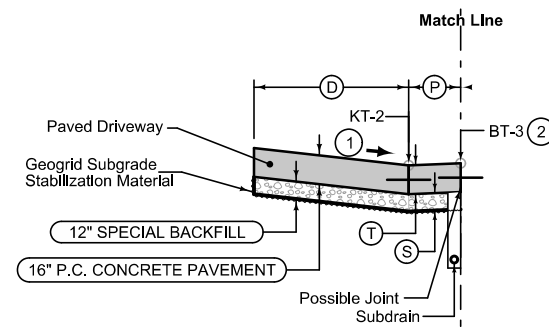
Normal section shown may be modified appropriately in areas of superelevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and backslopes.

### Mine Entrance Tie-Ins

Shoulder Jointing:  
Refer to the Typical on Sheet B.6 for details.

Mine Entrance Ties					
STATION TO STATION		(P)	(D)	(T)	(S)
		Feet	Feet	Inches	Inches
② 290+47.00	290+52.00	2	0-10	10	③ 12
291+01.00	291+06.00	2	10-0	10	④ 12



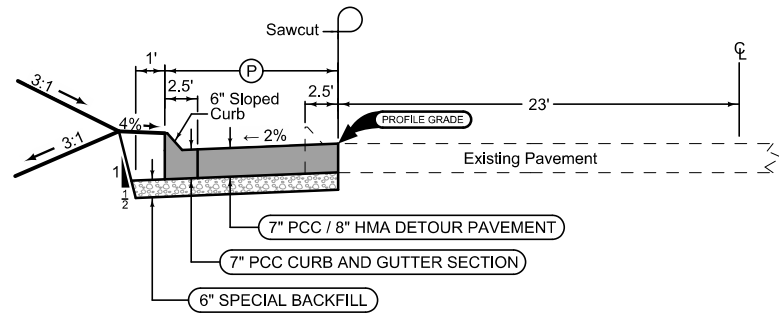
- ① Slope varies in order to match existing mainline pavement. Refer to Cross Sections for slopes.
- ② Only use BT-3 Joint to tie into existing mainline pavement.
- ③ Use 12" Special Backfill for subbase adjacent to existing mainline pavement. See Curbed Shoulder typical on Sheet B.2.
- ④ Use 10" Modified Subbase adjacent to 10" mainline pavement. See Curbed Shoulder typical on Sheet B.1

See Tab 100-24 for pavement quantities.  
See Tab 112-9 for shoulder quantities.

## IA 22 Linwood Mine Crossing

Design No. 514  
File No. 30687

Detour-1



\*\* 1.5" Sloped Curb at driveway.

Stage 1 (Temporary)		
STATION TO STATION	(P)	Feet
287+60.00	289+50.00	1.5-11
289+50.00	290+49.00	11
291+04.00	295+65.00	11
** 295+65.00	296+37.00	11
296+37.00	298+50.00	11
298+50.00	300+41.50	11-1.5

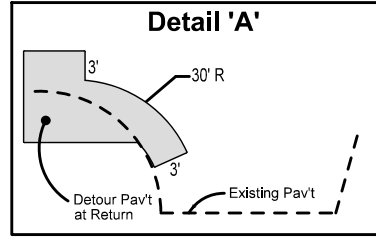
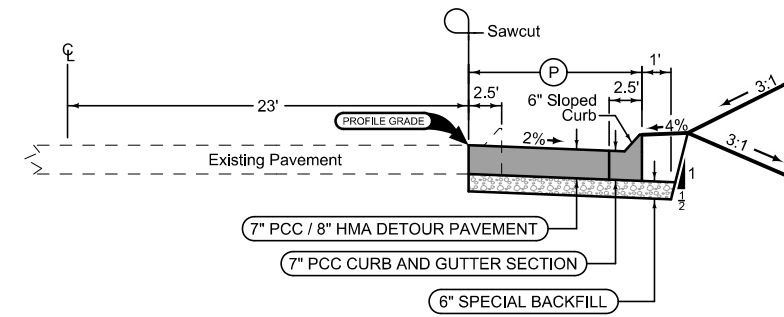
Normal section shown may be modified appropriately in areas of super-elevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and backslopes.

**DETOUR PAVING - STAGE 1**

Detour-4

- 1 Tie into existing return of south mine entrance. Refer to Detail 'A' and J Sheets for more information.



Normal section shown may be modified appropriately in areas of super-elevated curves or other locations specifically designated by the Engineer.

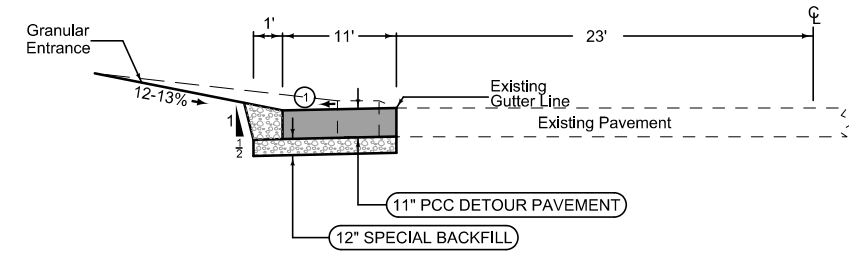
See plan & profile sheets and cross sections for additional details of ditches and backslopes.

**DETOUR PAVING - STAGE 2**

STATION TO STATION			(P)	Feet
287+70.00	289+50.00	2-11		
289+50.00	290+38.23	11		
290+38.23	290+45.11	1	1	

Detour-2

- 1 Refer to Cross Sections for slopes.



Normal section shown may be modified appropriately in areas of super-elevated curves or other locations specifically designated by the Engineer.

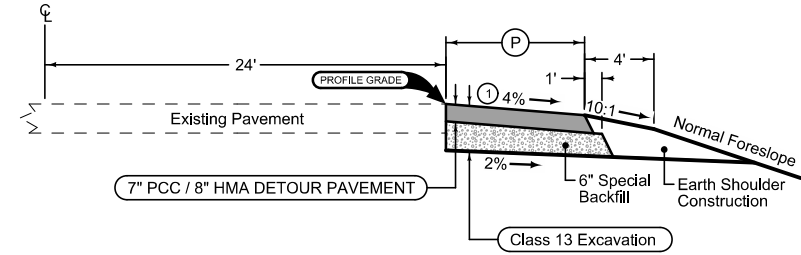
See plan & profile sheets and cross sections for additional details of ditches and backslopes.

**DETOUR PAVING - STAGE 1**

Stage 1 (Temporary)		
STATION TO STATION	(P)	Feet
290+49.00	291+04.00	

SHOULDER

- 1 Slope Varies. Tie into existing entrance pavement.



PCC Shoulder Jointing:  
 Longitudinal joint: BT-1 or BT-3  
 Transverse joints: C at 20' spacing  
 HMA Shoulder Jointing:  
 Longitudinal joint: B

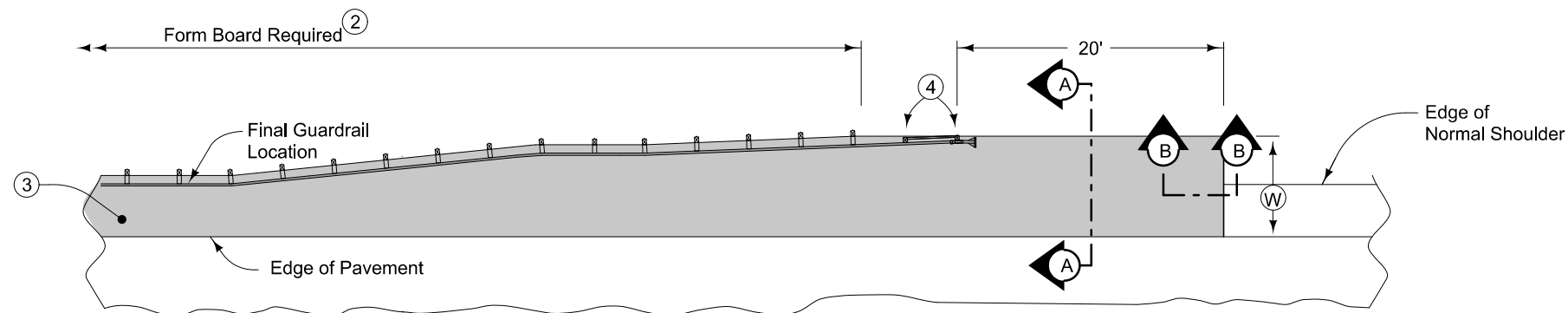
Normal section shown may be modified appropriately in areas of super-elevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and backslopes.

**Paved Shoulder Alternates - Stage 2**

Stage 2 (Final)		
STATION TO STATION	(P)	Feet
298+15.00	300+47.71	10

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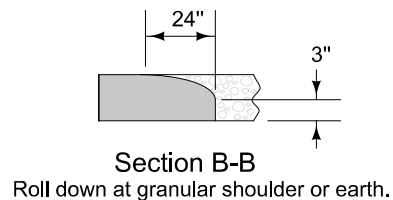
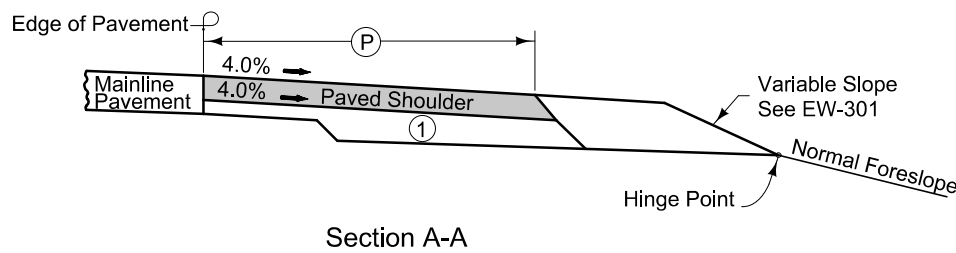
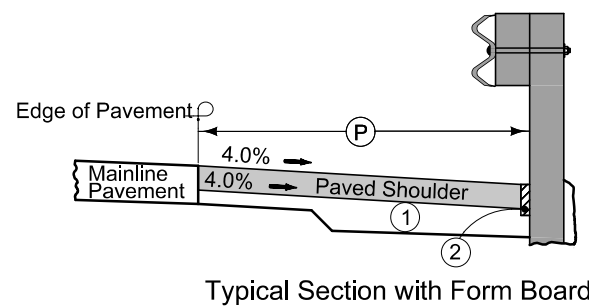
8" HMA Paved Shoulder at guardrail. 7" PCC may be substituted with the following jointing layout:

Match mainline pavement joint spacing. When mainline pavement is 8" or greater in thickness, place additional transverse joints in shoulder at mid-panel of the mainline pavement. Place longitudinal joint at W/2 from edge of mainline pavement when W is greater than 10' wide. Terminate longitudinal joint at transverse joint less than 10' in length.

Compaction of HMA is required to face of guardrail post. Hand compaction will be allowed under guardrail. Removal & reinstallation of guardrail will be allowed with no additional payment.

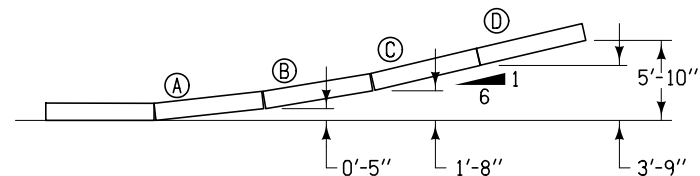
Refer to Shoulder tabulation (112-9) for quantities.

- ① 6" Special Backfill.
- ② When guardrail posts are installed prior to construction of paved shoulder, nail 1" x 6" untreated form boards along the face of guardrail posts for the length shown. This board is to prevent shoulder material from contacting the sides of the posts and altering the function of the guardrail. Form board not required for final 2 posts.
- ③ Continue paved shoulder to existing paved shoulder or 20' beyond the end of guardrail.
- ④ Shoulder may be notched for final 2 posts or post sleeves may be installed through pavement.



PAVED SHOULDER AT GUARDRAIL

TBR-FLARE



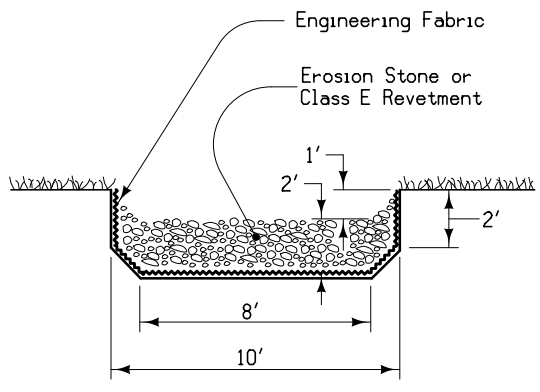
MAXIMUM BARRIER OFFSETS  
FOR FLARE SECTIONS

Design No. 514  
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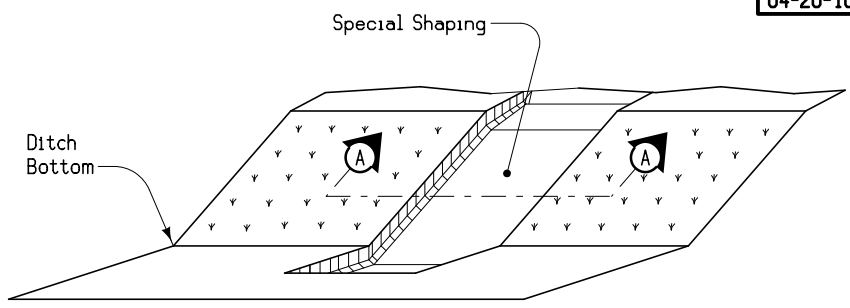


4403  
04-20-10

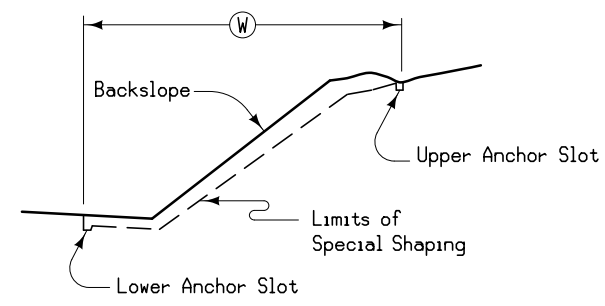
CROSSING  
JOINTING



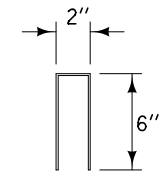
Section A-A



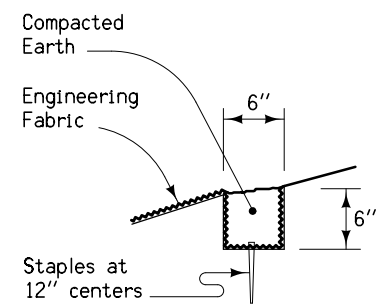
Isometric View



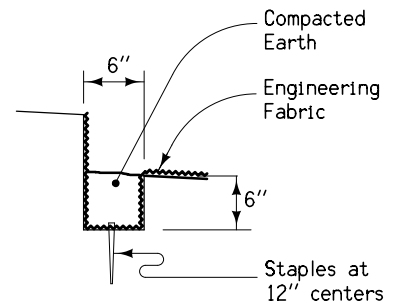
Typical Section



Staple  
(No. 11 wire)



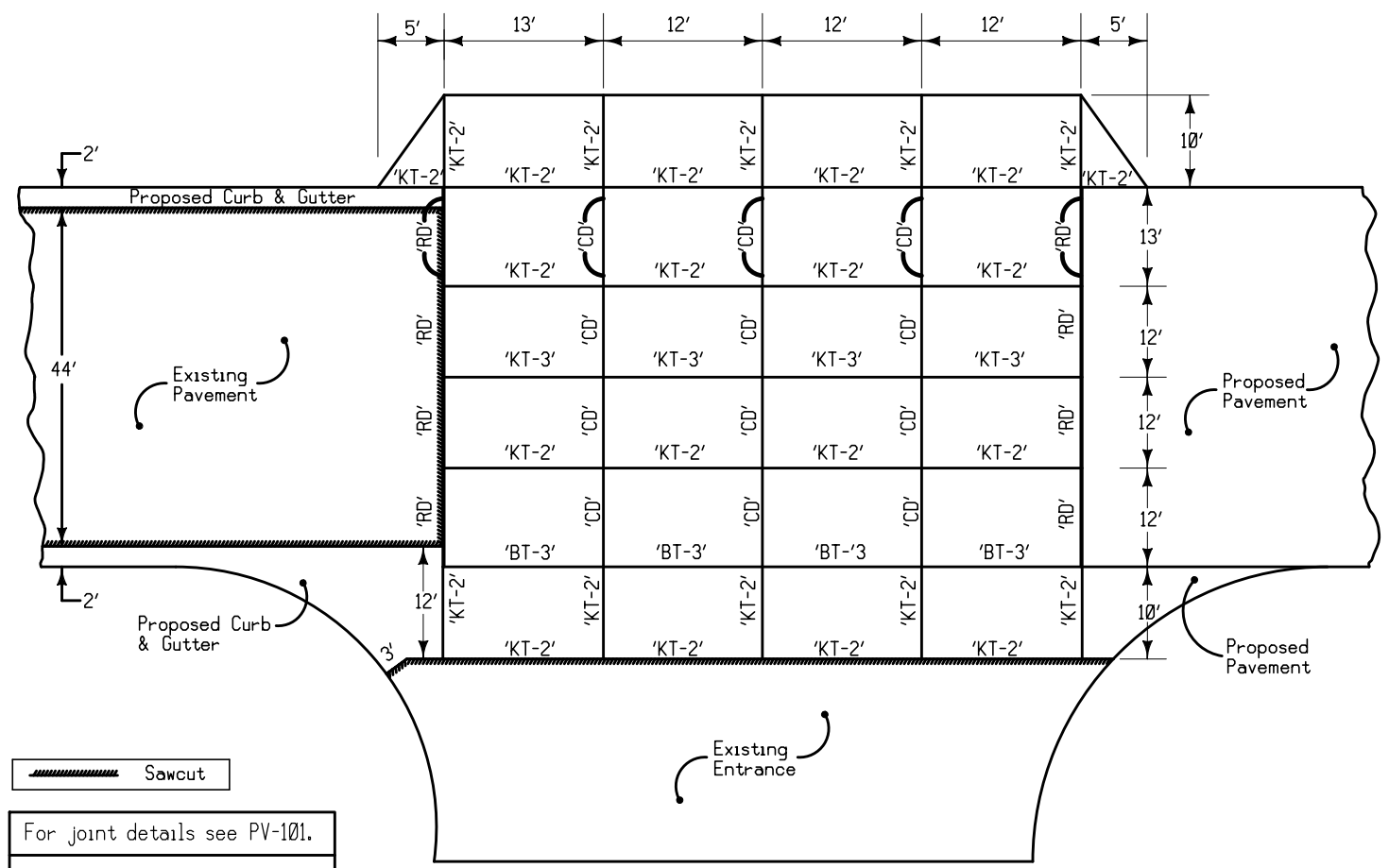
Upper Anchor Slot



Lower Anchor Slot

ROCK FLUME

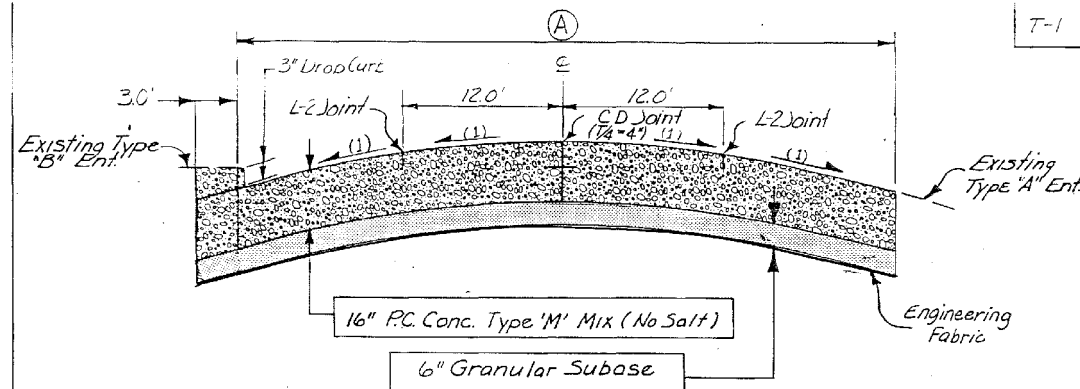
Refer to Tabulation 100-23 for additional information.



For joint details see PV-101.  
 'CD' Dowel Dimensions:  
 Pavement Thickness, T, is 16"  
 Dowel Height, DH, is 8"  
 Dowel Diameter is 1-1/2"

JOINTING DIAGRAM & DOWEL DETAILS  
FOR PCC PAVEMENT AT MINE CROSSING  
(Sta. 290+76.50)

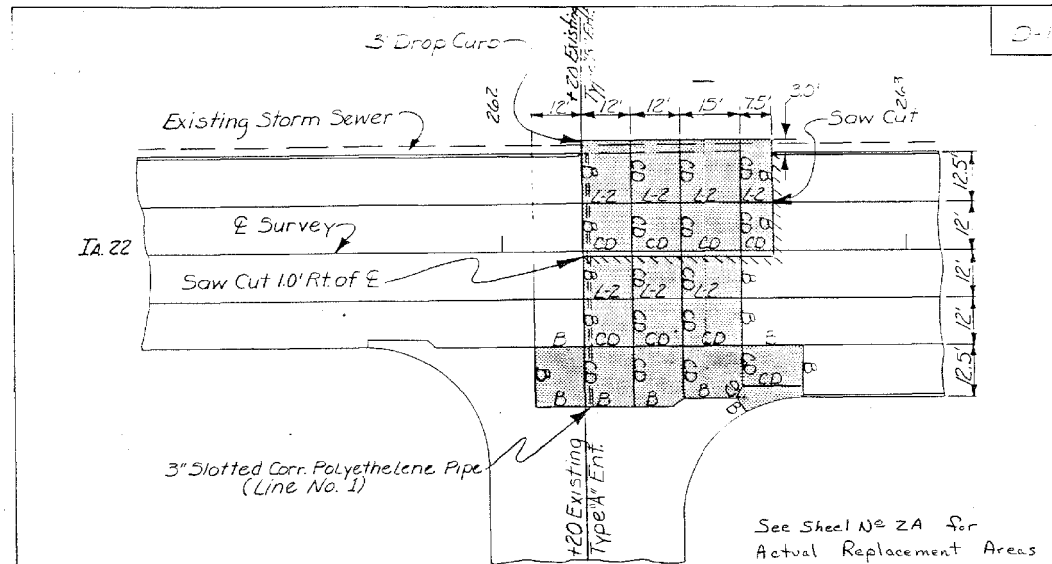
Design No. 514  
File No. 30687



RD. IDENT	STATION TO STATION	(A)	REMARKS
IA. 22	262+08±	262+50±	48.5'
IA. 22	290+49.3±	291+03.6±	48.5'

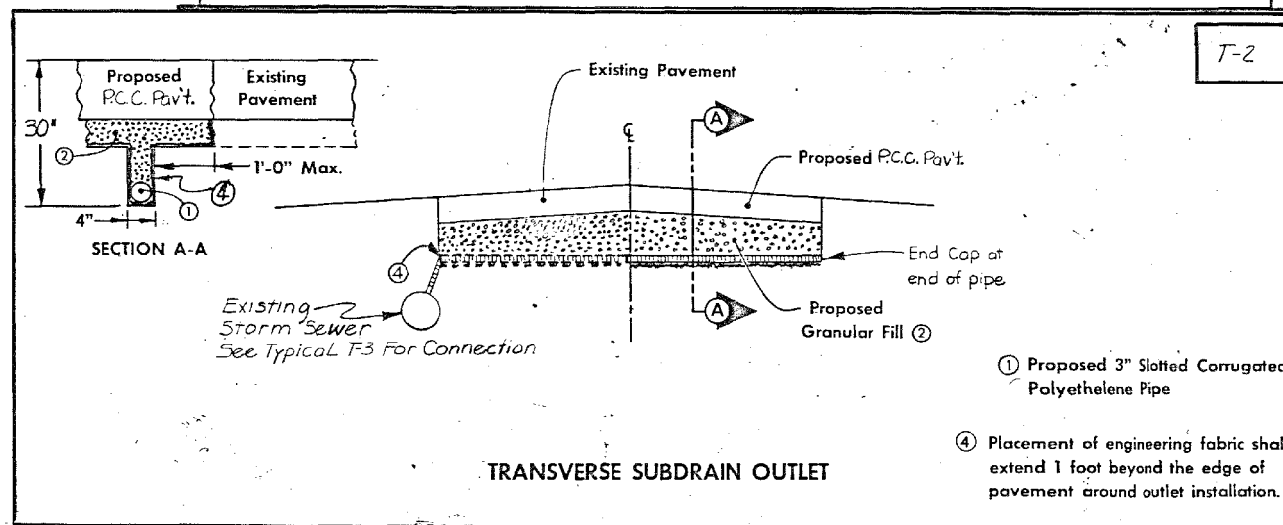
TYPICAL SECTION  
FOR FULL DEPTH P.C. CONC. PATCHING

(1) Contractor shall match existing profile for each lane.



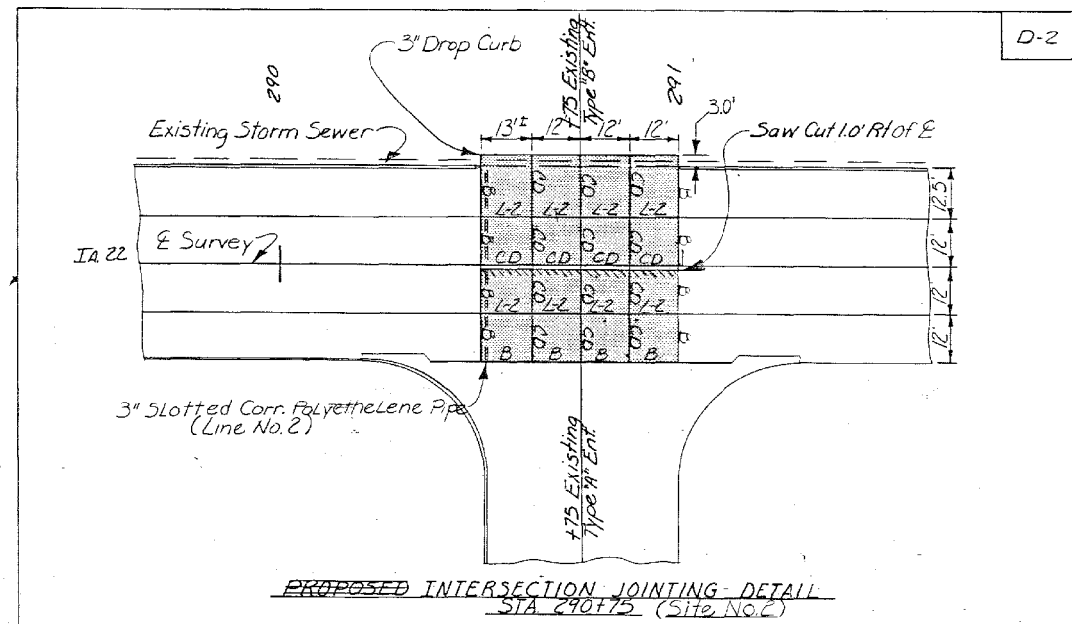
PROPOSED INTERSECTION JOINTING DETAIL  
AT STA 262+20 (Site No. 1)

See Sheet No. 2A for Actual Replacement Areas

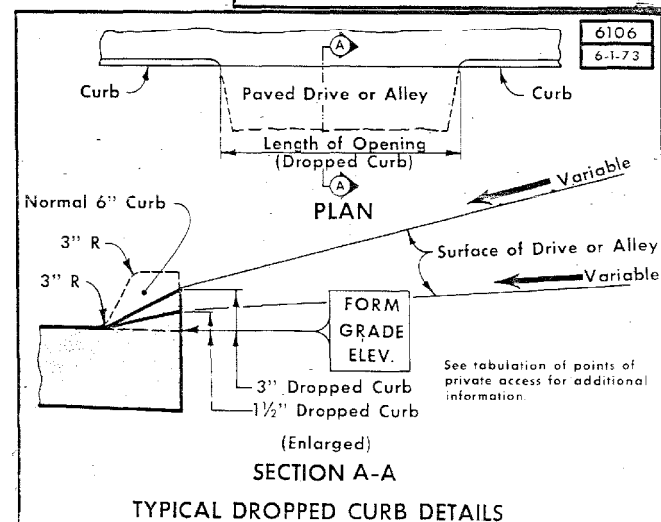


TRANSVERSE SUBDRAIN OUTLET

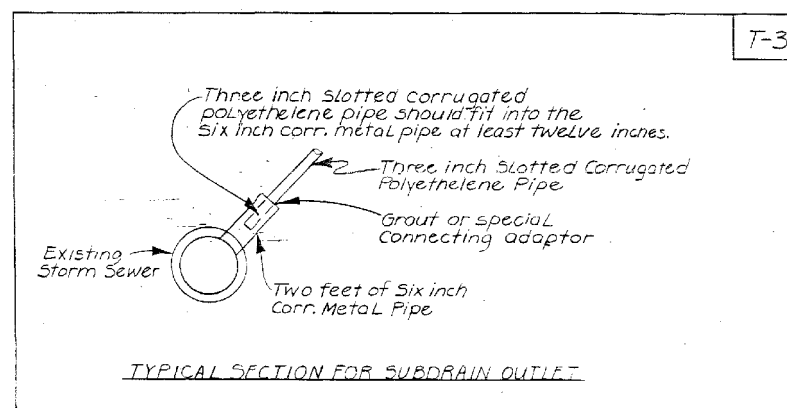
- ① Proposed 3" Slotted Corrugated Polyethylene Pipe
- ④ Placement of engineering fabric shall extend 1 foot beyond the edge of pavement around outlet installation.



PROPOSED INTERSECTION JOINTING DETAIL  
STA 290+75 (Site No. 2)



TYPICAL DROPPED CURB DETAILS



TYPICAL SECTION FOR SUBDRAIN OUTLET

Scott COUNTY

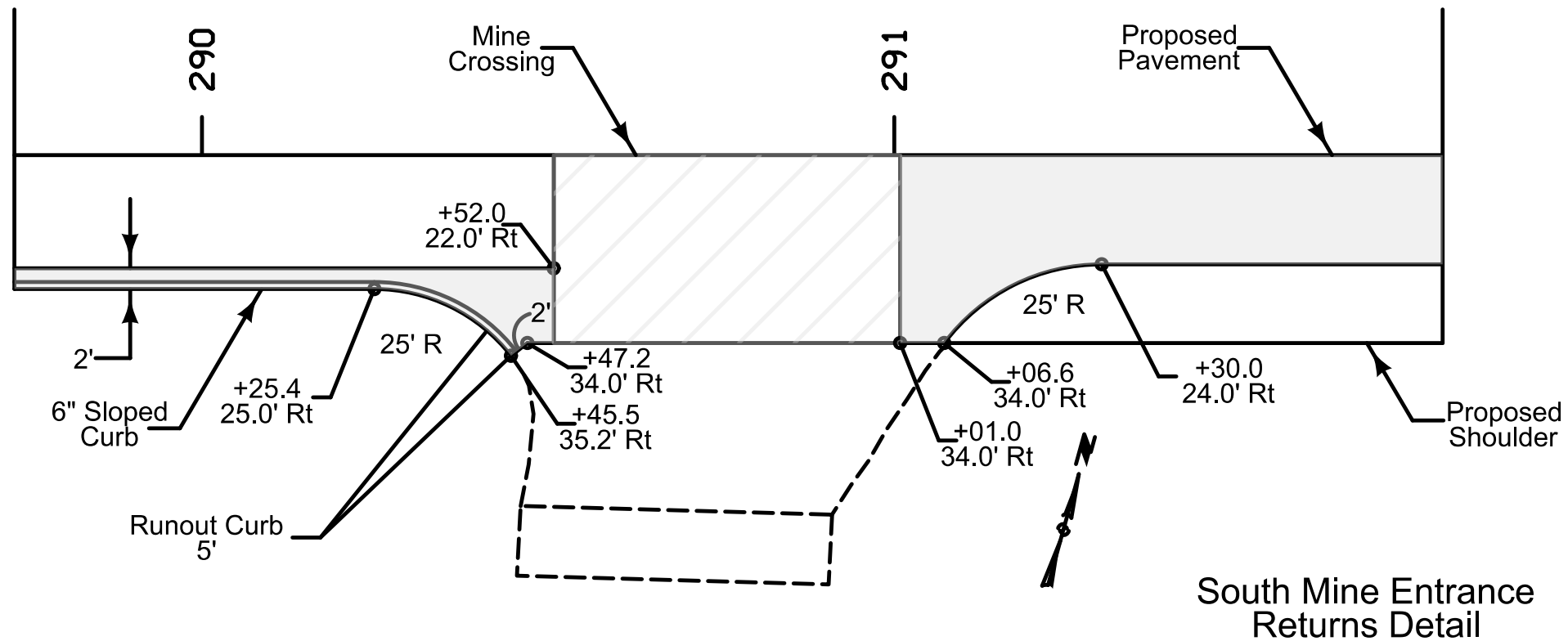
FN-22-5(15)--21-82

For Information Only

Design No. 514  
File No. 30687

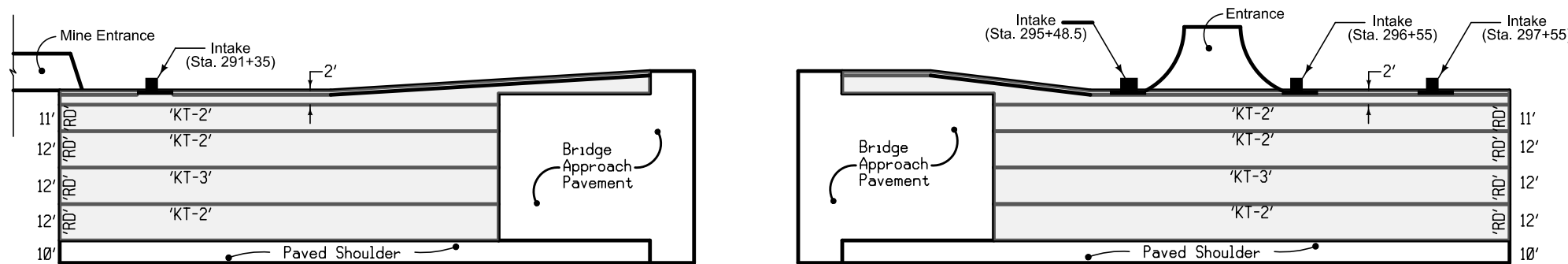
\*Use layout as guidance. UAC existing jointing.

RETURNS



South Mine Entrance Returns Detail

MAINLINE JOINTING



Mainline Jointing Details

For joint details see PV-101.  
 Transverse Joints: Place CD joints at 20' spacing. Adjust spacing at intakes and end of paving. A minimum spacing of 12' should be used.

Design No. 514  
 File No. 30687

100-1D  
10-18-05

**PROJECT DESCRIPTION**

This project involves the replacement of the IA 22 Bridge over Donaldson Creek 2.8 miles west of I-280. The existing structure will be replaced with a 100' X 68' pretensioned prestressed concrete beam bridge. New bridge approaches will be constructed. The roadway beyond the ends of the bridge approaches will be reconstructed in order to meet the new profile grade. Additional work to replace the Linwood Mine crossing at Sta. 290+77 are included with this project.

All roadway items will be included in Division 1. Items included in the construction of the Linwood Mine crossing will be a part of Division 2.

Traffic will be maintained by staged construction with at least one lane open in each direction on mainline. Access to the north and south entrances to Linwood Mine will be maintained at all times.

100-1C  
04-17-12

**ESTIMATED PROJECT QUANTITIES  
(UP TO A 5 DIVISION PROJECT)**

Item No.	Item Code	Item	Unit	Quantities																
				Estimated					Total	As Built										
				Division 1	Division 2	Division 3	Division 4	Division 5		Division 1	Division 2	Division 3	Division 4	Division 5						
1	2101-0050001	CLEARING AND GRUBBING	ACRE	0.2							0.2									
2	2102-0425070	SPECIAL BACKFILL	TON	899.5	240.2						1,139.7									
3	2102-2625000	EMBANKMENT-IN-PLACE	CY	598.0							598.0									
4	2102-2710000	EXCAVATION, CLASS 10, WASTE	CY	3,219.0							3,219.0									
5	2102-2712015	EXCAVATION, CLASS 12, BOULDERS OR ROCK FRAGMENTS	CY	20.0							20.0									
6	2102-2713000	EXCAVATION, CLASS 13, WASTE	CY	154.8							154.8									
7	2105-0425005	TOPSOIL, FURNISH AND SPREAD	CY	295.0							295.0									
8	2107-0075100	COMPACTION WITH MOISTURE CONTROL	CY	500.0							500.0									
9	2113-0001100	SUBGRADE STABILIZATION MATERIAL, POLYMER GRID	SY	2,994.1	381.2						3,375.3									
10	2115-0100000	MODIFIED SUBBASE	CY	831.7							831.7									
11	2122-5190501	PAVED SHOULDER, PORTLAND CEMENT CONCRETE (PAVED SHOULDER PANEL FOR BRIDGE END DRAIN)	SY	51.3							51.3									
12	2123-7450000	SHOULDER CONSTRUCTION, EARTH	STA	8.00							8.00									
13	2123-7450020	SHOULDER FINISHING, EARTH	STA	26.90							26.90									
14	2213-6745500	REMOVAL OF CURB	STA	14.40							14.40									
15	2301-0690200	BRIDGE APPROACH, RK-20	SY	860.0							860.0									
16	2301-1033100	STANDARD OR SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 10 IN.	SY	2,691.2							2,691.2									
17	2301-1033110	STANDARD OR SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 11 IN.	SY	67.2							67.2									
18	2301-6911722	PORTLAND CEMENT CONCRETE PAVEMENT SAMPLES	LS	1.00							1.00									
19	2303-0053503	HOT MIX ASPHALT MIXTURE (10,000,000 ESAL), SURFACE COURSE, 1/2 IN. MIX, FRICTION L-3	TON	270.0							270.0									
20	2303-0246428	ASPHALT BINDER, PG 64-28	TON	16.2							16.2									
21	2304-0100000	DETOUR PAVEMENT	SY	1,665.2							1,665.2									
22	2315-0275025	SURFACING, DRIVEWAY, CLASS A CRUSHED STONE	TON	19.7							19.7									
23	2416-0100024	APRONS, CONCRETE, 24 IN. DIA.	EACH	3							3									
24	2435-0140148	MANHOLE, STORM SEWER, SW-401, 48 IN.	EACH	1							1									
25	2435-0250000	INTAKE, SW-500	EACH	5							5									
26	2435-0250000	INTAKE, SW-500	EACH	1							1									
27	2502-0212034	SUBDRAIN, LONGITUDINAL, (SHOULDER) 4 IN. DIA.	LF	1,189.9							1,189.9									
28	2502-0220193	SUBDRAIN OUTLET (RF-19C)	EACH	11							11									
29	2502-0220196	SUBDRAIN OUTLET, RF-19E	EACH	5							5									
30	2503-0114215	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 2000D (CLASS III), 15 IN.	LF	103.5							103.5									
31	2503-0114224	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 2000D (CLASS III), 24 IN.	LF	564.5							564.5									
32	2503-0114424	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 3000D (CLASS IV), 24 IN.	LF	183.4							183.4									
33	2503-0200036	REMOVE STORM SEWER PIPE LESS THAN OR EQUAL TO 36 IN.	LF	489							489									
34	2503-0200341	STORM SEWER ABANDONMENT, FILL AND PLUG, LESS THAN OR EQUAL TO 36 IN. DIA.	LF	223							223									
35	2505-4000120	REMOVAL OF STEEL BEAM GUARDRAIL	LF	381.3							381.3									
36	2505-4000300	STEEL BEAM GUARDRAIL	LF	87.5							87.5									
37	2505-4000400	STEEL BEAM GUARDRAIL BARRIER TRANSITION SECTION	EACH	2							2									
38	2505-4021010	STEEL BEAM GUARDRAIL END ANCHOR, BOLTED	EACH	2							2									
39	2505-4021700	STEEL BEAM GUARDRAIL END TERMINAL	EACH	2							2									
40	2507-3250005	ENGINEERING FABRIC	SY	108.9							108.9									
41	2507-0020000	EROSION STONE	TON	84.0							84.0									
42	2510-6745050	REMOVAL OF PAVEMENT	SY	5,394.6	321.2						5,715.8									
43	2510-6750600	REMOVAL OF INTAKES AND UTILITY ACCESSES	EACH	5							5									

Design No. 514  
File No. 30687

**ESTIMATED PROJECT QUANTITIES  
(UP TO A 5 DIVISION PROJECT)**

Item No.	Item Code	Item	Unit	Quantities											
				Estimated					Total	As Built					
				Division 1	Division 2	Division 3	Division 4	Division 5		Division 1	Division 2	Division 3	Division 4	Division 5	
44	2512-1725206	CURB AND GUTTER, P.C. CONCRETE, 2.0 FT.	LF	800.5						800.5					
45	2515-2475008	DRIVEWAY, P.C. CONCRETE, 8 IN.	SY	160.7						160.7					
46	2515-6745600	REMOVAL OF PAVED DRIVEWAY	SY	138.6						138.6					
47	2510-6910000	SAFETY CLOSURE	EACH	8						8					
48	2520-3350015	FIELD OFFICE	EACH	1						1					
49	2527-9263109	PAINTED PAVEMENT MARKING, WATERBORNE OR SOLVENT-BASED	STA	188.36						188.36					
50	2527-9263100	PAVEMENT MARKINGS REMOVED	STA	141.88						141.88					
51	2528-8400048	TEMPORARY BARRIER RAIL, CONCRETE	LF	1,275.0						1,275.0					
52	2528-8400256	TEMPORARY TRAFFIC SIGNALS	EACH	2						2					
53	2528-8445110	TRAFFIC CONTROL	LS	1.00						1.00					
54	2528-8445113	FLAGGERS	EACH							See Proposal					
55	2528-9109020	TEMPORARY LANE SEPARATOR SYSTEM	LF	4,240.0						4,240.0					
56	2551-0000110	TEMP CRASH CUSHION	EACH	3						3					
57	2552-0000140	ROCK EXCAVATION	CY	49.3						49.3					
58	2595-0000012	INSURANCE WHEN WORKING IN RAILROAD RIGHT-OF-WAY, MODIFIED, FOR DAKOTA MINNESOTA & EASTERN RAILROAD DOING BUSINESS AS CANADIAN PACIFIC LIABILITY INSURANCE, MODIFIED, FOR DAKOTA MINNESOTA & EASTERN RAILROAD DOING BUSINESS AS CANADIAN PACIFIC	LS	1.00						1.00					
59	2595-0000013	LIABILITY INSURANCE, MODIFIED, FOR DAKOTA MINNESOTA & EASTERN RAILROAD DOING BUSINESS AS CANADIAN PACIFIC	LS	1.00						1.00					
60	2599-9999005	('EACH' ITEM) CLEANING OF ALL TEMPORARY TRAFFIC CONTROL SIGNS AND DEVICES	EACH	20						20					
61	2599-9999005	('EACH' ITEM) REMOVAL OF SILT BASINS	EACH	1						1					
62	2599-9999005	('EACH' ITEM) STEEL BEAM GUARDRAIL SPECIAL END ANCHOR	EACH	1						1					
63	2599-9999007	('HOURS' ITEM) TEMPORARY TRAFFIC SIGNAL OPERATOR	HOUR	1200						1200					
64	2599-9999018	('SQUARE YARDS' ITEM) STANDARD SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 16 IN.	SY		381.2					381.2					
65	2601-2643412	TURF REINFORCEMENT MAT, TYPE 2	SQ	4.0						4.0					
66	2601-2700010	OUTLET OR CHANNEL SCOUR PROTECTION	SF	32						32					
67	2602-0000020	SILT FENCE	LF	1,350.0						1,350.0					
68	2602-0000030	SILT FENCE FOR DITCH CHECKS	LF	352.5						352.5					
69	2602-0000050	SILT BASINS	EACH	1						1					
70	2602-0000071	REMOVAL OF SILT FENCE OR SILT FENCE FOR DITCH CHECKS	LF	657.5						657.5					
71	2602-0000101	MAINTENANCE OF SILT FENCE OR SILT FENCE FOR DITCH CHECK	LF	131.5						131.5					
72	2602-0000212	FLOATING SILT CURTAIN (HANGING)	LF	400.0						400.0					
73	2602-0000312	PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE, 12 IN. DIA.	LF	200.0						200.0					
74	2602-0000320	PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE, 20 IN. DIA.	LF	200.0						200.0					
75	2602-0000350	REMOVAL OF PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE	LF	400.0						400.0					
76	2602-0010010	MOBILIZATIONS, EROSION CONTROL	EACH	1						1					
77	2602-0010020	MOBILIZATIONS, EMERGENCY EROSION CONTROL	EACH	1						1					
78	2102-0425070	ALT 'AA' OPTION 1: HMA PAVED SHOULDER SPECIAL BACKFILL	TON	442.2											
79	2122-5500000	PAVED SHOULDER, HOT MIX ASPHALT MIXTURE, 8 IN.	SY	854.8											
80	2102-0425070	ALT 'AA' OPTION 2: PCC PAVED SHOULDER SPECIAL BACKFILL	TON	486.9											
81	2122-5190007	PAVED SHOULDER, P.C. CONCRETE, 7 IN.	SY	854.8											

Design No. 514  
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**ESTIMATE REFERENCE INFORMATION**

Item No.	Item Code	Description
1	2101-0850001	<b>CLEARING AND GRUBBING</b> Includes clearing and grubbing required for the bridge replacement.
-	-	-
2	2102-0425070	<b>SPECIAL BACKFILL</b> Refer to the Typical on Sheets B.1-B.5 and Tabs. 100-24 and Curb & Gutter for locations and details. Also, refer to the Linwood Mine Crossing typical on Sheet B.3 and Tab. 102-3.  Quantity includes 621.1 tons of material for detour pavement. Quantity also includes 278.4 tons of material for final curb subbase. Quantity also includes 240.2 tons of material for the Linwood Mine Crossing.
-	-	-
3	2102-2625000	<b>EMBANKMENT-IN-PLACE</b> Refer to 'T' sheets.  Quantity includes 560 cu. yds. of Class 10 Suitable Contractor borrow material for roadway. Also includes 38 cu. yds. of borrow material for steel beam guardrail installations. Refer to Tab. 107-23 for locations and details. Shrink is not included for this item.  Special attention should be given to Section 2107.03.C of the current Standard Specifications on this project.  Overhaul will not be measured or paid for, but shall be considered incidental to roadway excavation on this project.
-	-	-
4	2102-2710090	<b>EXCAVATION, CLASS 10, WASTE</b> Refer to the 'T' Sheets for locations and details. All class 10 excavation material to become property of the Contractor.  All excavated material shall not be used as fill on for the project.
-	-	-
5	2102-2712015	<b>EXCAVATION, CLASS 12, BOULDERS OR ROCK FRAGMENTS</b> Refer to Tab. 103-7 for additional information.  Dispose of excess material according to Article 1106.07 of the current specifications.
-	-	-
6	2102-2713090	<b>EXCAVATION, CLASS 13, WASTE</b> Refer to Typical SHOULDER on Sheet B.4 and Tab. 112-9 for locations and details.
-	-	-
7	2105-8425005	<b>TOPSOIL, FURNISH AND SPREAD</b> Refer to Tab. 103-4.  The Contractor shall provide all the required topsoil and follow provisions in Section 2105 of the current specifications.  Method of Measurement: The quantity of topsoil furnished and spread will be measured in cubic yards and will be computed on the depth of topsoil specified in the contract document over the area involved. Sufficient field measurements will be taken to assure reasonable conformity with the required final thickness of topsoil in place.  Basis of Payment: The Contractor will be paid the contract unit price for topsoil, furnish and spread per cubic yard of topsoil placed, measured as provided above.  Overhaul will not be paid for on this item. Shrink is not included for this item.
-	-	-
8	2107-0875100	<b>COMPACTION WITH MOISTURE CONTROL</b> Refer to Tab. 103-6 and the 'T' sheets for locations and details.  Cubic yards shown on the contract documents as determined by the template fill volume. Shrinkage will not be included in the moisture control quantity.
-	-	-
9	2113-0001100	<b>SUBGRADE STABILIZATION MATERIAL, POLYMER GRID</b> Refer to the Typical on Sheet B.1 for locations and details. Material to be placed below all 10" Modified Subbase.  Refer to the Typical for the Linwood Mine Crossing on Sheet B.3 for locations and details. Material is to be placed beneath the 12" Special Backfill subbase under the 16" PCC pavement.
-	-	-
10	2115-0100000	<b>MODIFIED SUBBASE</b> Refer to Typical on Sheets B.1 and B.3 and Tab. 100-24 for locations and details.
-	-	-
11	2122-5190501	<b>PAVED SHOULDER, PORTLAND CEMENT CONCRETE (PAVED SHOULDER PANEL FOR BRIDGE END DRAIN)</b> Refer to Tab. 104-8A for locations and details.
-	-	-
12	2123-7450000	<b>SHOULDER CONSTRUCTION, EARTH</b> Refer to Typical on the B Sheets and Tab. 112-9 for locations and details. At least 4" of topsoil shall be placed on permanent shoulders.

Design No. 514  
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**ESTIMATE REFERENCE INFORMATION**

Item No.	Item Code	Description
13	2123-7450020	SHOULDER FINISHING, EARTH Refer to the Typical on the B Sheets, Tab. 100-24, and Tab. Curb & Gutter for locations and details.  Stage 1 includes 11.5 STAs of construction. Stage 2 includes 2.8 STAs of construction. Stage 3 includes 9.8 STAs of construction. Stage 4 includes 2.8 STAs of construction.  Material for the earth shoulder finishing is included in the template quantity.
14	2213-6745500	REMOVAL OF CURB Refer to Tab. 110-4 for locations and details.
15	2301-0690200	BRIDGE APPROACH, RK-20 Refer to Tab. 112-6 for locations and details.
16	2301-1033100	STANDARD OR SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 10 IN. Refer to Tab. 100-24 for locations and details.
17	2301-1033110	STANDARD OR SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 11 IN. Refer to Tab. 100-24 for location and details.
18	2301-6911722	PORTLAND CEMENT CONCRETE PAVEMENT SAMPLES
19	2303-0053503	HOT MIX ASPHALT MIXTURE (10,000,000 ESAL), SURFACE COURSE, 1/2 IN. MIX, FRICTION L-3
20	2303-0246428	ASPHALT BINDER, PG 64-28 Refer to Typical on Sheet B.1 and Tab. 100-25 for locations and details.
21	2304-0100000	DETOUR PAVEMENT Refer to the Detour Typical on the B Sheets and Tab. 100-24 for locations and details.
22	2315-8275025	SURFACING, DRIVEWAY, CLASS A CRUSHED STONE Refer to Tab. 102-3 for locations and details.
23	2416-0100024	APRONS, CONCRETE, 24 IN. DIA.
24	2435-0140148	MANHOLE, STORM SEWER, SW-401, 48 IN.
25	2435-0250800	INTAKE, SW-508
26	2435-0250900	INTAKE, SW-509 Refer to Tab. 104-5B and the 'M' sheets for locations and details.
27	2502-8212034	SUBDRAIN, LONGITUDINAL, (SHOULDER) 4 IN. DIA.
28	2502-8220193	SUBDRAIN OUTLET (RF-19C)
29	2502-8220196	SUBDRAIN OUTLET, RF-19E Refer to Tab. 104-9 for locations and details.
30	2503-0114215	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 2000D (CLASS III), 15 IN.
31	2503-0114224	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 2000D (CLASS III), 24 IN. Refer to Tab. 104-5B and the 'M' sheets for locations and details.
32	2503-0114424	STORM SEWER GRAVITY MAIN, TRENCHED, REINFORCED CONCRETE PIPE (RCP), 3000D (CLASS IV), 24 IN. Refer to Tab. 104-5B and the 'M' sheets for locations and details.  Connected pipe joints will be used for this pipe. Placement of pipe will need to be coordinated with the construction of the west abutment piling.
33	2503-0200036	REMOVE STORM SEWER PIPE LESS THAN OR EQUAL TO 36 IN.
34	2503-0200341	STORM SEWER ABANDONMENT, FILL AND PLUG, LESS THAN OR EQUAL TO 36 IN. DIA. Refer to Tab. 110-14 and the 'M' Sheets for locations and details.
35	2505-4008120	REMOVAL OF STEEL BEAM GUARDRAIL Refer to Tab. 110-7A for locations and details.  Remove guardrail adjacent to EB lanes in Stage 2. Remove guardrail adjacent to WB lanes in Stage 3.
36	2505-4008300	STEEL BEAM GUARDRAIL
37	2505-4008400	STEEL BEAM GUARDRAIL BARRIER TRANSITION SECTION
38	2505-4021010	STEEL BEAM GUARDRAIL END ANCHOR, BOLTED
39	2505-4021700	STEEL BEAM GUARDRAIL END TERMINAL Refer to Tab. 108-8A for locations and details.
40	2507-3250005	ENGINEERING FABRIC
41	2507-8029000	EROSION STONE Refer to Tab. 100-23 for locations and details.
42	2510-6745850	REMOVAL OF PAVEMENT Refer to Tabs. 110-1 and 102-5 and the As-Built Typical on Sheet B.7 for locations and details.

**ESTIMATE REFERENCE INFORMATION**

Item No.	Item Code	Description
43	2510-6750600	REMOVAL OF INTAKES AND UTILITY ACCESSES Refer to Tab. 110-15 and the 'M' Sheets for locations and details.  Item includes two 4 sq. yd. steel plates to cover the intakes during Stages 1-2. Refer to Tab. 100-26 for locations and details.
44	2512-1725206	CURB AND GUTTER, P.C. CONCRETE, 2.0 FT. Refer to the Typical on the B Sheets and Tab. Curb & Gutter for locations and details.
45	2515-2475008	DRIVEWAY, P.C. CONCRETE, 8 IN. Refer to Tab. 102-3 for locations and details.
46	2515-6745600	REMOVAL OF PAVED DRIVEWAY Refer to Tab. 110-8 for location and details. Requires 35.2 linear foot of full depth saw cut.
47	2518-6910000	SAFETY CLOSURE Refer to Tab. 108-13A for locations and details.
48	2520-3350015	FIELD OFFICE
49	2527-9263109	PAINTED PAVEMENT MARKING, WATERBORNE OR SOLVENT-BASED
50	2527-9263180	PAVEMENT MARKINGS REMOVED Refer to Tab. 108-22 for locations and details.
51	2528-8400048	TEMPORARY BARRIER RAIL, CONCRETE Refer to the J Sheets and Tab. 108-33 for locations and details. Also see Typical TBR_FLARE on Sheet B.5.
52	2528-8400256	TEMPORARY TRAFFIC SIGNALS Refer to Tab. 108-28 for locations and details. Maintain a minimum clearance of 16.5' from the top of pavement to the bottom of each signal.
53	2528-8445110	TRAFFIC CONTROL Cleaning of temporary traffic control signs and devices will be paid separately.
54	2528-8445113	FLAGGERS
55	2528-9109020	TEMPORARY LANE SEPARATOR SYSTEM Refer to Tab. 108-35 for locations and details.
56	2551-0000110	TEMP CRASH CUSHION Refer to the J Sheets and Tab. 108-30 for locations and details.
57	2552-0000140	ROCK EXCAVATION Quantity includes rock excavation for storm sewer pipe construction. Refer to 'M' Sheets for locations and details.
58	2595-0000012	INSURANCE WHEN WORKING IN RAILROAD RIGHT-OF-WAY, MODIFIED, FOR DAKOTA MINNESOTA & EASTERN RAILROAD DOING BUSINESS
59	2595-0000013	LIABILITY INSURANCE, MODIFIED, FOR DAKOTA MINNESOTA & EASTERN RAILROAD DOING BUSINESS AS CANADIAN PACIFIC
60	2599-9999005	('EACH' ITEM) CLEANING OF ALL TEMPORARY TRAFFIC CONTROL SIGNS AND DEVICES Item is for cleaning all temporary traffic control signs and devices. Cleaning shall be per Article 2528.03, L, 6, of the Standard Specifications or as directed by the Engineer.  Method of Measurement: Each cleaning of all temporary traffic control signs and devices will be counted.  Basis of Payment: Payment is full compensation for furnishing all materials, labor, and equipment needed to clean all temporary traffic control signs and devices.

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**ESTIMATE REFERENCE INFORMATION**

Item No.	Item Code	Description
61	2599-9999005	<p>( 'EACH' ITEM) REMOVAL OF SILT BASINS                      Fill the silt basin with Class 10 material and a minimum of 4 inches of topsoil. Furnish, place, and compact Class 10 material according to Section 2107 of the current Standard Specifications for Type A compaction. Furnish, place, and compact topsoil according to Section 2105 of the Standard Specifications. Smooth the surface of the topsoil and leave in a finished condition so that it will drain properly. This will be incidental to the bid item.</p> <p>Method of Measurement:                      The number of silt basins removed will be counted by the Engineer.</p> <p>Basis of Payment:                      The Contractor will be paid for the number of silt basins removed. Payment is full compensation for providing, preparing, transporting, and placing the Class 10 material and topsoil. The Contractor may strip and stockpile the topsoil and Class 10 material from constructing the silt basin for later use in removing the silt basin. Overhaul will not be paid for this item.</p>
62	2599-9999005	<p>( 'EACH' ITEM) STEEL BEAM GUARDRAIL SPECIAL END ANCHOR                      Refer to Sheet U.1 for details.</p> <p>A portion of the existing WB guardrail will be removed to construct the proposed temporary intake at Sta. 293+00.45. This item will be installed to the next section of existing guardrail that remains. The anchor will need to match the existing height of the guardrail.</p> <p>Method of Measurement: The Engineer will count the quantity of each type of beam guardrail end anchors and terminal devices constructed. Installations continued across a bridge will not be counted as end anchors.</p> <p>Basis of Payment: The Contractor will be paid the contract unit price for each type of beam guardrail end anchor or terminal device.</p>
63	2599-9999007	<p>( 'HOURS' ITEM) TEMPORARY TRAFFIC SIGNAL OPERATOR                      Item is for controlling the temporary traffic signals at the Linwood Mining entrance (Sta. 290+77). Article 2528.03, H, 4, of the Standard Specifications shall apply.</p> <p>Method of Measurement:                      By count for duration, in hours, needed to control the temporary traffic signals during mining operations or as directed by the Engineer.</p> <p>Basis of Payment:                      Payment is full compensation for trained traffic signal operators.</p>
64	2599-9999018	<p>( 'SQUARE YARDS' ITEM) STANDARD SLIP FORM PORTLAND CEMENT CONCRETE PAVEMENT, CLASS C, CLASS 3 DURABILITY, 16 IN.                      Refer to Tab. 100-24 for locations and details.</p> <p>Refer to Section 2301 of the current Standard Specifications for details and information, including Method of Measurement and Basis of Payment.</p>
65	2601-2643412	<p>TURF REINFORCEMENT MAT, TYPE 2                      Refer to Tab. 104-8A for locations and details.</p>
66	2601-2700010	<p>OUTLET OR CHANNEL SCOUR PROTECTION                      Refer to Tab. 104-8A for locations and details.</p> <p>Method of Measurement will be in square feet of actual area covered. Basis of Payment will include all material, equipment, and labor needed to install the Scourstop as defined by Standard Road Plan RF-39.</p>
67	2602-0000020	<p>SILT FENCE                      Refer to Tab. 100-17.                      The tabulation includes estimated locations for placement of "Silt Fence" to address erosion to be encountered during construction. Verify the specific locations with the Engineer prior to beginning placement. Bid item includes 25% additional quantity for field adjustments and replacements.</p>
68	2602-0000030	<p>SILT FENCE FOR DITCH CHECKS                      Refer to Tab 100-18.                      The tabulation includes estimated locations for placement of "Silt Fence for Ditch Checks" to address erosion to be encountered during construction. Verify the specific locations with the Engineer prior to beginning placement. Bid item includes 50% additional quantity for field adjustments and replacements.</p>
69	2602-0000050	<p>SILT BASINS                      Refer to Tab. 100-14.</p> <p>The tabulation includes estimated locations for placement of "Silt Basins" to address erosion to be encountered during construction. Verify the specific locations with the Engineer prior to beginning placement. Bid item includes 100% additional quantity for field adjustment and maintenance.</p>

**ESTIMATE REFERENCE INFORMATION**

Item No.	Item Code	Description
70	2602-0000071	<p>REMOVAL OF SILT FENCE OR SILT FENCE FOR DITCH CHECKS                      This item is included for silt fence and silt fence for ditch check removal required for staging reasons, removal to allow for replacement (replacement to be paid separately), or for areas that have achieved 70% permanent growth.</p>
71	2602-0000101	<p>MAINTENANCE OF SILT FENCE OR SILT FENCE FOR DITCH CHECK                      This item is included for clean-out and repair of the silt fence and silt fence for ditch checks during the project.</p>
72	2602-0000212	<p>FLOATING SILT CURTAIN (HANGING)                      Refer to Tab. 100-10 for locations and details.</p>
73	2602-0000312	<p>PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE, 12 IN. DIA.</p>
74	2602-0000320	<p>PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE, 20 IN. DIA.                      Refer to Standard Road Plan EC-204.</p> <p>Item is included for temporary perimeter sediment control, inlet protection, and water velocity reduction on slopes or ditches at locations to be determined during construction. Verify specific locations with the Engineer prior to beginning placement.</p>
75	2602-0000350	<p>REMOVAL OF PERIMETER AND SLOPE SEDIMENT CONTROL DEVICE                      Included for removal of perimeter and sediment control devices. All material shall become the property of the Contractor.</p> <p>Method of Measurement: Perimeter and Sediment Control Devices which have been properly removed will be measured in linear feet to the nearest foot.</p> <p>Basis of Payment: Payments for the length of Perimeter and Slope Sediment Control Devices which have been properly removed, the Contractor will be paid the contract unit price per foot.</p>
76	2602-0010010	<p>MOBILIZATIONS, EROSION CONTROL</p>
77	2602-0010020	<p>MOBILIZATIONS, EMERGENCY EROSION CONTROL</p>
78	2102-0425070	<p>ALT 'AA' OPTION 1: HMA PAVED SHOULDER                      SPECIAL BACKFILL</p>
79	2122-5500080	<p>PAVED SHOULDER, HOT MIX ASPHALT MIXTURE, 8 IN.                      Refer to B Sheets and Tab. 112-9 for locations and details.</p>
80	2102-0425070	<p>ALT 'AA' OPTION 2: PCC PAVED SHOULDER                      SPECIAL BACKFILL</p>
81	2122-5190007	<p>PAVED SHOULDER, P.C. CONCRETE, 7 IN.                      Refer to B Sheets and Tab. 112-9 for locations and details.</p>

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10-18-11

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100-14	SILT BASINS	C.13
100-17	TABULATION OF SILT FENCES	C.13
100-18	TABULATION OF SILT FENCES FOR DITCH CHECKS	C.13
100-23	ROCK DITCH CHECKS/DITCHES/FLUMES/SPLASH BASINS/SLOPE PROTECTION	C.13
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110-1	REMOVAL OF PAVEMENT	C.7
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110-14	SANITARY OR STORM SEWER ABANDONMENT OR REMOVAL	C.8
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112-6	BRIDGE APPROACH SECTION	C.10
112-9	SHOULDERS	C.12

**STANDARD ROAD PLANS**

105-4  
10-18-11

The following Standard Road Plans apply to construction work on this project.

Number	Date	Title
BA-200	10-18-11	Steel Beam Guardrail Components
BA-201	10-19-10	Steel Beam Guardrail Barrier Transition Section
BA-202	10-18-11	Steel Beam Guardrail Bolted End Anchor
BA-205	10-18-11	Steel Beam Guardrail End Terminal
BA-250	10-18-11	Steel Beam Guardrail Installation at Concrete Barrier or Bridge End Post
BA-401	04-16-13	Temporary Barrier Rail (Precast Concrete)
EC-101	04-20-10	Wood Excelsior Mat for Ditch Protection
EC-201	04-20-10	Silt Fence
EC-202	04-17-12	Floating Silt Curtain
EC-204	10-16-12	Perimeter and Slope Sediment Control Devices
EW-202	04-17-12	Bridge Berm Grading without Recoverable Slope (Non-Barnroof Section)
EW-301	04-19-11	Guardrail Grading
EW-401	10-15-13	Temporary Stream Crossing, Causeway, or Equipment Pad
EW-403	10-15-13	Temporary Erosion Control Measures
MI-210	10-15-13	PCC Driveways and Alleys
PM-110	04-16-13	Line Types
PV-101	10-15-13	Joints
PV-102	04-16-13	PCC Curb Details
RF-2	10-18-11	Construction of Type "C" Concrete Adaptors for Pipe Culvert Connections
RF-3	10-15-13	Concrete Aprons
RF-14	04-16-13	Connected Pipe Joints
RF-40	10-15-13	Rock Flume for Bridge End Drain
RK-20	04-16-13	Double Reinforced 12" Approach
RK-21	10-16-12	Bridge Approach (abutting PCC or Composite Pavement)
RM-37	10-21-08	Junction Box (Cast Iron)
SI-173	04-20-10	Object Markers
SI-211	10-19-10	Object Marker and Delineator Placement with Guardrail
SW-211	10-16-12	Special Pipe Connections for Storm Sewer
SW-401	04-21-09	Circular Storm Sewer Manhole
SW-508	10-20-09	Single Open-Throat Intake, Large Box
SW-509	10-18-11	Double Open-Throat Curb Intake, Small Box
TC-1	04-16-13	Work Not Affecting Traffic (Two-Lane or Multi-Lane)
TC-213	04-17-12	Lane Closure with Flaggers
TC-402	10-15-13	Shoulder Closure (Multi-Lane)
TC-419	10-15-13	Lane Closure on Undivided Highway
TC-423	10-16-12	Closure of Two Adjacent Lanes on Undivided Highway

SN-1

**STEEL PLATE**

A 1" thick steel plate shall be placed over the wells of the existing intakes at Sta. 290+56 and Sta. 292+97 for protection before placing detour pavement during Stage 1. These plates shall be removed during later stages when the adjacent pavement is placed. The placement and removal of the steel plate is incidental to the Removal of Intakes and Utility Accesses bid item. Refer to Tab. 100-26 for additional information.

232-3C  
Modified

**EROSION CONTROL  
(NATIVE GRASS SEEDING)**

Following the completion of work in a disturbed area, place seed and mulch on the disturbed area lying 8 feet or more beyond the shoulder as follows:

SEEDING MIXTURE:

Big bluestem (Andropogon gerardii)	6 lbs. PLS/Acre (7.0 kg/ha)
Indiangrass (Sorghastrum nutans)	6 lbs. PLS/Acre (7.0 kg/ha)
Little bluestem (Schizachyrium scoparium)	6 lbs. PLS/Acre (7.0 kg/ha)
Partridge Pea (Chamaecrista fasciculata)	4 lbs. PLS/Acre (4.5 kg/ha)
Sideoats grama (Bouteloua curtipendula)	4 lbs. PLS/Acre (4.5 kg/ha)
Canada wildrye (Elymus canadensis)	2 lbs. PLS/Acre (2.2 kg/ha)
Switchgrass (Panicum virgatum)	1 lbs. PLS/Acre (1.1 kg/ha)
Oats (Avena sativa)	32 lbs./Acre (36.0 kg/ha)

Big bluestem, Indiangrass, Canada wildrye and Little bluestem shall be debarbed or equal to facilitate the application of seed.

Use mulch meeting the requirements of Sections 2601.03,E,2,a and 4169.07,A of the Standard Specifications.

Preparing the seedbed and furnishing and applying seed and mulch is incidental to mobilization and will not be paid for separately.

262-6  
10-18-05

**UTILITIES  
(NOT A POINT 25 PROJECT)**

This is NOT a POINT 25 project and is not subject to the provisions of IAC 761-115.25.

281-1  
10-15-13

**SECTION 404 PERMIT AND CONDITIONS**

Construct this project according to the requirements of U.S. Army Corps of Engineers MWP #14, Permit No. 2013-1410. A copy of this permit is available from the Iowa DOT website (<http://envpermits.iowadot.gov/CMEPortalENV/Home.aspx>). The U.S. Army Corps of Engineers reserves the right to visit the site without prior notice.

232-10  
Modified

**EMERALD ASH BORER**

Dispose of all wood material generated as a result of clearing and/or grubbing according to the Iowa Department of Agriculture and Land Stewardship's Emerald Ash Borer (EAB) Quarantine Order. For more information refer to [http://www.iowatreepests.com/eab\\_regulations.html](http://www.iowatreepests.com/eab_regulations.html).

232-11  
Modified

**EROSION CONTROL  
(STABILIZING CROP SEEDING)**

Following the completion of work in a disturbed area, place stabilizing crop, fertilizer, and mulch on the disturbed area as follows:

Use seed mix and fertilizer meeting the requirements of Section 2601.03,C,1 of the Standard Specifications.

Use mulch meeting the requirements of Sections 2601.03,E,2,a and 4169.07,A of the Standard Specifications.

Preparing the seedbed and furnishing and applying seed, fertilizer, and mulch is incidental to mobilization and will not be paid for separately.

232-3A  
Modified

**EROSION CONTROL  
(RURAL SEEDING)**

Following the completion of work in a disturbed area, place seed, fertilizer, and mulch on the disturbed area lying 8 feet adjacent to shoulder and median as follows:

Use seed mix and fertilizer meeting the requirements of Section 2601.03,C,3 of the Standard Specifications.

Use mulch meeting the requirements of Sections 2601.03,E,2,a and 4169.07,A of the Standard Specifications.

Preparing the seedbed and furnishing and applying seed, fertilizer, and mulch is incidental to mobilization and will not be paid for separately.

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**EXISTING PAVEMENT**

No.	Location					Year	Type	Project Number	Surface		Base		Subbase		Removal		Coarse Aggregate			Reinforcement	Remarks
	County	Route	Dir. of Travel	Begin Milepost	End Milepost				Type	Depth	Type	Depth	Type	Depth	Type	Depth	Source	Type	Durability Class	Type	
1	82	22	1	92.02	93.65	2010 1974		STPN-022-6(27)--23-82 FN-22-5(2)--21-82	HMA PCC	2 9				SCR	0.5		LINWOOD		C. Lst	I	

**REMOVAL OF PAVEMENT**

110-1  
04-16-13

Refer to Tabulation 102-5

\* Not a Bid Item

Begin Station	End Station	Side	Pavement Type	Area	Saw Cut*	Remarks
				SY	LF	
<b>IA 22 - Existing Pavement</b>						
291+01.00	298+15.00	Both	PCC/HMA	3662.2	86.6	
287+60.00	300+41.50	Lt	PCC		1286.5	Curb Removal; See Tab. 110-4
287+57.00	290+45.11	Rt	PCC		289.3	Curb Removal; See Tab. 110-4
<b>IA 22 - Detour Pavement</b>						
287+60.00	290+49.00	Lt	PCC	252.9		Remove in Stage 3
290+49.00	291+04.00	Lt	PCC	67.2		Remove in Stage 3; 11" Pavement
291+04.00	300+41.50	Lt	PCC	1168.6		Remove in Stage 3
287+70.00	290+45.11	Rt	PCC	243.6		Remove in Stage 4
Div. 1 Totals				5394.6	1662.4	
290+52.00	291+01.00	Both	PCC	321.2	93.0	Linwood Mine Crossing (Division 2)
Div. 2 Totals				321.2	93.0	

**CURB REMOVAL**

110-4  
08-01-08

Begin Station	End Station	Side	Length STA	Remarks
<b>IA 22</b>				
<b>Stage 1</b>				
287+60.00	290+36.50	Lt	2.8	
291+17.00	295+82.00	Lt	4.7	
296+21.00	300+41.50	Lt	4.2	
<b>Stage 2</b>				
287+70.00	290+43.85	Rt	2.7	
Total			14.4	

**REMOVAL OF CONCRETE DRIVES**

110-8  
08-01-08

Location Station	Side	Area SY	Remarks
296+00.70	Lt	138.6	35.2' of sawcut required
Total		138.6	

**PAVEMENT SMOOTHNESS + PCC TEXTURE**

100-27  
10-20-09

Road Identification	Begin Station	End Station	Proposed Posted Speed			Remarks
			35 or less	40 - 45	over 45	
IA 22	290+52.00	292+45.78			X	
	294+90.82	298+15.00			X	

**INCIDENTAL ITEMS**

100-26  
10-15-13

Special or unique items where method of measurement / basis of payment is not indicated in the specifications or other contract documents.

No.	Incidental Item	Unit	Quantity	Incidental To		Remarks
				Item Code	Item	
1	Steel Plate		1	2510-6750600	REMOVAL OF INTAKES AND UTILITY ACCESSES	(1)
2	Steel Plate		1	2510-6750600	REMOVAL OF INTAKES AND UTILITY ACCESSES	(1)

(1) Cover well with a Grade 36 steel plate to facilitate stage construction. See Note SN-1 on Sheet C.5 and Tab. 104-5B on Sheet M.1 for additional information. The intakes are located at Sta. 290+56 and Sta. 292+97.

**REMOVAL OF INTAKES AND UTILITY ACCESSES**

110-15  
04-16-13

No.	Location/Description	Type	Remarks
1	Sta. 290+56 Lt	Intakes	Remove in Stage 3
2	Sta. 292+97 Lt	Intakes	Remove Top in Stage 1 and Well in Stage 3
3	Sta. 293+00.45 Lt	Intakes	Remove in Stage 3
4	Sta. 295+50 Lt	Intakes	Remove in Stage 1
5	Sta. 298+80 Lt	Intakes	Remove in Stage 1

**ACCESS POINTS AND SAFETY RAMPS**

102-3  
10-15-13

Refer to Cross-Sections

Length of unclassified pipe calculated is based on using Reinforced Concrete Pipe.

① Refer to MI-210

② Refer to EW-501.

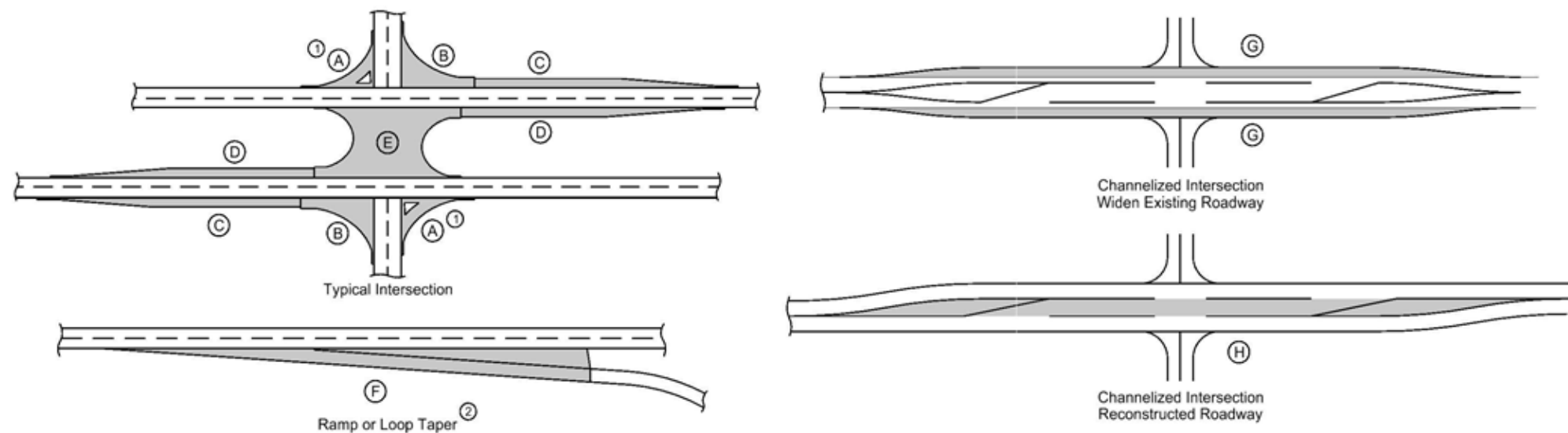
③ Refer to EW-501 or EW-502.

\*Predetermined for access point not constructed with this project.

Location		Type	Length of Opening ①			Pipe Culvert ③			Aprons	Driveway Surface Area		Driveway Surfacing Material	Remarks			
Station	Side	A, B, C, Safety Ramp, or Predetermined*	Case	1 1/2" Dropped Curb	3" Dropped Curb	W	① PR	② SR		H	Size			Pipe Length	Lt.	Rt.
			1 or 2	LF	LF	FT	FT	FT		FT	IN			LF	LF	LF
<b>Stage 1</b>																
290+77.00	Lt	B				49.0										
296+00.70	Lt	B	2	72.0		35.0										
<b>Stage 3</b>																
290+77.00	Lt	B				49.0										
296+00.70	Lt	B	1	79.6		35.0	30.0									
290+77.00	Rt	B				49.0										

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HMA PAVEMENT



- ① Does not include island area or curb. Refer to tabulation 112-4 for quantities.
- ② Refer to PV-410, PV-411, PV-412, and PV-414.

Calculations assume a surface course unit weight (lbs/cf) of 147, an intermediate course unit weight (lbs/cf) of 8, a base course unit weight (lbs/cf) of 8, and a special backfill unit weight (lbs/cf) of 140.

Road Identification	Direction of Travel	Location Station to Station	Mainline		Area								Hot Mix Asphalt Pavement						Remarks						
			Width FT	Length FT	Area SY	A <sup>①</sup> SY	B SY	C SY	D SY	E SY	F SY	G SY	H SY	Surface			Binder								
														Surface TONS	Intermediate SY	Base TONS	Surface TONS	Intermediate TONS		Base TONS	Special Backfill TONS	Modified Subbase CY	Granular Subbase SY		
IA 22	Both	291+01.00 to 292+45.78	47.0	144.8	756.1									83.4	756.1				5.001						
	Both	294+00.82 to 298+15.00	47.0	324.2	1692.9									186.6	1692.9				11.199						
<b>Totals</b>													<b>270.0</b>	<b>2449.0</b>				<b>16.200</b>							

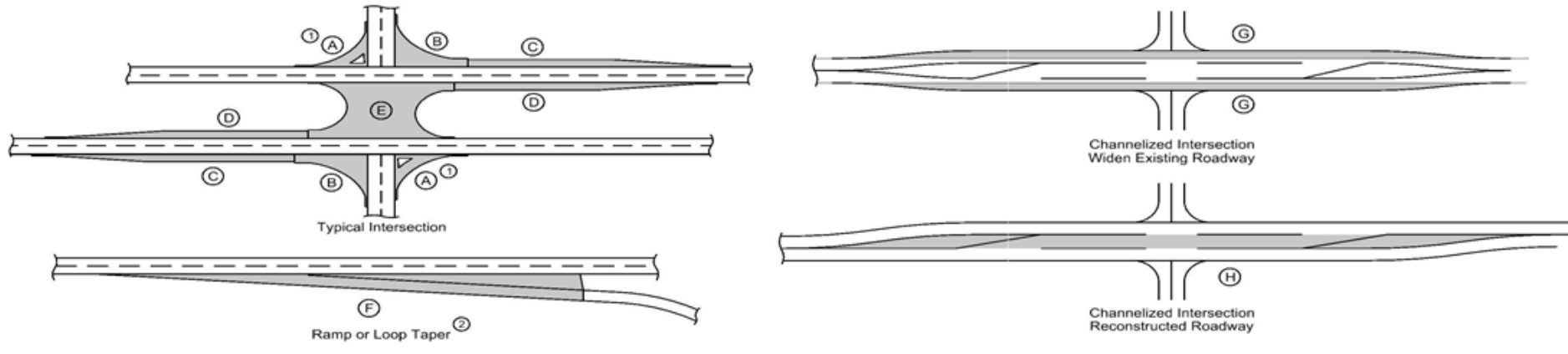
SANITARY OR STORM SEWER ABANDONMENT OR REMOVAL

\* Not a bid item

Location/Description	Sanitary or Storm Sewer	Abandonment, Plug Only or Abandonment, Plug and Fill or Removal	Length of Pipe		Fill Material*	Remarks
			≤ 36 inch diameter	> 36 inch diameter	Flowable Mortar or CLSM	
			LF	LF	CY	
IA 22						
Sta. 290+57.83 to 291+50.00; Lt	Storm Sewer	Removal	92			Stage 3
Sta. 291+50.00 to 292+92.68; Lt	Storm Sewer	Abandonment, Plug and Fill	143		175.0	
Sta. 293+01.83 to 294+45.00; Lt	Storm Sewer	Removal	143			
Sta. 294+45.00 to 295+25.00; Lt	Storm Sewer	Abandonment, Plug and Fill	80		98.1	
Sta. 295+25.00 to 295+47.62; Lt	Storm Sewer	Removal	23			
Sta. 291+33.00 to 293+51.45; Rt	Storm Sewer	Removal	219			
Sta. 293+00.43 Lt; Temporary Pipe T-1 (Stages 1-2)	Storm Sewer	Removal	8			
Sta. 298+82.40 to 298+86.40; Lt	Storm Sewer	Removal	4			Cap remaining 1.5' of existing pipe for Stages 1-2.
		Totals				
		Removal	489			
		Abandonment, Plug and Fill	223		273.1	

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PCC PAVEMENT



Road Identification	Location		Mainline			Area (3)								Total Area By Pavement Thickness		Special Backfill	Modified Subbase	Granular Subbase	Remarks			
	Direction of Travel	Station to Station	Width	Length	Area	A	B	C	D	E	F	G	H	SY						TONS	CY	SY
														10 IN	8 IN							
IA 22 - Final Pavement	WB/EB	291+01.00	291+90.38	49.0	89.4	486.6									486.6		148.6					
	WB/EB	291+90.38	292+45.78	Var	55.4	318.7									318.7		98.8					
	WB	292+45.78	292+95.78	Var	50.0	50.2									50.2		18.6	Pavement adjacent to approach pav't				
	WB	294+40.82	294+70.00	11.5	29.2	37.4									37.4		13.1	Pavement adjacent to approach pav't				
	WB	294+70.00	294+90.82	Var	20.8	21.9									21.9		8.0	Pavement adjacent to approach pav't				
	WB/EB	294+90.82	295+22.70	Var	31.9	184.9									184.9		57.3					
	WB/EB	295+22.70	298+15.00	49.0	292.3	1591.4									1591.4		487.4					
Linwood Mine Crossing	WB/EB	290+52.00	291+01.00	49.0	49.0	266.8								*266.8	*168.1		*16" PCC Pav't; 12" Subbase					
Linwood Mine North Entrance		290+47.00	291+06.00	Var	59.0	60.0								*60	*37.8		*16" PCC Paved Entr.' 12" Subbase					
Linwood Mine South Entrance		290+52.00	291+01.00	10.0	49.0	54.4								*54.4	*34.3		*16" PCC Paved Entr.' 12" Subbase					
Total Final Pavement															2691.2		831.7					
16" PCC Pavement (Div. 2)															381.2		240.2					
IA 22 - Detour Pavement																						
Stage 1	WB	287+60.00	289+50.00	Var	190.0	131.9									131.9	48.2						
	WB	289+50.00	290+49.00	11.0	99.0	121.0									121.0	41.6						
	WB	290+49.00	291+04.00	11.0	55.0	67.2									*67.2	*46.2		*11" PCC Pav't; 12" Subbase				
	WB	291+04.00	298+50.00	11.0	746.0	911.8									911.8	313.3						
	WB	289+50.00	300+41.50	Var	1091.5	132.0									132.0	48.6						
		295+65.00	296+37.00	Var	72.0	124.8									124.8	36.8		Driveway Detour Pavement				
Stage 2	EB	287+70.00	289+50.00	Var	180.0	130.0									130.0	47.3						
	EB	289+50.00	290+38.23	11.0	88.2	107.8									107.8	37.1						
	EB	290+38.23	290+45.11	Var	6.9	5.7									5.7	2.1		Tie into existing return pavement				
8" Detour Pavement															1665.2		574.9					
11" PCC Pavement															67.2		46.2					
Total Special Backfill																	621.1					

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107-23  
10-18-11

### GRADING FOR GUARDRAIL INSTALLATIONS

① Lane(s) to which the installation is adjacent.

Refer to EW-301

No.	Direction of Traffic	Location			Foreslope at Guardrail	Dimensions (Feet)									Earthwork		Remarks		
		Station	Side			X1	Y1	X2	Y2	X3	Y3	X4	Y4	Z	Excavation Class 10	Embankment In Place			
																		CY	CY
1		293+03.78	Rt		10:1	77.5	5.0	149.8	7.5							48.5		28.0	
2		294+37.80	Lt		UAC	27.5	5.0	64.8	8.8	114.5	10.8	134.5	10.8			(1)		10.0	
																			(1) Tie-in with proposed entrance at Sta. 296+00

110-7A  
04-17-12

### REMOVAL OF STEEL BEAM GUARDRAIL

① Lane(s) to which the installation is adjacent.

② Includes length of End Terminals and End Anchors.

No.	Direction of Traffic	Location			Removal of Guardrail LF
		Station to Station	Side		
1		293+70.00	295+57.50	Rt	187.5
2		293+70.00	295+57.50	Lt	187.5
3		End Anchor	293+15.00	Lt	6.3
		Total			381.3

108-8A  
10-19-10

### STEEL BEAM GUARDRAIL AT CONCRETE BARRIER OR BRIDGE END POST

Refer to BA-200, BA-201, BA-202, BA-205, BA-250, SI-172, SI-173 and SI-211.

① See Standards for list of materials.

No.	Station	Offset	Layout Lengths				Delineators and Object Markers				Bid Items ①					Remarks										
			VF	VF	VT2	ET Terminal	Type	Delineator Type 1	Object Marker		End Anchor Bolted	Barrier Transition Section	Steel Beam Guardrail	End Terminal			Adapter									
									Type 2	Type 3				Standard	Flared for Cable Connection											
										White No.								OM-3L No.	OM-3R No.	BA-202 Type	BA-201 No.	BA-200 LF	BA-205 No.	BA-206 No.	BA-210 No.	
1	293+03.78	34.8' Rt	78.125										50.0	2				1								
2	294+37.80	34.8' Lt	28.125	37.50									50.0	2				1								
							Totals																			

104-8A  
04-20-10  
① Not a Bid Item

### SCOUR PROTECTION OR ROCK FLUME FOR BRIDGE END DRAIN

Refer to Standard Road Plan RF-39 or RF-40

Location		Shoulder			Rock Flume RF-40				Scour Protection RF-39			Remarks
Bridge Station	Bridge Corner	Distance DI-1 or DI-2	Panels Required	PCC	Polymer Grid	Modified Subbase	Macadam Stone Base	Engineering Fabric	Erosion Stone	Outlet or Channel Scour Protection	Turf Reinforced Mat (TRM)	
			A B C or D	Sq.Yds.	Sq.Yds.	Tons	Material Tons	Sq.Yds.	Tons	Sq. Feet	Squares	
293+68.30	SW	45.0	C, B	51.3	60.2	37.900				32.0	4.0	

112-6  
04-16-13

### BRIDGE APPROACH SECTION

Refer to the RK-Series.

\* Not a bid item

Location		Skew Ahead Degrees	Approach Pavement					Fixed or Movable Abutment	Subdrain							Remarks			
Bridge Station	End		T Thickness	Pay Length	Non-Reinf. Pavement Area	Single-Reinf. Pavement Area	Double-Reinf. Pavement Area		Perforated Subdrain 4"	Subdrain Outlet		Porous Backfill	Class 'A' Crushed Stone Backfill	Modified Subbase	Polymer Grid				
										LEFT	RIGHT						LF	STA	Side
										Inches	FT						SY	SY	SY
293+68.30	W	10	12.0	70.0	53.3	106.7	270.8	M	100.0	292+55.51	Rt	17.0							
293+68.30	E	10	12.0	70.0	53.3	106.7	269.1	M	100.0	294+81.09	Rt	17.0							

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### PAVEMENT MARKING LINE TYPES

See PM-110

\*BCY4 - Place on the same side of the roadway to match existing markings near the project.

\*\*\*NPY4 - Factor of 1.00 as value includes number of 4-inch passes to cover median nose area.

\*\*NPY4 - For estimating purposes only. No Passing Zone Lines will be located in the field.

BCY4: Broken Centerline (Yellow) @ 0.25

DCY4: Double Centerline (Yellow) @ 2.00

NPY4: No Passing Zone Line (Yellow) @ 1.25

BLW4: Broken Lane Line (White) @ 0.25

ELW4: Edge Line Right (White) @ 1.00

ELY4: Edge Line Left (Yellow) @ 1.00

Road ID	Station to Station		Dir. of Travel	Marking Type	Side			Length by Line Type (Unfactored)										Remarks					
					L	C	R	BCY4*	DCY4	NPY4**	BLW4	ELW4	ELY4										
					STA	STA	STA	STA	STA	STA	STA	STA	STA	STA	STA	STA	STA		STA	STA	STA	STA	
Stage 2																							
IA-22	283+07.48	313+00.00	WB	Removal Of Paint		X																	
	285+30.00	291+05.00	BOTH	Removal Of Paint		X			5.75														
	298+15.00	313+00.00	BOTH	Removal Of Paint		X			14.85														
	283+07.48	291+05.00	EB	Removal of Paint		X							7.98										
	298+15.00	304+90.00	EB	Removal of Paint		X							6.75										
	287+90.00	300+10.00	WB	Waterborne/Solvent Paint			X							12.20						Temporary Markings			
	302+90.00	313+00.00	WB	Waterborne/Solvent Paint		X									10.10					Temporary Markings			
	305+30.00	313+00.00	EB	Waterborne/Solvent Paint		X									7.70					Temporary Markings			
	283+07.48	304+90.00	EB	Waterborne/Solvent Paint			X							21.83						Temporary Markings			
Stage 3																							
IA-22	302+90.00	313+00.00	WB	Removal of Paint		X									10.10								
	313+00.00	314+00.00	WB	Removal of Paint			X																
	283+07.48	285+30.00	BOTH	Removal of Paint		X			2.23														
	305+30.00	313+00.00	EB	Removal of Paint		X																	
	283+07.48	304+90.00	EB	Removal of Paint			X																
	285+50.00	313+95.00	WB	Waterborne/Solvent Paint				X															
	283+07.48	285+30.00	WB	Waterborne/Solvent Paint			X																
	283+07.48	285+30.00	WB	Waterborne/Solvent Paint		X																	
	283+07.48	287+70.00	EB	Waterborne/Solvent Paint		X																	
	287+90.00	300+35.00	EB	Waterborne/Solvent Paint				X															
	300+35.00	304+90.00	EB	Waterborne/Solvent Paint				X															
Stage 4																							
IA-22	285+50.00	313+95.00	WB	Removal of Paint				X															
	283+07.48	285+30.00	WB	Removal of Paint		X																	
	283+07.48	287+70.00	EB	Removal of Paint		X																	
	290+46.11	300+35.00	EB	Removal of Paint				X															
	285+30.00	313+95.00	WB	Waterborne/Solvent Paint			X																
	283+07.48	314+00.00	BOTH	Waterborne/Solvent Paint			X																
	283+07.48	305+55.00	EB	Waterborne/Solvent Paint			X																
	291+30.00	300+35.00	EB	Waterborne/Solvent Paint				X															
Factored Total: Waterborne/Solvent Paint									-	61.85	-	13.34	88.53	24.65	-	-	-	-	-	-			
Factored Total: Removal of Paint									-	45.65	-	11.41	60.16	24.65	-	-	-	-	-	-			
Bid Quantity: Painted Pavement Markings, Waterborne or Solvent-Based													188.36										
Bid Quantity: Pavement Markings Removed													141.88										

### CRASH CUSHIONS

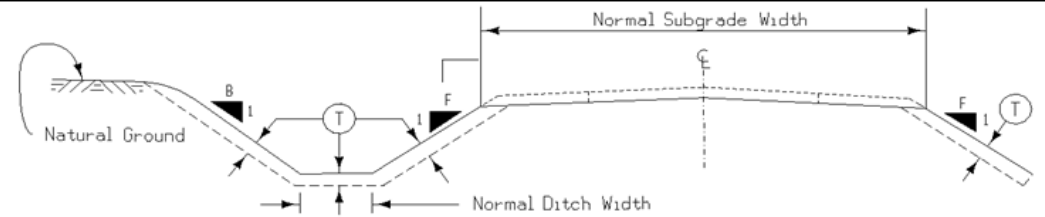
- \* Bid Item
- ① Lane(s) to which the installation is adjacent.
- ② Complete this section when using the Temporary Crash Cushion bid item and Earthwork is needed for Sand Barrel placement. Refer to BA-500

No.	Direction of Traffic	Location Station	Side	Obstacle Width FT	Crash Cushion (Select One)*					Sand Barrel Details ②					Earthwork*		Spare Parts Kit (Select One)*		Obstacle Description	Remarks
					Temporary	Temporary Redirective	Temporary Severe Use	Permanent	Permanent Severe Use	V	W	X	Y	Z	Excavation Class 10	Embankment in Place	Permanent	Permanent Severe Use		
					Length	Length	Length	Length	Length	Length	Length	Length	Length	Length	CY	CY	EACH	EACH		
Stage 2																				
1	EB	291+60.00	Rt	1.88	X															
Stage 3																				
2	WB	295+25.00	Rt	1.88	X															
3	WB	298+50.00	Rt	1.88	X															

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**TABULATION OF SPREADING TOPSOIL**



Perform this work according to Section 2105. Prior to placing topsoil on any cohesive soil, scarify the area to be covered to a minimum depth of 3 inches.

Appropriate adjustments have been made in the template quantities to reflect the placement of topsoil on foreslope, backslope and ditch bottom as detailed hereon.

Placement Description							Topsoil Excavation Available From			
Area	Quantity	Location		Side	Slope	(T)	Remarks	Amount Reserved	Station to Station	Remarks
No.	CY	Station to Station		L. or R.	B. or F.	IN		CY		
1	49.0	291+06.59	298+58.00	R	F	6.0	Stage 2 - Final Construction			Provide by Contractor
2	193.0	288+54.00	304+65.00	L	F	6.0	Stage 3 - Final Construction			
3	28.0	287+57.00	290+43.85	R	F	6.0	Stage 4 - Final Construction			
4	25.0	298+58.00	299+60.00	R	F	6.0	Stage 4 - Final Construction			
<b>Total</b>	<b>295.0</b>									

**TABULATION OF SILT FENCES**

Refer to EC-201

Location		Side	Length LF	Remarks
Begin Station	End Station			
IA 22				
Stage 2				
291+20.00	292+10.00	Rt	110.0	
294+00.00	296+00.00	Rt	220.0	
296+00.00	297+30.00	Rt	150.0	
297+30.00	298+50.00	Rt	140.0	
299+25.00	299+75.00	Rt	70.0	
Stage 3				
294+25.00	294+75.00	Lt	70.0	
301+85.00	303+35.00	Lt	170.0	
303+35.00	304+65.00	Lt	150.0	
<b>Total</b>			<b>1080.0</b>	
<b>Total + 25%</b>			<b>1350.0</b>	

**TABULATION OF SILT FENCES FOR DITCH CHECKS**

Refer to EC-201

Location Station	Side	Length LF	Remarks
IA 22			
Stage 2			
292+70.00	Rt	24.0	
293+15.00	Rt	15.0	Adjacent to Bridge Berm
293+80.00	Rt	15.0	Adjacent to Bridge Berm
295+30.00	Rt	21.0	
296+80.00	Rt	29.0	
298+58.00	Rt	29.0	
299+25.00	Rt	29.0	
Stage 3			
294+50.00	Lt	25.0	Adjacent to Bridge Berm
302+00.00	Lt	24.0	
303+50.00	Lt	24.0	
<b>Total</b>		<b>235.0</b>	
<b>Total + 50%</b>		<b>352.5</b>	

\* Design shown for mandatory locations is the minimum allowed.

**ROCK DITCH CHECKS/DITCHES/FLUMES/SPLASH BASINS/SLOPE PROTECTION**

Refer to Typical 4401, 4402, 4403, 4404, and 4405

Location		Type										Remarks		
Road Identification	Station	Side	Mandatory* Location (yes or no)	Rock Ditch Check	Rock Ditch	Rock Flume	Rock Splash Basin	Rock Slope Protection	(L)	(W)	Erosion Stone		Class E Revetment	Eng. Fabric
		Lt./Rt.							FT	FT	TON		TON	SY
Stages 1 & 2														
IA 22	294+80.00	Lt.	Yes			X				40.0		48.0		62.2
	298+75.00	Lt.	Yes			X				30.0		36.0		46.7
<b>Totals</b>											<b>84.0</b>		<b>108.9</b>	

**CURB & GUTTER**

Refer to PV-102 and 6000s Detail Series.

Curb and Gutter							Remarks
Station	Station	Offset	Curb Type	Gutter Width	Length (1) LF	Special Backfill (1) TON	
287+60.00	287+65.00	Lt	Transition to Existing	2	5.0	2.1	
287+65.00	290+52.00	Lt	6" Sloped PCC	2	287.0	100.5	
298+15.00	300+36.50	Lt	6" Sloped PCC	2	221.5	77.5	
300+36.50	300+41.50	Lt	Transition to Existing	2	5.0	8.9	
287+70.00	287+75.00	Rt	Transition to Existing	2	5.0	1.8	
287+75.00	290+25.40	Rt	6" Sloped PCC	2	250.4	87.6	
290+25.40	290+52.00	Rt	Transition to Existing	2	26.6	18.7	See typical on Sheet B.8
<b>Total</b>					<b>800.5</b>	<b>278.4</b>	

**FLOATING SILT CURTAINS**

Refer to EC-202

Station	Hanging LF	Containment LF	Clean-out (Containment) LF	Remarks
293+68.30	400.0			

**SILT BASINS**

Refer to EW-403

Location Station	Side	Remarks
293+00.00	Rt	Exist. Ditch outletting to Donaldson Creek

Design No. 514  
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**LONGITUDINAL SUBDRAIN SHOULDER AND BACKSLOPE**

Refer to Soils Sheets

① Refer to EW-203, EW-204, or EW-211.  
\*Not a bid item

Line No.	Road or Lane	Location		Side	Depth	Longitudinal Subdrain (RF-19C)			Subdrain Outlet			Porous* Backfill	Class "A"* Crushed Stone	Remarks		
		Station to Station				Shoulder	Backslope	Bridge Berm ①	RF-19C, RF-19E, or RF-19F							
1	IA 22	290+57.83	291+35.00	LT	24.0	4.0	81.2				290+57.83	6.0	RF-19C	3.8	0.2	UAC INTAKE
											291+35.00	6.0	RF-19C			INTAKE #5
2	IA 22	291+35.00	292+46.00	LT	24.0	4.0	113.0				291+35.00	6.0	RF-19C	5.2	0.2	INTAKE #5
3	IA 22	294+92.00	295+48.50	LT	24.0	4.0	58.5				295+48.50	6.0	RF-19C	2.7	0.2	INTAKE #3
4	IA 22	295+48.50	296+55.07	LT	24.0	4.0	110.6				295+48.50	6.0	RF-19C	5.1		INTAKE #3
											296+55.07	6.0	RF-19C		0.2	INTAKE #2
5	IA 22	296+55.07	297+55.00	LT	24.0	4.0	103.9				296+55.07	6.0	RF-19C	4.8		INTAKE #2
											297+55.00	6.0	RF-19C		0.2	INTAKE #1
6	IA 22	297+55.00	298+79.77	LT	24.0	4.0	128.8				297+55.00	6.0	RF-19C	6.0		INTAKE #1
											298+79.77	6.0	RF-19C		0.2	INTAKE #8
7	IA 22	291+35.00	292+46.00	RT	24.0	4.0	113.0				291+35.00	6.0	RF-19C	5.2	0.2	INTAKE #9
8	IA 22	294+92.00	296+75.00	RT	24.0	4.0	223.0				294+92.00	6.0	RF-19E	10.3		
											296+75.00	6.0	RF-19E		0.2	
9	IA 22	296+75.00	298+73.00	RT	24.0	4.0	238.0				296+75.00	6.0	RF-19E	11.0		
											298+73.00	6.0	RF-19E		0.2	
10	IA 22	ADD OUTLET TO EXISTING SUBDRAIN		RT			20.0				298+73.00	6.0	RF-19E	0.0	0.2	NOTE #3
<b>Totals</b>							1189.9					16	54.1	2.0		

NOTE 1: LONGITUDINAL SUBDRAINS 1-7 ARE TYPE 12  
 NOTE 2: LONGITUDINAL SUBDRAINS 8-10 ARE TYPE 7 WITH PCC OR TYPE 8 WITH HMA (ACC)  
 NOTE 3: RECORDS INDICATE THAT LONGITUDINAL SUBDRAINS EXIST WITHIN THE PAVEMENT RECONSTRUCTION AREA. IF ANY LONGITUDINAL SUBDRAINS ARE ENCOUNTERED, THEY SHALL BE OBLITERATED WITHIN THE PAVEMENT RECONSTRUCTION AREA. NEW OUTLETS SHALL BE INSTALLED IN THE UNDISTURBED AREA OUTSIDE THE PAVEMENT AREA AT APPROXIMATE STATION RECONSTRUCTION LISTED AND AS APPROVED BY ENGINEER, TO MAINTAIN DRAINAGE OF ANY EXISTING SUBDRAINS IN THE UNAFFECTED AREAS. OBLITERATION IS INCIDENTAL TO THE INSTALLATION OF THE NEW OUTLETS.  
 NOTE 4: 11 SUBDRAIN OUTLET'S ARE TYPE RF-19C, AND 5 SUBDRAIN OUTLET'S ARE TYPE RF-19E

103-6  
04-19-11

**EMBANKMENT WITH MOISTURE CONTROL**

Moisture content shall be within the limits of minus 2 and plus 2 percentage points of Optimum Moisture Content for maximum density within the area described and listed below.

Moisture Control is required for all Class 10 fill placed in all locations and depths. Stability berms placed outside the normal foreslope template and topsoil will not require Moisture Control.

103-7  
08-01-08

**SHRINKAGE DATA**

Material	%	Remarks
Topsoil	40%	
Class 10	30%	
Boulder Estimate*		20 CY
*Excluding rock excavation		

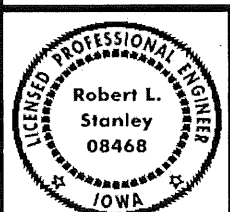
**SPECIAL ATTENTION-SLIVER FILL**

Special attention should be given to Section 2107.03.C, Standard Specification Series of 2012, on this project.

Design No. 514  
File No. 30687

**GEOTECHNICAL DESIGN**

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.



Signature: *Robert Stanley* Date: 11-27-13

Printed or Typed Name: Robert L. Stanley  
My license renewal date is December 31, 2014

Pages or sheets covered by this seal: CS.1

### SURVEY SYMBOLS

- FW Wire Fence
- PR Electric Riser Pole
- LUM Luminaire
- TA Tower Anchor
- RRS Railroad Signal
- PPA Power Pole Co. 1
- GDL Guard Rail Steel
- SI Sign
- OUT Tile Outlet
- BB Billboard
- SL Speed Limit Sign
- MM Mile Marker Post
- TPD Telephone Pedestal
- RRB Railroad Signal Box
- TIL Tile Line
- DU Centerline Draw or Stream (Up)
- D Centerline Draw or Stream (Down)
- EW Edge of Water
- BNK Stream Bank
- RIP Rip-Rap
- RR Centerline of Railroad Tracks
- TLA Underground Telephone Line Co. 1
- ELB Underground Electric Line Co. 2
- WLA Underground Water Line Co. 1
- GHA Underground High Pres Gas Co 1
- IN Storm Sewer Intake
- RET Retaining Walls
- TLNR Tree Line Right
- TLNL Tree Line Left
- STA Storm Sewer Line Co. 1

### UTILITY LEGEND

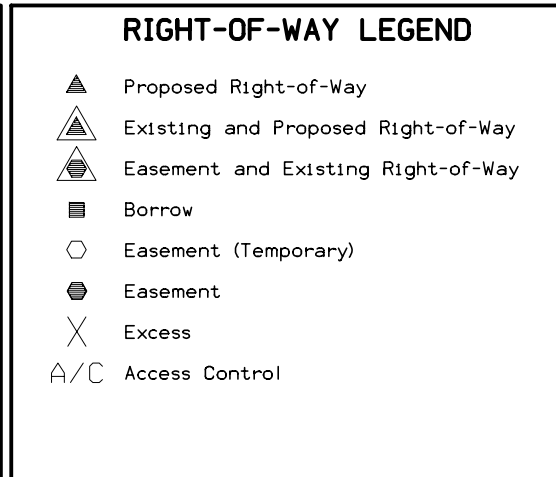
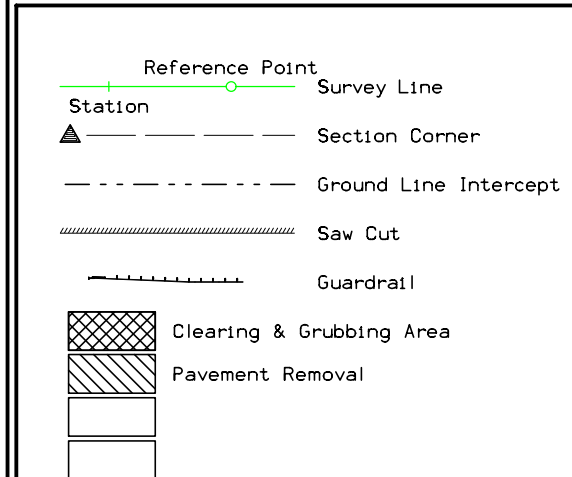
- DME RailRoad
- MidAmerican Energy (Gas)**  
Scott Bull  
2811 5th Avenue  
Rock Island, IL 61201  
309-793-3763  
sabull@midamerican.com
- MidAmerican Energy (Electrical Transmission)**  
Tom Alberson  
106 East 2nd Street  
Davenport, IA 52801  
563-338-8155  
ktalbertson@midamerican.com
- MidAmerican Energy (Electrical Distribution)**  
Jeff Thomas  
2811 5th Avenue  
Rock Island, IL 61201  
309-793-3763  
jwthomas@midamerican.com
- Centurylink**  
Steven Parker  
320 2nd Avenue SW  
Rochester, MN 55902  
507-285-2335  
steven.parker4@quest.com
- City of Buffalo**  
Al Horst  
329 Dodge Street  
Buffalo, IA 52728  
563-381-2226  
bufpublicworks@mchsi.com
- Linnwood Mining**  
Abandoned Water Line

### PLAN VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

LINEWORK	Design Color No.	
Green	(2)	Existing Topographic Features and Labels
Blue	(1)	Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation
Magenta	(5)	Existing Utilities
SHADING	Design Color No.	
Yellow	(4)	Highlight for Critical Notes or Features
Red	(3)	Delineates Restricted Areas
Lavender	(9)	Temporary Pavement Shading
Gray, Light	(48)	Proposed Pavement Shading
Gray, Med	(80)	Proposed Granular Shading
Gray, Dark	(112)	Proposed Grade and Pave Shading
Brown, Light	(236)	Grading Shading
Tan	(8)	Proposed Sidewalk Shading
Blue, Light	(230)	Proposed Sidewalk Landing Shading
Pink	(11)	Proposed Sidewalk Ramp Shading

### PROFILE VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

LINEWORK	Design Color No.	
Green	(2)	Existing Ground Line Profile
Blue	(1)	Proposed Profile and Annotation
Magenta	(5)	Existing Utilities
Blue, Light	(230)	Proposed Ditch Grades, Left
Black	(0)	Proposed Ditch Grades, Median
Rust	(14)	Proposed Ditch Grades, Right



Design No. 514  
File No. 30687

## PLAN AND PROFILE LEGEND AND SYMBOL INFORMATION SHEET

(COVERS SHEET SERIES D, E, F, & K)

BUFFALO TWP.  
T-77N R-2E  
SEC. 24

POT Sta 283+07.48

285

5' Transition to  
6" Standard Curb

290

+77 Prop.  
Type "B" Ent.

295

+01 Prop.  
Type "B" Ent.

BEGIN CONSTRUCTION  
STA 287+60.00

BEGIN PAVING  
STA 290+52.00

5' Transition to  
6" Standard Curb

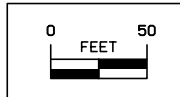
IA 22 (ML022)

DAKOTA MINNESOTA  
EASTERN RR

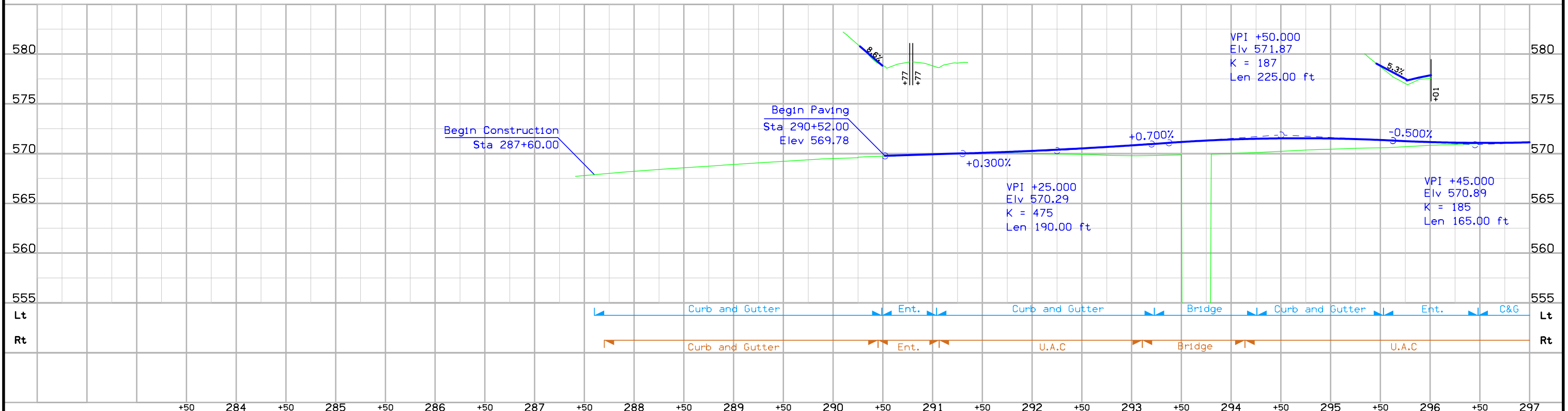
DONALDSON  
CREEK

Sta. 293+65.8  
30'x83.5' Conc. Arch Bridge  
D.A. = 2100 Ac. R - H

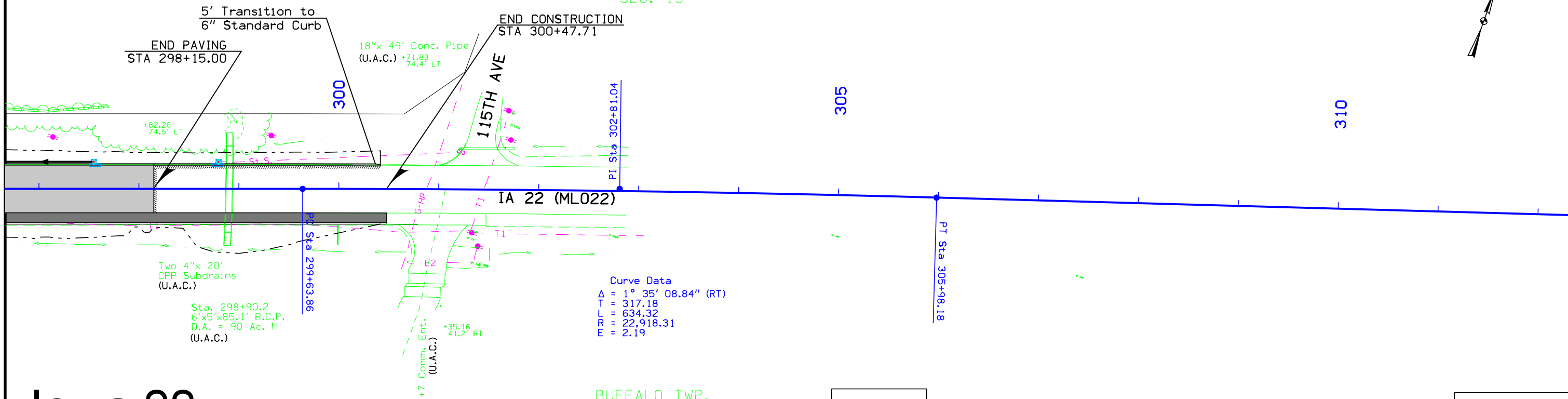
# Iowa 22



Design No. 514  
File No. 30687

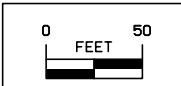


BUFFALO TWP.  
T-77N R-2E  
SEC. 13

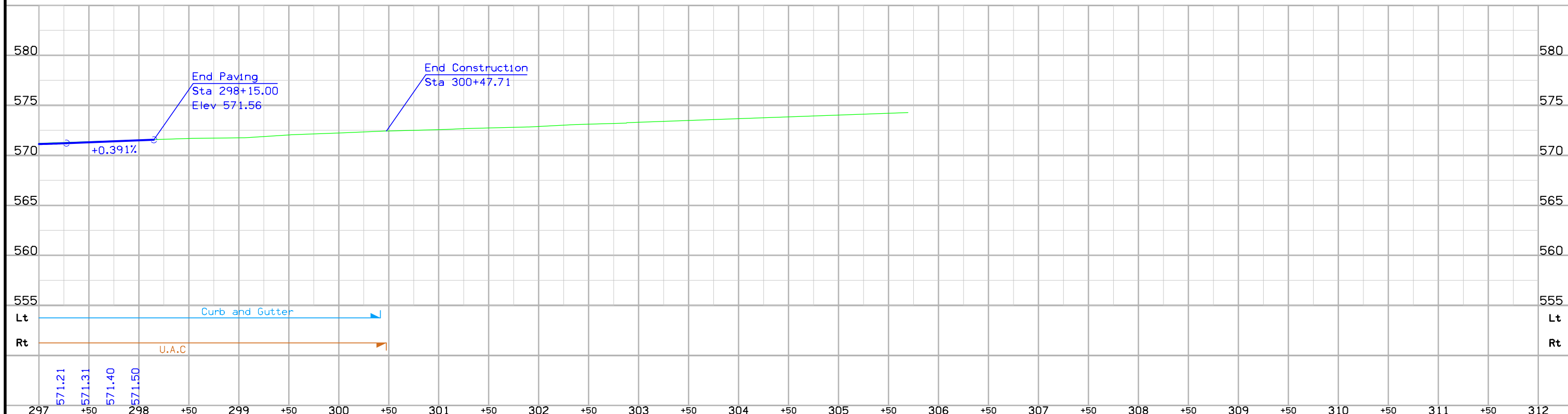


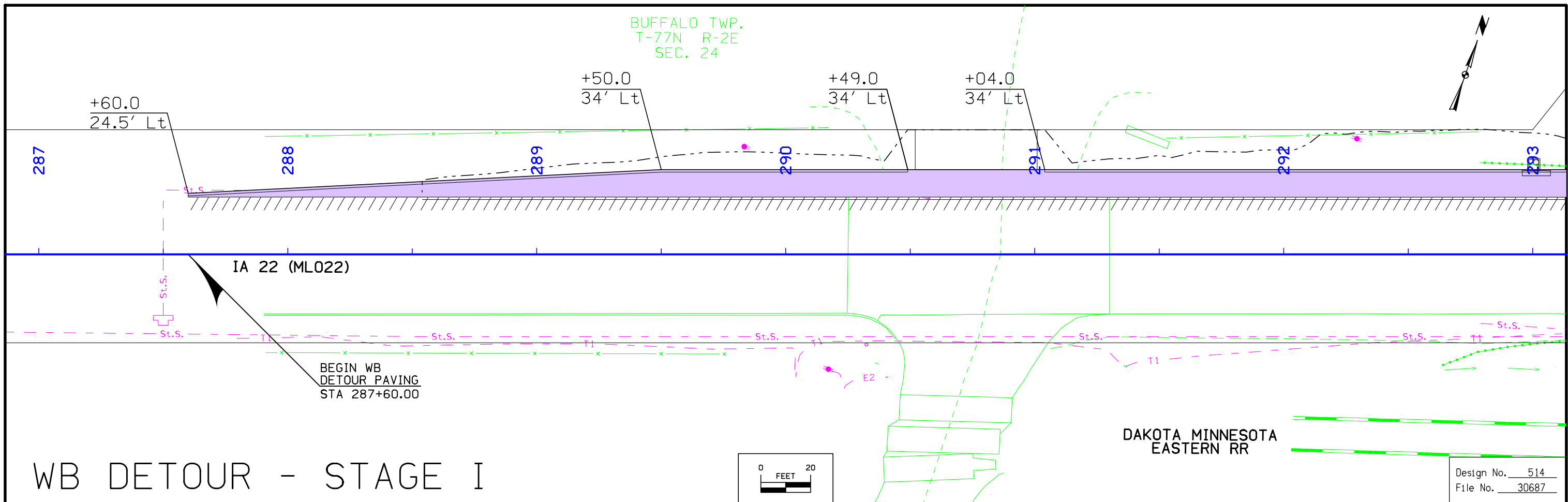
# Iowa 22

BUFFALO TWP.  
T-77N R-2E  
SEC. 24

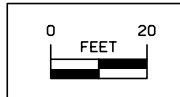


Design No. 514  
File No. 30687

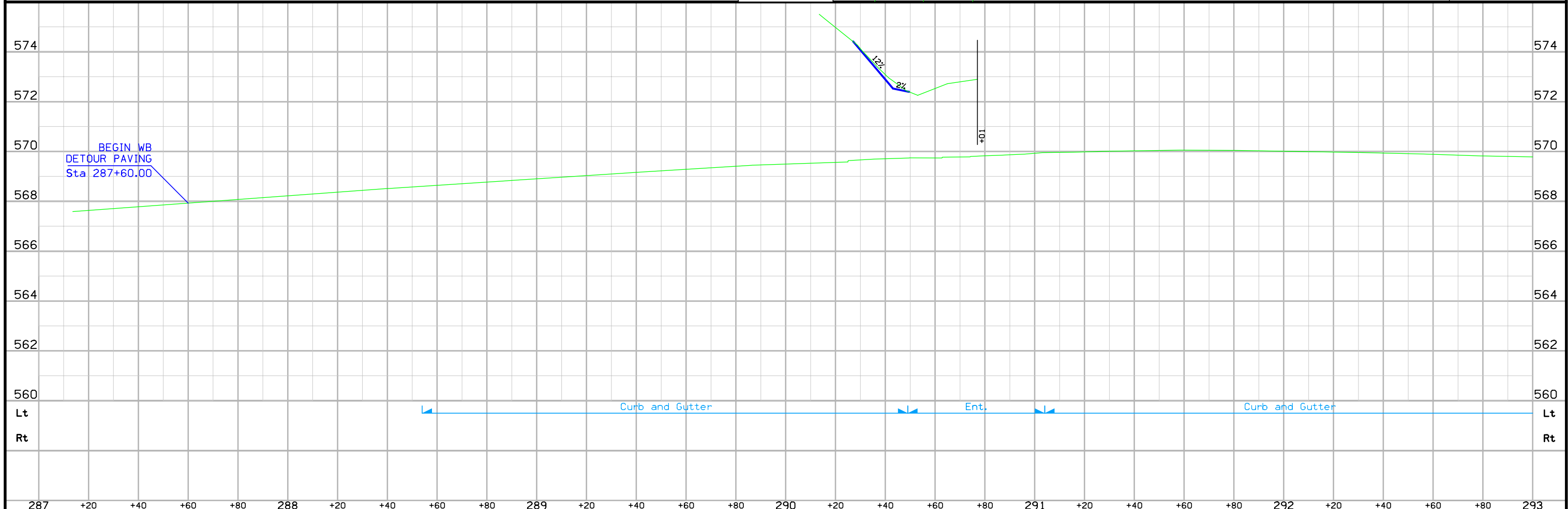


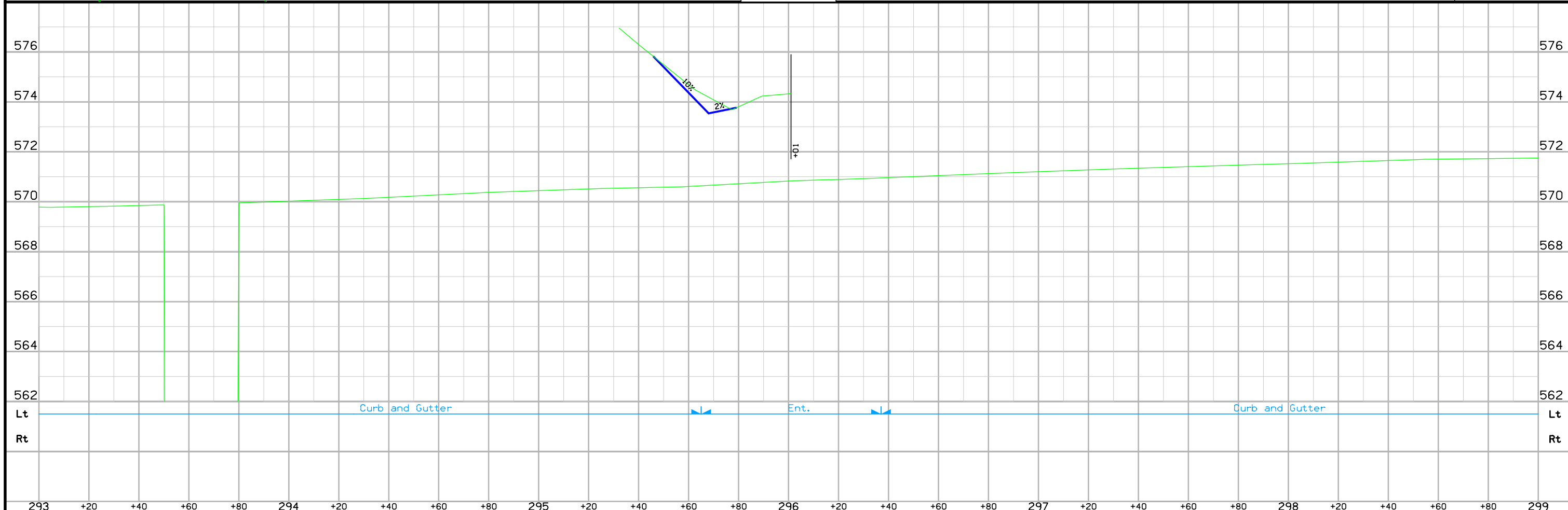
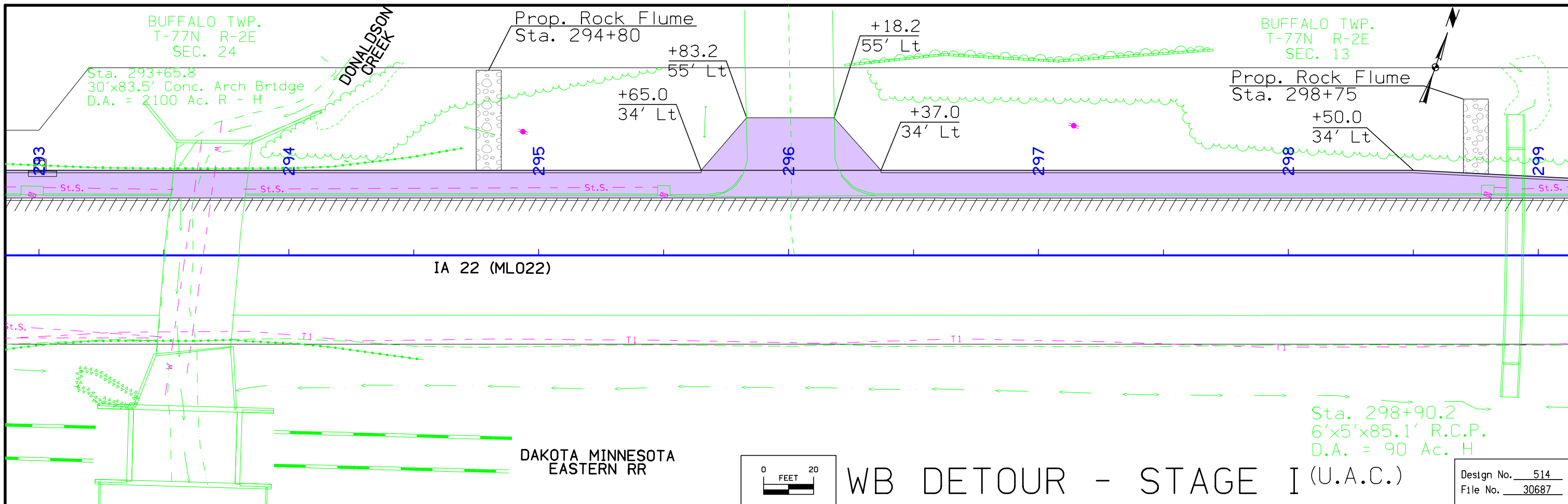


# WB DETOUR - STAGE I

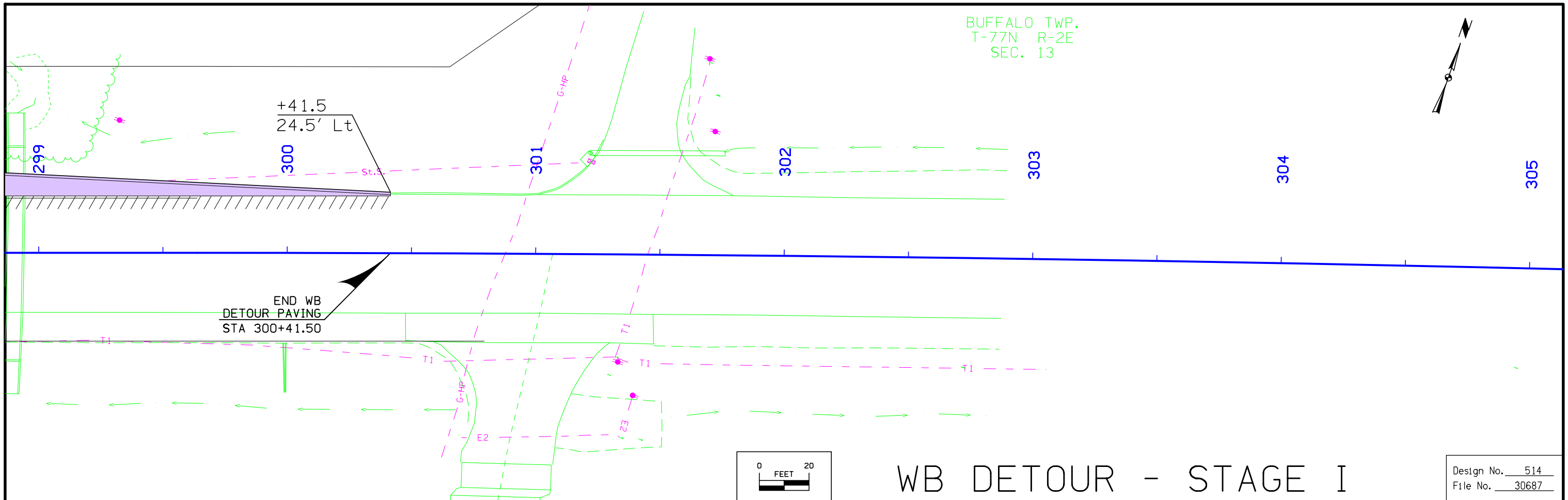


Design No. 514  
File No. 30687



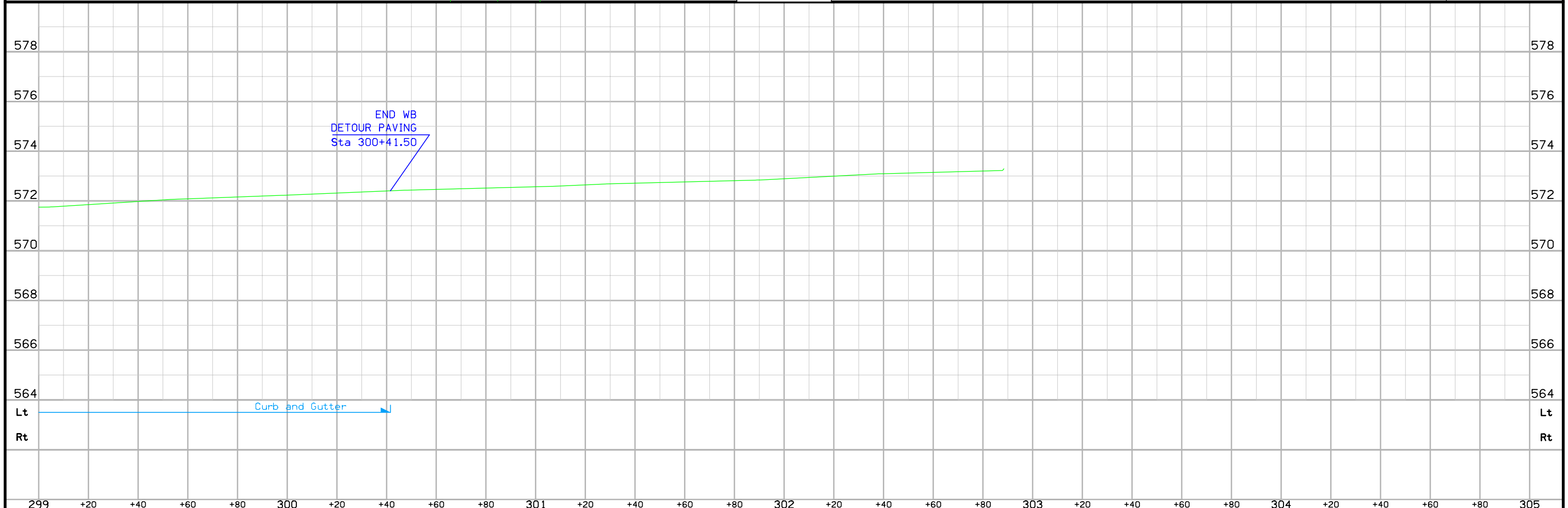


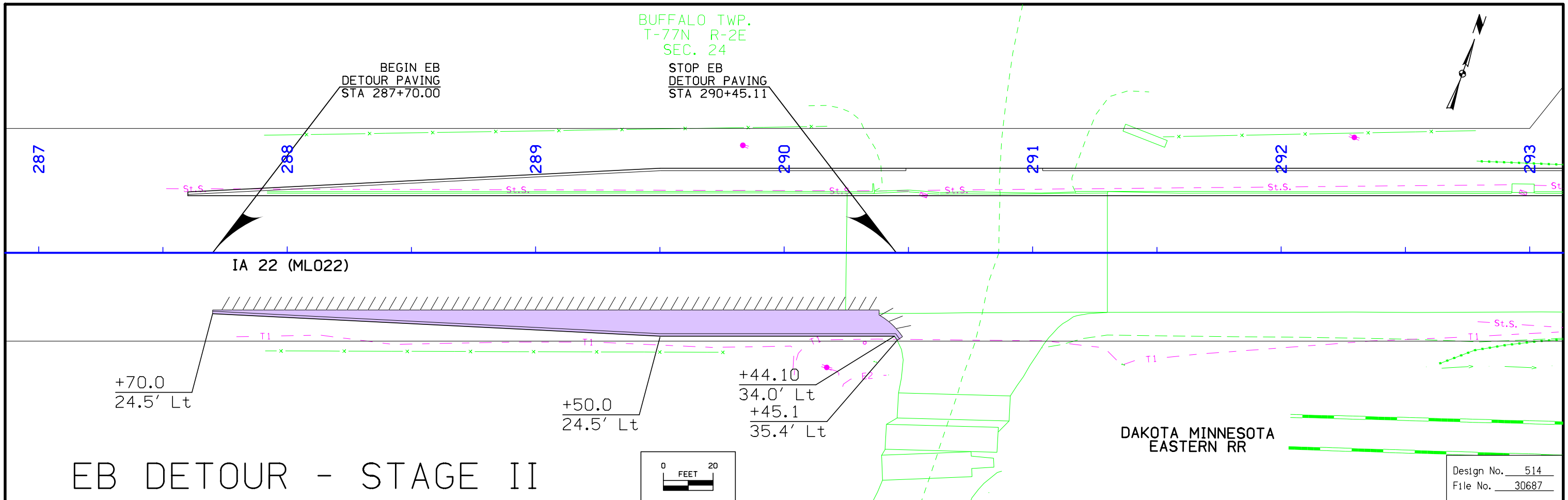
BUFFALO TWP.  
T-77N R-2E  
SEC. 13



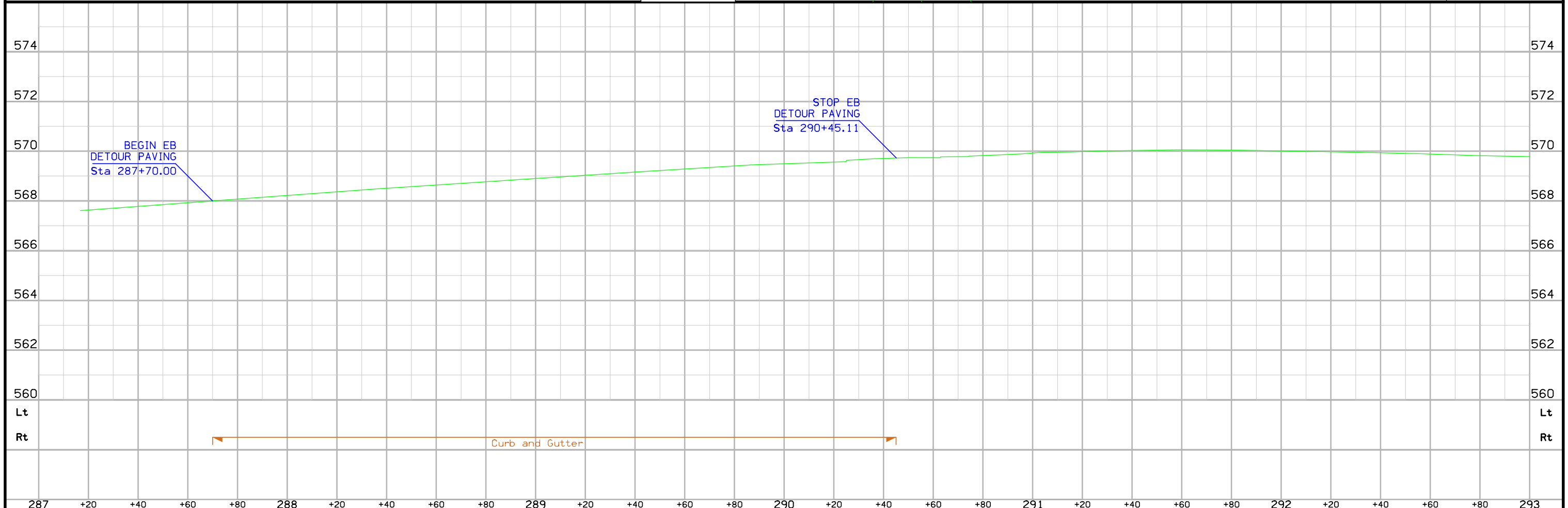
# WB DETOUR - STAGE I

Design No. 514  
File No. 30687





# EB DETOUR - STAGE II





# Survey Information

Scott County  
 SAP 0699  
 BRFN-022-5(29)--39-82  
 IA22 Bridge over Donaldson Creek 2.8 miles west of the junction of I-280  
 PIN 10-82-022-020

**General Information**

This survey is in English Units.

Control Information -Opus & RTN observations were utilized as a basis for project control. 11 Control Points were held fixed in the RTK calibration.

2 held fixed Vertically; 601,605  
 9 held fixed Vertically and Horizontally; 49,50,51,52,53,54,600,603,606

**Vertical Datum**

This survey is relative to NAVD 88 vertical datum.

At the project location, CP49 was observed with both a IaRTN observation and in a four-hour static GPS observation. The vertical difference between the two observations was 0.15 feet. The OPUS elevation value of 567.578 was held fixed in a bench level loop. CP54 was observed with both a IaRTN observation and in a four-hour static GPS observation. The vertical difference between the two observations was also 0.27 feet. The Opus elevation value of 572.325 was held fixed in a bench level loop.

A bench level loop originated and terminated on CP49, elevation 567.578 on BM600 was computed by backing elevations west from CP49.

The adjusted elevations of all CP and BM points along the route were held fixed in the RTK vertical calibration. The total length of the loop was 1.5 mile with a missed closure of 0.0061 feet.

The estimated vertical difference between NAVD88 to NGVD29 at the project site is 0.19' were (29 higher is than 88).

**Equations:**

BM#603 this survey	Elev. =566.498
BM#133A	Elev. =567.25 1973 Situation Plan FN-22-5(2)--21-82
BM#603	Elev. =566.688 (Computed NGVD 29)
BM#605 this survey	Elev. =567.145
BM#133	Elev. =567.90 1973 AB Plan FN-22-5(2)--21-82 Sheet 55 of 419
BM#605	Elev. =567.335 (Computed NGVD 29)
BM#606 this survey	Elev. =568.934 (US Corps of Engineers BM 16-R-5...Fd 2" Pipe w/Cap)
BM# 16-R-5	Elev. =571.75 1987 Storm Sewer AB Plan FN-22-5(17)--21-82 Sheet G01 of 22
BM# 16-R-5	Elev. =571.75 1975 Grading/Pave AB Plan FN-22-5(2)--21-82 Sheet 56 of 419
BM# 16-R-5	Elev. =569.653 F-22-5(2)-20-82 H.C. Brockman Bench Levels Notes Page 15
BM#606	Elev. =569.22 (Computed NGVD 29)

**Horizontal Datum**

**Project Coordinate Transformation**

Iowa State Plane South Zone coordinates in US feet were transformed to project ground coordinates using a 1/combined scale factor broadcast about a held point. The held State Plane coordinate and project coordinate at CP16 are N= 542164.037 E=2395111.853

1 / GRID = 1.000063684

VERTICAL DATUM = NAVD 88 <-> HORIZONTAL DATUM = NAD 83 (IARTN)

**Local Project Plane Coordinate Conversion Equation:**

- a. Local Project Coord y = [(State Plane y - hold point y) 1/grid factor] + hold point y
- b. Local Project Coord x = [(State Plane x - hold point x) 1/grid factor] + hold point x

**ALL COORDINATES CONVERTED TO ENGLISH UNITS**

POINT	STATE PLANE COORD(Y)	STATE PLANE COORD(X)	POINT SCALE FACTOR	LOCAL PROJECT PLANE COORD(Y)	LOCAL PROJECT PLANE COORD(X)	ESTIMATED GPS DERIVED ORTHOMETRIC HEIGHT
16	542164.037	2395111.853	0.999958200	542164.037	2395111.853	567.674
49	545271.503	2410839.412	0.999958760	545271.701	2410840.414	567.577
50	545654.368	2411620.108	0.999958830	545654.590	2411621.159	566.309
51	545888.072	2412475.012	0.999958860	545888.309	2412476.118	569.589
52	545993.312	2412803.102	0.999958900	545993.556	2412804.229	566.817
53	546068.372	2412791.123	0.999958900	546068.621	2412792.249	570.012
54	546360.898	2413901.196	0.999958980	546361.165	2413902.393	572.327
600	545016.905	2410205.754	0.999958760	545017.087	2410206.715	567.603
601	545512.137	2411392.554	0.999958760	545512.350	2411393.591	567.051
602	545810.193	2412237.255	0.999958830	545810.425	2412238.346	567.838
603	546089.722	2412813.537	0.999958900	546089.972	2412814.664	566.498
604	546129.851	2412916.034	0.999958900	546130.104	2412917.168	571.059
605	546173.140	2413319.925	0.999958980	546173.395	2413321.085	567.145
606	546175.157	2413555.122	0.999958980	546175.412	2413556.297	568.934

**Alignment**

The mainline alignment is a retrace of the existing alignment found on the FN-22-5(2)--21-82 AB Plan. Stationing was backed up & carried forward from a Mag nail found at PC Sta 299+64.35 without equation.

**Alignment Equations**

PC Sta 299+64.35 This Survey  
 =PC Sta 299+64.35 FN-22-5(2)--21-82 AB Plan

**BENCHMARKS**

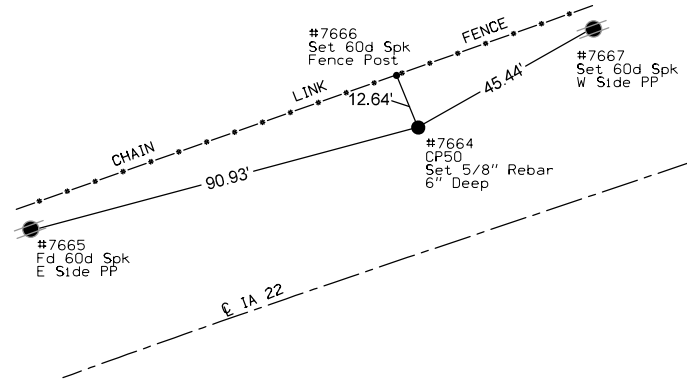
	ELEVATION
No. 602 Sta. 287+47.228 28.098 Rt. Y: 545810.425 X:2412238.346	Cut a square on SW Corner intake----- 567.838
No. 603 Sta. 293+83.546 45.303 Lt. Y: 546089.972 X:2412814.664	Fd IHC BM on InHdwl of Arch Brg----- 566.498
No. 604 Sta. 294+93.554 49.308 Lt. Y: 546130.104 X:2412917.168	Fd RR Spk S Side PP----- 571.059
No. 605 Sta. 298+89.086 43.306 Rt. Y: 546173.395 X:2413321.085	Fd IHC BM on Outlet Hdwl----- 567.145
No. 606 Sta. 301+12.522 118.648 Rt. Y: 546175.412 X:2413556.297	Fd US CORPS BM A STANDARD CAP on a 2" Pipe access thru a hinged cap on a 4" pipe=US Corps BM# 16-R-5 Pipe----- 568.934

**MISCELLANEOUS LOCATIONS**

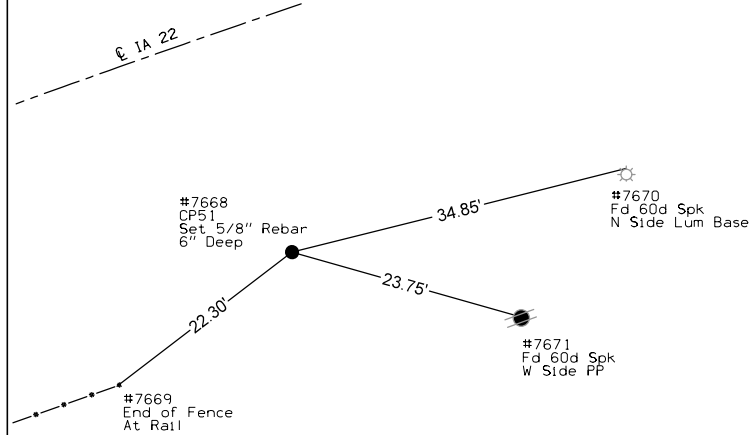
No. 600 Sta.*****	Y: 545017.087 X:2410206.715	Fd "X" SE Wing of Br-----	567.603
No. 601 Sta.*****	Y: 545512.350 X:2411393.591	Cut a square on SE Corner intake-----	567.051

Design No. 514  
 File No. 30687

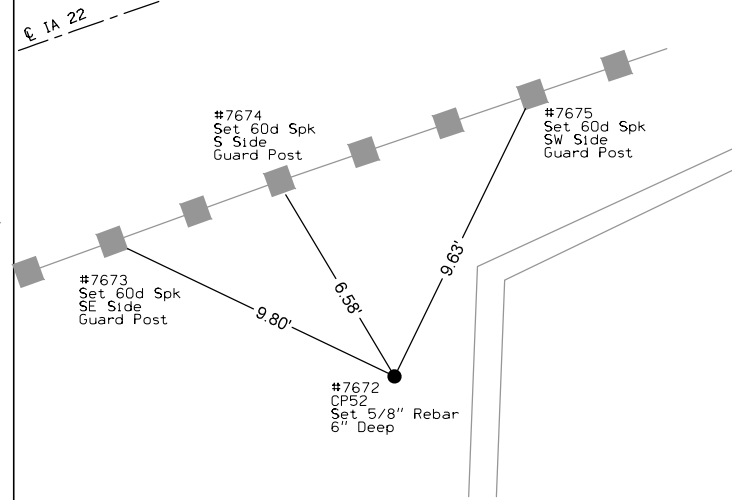
C.P. NO STATIONING  
 C.P. 50, Set 5/8" Rebar 6" Deep  
 N=545654.590 E=2411621.159



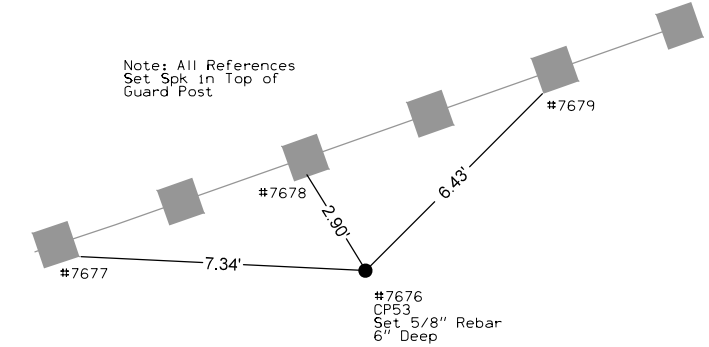
C.P. 289+97.38 RIGHT 33.16  
 C.P. 51, Set 5/8" Rebar 6" Deep  
 N=545888.309 E=2412476.118



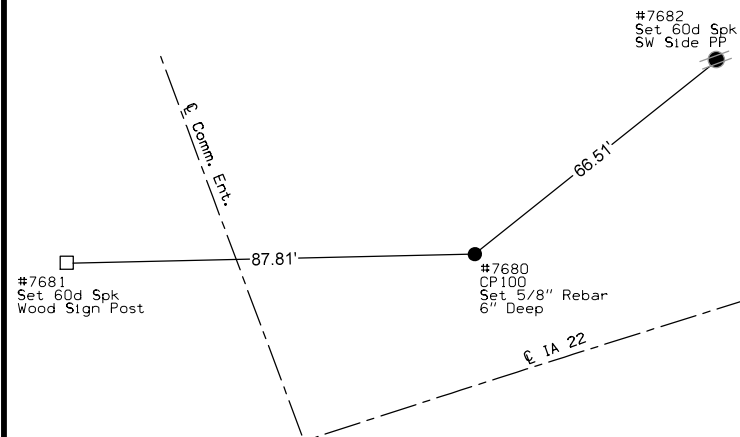
C.P. 293+41.84 RIGHT 42.25  
 C.P. 52, Set 5/8" Rebar 6" Deep  
 N=545993.556 E=2412804.229



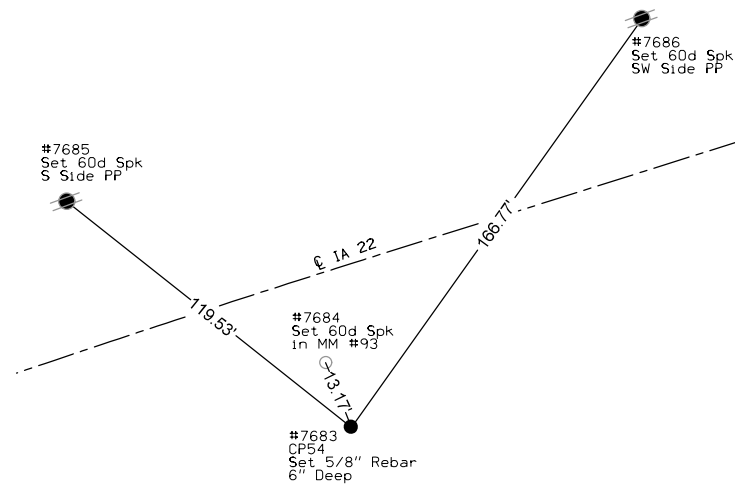
C.P. 293+55.34 LEFT 32.56  
 C.P. 53, Set 5/8" Rebar 6" Deep  
 N=546068.621 E=2412792.249



C.P. 296+50.59 LEFT 30.21  
 C.P. 100, Set 5/8" Rebar 6" Deep  
 N=546163.972 E=2413071.688



C.P. 305+01.05 RIGHT 51.89  
 C.P. 54, Set 5/8" Rebar 6" Deep  
 N=546361.165 E=2413902.393



Design No. 514  
 File No. 30687

**ALIGNMENT COORDINATES**

101-16  
10-20-09

Name	Location	Point on Tangent			Begin Spiral			Begin Curve			Simple Curve PI or Master PI of SCS			End Curve			End Spiral		
		Station	Coordinates		Station	Coordinates		Station	Coordinates		Station	Coordinates		Station	Coordinates		Station	Coordinates	
			Y (Northing)	X (Easting)		Y (Northing)	X (Easting)		Y (Northing)	X (Easting)		Y (Northing)	X (Easting)		Y (Northing)	X (Easting)		Y (Northing)	X (Easting)
3	ML022	283+07.48	545,691.63	2,411,814.02															
C1	ML022						299+63.86	546,238.98	2,413,377.35	302+81.04	546,343.79	2,413,676.71	305+98.18	546,440.27	2,413,978.86				
5	ML022	326+93.81	547,077.77	2,415,975.17															

**SPIRAL OR CIRCULAR CURVE DATA**

101-17  
04-19-11

Name	Location	$\Delta_{scs}$	Horizontal Alignment Data													Remarks			
			Spiral Data						Curve Data										
			$\theta_s$	Ls	Ts	Es	Xc	Yc	L.T.	S.T.	$\Delta_c$	T	L	R	E				
C1	ML022													1° 35' 08.84" RT	317.18'	634.32'	22,918.31'	2.19'	

Design No. 514  
File No. 30687

①  
 LINWOOD STONE PRODUCTS CO, INC.  
 & LINWOOD MINING & MINERALS CORP.

①  
 LINWOOD STONE PRODUCTS CO, INC.  
 & LINWOOD MINING & MINERALS CORP.

FLINT HILLS RESOURCES  
 PINE BEND, LLC

APPROX. SEC. LINE

Two 96" X 70'  
 Cast Iron Pipes  
 (U.A.C.)

PERMANENT EASEMENT FOR  
 EROSION CONTROL MEASURES

BUFFALO TWP.  
 T-77N R-2E  
 SEC. 24

TEMPORARY EASEMENT TO  
 CONSTRUCT ENTRANCE

TEMPORARY EASEMENT  
 TO SHAPE

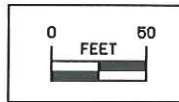
TEMPORARY EASEMENT  
 TO SHAPE

**Right of Way Design Information**  
**THIS SHEET INCLUDED**  
**FOR INFORMATION ONLY**

**ROW Team: GETTINGS/DUMDEI/GOGERTY**  
**ROW #: STPN-22-5(32)-2J-82**  
**Plan Date: 1/15/13**

**Color Legend:**  
 **Property Lines**  
 **Temporary Easement**  
 **Permanent Acquisition**

For Mainline Details  
 Refer to Sheet No. D.2  
 For Detour Details  
 Refer to Sheets No. F.1, F2 & F4



POT Sta 283+07.48

285

BEGIN CONSTRUCTION  
 STA 287+57.00

27+55  
 ±40'

BEGIN PAVING  
 STA 291+30.00

291+25  
 ±36'±R

291+39±R  
 ±40'

+86 Quarry Ent.

+86 Comm. Ent.

290+40  
 ±60

291+10  
 ±60

291+85  
 ±50'±R

293+20  
 ±75'(Exist. Corner)

290

+77 Prop. Type "B" Ent.

292+90  
 ±50'

291+70  
 ±50'

293+20  
 ±59'

293+20  
 ±63'

294+00  
 ±65'

294+00  
 ±59'

294+35  
 ±140'

294+80  
 ±135'

295

+01 Prop. Type "B" Ent.

DAKOTA MINNESOTA  
 EASTERN RR

294+90  
 ±55'

②R  
 DAKOTA, MINNESOTA  
 & EASTERN R.R.

Sta. 293+65.8  
 30'x23' Conc. Arch Bridge  
 U.A. 2.00 Ac. R - H  
 (REMOVE)  
 Sta 293+65.09  
 Build 100'-0 x 68'-0  
 Concrete Beam Bridge

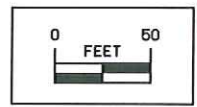
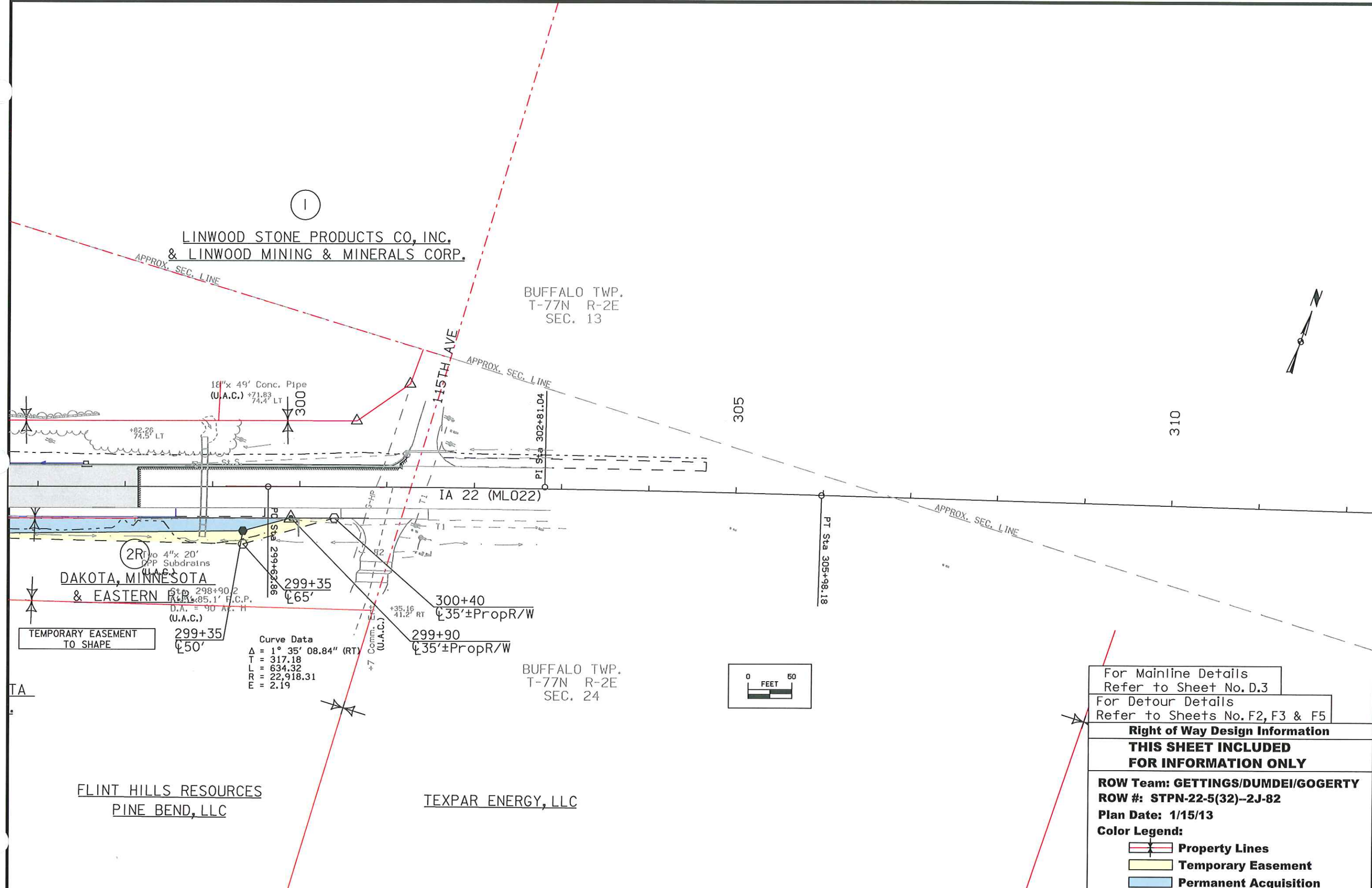
DONALDSON  
 CREEK

IA 22 (ML022)

St.S.

LINWOOD STONE PRODUCTS CO, INC.  
& LINWOOD MINING & MINERALS CORP.

BUFFALO TWP.  
T-77N R-2E  
SEC. 13



Curve Data  
 $\Delta = 1^\circ 35' 08.84''$  (RT)  
 $T = 317.18$   
 $L = 634.32$   
 $R = 22,918.31$   
 $E = 2.19$

For Mainline Details  
Refer to Sheet No. D.3

For Detour Details  
Refer to Sheets No. F.2, F.3 & F.5

**Right of Way Design Information**

**THIS SHEET INCLUDED FOR INFORMATION ONLY**

**ROW Team: GETTINGS/DUMDEI/GOGERTY**  
**ROW #: STPN-22-5(32)-2J-82**  
**Plan Date: 1/15/13**

**Color Legend:**

- Property Lines
- Temporary Easement
- Permanent Acquisition

108-23A  
08-01-08

**TRAFFIC CONTROL PLAN**

Traffic will be maintained on IA 22 by staged construction with at least one lane open in each direction at all times.  
 Maintain access to Linwood Mine entrances (Sta. 290+76.50) at all times. The mine crossing traffic will be controlled by temporary traffic signals.  
 Refer to Staging Notes for Traffic Control Plan details.

102-15  
08-01-08

**TABULATION OF SPECIAL EVENTS**

Event	Location	Date
Buffalo Days	Buffalo, IA	July 4-6

108-26A  
08-01-08

**STAGING NOTES**

2014 Construction Season  
 Stage I  
 Traffic Control:  
 1. Maintain traffic on IA 22 with two 12' EB lanes and one 12' WB lane.  
 2. Maintain access to entrances at all times.

Construction:  
 1. Construct detour pavement adjacent to the WB lanes of IA 22.  
 2. Construct entrances at Sta. 290+76.50 and Sta. 296+00.70.  
 3. Construct intake and temporary storm sewer pipe at Sta. 293+00.45.  
 4. Construct curb dropoffs and drainage flumes at Sta. 204+80 and Sta. 298+75.

Stage II  
 Traffic Control:  
 1. Two-lane, two-way traffic will be maintained with 11' lanes on the existing north lane and the adjacent detour pavement on IA 22.  
 2. Maintain access to entrances at all times.  
 3. Maintain mine traffic during construction of the crossing on mainline IA 22 at all times.

Construction:  
 1. Construct the proposed EB lanes and shoulder on IA 22.  
 2. Construct the Linwood Mine Crossing on EB lanes of IA 22.  
 3. Construct the EB steel beam guardrail.  
 4. Construct detour pavement on the EB shoulder, south of the proposed bridge.  
 5. Remove existing storm sewer.

Stage III  
 Traffic Control:  
 1. Two-lane, two-way traffic will be maintained with 11' lanes on the detour pavement and the newly constructed EB lanes and shoulder on IA 22.  
 2. Maintain access to entrances at all times.  
 3. Maintain mine traffic during construction of the crossing on mainline IA 22 at all times.

Construction:  
 1. Remove detour pavement adjacent to the WB lanes.  
 2. Construct the WB lanes on IA 22.  
 3. Construct the Linwood Mine Crossing on WB lanes of IA 22.  
 4. Construct the curb and gutter adjacent to the WB lanes (Sta. 287+60.0 to Sta. 290+52.0).  
 5. Construct the curb and gutter adjacent to the WB lanes (Sta. 298+15.0 to Sta. 300+41.5).  
 6. Construct WB steel beam guardrail.  
 7. Reconstruct the entrances at Sta. 290+76.50 and Sta. 296+00.70.  
 8. Remove existing storm sewer.  
 9. Construct proposed intakes and storm sewer pipes on north end of IA 22.

Stage IV  
 Traffic Control:  
 1. Maintain traffic on IA 22 with two 12' WB lanes and one 12' EB lane.  
 2. Maintain access to entrances at all times.

Construction:  
 1. Remove detour pavement along the EB lanes.  
 2. Construct curb and gutter along the EB lanes, south of the proposed bridge (Sta. 287+70.0 TO Sta. 290+52.0)

111-01  
04-17-12

**COORDINATED OPERATIONS**

Other work in progress during the same period of time will include the construction of the projects listed. Coordinate operations with those of other contractors working within the same area.










Project	Type of Work
BRFN-22-5(28)--39-82	BRIDGE REPLACEMENT - OTHER

Design No. 514  
 File No. 30687

**CROSS SECTION VIEW COLOR LEGEND  
OF TRAFFIC CONTROL AND STAGING SHEETS**

SHADING	Design Color No.	
Green, Light	(225)	Existing Pavement Shading
Gray, Light	(48)	Previously Constructed Pavement Shading
Gray, Med	(80)	Previously Constructed Granular Surface Shading
Blue, Light	(230)	Proposed Pavement Shading
Lavender	(9)	Temporary Pavement Shading
Brown, Med	(237)	Future Proposed Pavement Shading

**CROSS SECTION VIEW PATTERN AND SYMBOL LEGEND  
OF TRAFFIC CONTROL AND STAGING SHEETS**

	Pavement Removal		Proposed Granular Shoulder
	Proposed Granular Subbase		Temporary Shoulder
	Proposed Special Backfill		Existing Shoulder Strengthening
	Temporary Barrier Rail		Permanent Barrier Rail
			Channelizing Device



**PLAN VIEW COLOR LEGEND OF TRAFFIC CONTROL AND STAGING SHEETS**

LINEWORK	Design Color No.	
Green	(2)	Existing Topographic Features and Labels
Magenta	(5)	Pavement Marking Call Outs
Blue	(1)	Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation
Yellow	(4)	Pavement Markings, Yellow
Off White	(254)	Pavement Markings, White

SHADING	Design Color No.	
Green, Light	(225)	Existing Pavement Shading
Gray, Light	(48)	Previously Constructed Pavement Shading
Gray, Med	(80)	Previously Constructed Granular Surface Shading
Blue, Light	(230)	Proposed Pavement Shading
Lavender	(9)	Temporary Pavement Shading
Brown, Light	(236)	Proposed Grading Limits Shading
Pink, Dark	(13)	Proposed MSE or CIP Wall Shading
Red	(3)	Proposed Bridge Shading and Sign Trusses
Black w/Gray, Light Fill	(0,48)	Previously Constructed Structure

**PLAN VIEW PATTERN AND SYMBOL LEGEND  
OF TRAFFIC CONTROL AND STAGING SHEETS**

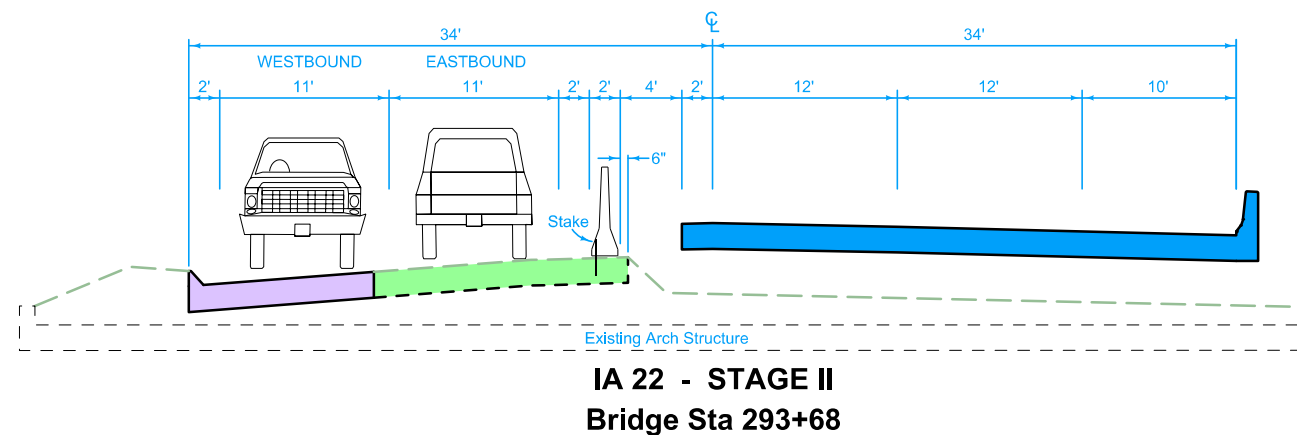
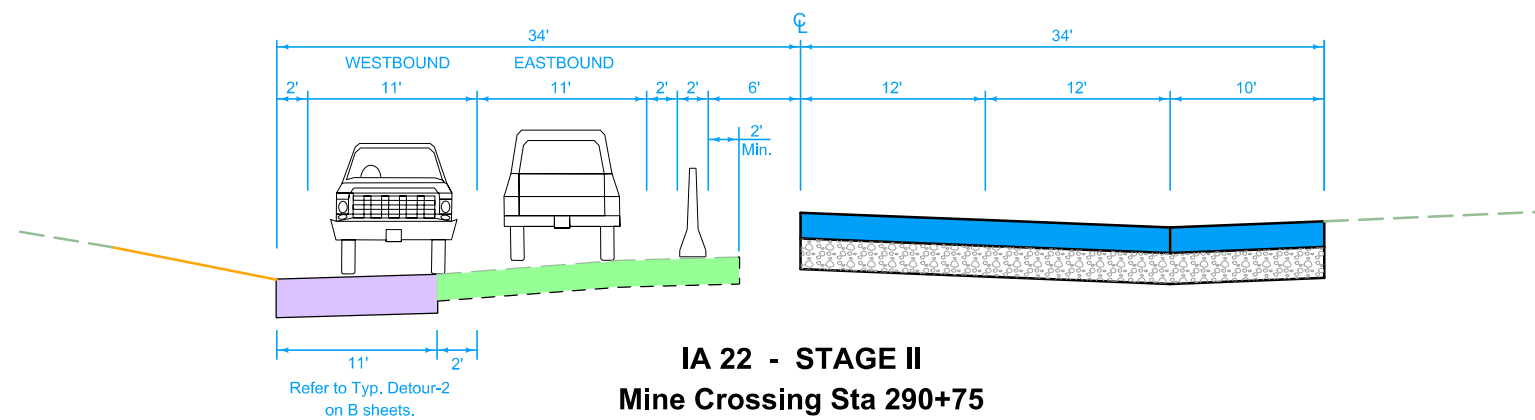
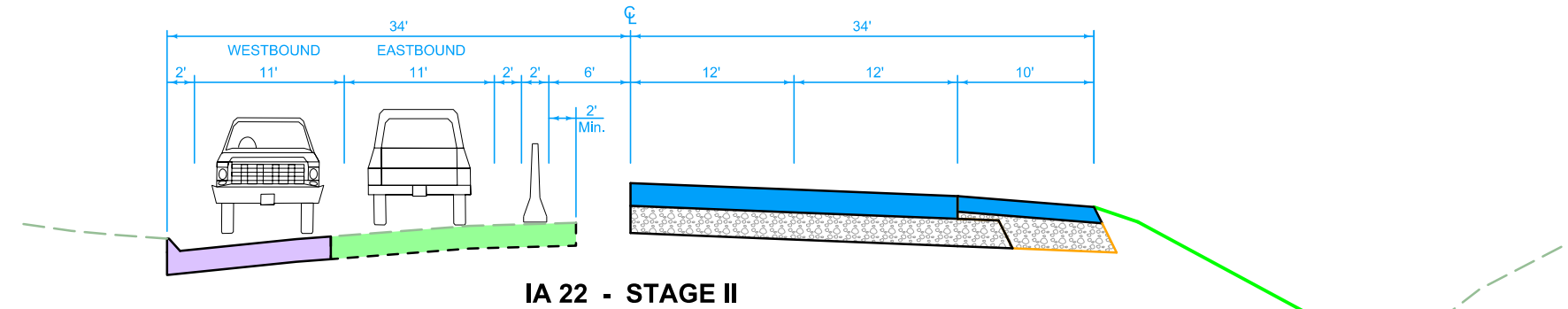
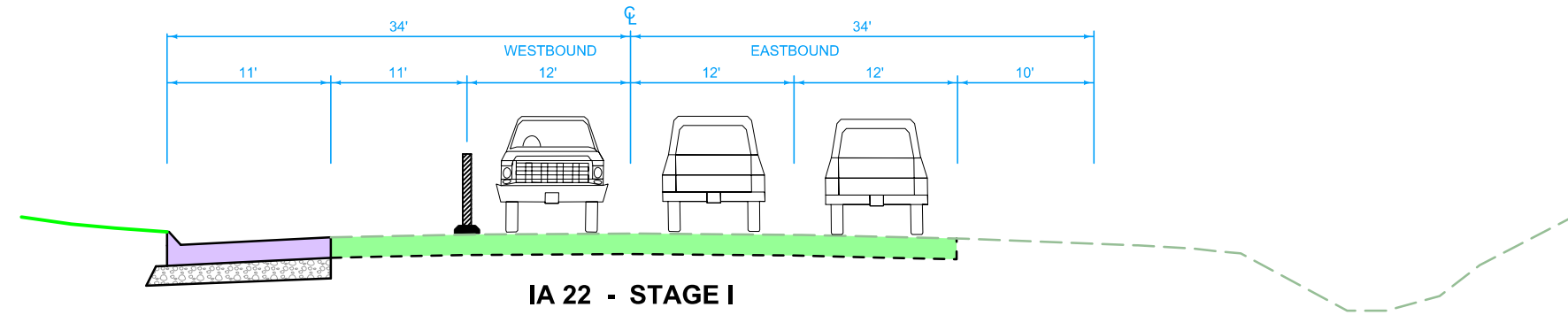
●	Channelizing Device	■	Crash Cushion
✕	Drum	○→	Traffic Signal
■	Temporary Lane Separator	♯	Flagger
◆	Tubular Marker	○●	Temporary Floodlighting
♦	Channelizer Marker	†	Traffic Sign
△	Concrete Barrier Marker	⋮	Type III Barricade
◁	Delineator	☀	Type A Warning Light
≡	Temporary Barrier Rail	←	Direction of Traffic
	Pavement Removal		Safety Closure

NOTE: Device spacing according to Standard Road Plans unless specifically dimensioned.

**TRAFFIC CONTROL  
AND  
STAGING  
LEGEND AND SYMBOL  
INFORMATION SHEET**

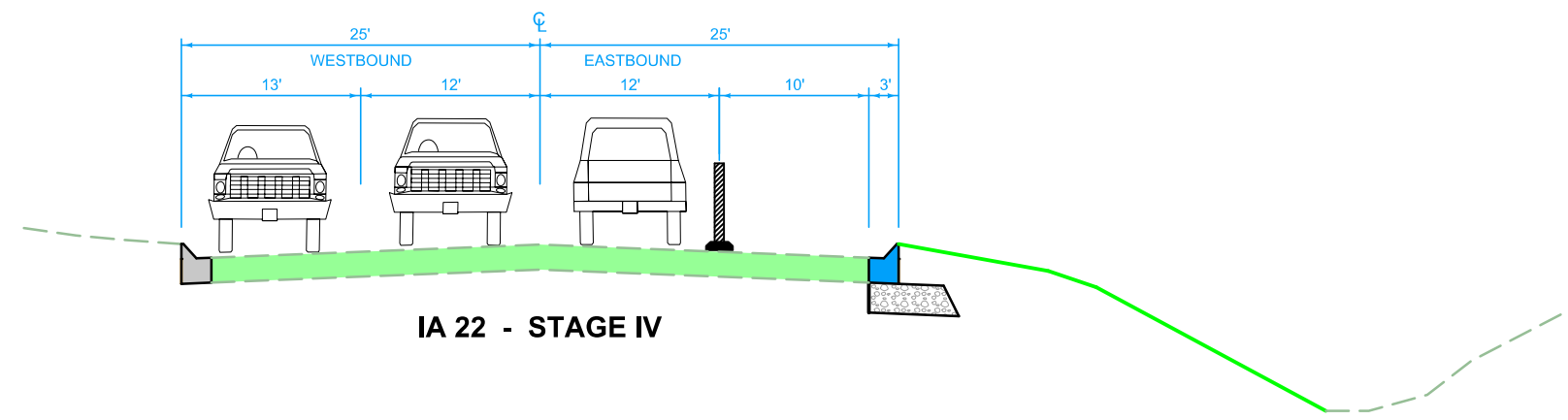
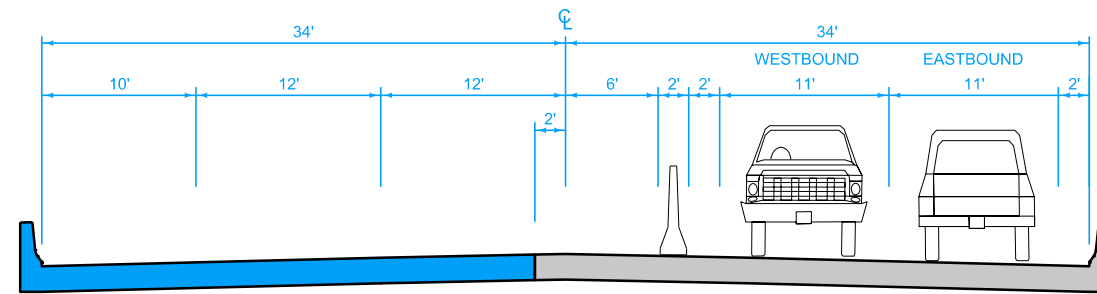
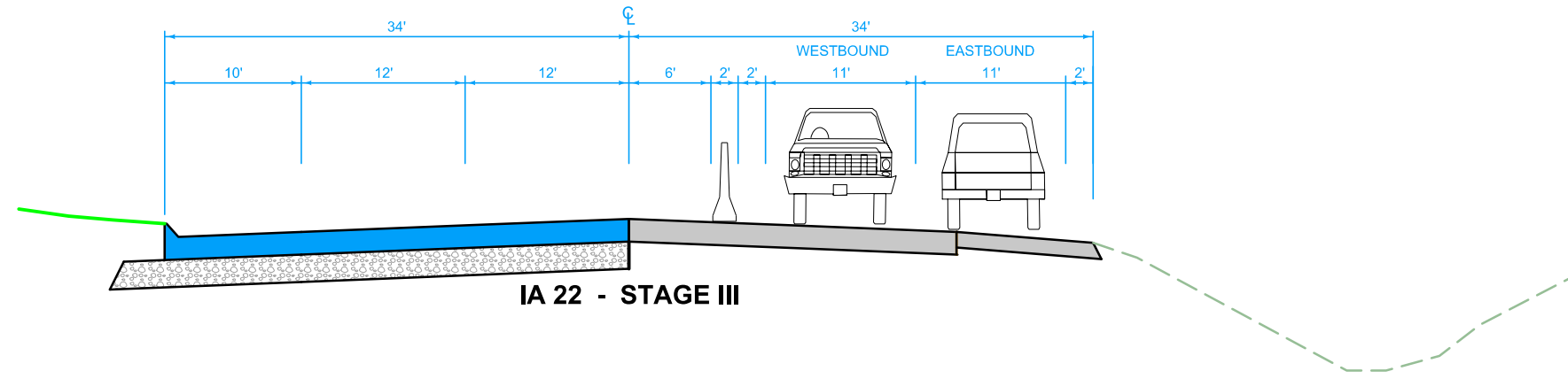
(COVERS SHEET SERIES J)

Design No. 514  
File No. 30687

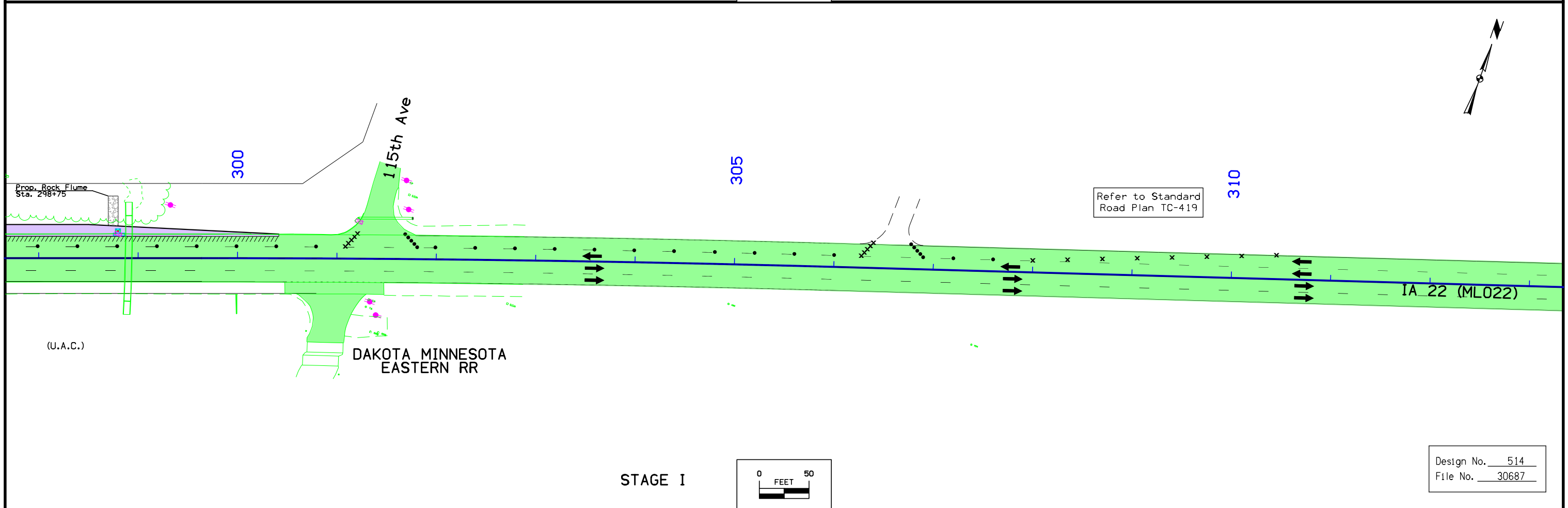
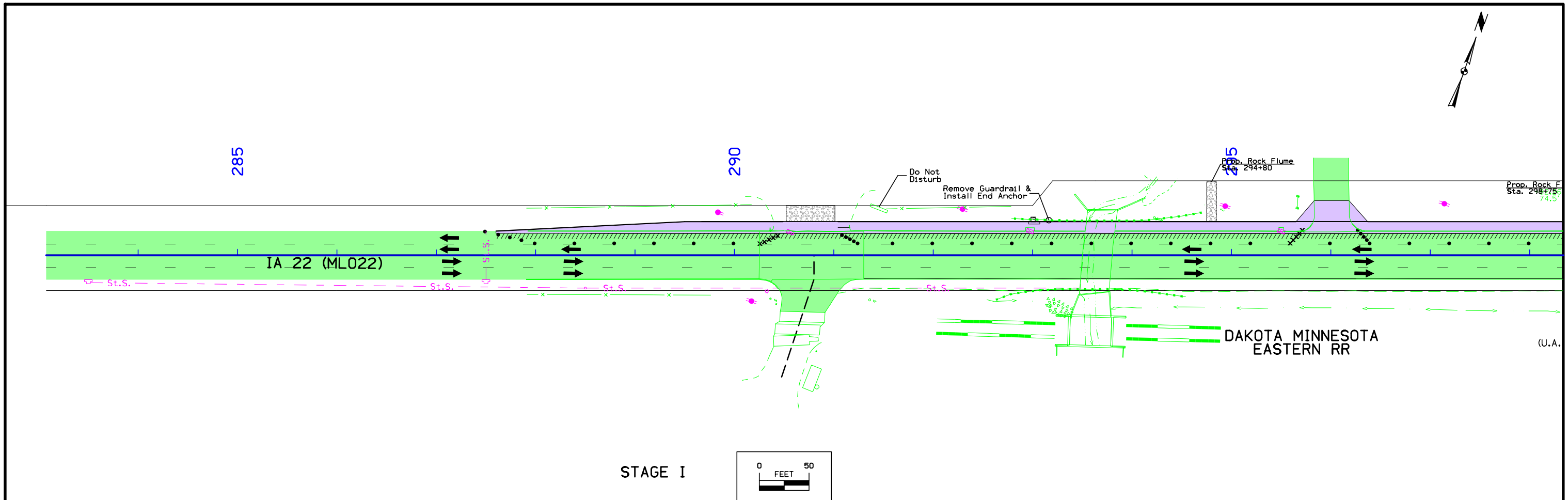


Design No. 514  
File No. 30687

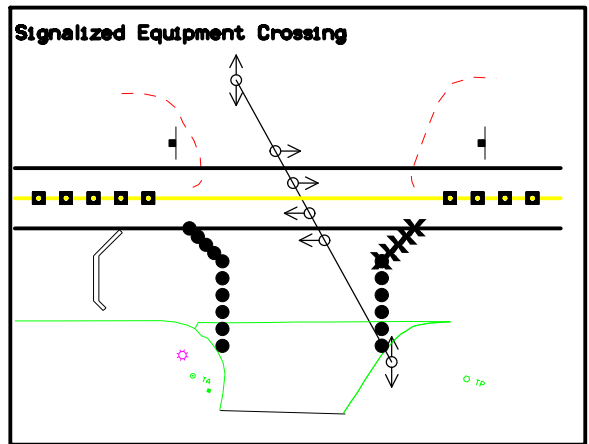
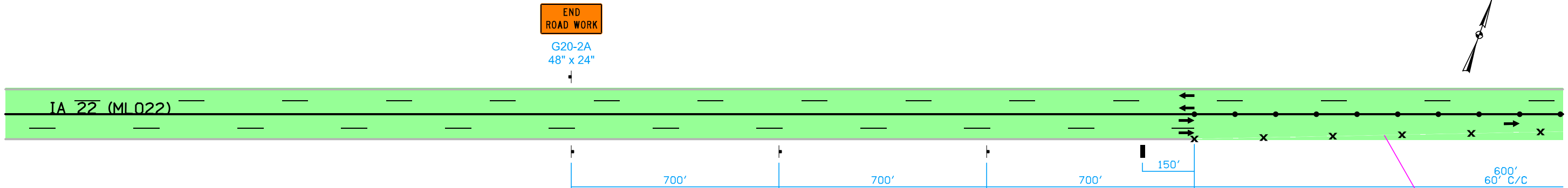




Design No. 514  
File No. 30687



Design No. 514  
File No. 30687



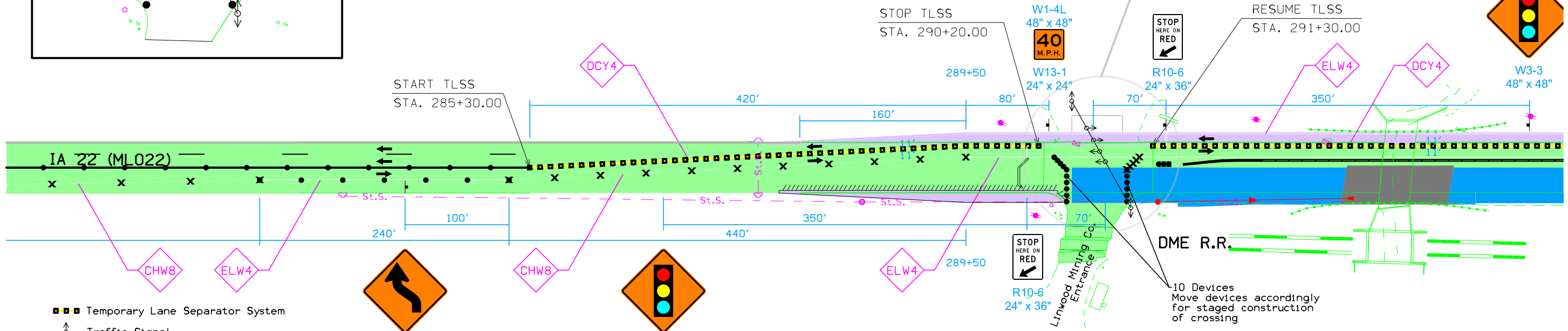
**SIGNAL NOTES:**  
 No detection area required.

Set signal timing as follows: For traffic from all directions, set the yellow indication at approximately 5 seconds and the all-red clearance interval at approximately 2 seconds. Set the minimum green interval on the main road at 20 seconds. Set the green interval for haul road traffic at 12 seconds but may be extended up to a maximum of 30 seconds.

Locate the Stop bars 70 feet in advance of each lane's nearest signal head.

Required only if haul road is used during nighttime hours.

When the equipment crossing is not in use, place Type III Barricades as shown, and cover SIGNAL AHEAD and STOP HERE ON RED signs.

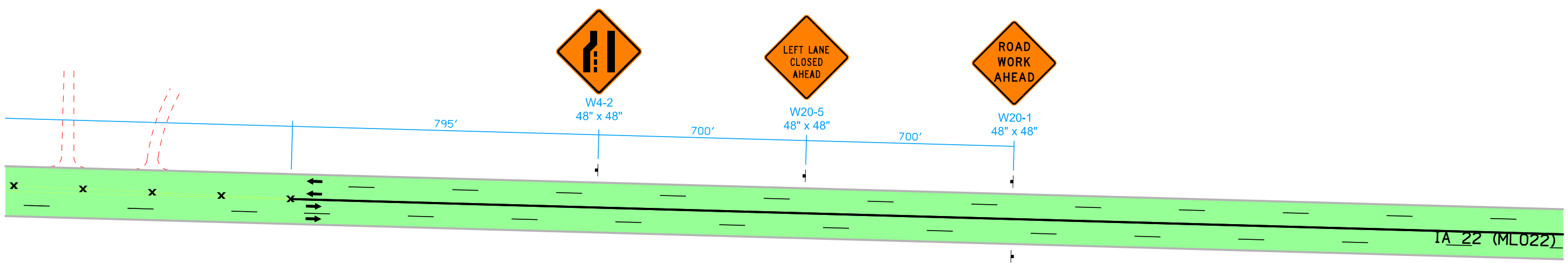
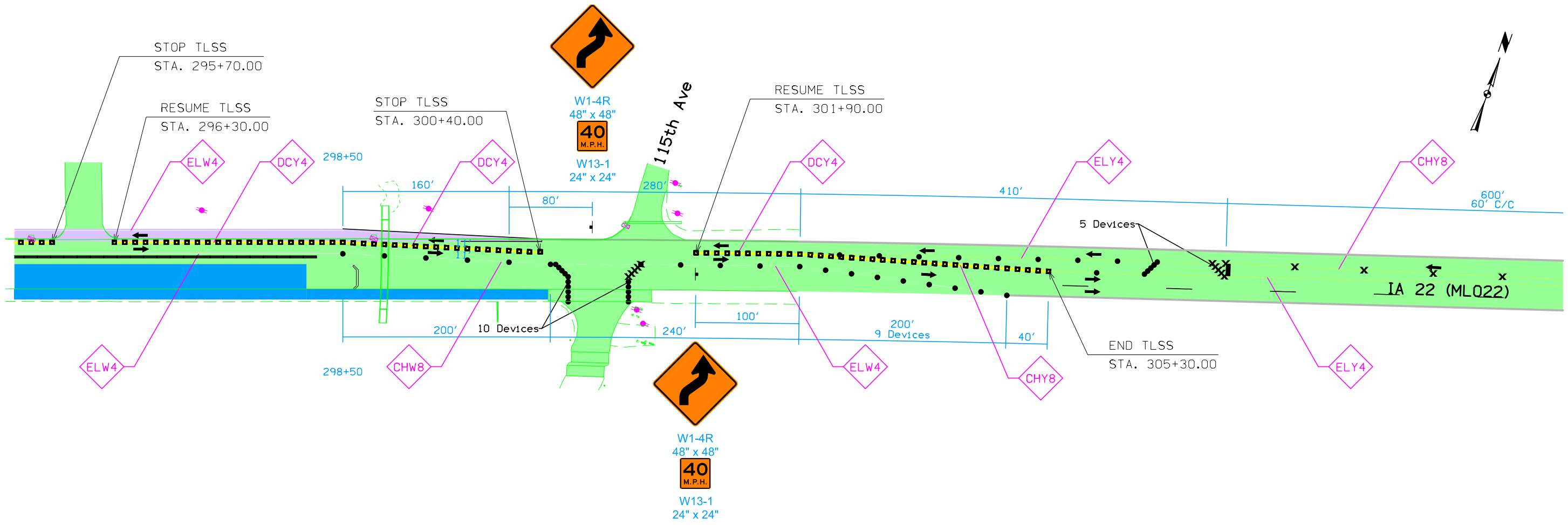


- ■ ■ Temporary Lane Separator System
- ⬆ Traffic Signal
- ⬆ Traffic Sign
- x Drum
- 42" Channelizer
- ▬ Arrow Board
- ← Direction of Traffic

① Spacing equals 40' C/C except where noted

**STAGE II**  
 (Sheet 1 of 2)

Design No. 514  
 File No. 30687



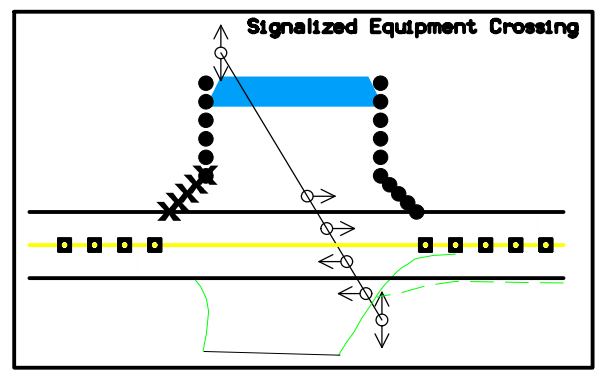
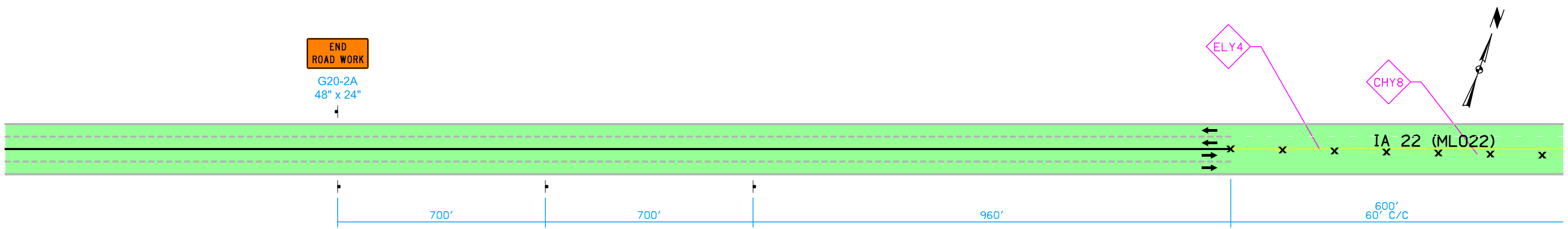
- ■ ■ Temporary Lane Separator System
- ⊣ Traffic Sign
- × Drum
- 42" Channelizer
- ▬ Arrow Board
- ← Direction of Traffic

① Spacing equals 40' C/C except where noted

END ROAD WORK  
G20-2A  
48" x 24"

STAGE II  
(Sheet 2 of 2)

Design No. 514  
File No. 30687

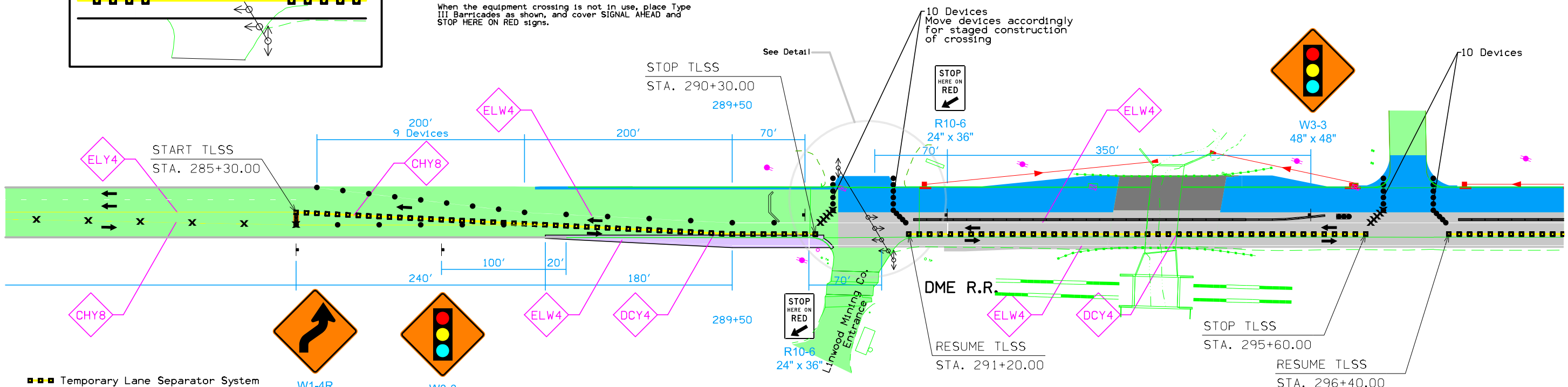


**SIGNAL NOTES:**  
 No detection area required.

Set signal timing as follows: For traffic from all directions, set the yellow indication at approximately 5 seconds and the all-red clearance interval at approximately 2 seconds. Set the minimum green interval on the main road at 20 seconds. Set the green interval for haul road traffic at 12 seconds but may be extended up to a maximum of 30 seconds.

Locate the Stop bars 70 feet in advance of each lane's nearest signal head.  
 Required only if haul road is used during nighttime hours.

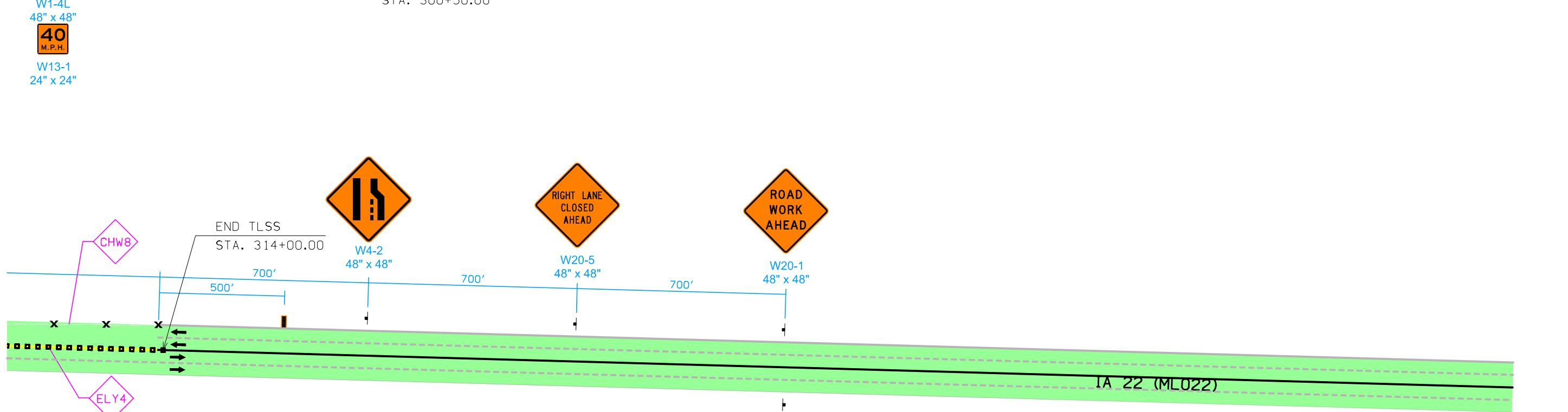
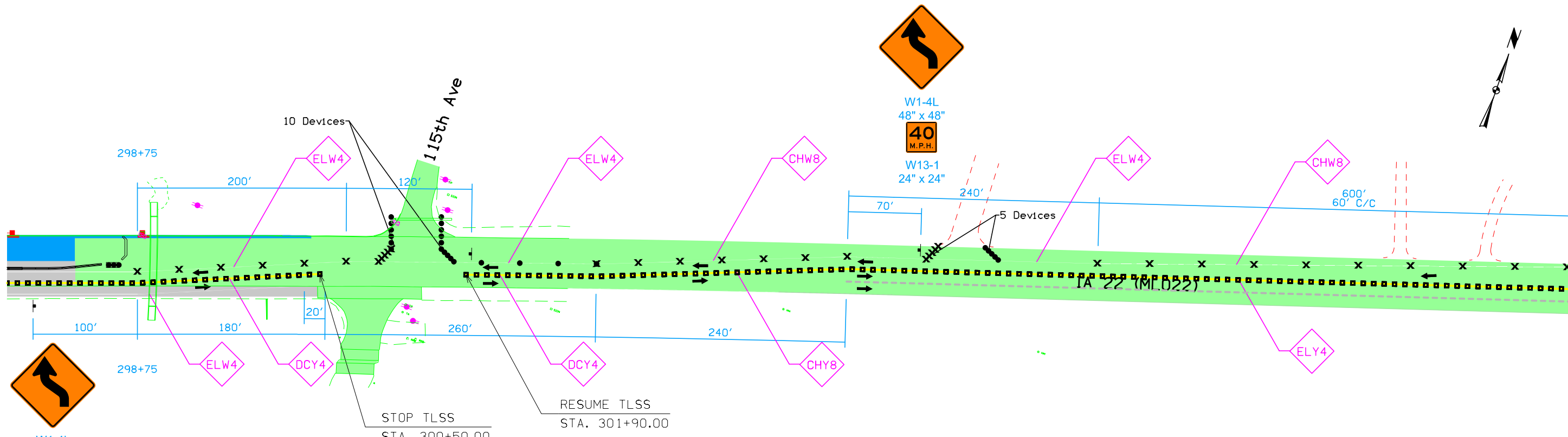
When the equipment crossing is not in use, place Type III Barricades as shown, and cover SIGNAL AHEAD and STOP HERE ON RED signs.



- ■ ■ Temporary Lane Separator System
  - ⊕ Traffic Signal
  - ⊥ Traffic Sign
  - x Drum
  - 42" Channelizer
  - ▬ Arrow Board
  - ← Direction of Traffic
- W1-4R 48" x 48"
- W3-3 48" x 48"
- W13-1 24" x 24"
- ① Spacing equals 40' C/C except where noted

**STAGE III**  
 (Sheet 1 of 2)

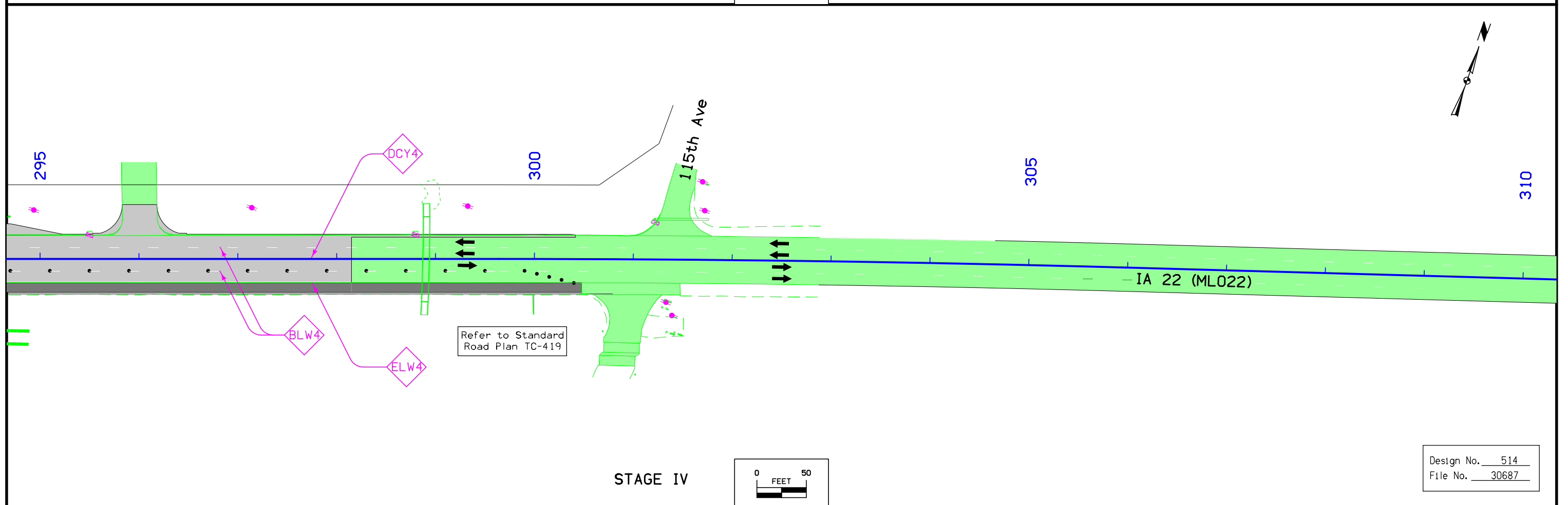
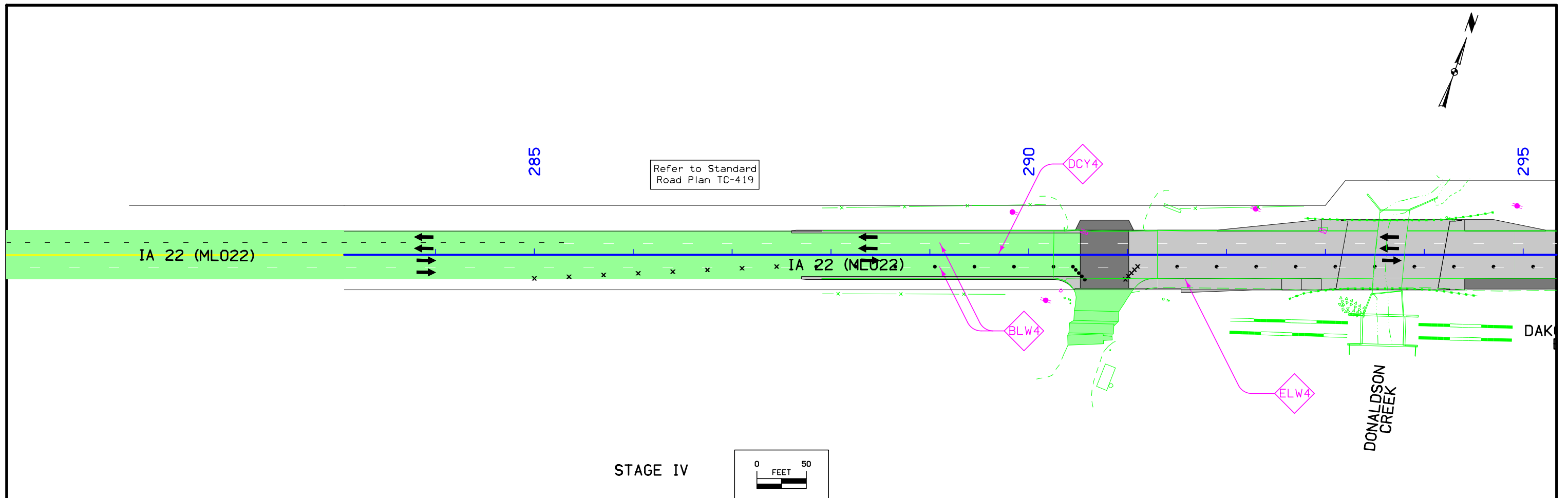
Design No. 514  
 File No. 30687



- ■ ■ Temporary Lane Separator System
  - ⊕ Traffic Signal
  - ⊣ Traffic Sign
  - x Drum
  - 42" Channelizer
  - ▬ Arrow Board
  - ← Direction of Traffic
- ① Spacing equals 40' C/C except where noted

STAGE III  
(Sheet 2 of 2)

Design No. 514  
File No. 30687



Design No. 514  
File No. 30687

### STORM SEWER

\* Bid Item  
\*\* For SW-545

INTAKES AND UTILITY ACCESSES						PIPES													
						Design Length, Slope, and Flowlines are calculated from inside wall to inside wall along CL of pipe. An additional 6 ft length is added to Design Length to account for estimated length to center of structures.													
No.	Location Station and Offset	*Type or Standard Road Plan	Form Grade	Bottom Well	Extension Length**	Notes	Line Number	Intake/Utility Access No.		Class 'D'	Pipe Diameter	Bid* Length	Design Length	Slope %	Flow Lines			Pipe Profile Sheet No.	Notes
			Elev.	Elev.	FT			From	To		IN	FT	FT		Inlet Elevation	Outlet Elevation	Other Elevation		
1	297+55.00; 25.00' LT	SW-508	570.76	565.25			P-1	1	2	2000	24	101.9	95.9	2	565.76	563.84			
2	296+55.07; 25.00' LT	SW-508	570.51	563.04			P-2	2	3	2000	24	106.6	100.6	2	563.54	561.53			
3	295+48.50; 25.00' LT	SW-509	570.81	560.69			P-3	3	4	2000	24	135	132	3.1	561.23	557.18			
3	295+48.50; 25.00' LT	SW-509	570.81	560.69			P-3	3	4	2000	24	135	132	3.1	561.23	557.18			
4	294+10.00; 58.15' LT	24" RF-3					P-3	3	4	2000	24	135	132	3.1	561.23	557.18			
5A	290+53.55; 24.84' LT	RF-2, 'C-1'					P-5A	5A	5	2000	15	82.5	79.5	1.5	562.67	561.48			Use RF-2, Type 'C-1' pipe connection
5	291+35.00; 25.00' LT	SW-508	569.46	560.25			P-5	5	7	2000	24	221	218	3	560.75	554.19			(1) (4); Stage 3 Construction
6	293+00.45; 34.00' LT	SW-508	568.98	560.13	(3)		T-1	6	Exist	2000	15	14	8	1.5	560.63	560.51			(1); Stage 1 Construction
7	293+60.00; 49.70' LT	24" RF-3					P-8	8	Exist	2000	15	7	4	0.4	564.81	564.8			(1) (4)
8	298+79.77; 25.00' LT	SW-508	571.09	564.31			Exist	Exist	9	Exist	Exist	Exist	Exist	Exist	Exist	Exist			(1)
9	291+35.00; 33.00' RT	SW-401 48"	569.13	559.83	(1)		P-9	9	10	3000	24	183.4	180.4	0.6	560.33	559.21			(2) (4) (5)
10	293+23.47; 29.62' RT	24" RF-3			(2)														
E1	298+86.40; 27.00' RT	SW-211																	
Totals:										Totals:									
SW-508										15" 2000D									
SW-509										24" 2000D									
48" SW-401										24" 3000D									
24" RF-3 Aprons																			
SW-211																			

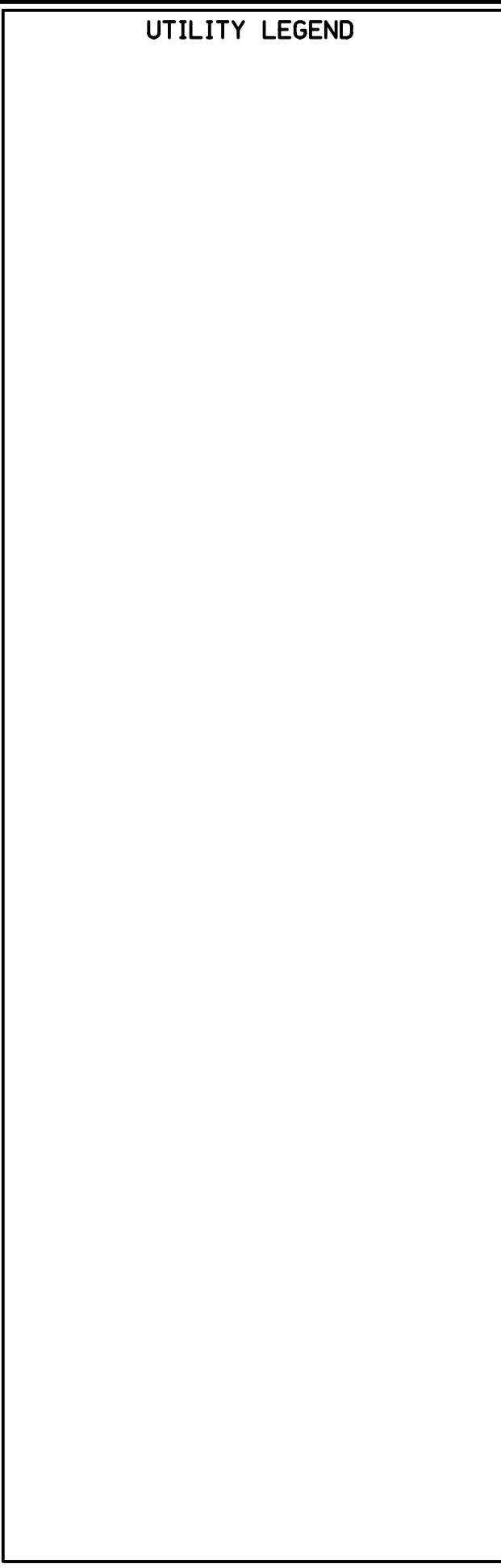
- (1) Retrofit the cut existing pipe to Manhole Intake #9 with SRP RF-2, Type 'C-2' adaptor.
- (2) Coordinate storm sewer construction with the construction of the abutment piles.
- (3) Temporary intake to be used in Stages 1-2. Remove during Stage 3.
- (4) Use RF-14 connected pipe joints.
- (5) Adjust the Inlet Elevation to provide a minimum of a 0.3' drop from the existing pipe flow line that outlets into Manhole #9.



### SURVEY SYMBOLS

- x — FW Wire Fence
- PR Electric Riser Pole
- ⊛ LUM Luminaire
- ⊙ TA TA Tower Anchor
- RRS Railroad Signal
- PPA Power Pole Co. 1
- +—+—+—+— GDL Guard Rail Steel
- SIGN SI Sign
- ⊙ OUT Tile Outlet
- BB BB Billboard
- SIGN SL Speed Limit Sign
- ⊙ MM MM Mile Marker Post
- ⊙ TP TPD Telephone Pedestal
- RRB Railroad Signal Box
- Tile · TIL Tile Line
- ← DU Centerline Draw or Stream (Up)
- D Centerline Draw or Stream (Down)
- ····· EW Edge of Water
- BNK Stream Bank
- ▲▲▲▲▲▲▲▲ RIP Rip-Rap
- RR Centerline of Railroad Tracks
- T1 · TLA Underground Telephone Line Co. 1
- E2 · ELB Underground Electric Line Co. 2
- W · WLA Underground Water Line Co. 1
- GHA Underground High Pres Gas Co 1
- ▣ IN Storm Sewer Intake
- ===== RET Retaining Walls
- TLNR Tree Line Right
- TLNL Tree Line Left
- St.S. · STA Storm Sewer Line Co. 1

### UTILITY LEGEND



### PLAN VIEW COLOR LEGEND OF STORM SEWER SHEETS

LINEWORK	Design Color No.	Description
Gray, Dark	(112)	Existing Topographic Features, Utilities, and Labels
Black	(17)	Proposed Storm Sewer Details, Alignment, Stationing, Tic Marks, and Alignment Annotation
SHADING	Design Color No.	Description
Gray, Light	(48)	Proposed Pavement Shading

### PROFILE VIEW COLOR LEGEND OF STORM SEWER SHEETS

LINEWORK	Design Color No.	Description
Gray, Dark	(112)	Existing Ground Line Profile and Existing Utilities Information
Black	(17)	Proposed Pipes and Intakes

### PLAN VIEW LINE STYLE LEGEND OF STORM SEWER SHEETS

- ▶---▶---▶---▶---▶ Plug and Abandon Existing Pipe or Structure
- - - - - Removal of Existing Pipe or Structure
- Previously Constructed Pipe or Structure
- ▶ Direction of Pipe Flow

### PROFILE VIEW LINE STYLE LEGEND OF STORM SEWER SHEETS

- Existing Ground
- Proposed Ground
- Previously Constructed Pipe or Structure
- Proposed Pipe or Structure

Reference Point

Station

▲ Section Corner

----- Ground Line Intercept

////// Saw Cut

----- Guardrail

▣ Clearing & Grubbing Area

▣ Pavement Removal

▣

▣

### RIGHT-OF-WAY LEGEND

- ▲ Proposed Right-of-Way
- ▲ Existing and Proposed Right-of-Way
- ▲ Easement and Existing Right-of-Way
- ▣ Borrow
- Easement (Temporary)
- Easement
- X Excess
- A/C Access Control

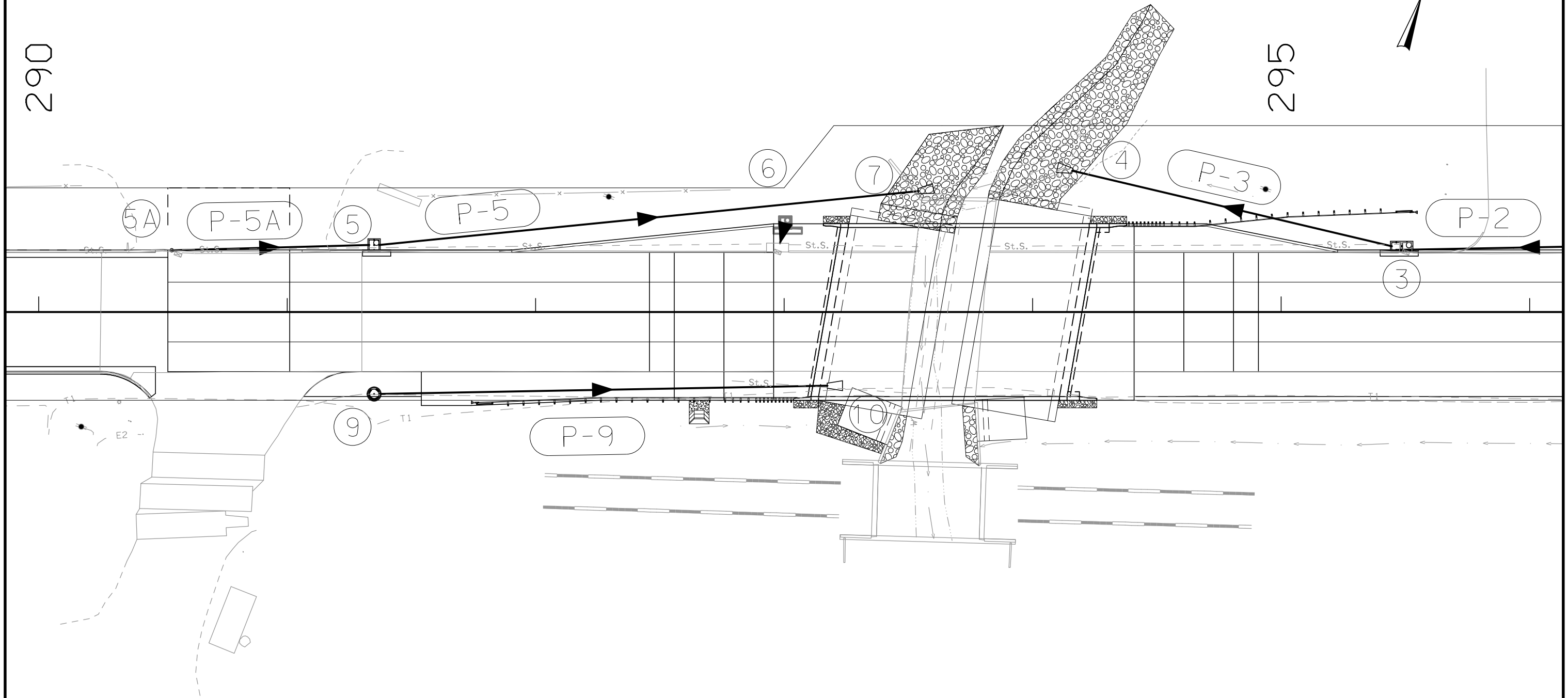
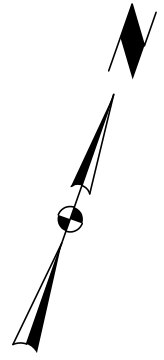
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# STORM SEWER LEGEND AND SYMBOL INFORMATION SHEET

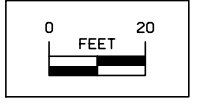
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290

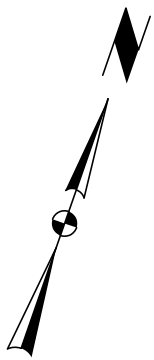
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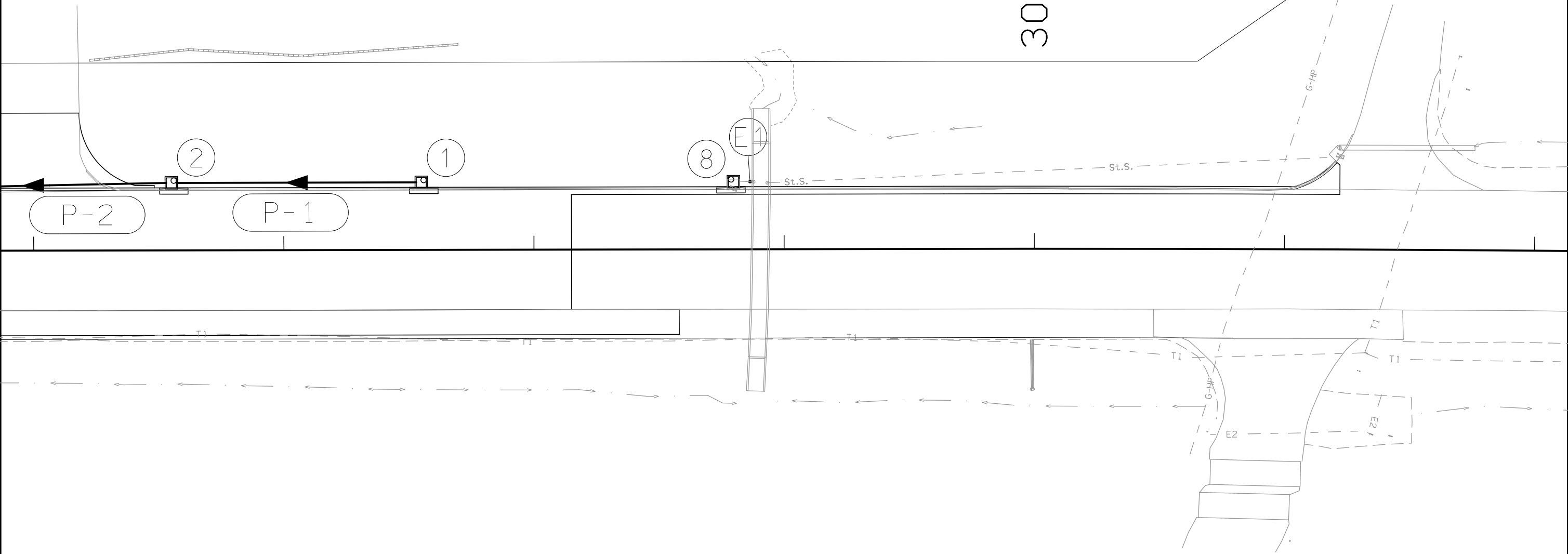
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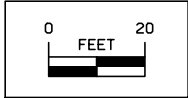
# Storm Sewer Design



300

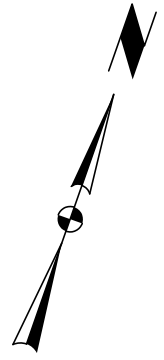


Design No. 514  
 File No. 30687



# Storm Sewer Design

- - - - - Remove St.S Pipe  
 ▶▶▶▶▶ Plug & Abandon St.S Pipe



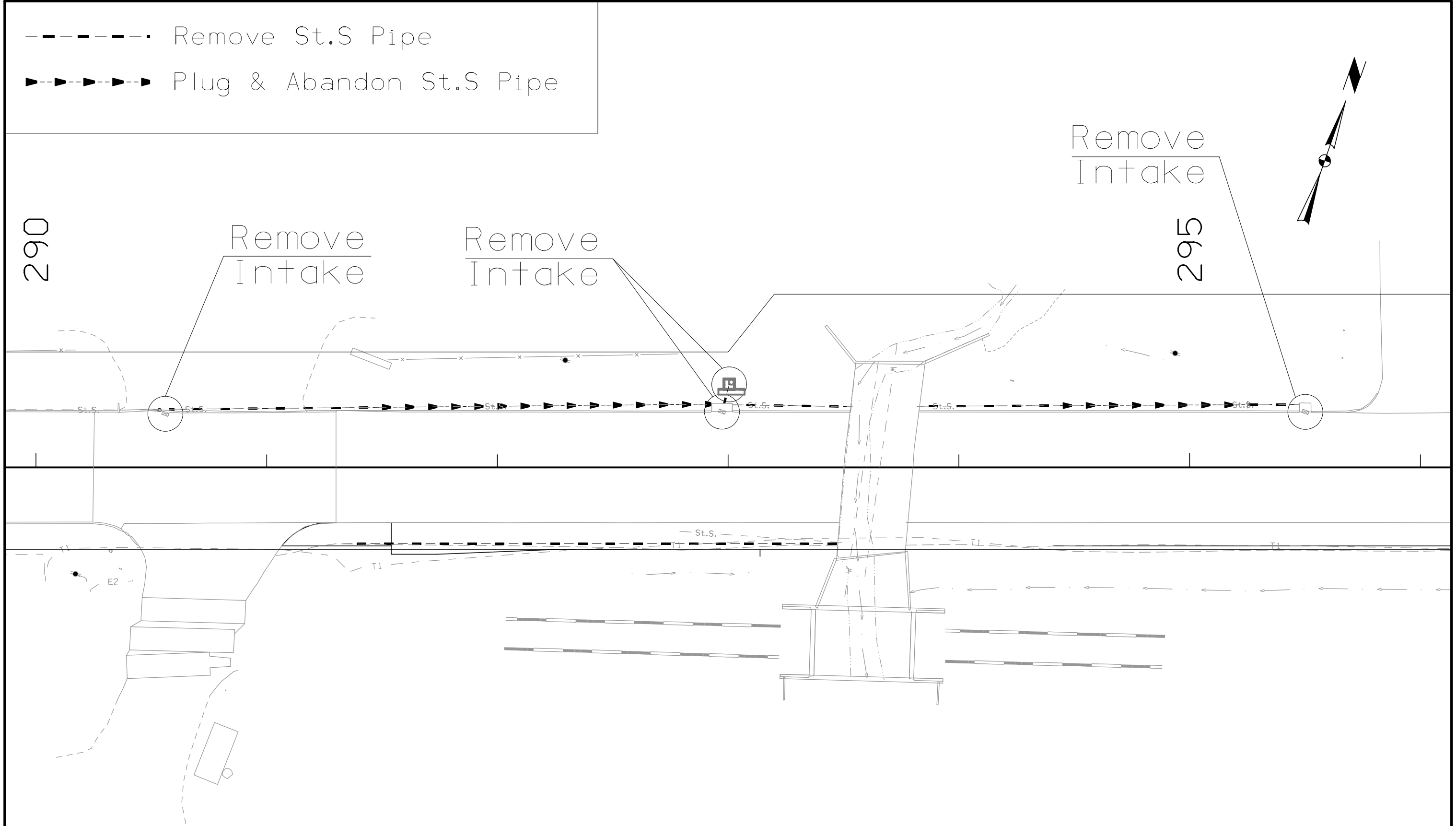
290

Remove Intake

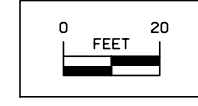
Remove Intake

295

Remove Intake

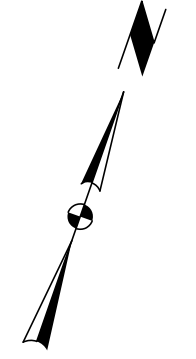


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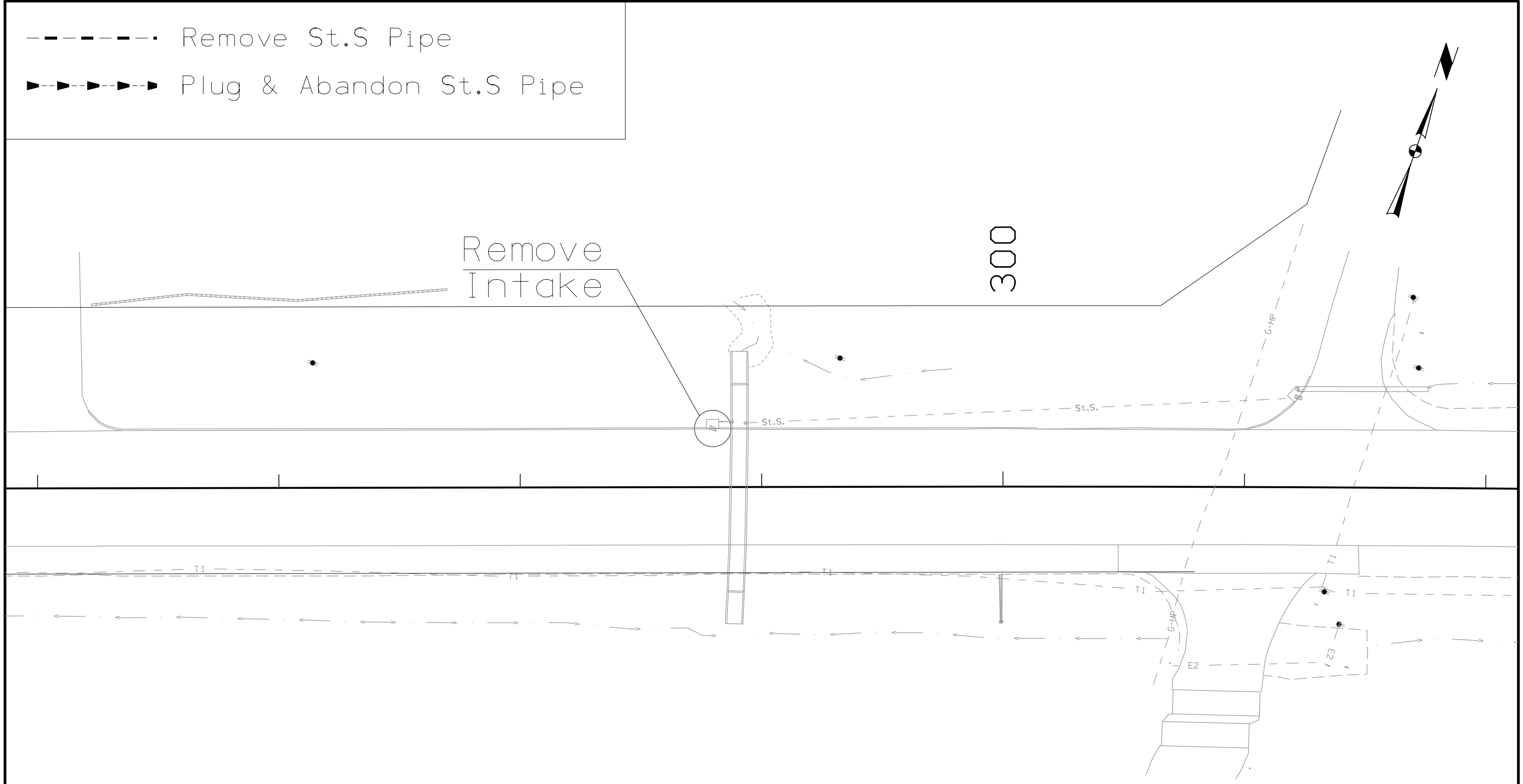
# Storm Sewer Removals

- Remove St.S Pipe
- ▶▶▶▶▶ Plug & Abandon St.S Pipe

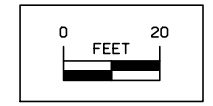


Remove Intake

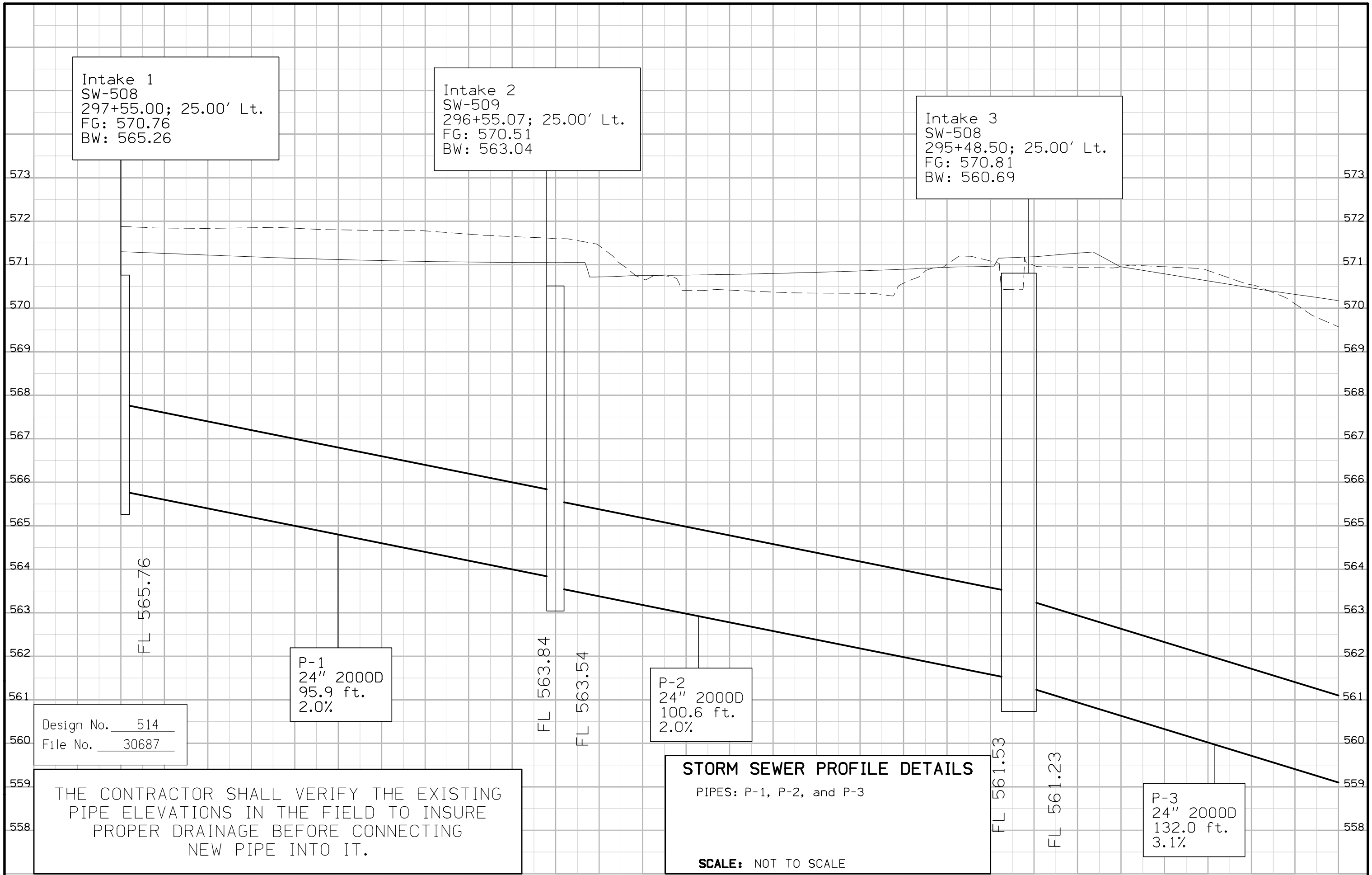
300



Design No. 514  
File No. 30687



# Storm Sewer Removals



Intake 1  
 SW-508  
 297+55.00; 25.00' Lt.  
 FG: 570.76  
 BW: 565.26

Intake 2  
 SW-509  
 296+55.07; 25.00' Lt.  
 FG: 570.51  
 BW: 563.04

Intake 3  
 SW-508  
 295+48.50; 25.00' Lt.  
 FG: 570.81  
 BW: 560.69

P-1  
 24" 2000D  
 95.9 ft.  
 2.0%

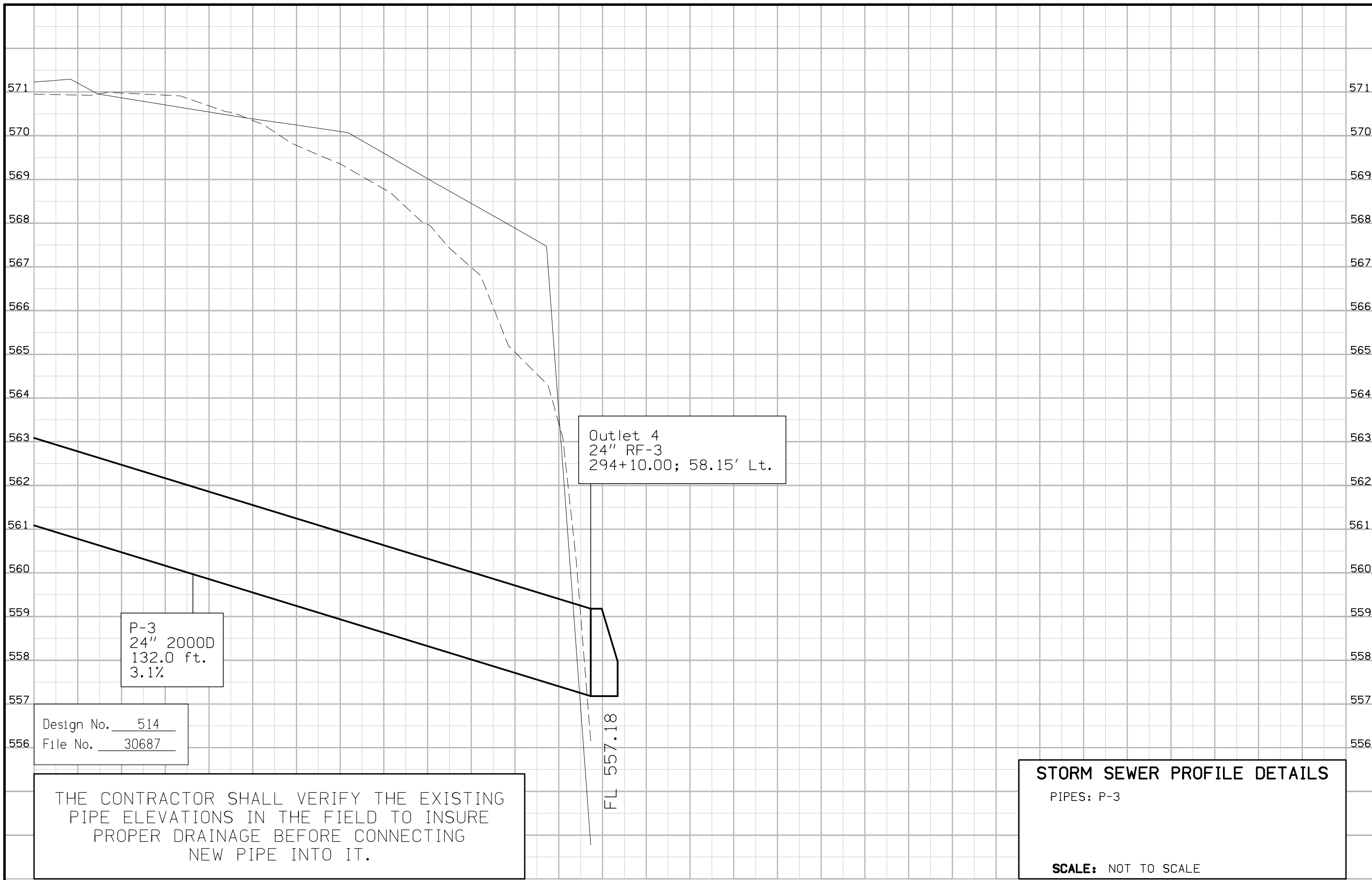
P-2  
 24" 2000D  
 100.6 ft.  
 2.0%

P-3  
 24" 2000D  
 132.0 ft.  
 3.1%

Design No. 514  
 File No. 30687

THE CONTRACTOR SHALL VERIFY THE EXISTING PIPE ELEVATIONS IN THE FIELD TO INSURE PROPER DRAINAGE BEFORE CONNECTING NEW PIPE INTO IT.

**STORM SEWER PROFILE DETAILS**  
 PIPES: P-1, P-2, and P-3  
 SCALE: NOT TO SCALE



Outlet 4  
 24" RF-3  
 294+10.00; 58.15' Lt.

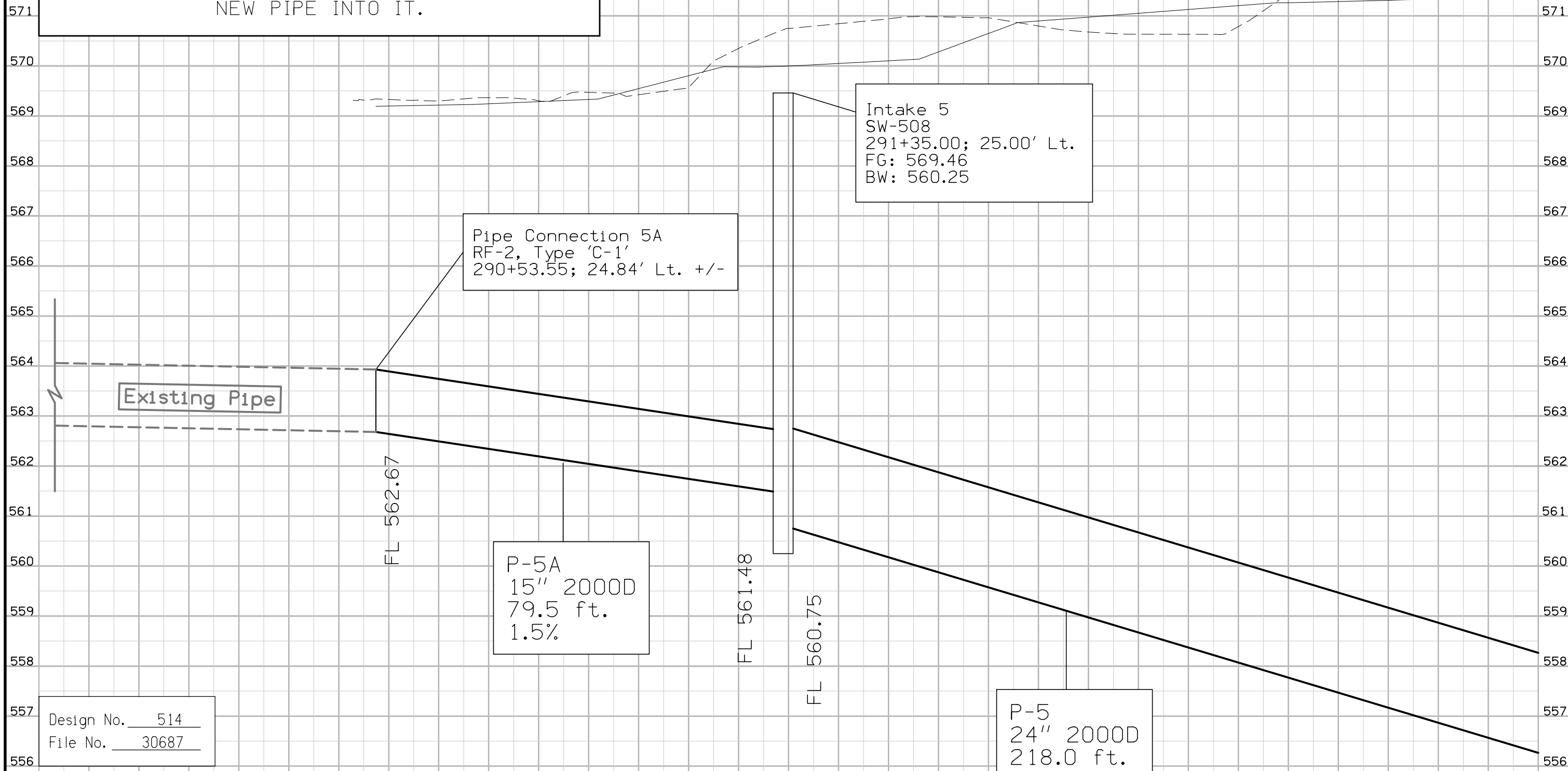
P-3  
 24" 2000D  
 132.0 ft.  
 3.1%

Design No. 514  
 File No. 30687

THE CONTRACTOR SHALL VERIFY THE EXISTING  
 PIPE ELEVATIONS IN THE FIELD TO INSURE  
 PROPER DRAINAGE BEFORE CONNECTING  
 NEW PIPE INTO IT.

**STORM SEWER PROFILE DETAILS**  
 PIPES: P-3  
  
**SCALE:** NOT TO SCALE

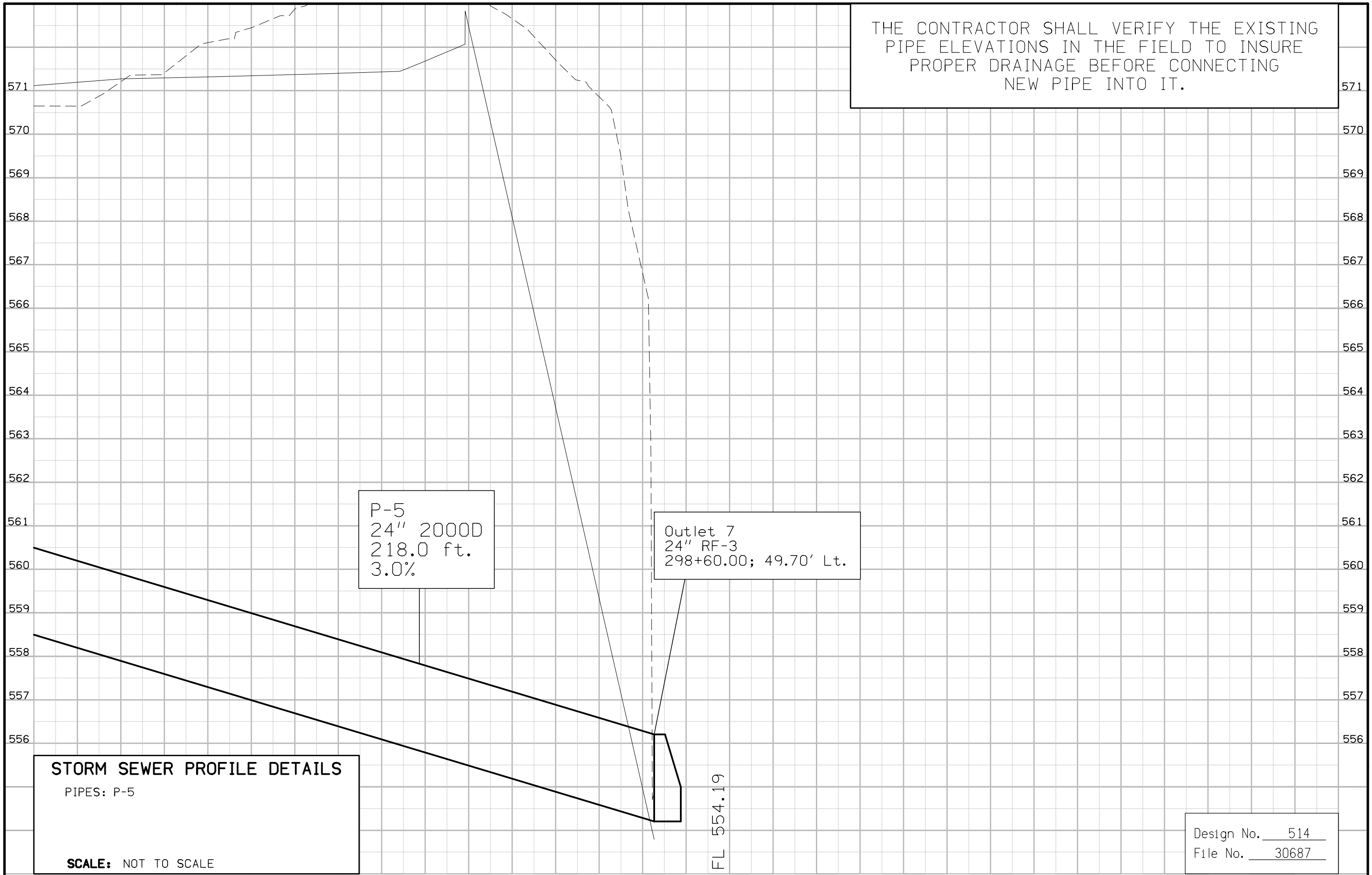
THE CONTRACTOR SHALL VERIFY THE EXISTING PIPE ELEVATIONS IN THE FIELD TO INSURE PROPER DRAINAGE BEFORE CONNECTING NEW PIPE INTO IT.

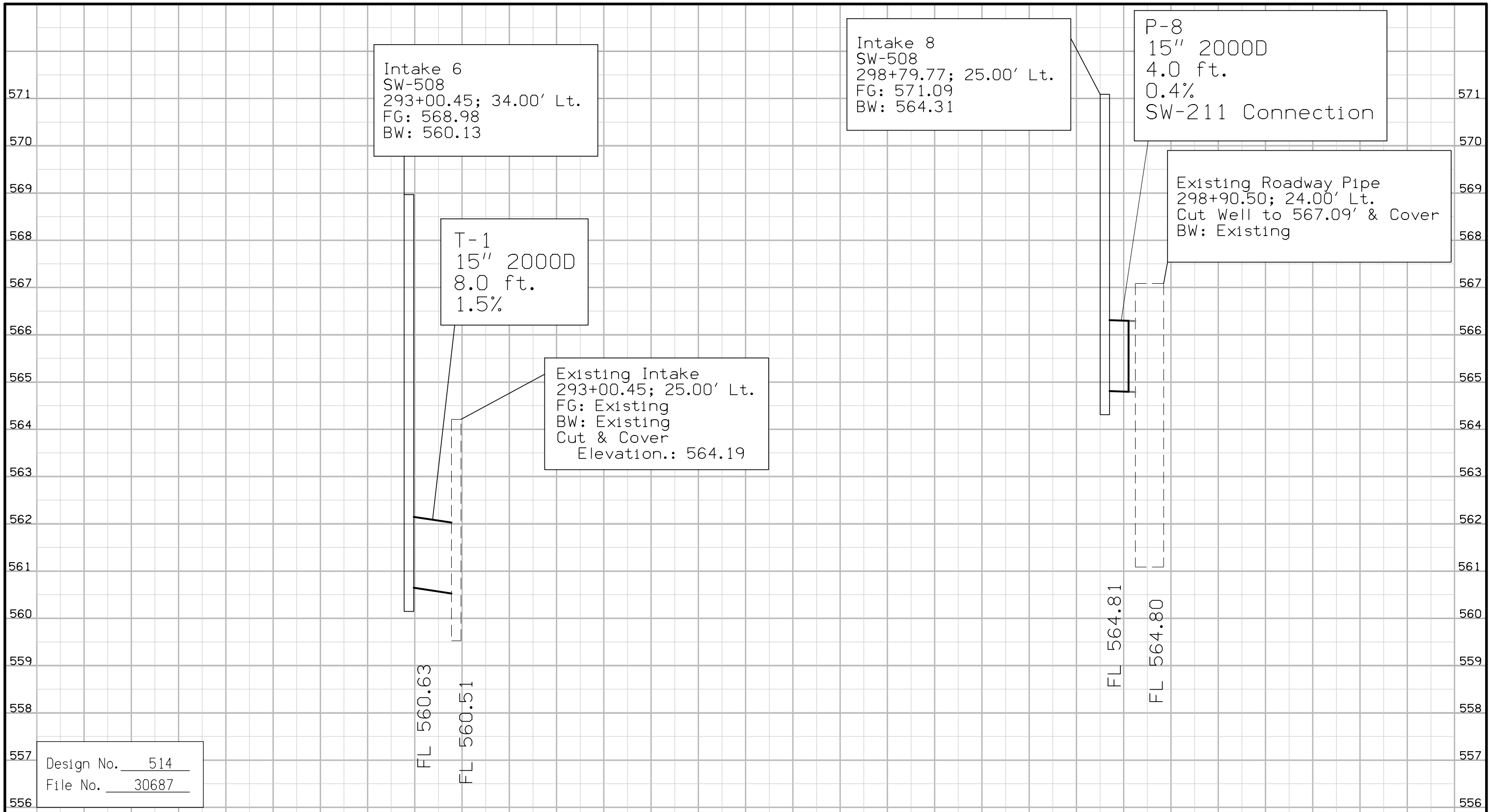


Design No. 514  
File No. 30687

**STORM SEWER PROFILE DETAILS**  
PIPES: P-5A and P-5  
  
**SCALE:** NOT TO SCALE



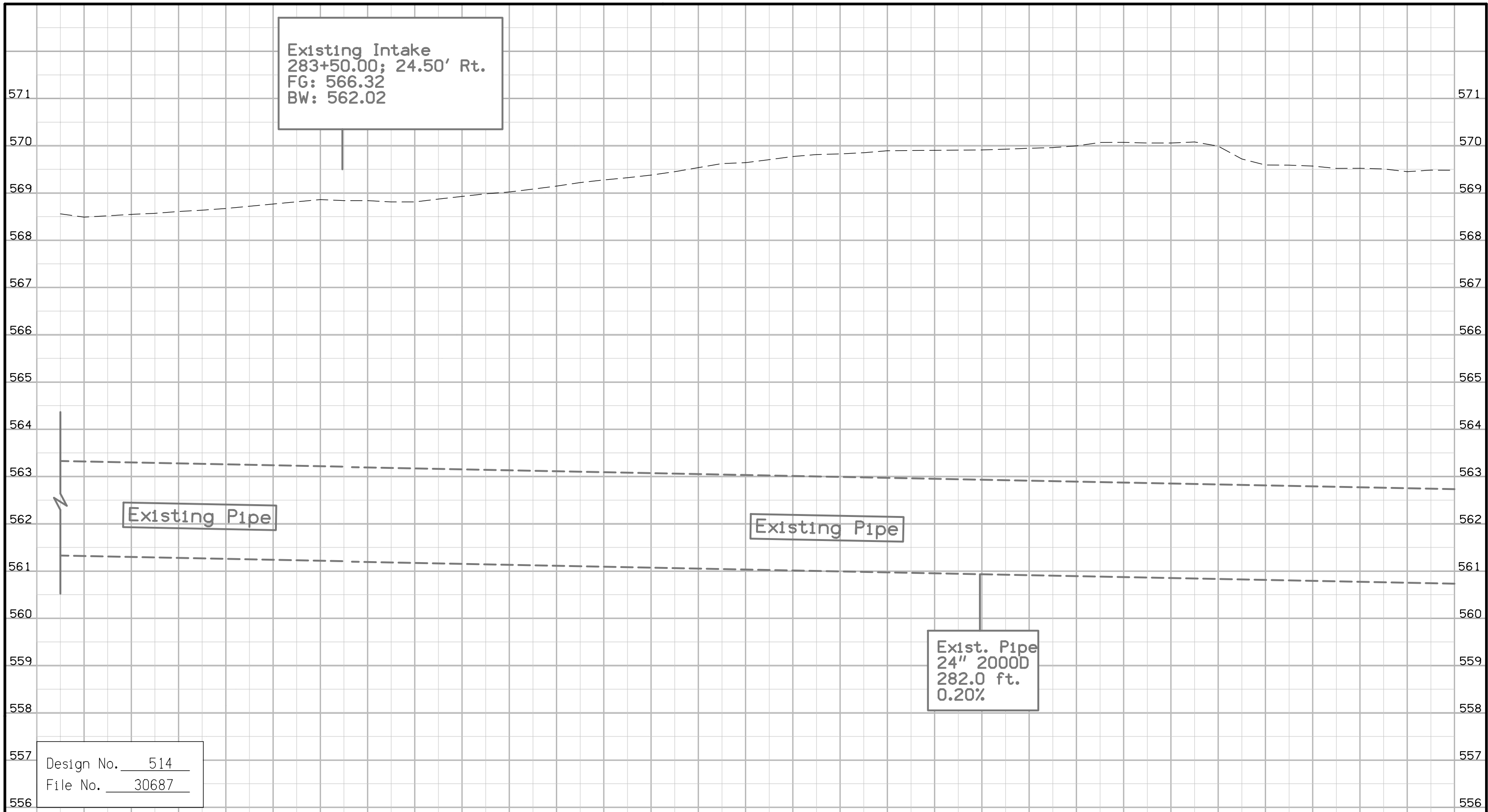




Design No. 514  
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**STORM SEWER PROFILE DETAILS**  
PIPES: T-1 and P-8  
  
**SCALE:** NOT TO SCALE

THE CONTRACTOR SHALL VERIFY THE EXISTING PIPE ELEVATIONS IN THE FIELD TO INSURE PROPER DRAINAGE BEFORE CONNECTING NEW PIPE INTO IT.



Existing Intake  
 283+50.00; 24.50' Rt.  
 FG: 566.32  
 BW: 562.02

Existing Pipe

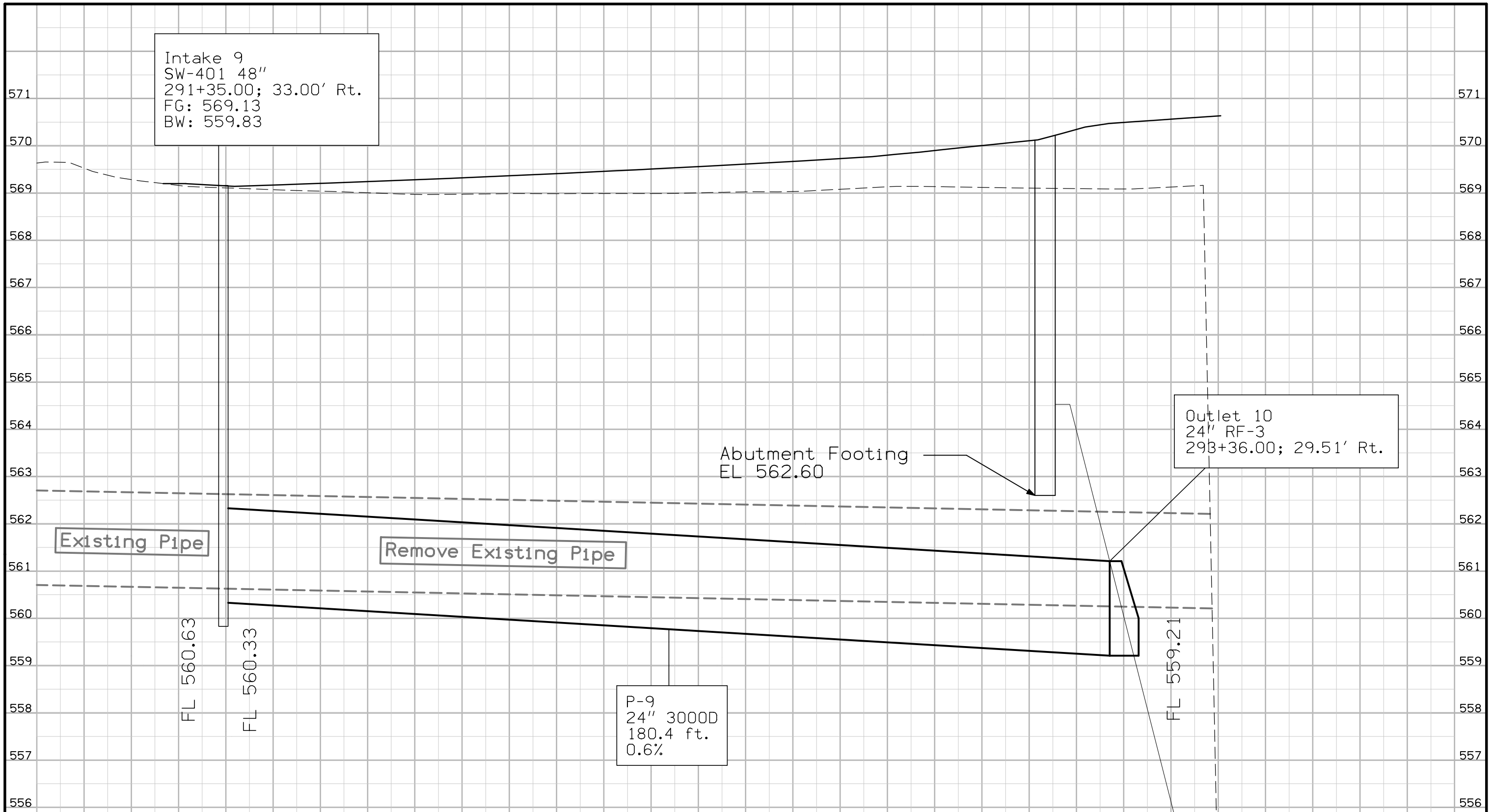
Existing Pipe

Exist. Pipe  
 24" 2000D  
 282.0 ft.  
 0.20%

Design No. 514  
 File No. 30687

**STORM SEWER PROFILE DETAILS**  
 PIPES: Existing Pipe  
  
**SCALE:** NOT TO SCALE

THE CONTRACTOR SHALL VERIFY THE EXISTING PIPE ELEVATIONS IN THE FIELD TO INSURE PROPER DRAINAGE BEFORE CONNECTING NEW PIPE INTO IT.



Intake 9  
 SW-401 48"  
 291+35.00; 33.00' Rt.  
 FG: 569.13  
 BW: 559.83

Outlet 10  
 24" RF-3  
 293+36.00; 29.51' Rt.

Abutment Footing  
 EL 562.60

Existing Pipe

Remove Existing Pipe

P-9  
 24" 3000D  
 180.4 ft.  
 0.6%

FL 560.63

FL 560.33

FL 559.21

**STORM SEWER PROFILE DETAILS**  
 PIPES: Existnig Pipe and P-9  
 SCALE: NOT TO SCALE

Design No. 514  
 File No. 30687

THE CONTRACTOR SHALL VERIFY THE EXISTING PIPE ELEVATIONS IN THE FIELD TO INSURE PROPER DRAINAGE BEFORE CONNECTING NEW PIPE INTO IT.

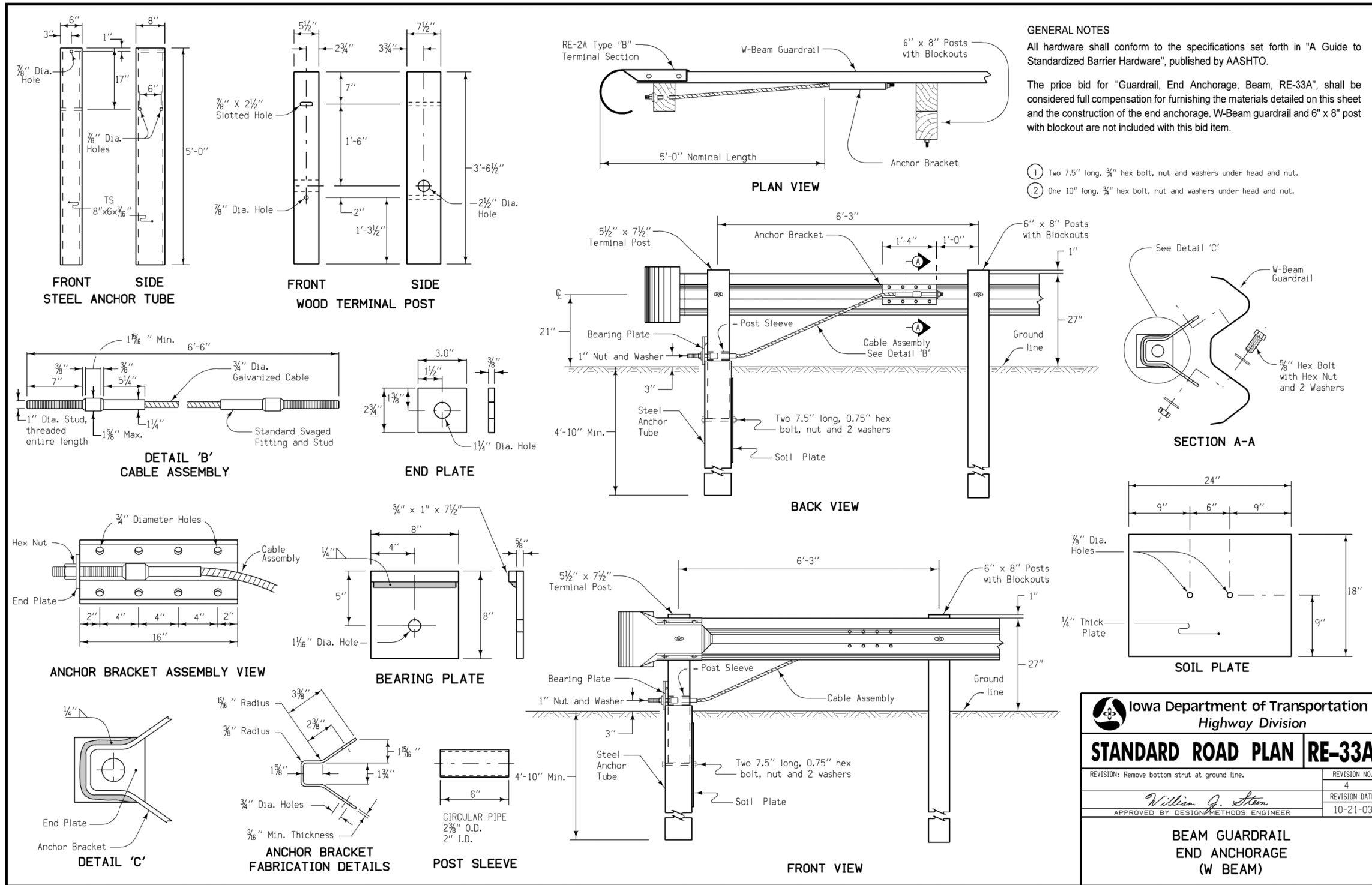


### TABULATION OF TEMPLATE QUANTITIES AND ADJUSTMENTS

Refer to Standard Plan EW-101 and RL-1B.

STATION	TOTAL CUT	CLASS 10 UNSUIT CUT	ADJUSTED CLASS 10 TOTAL	TOTAL FILL	CLASS 10 SUITABLE + 0% SHRINK	TOTAL FILL WITH SHRINK	APPROX. FILL VOLUME BELOW 3 FT	APPROX. FILL VOLUME BELOW 5 FT					STATION	TOTAL CUT	CLASS 10 UNSUIT CUT	ADJUSTED CLASS 10 TOTAL	TOTAL FILL	CLASS 10 SUITABLE + 0% SHRINK	TOTAL FILL WITH SHRINK	APPROX. FILL VOLUME BELOW 3 FT	APPROX. FILL VOLUME BELOW 5 FT					
293+00.00	19	19	19	5	5	5	0	0																		
293+25.00	9	9	9	3	3	3	0	0																		
293+50.00																										
294+00.00																										
294+50.00	7	7	7	14	14	14	0	0																		
294+75.00	7	7	7	10	10	10	0	0																		
295+00.00	4	4	4	6	6	6	0	0																		
295+25.00	3	3	3	7	7	7	0	0																		
295+50.00	6	6	6	10	10	10	0	0																		
295+75.00	11	11	11	9	9	9	0	0																		
296+00.00	16	16	16	8	8	8	0	0																		
296+25.00	21	21	21	6	6	6	0	0																		
296+50.00	25	25	25	6	6	6	0	0																		
296+75.00	28	28	28	7	7	7	0	0																		
297+00.00	30	30	30	7	7	7	0	0																		
297+25.00	31	31	31	6	6	6	0	0																		
297+50.00	30	30	30	6	6	6	0	0																		
297+75.00	30	30	30	6	6	6	0	0																		
298+00.00	30	30	30	6	6	6	0	0																		
298+25.00	19	19	19	6	6	6	0	0																		
298+50.00	7	7	7	6	6	6	0	0																		
298+75.00	7	7	7	6	6	6	0	0																		
299+00.00	7	7	7	6	6	6	0	0																		
299+25.00	7	7	7	6	6	6	0	0																		
299+50.00	7	7	7	6	6	6	0	0																		
299+75.00	7	7	7	6	6	6	0	0																		
300+00.00	7	7	7	6	6	6	0	0																		
300+25.00	7	7	7	6	6	6	0	0																		
300+50.00	7	7	7	7	7	7	0	0																		
300+75.00	7	7	7	8	8	8	0	0																		
301+00.00	7	7	7	7	7	7	0	0																		
302+00.00	46	46	46	14	14	14	0	0																		
302+25.00	16	16	16				0	0																		
302+50.00	16	16	16				0	0																		
302+75.00	17	17	17				0	0																		
Stage 3 Quantities	708	708	708	300	300	300	0	0																		
Stage 4																										
288+00.00							0	0																		
288+25.00	7	7	7	5	5	5	0	0																		
288+50.00	7	7	7	6	6	6	0	0																		
288+75.00	7	7	7	6	6	6	0	0																		
289+00.00	7	7	7	7	7	7	0	0																		
289+25.00	7	7	7	7	7	7	0	0																		
289+50.00	7	7	7	7	7	7	0	0																		
289+75.00	7	7	7	7	7	7	0	0																		
290+00.00	7	7	7	7	7	7	0	0																		
290+25.00	7	7	7	7	7	7	0	0																		
298+75.00	4	4	4	4	4	4	0	0																		
299+00.00	1	1	1	7	7	7	0	0																		
299+25.00	2	2	2	22	22	22	0	0																		
299+50.00							0	0																		
Stage 4 Quantities	70	70	70	92	92	92	0	0																		
Total Quantities	3219	3219	3219	560	560	560	0	0																		

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<b>STANDARD ROAD PLAN RE-33A</b>	
REVISION: Remove bottom strut at ground line.	REVISION NO. 4
	REVISION DATE 10-21-03
APPROVED BY DESIGN METHODS ENGINEER	
<b>BEAM GUARDRAIL END ANCHORAGE (W BEAM)</b>	

**LINE STYLE LEGEND OF CROSS SECTION SHEETS (ROAD)**

- - - - - - Existing Ground Line
- Proposed Template
- Proposed Topsoil Placement
- - - - - Additional Topsoil Removal
- Subgrade Treatment
- - - - - Granular Shoulder
- Pavement
- - - - - Existing Pipe\RCB
- Proposed Pipe\RCB
- Proposed Dike
- All Elements Associated with Proposed Entrances

**LINE STYLE LEGEND OF CROSS SECTION SHEETS (SOILS)**

- TS——— Topsoil (Class 10)
- TS A——— Topsoil (Type A Disposal)
- TS B——— Topsoil (Type B Disposal)
- TS C——— Topsoil (Type C Disposal)
- CL 10——— Class 10 Materials
- SEL LO——— Select Loams And Clay-Loams
- SEL SA——— Select Sand
- UNS A——— Unsuitable Type A Disposal
- UNS B——— Unsuitable Type B Disposal
- UNS C——— Unsuitable Type C Disposal
- SHALE——— Shale
- WASTE——— Waste
- B&W LS——— Broken and Weathered Rock
- ROCK——— Solid Rock
- BLDRS——— Boulders

Note: All layer lines and descriptions identify layers above the line.

Note: Vertical or near vertical lines connecting soil layers at edges of cross sections are only for the purpose of calculating template quantities and do not depict soil stratification.

**SYMBOL LEGEND OF CROSS SECTION SHEETS**

- Existing ROW  
|  
Existing Right-of-Way Limit
- Proposed ROW  
|  
Proposed Right-of-Way Limit
- Temporary ROW  
|  
Temporary Right-of-Way Limit

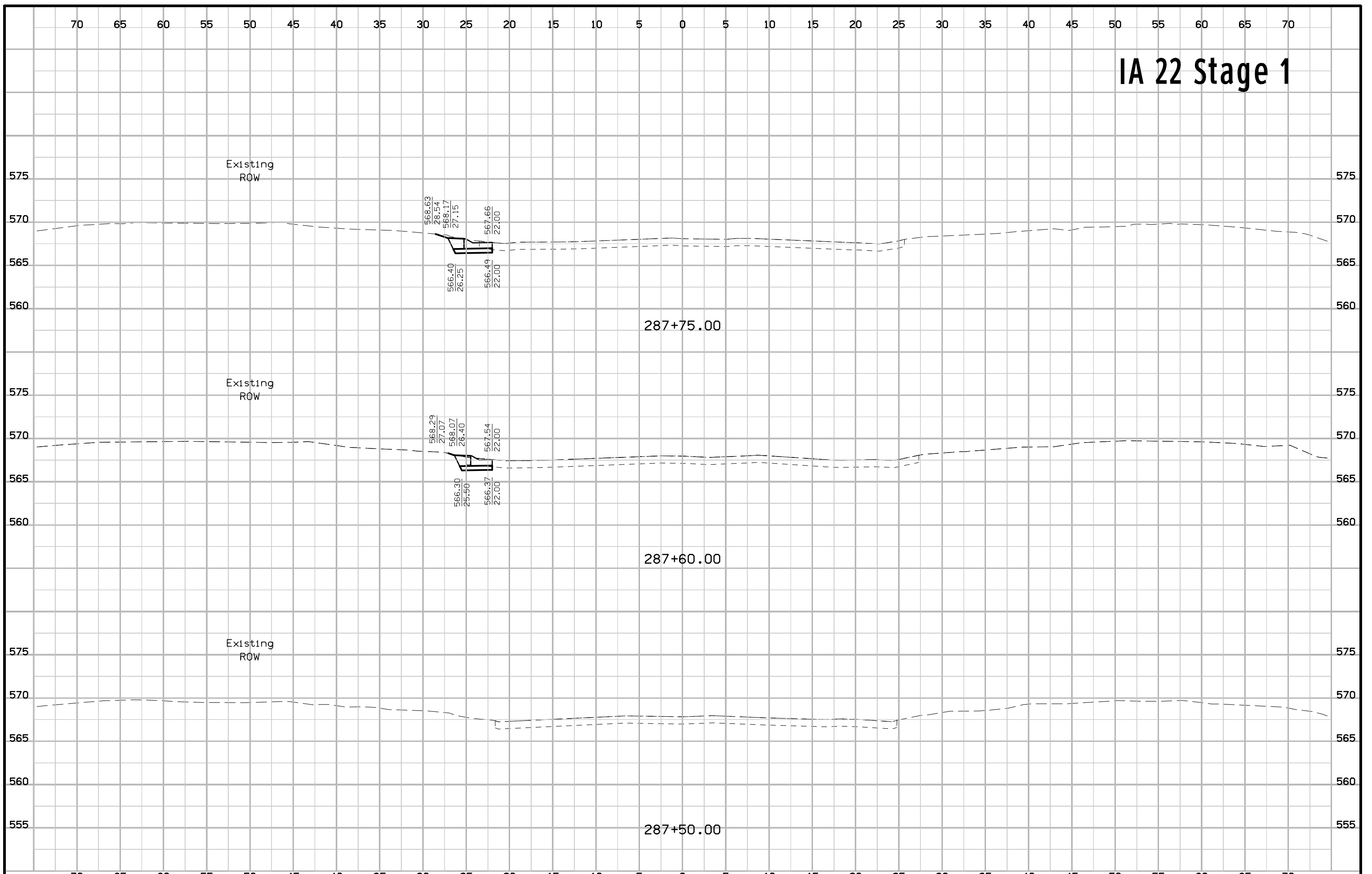
**CROSS SECTION  
LEGEND AND SYMBOL  
INFORMATION SHEET**

(COVERS SHEET SERIES W, X, Y, & Z)

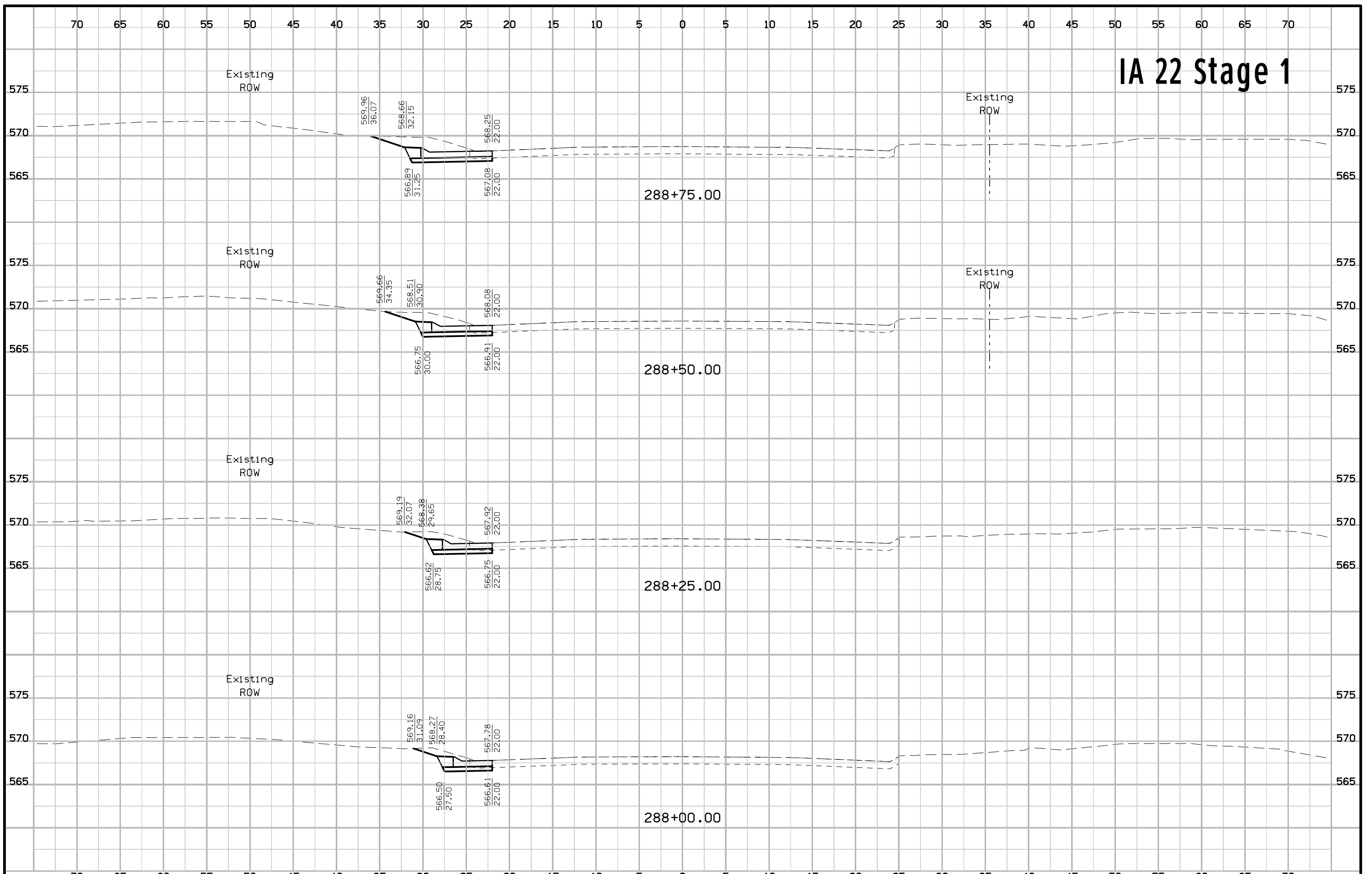
Design No. 514  
File No. 30687



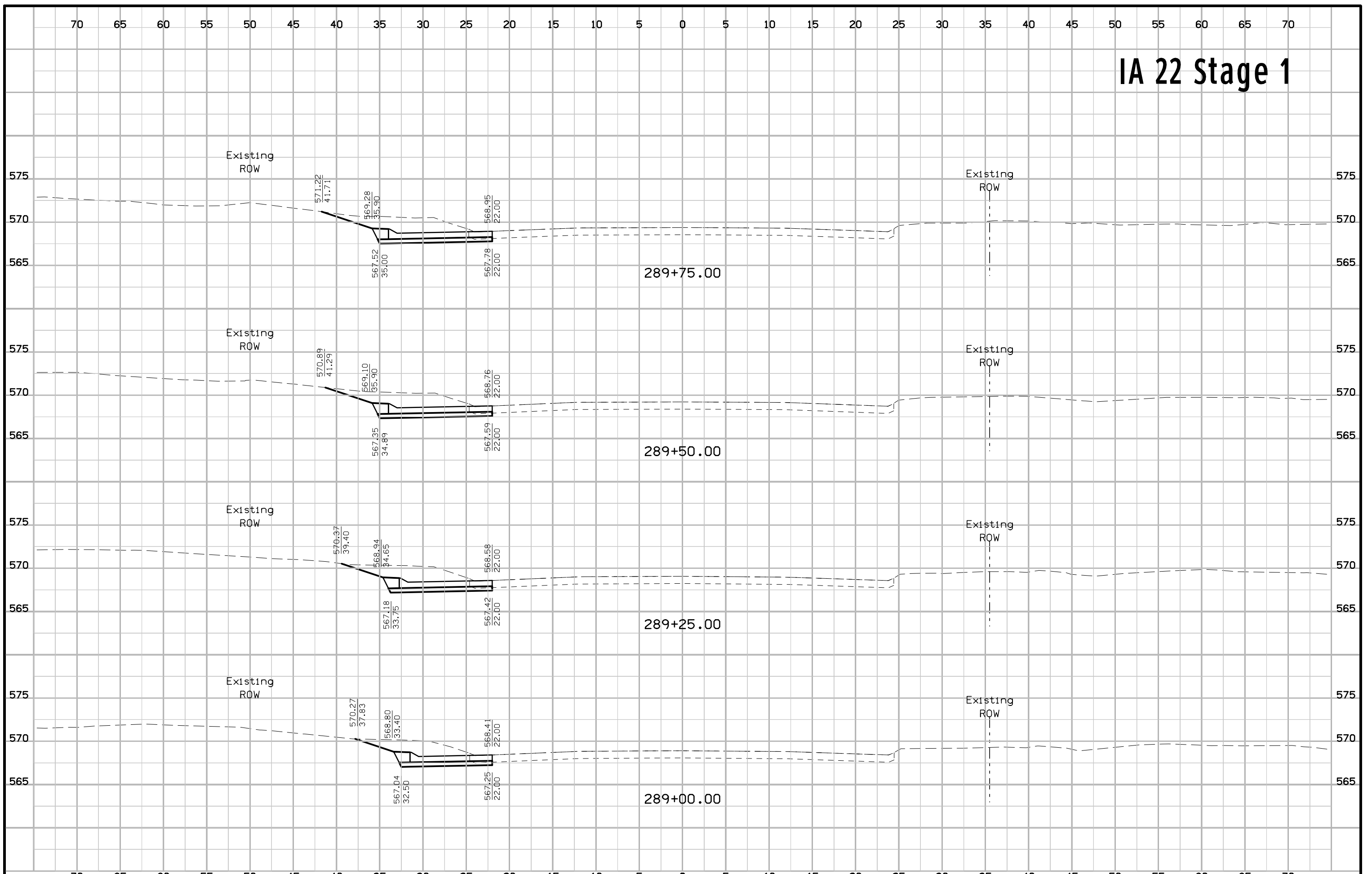
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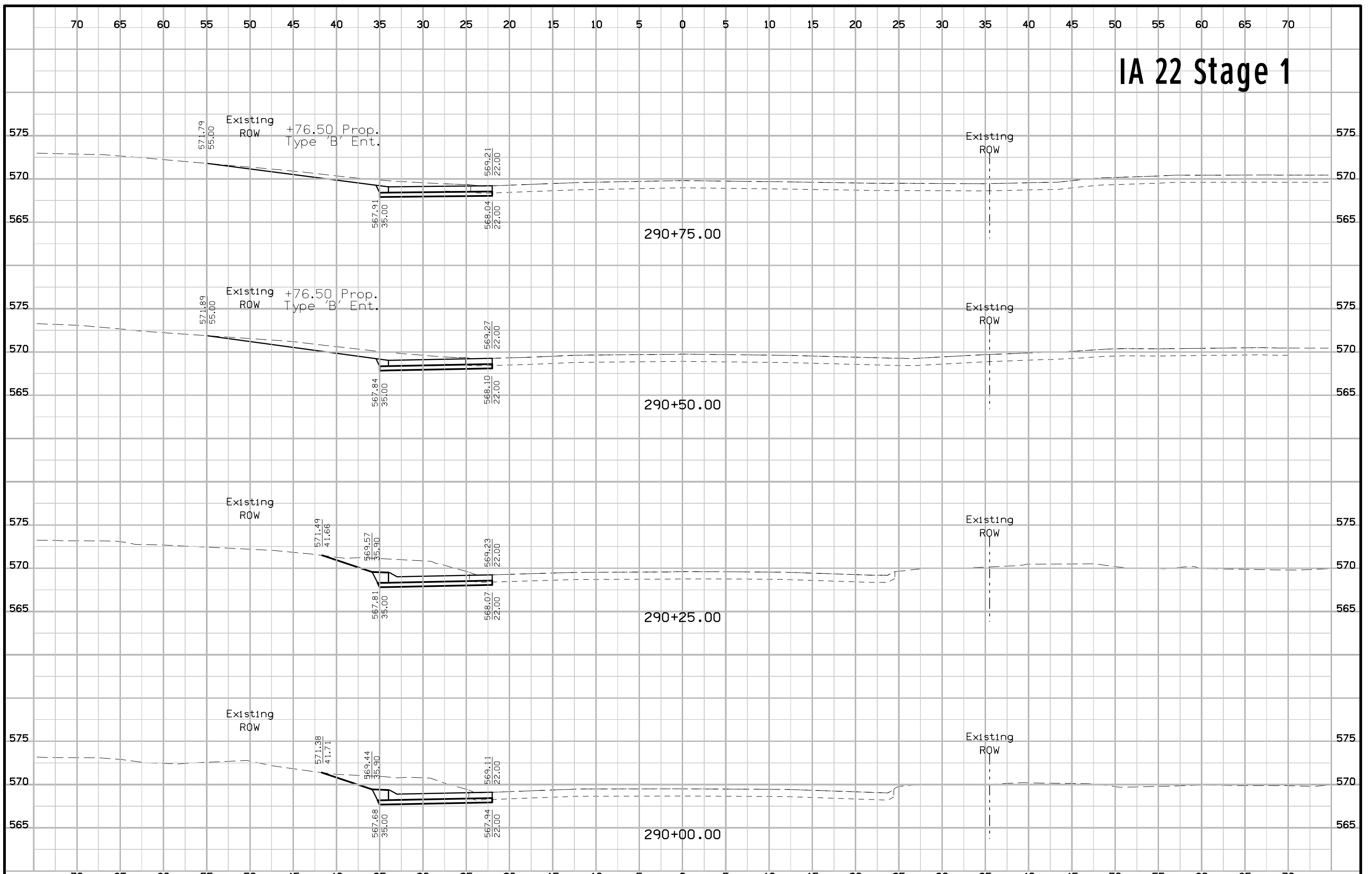
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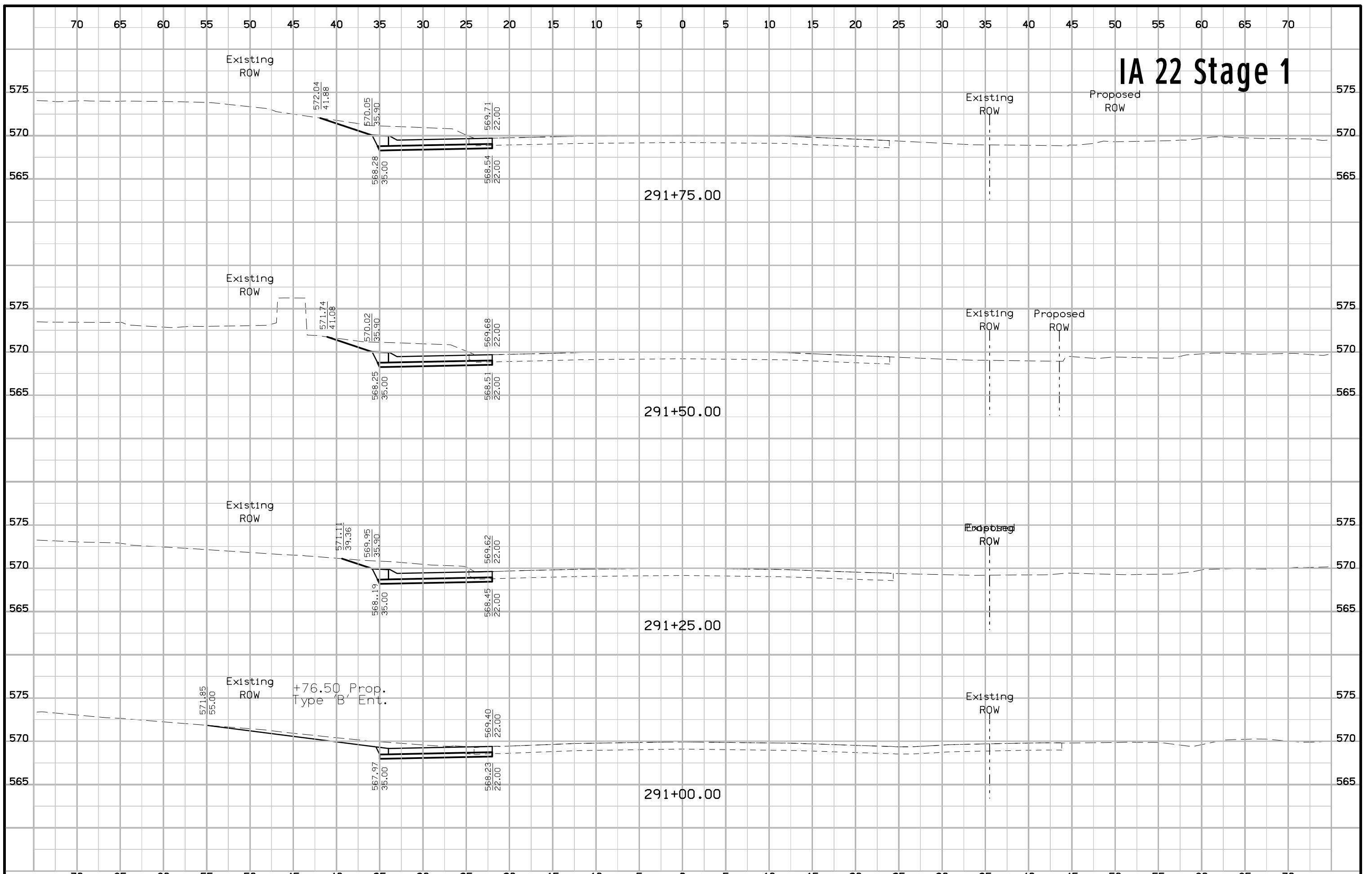
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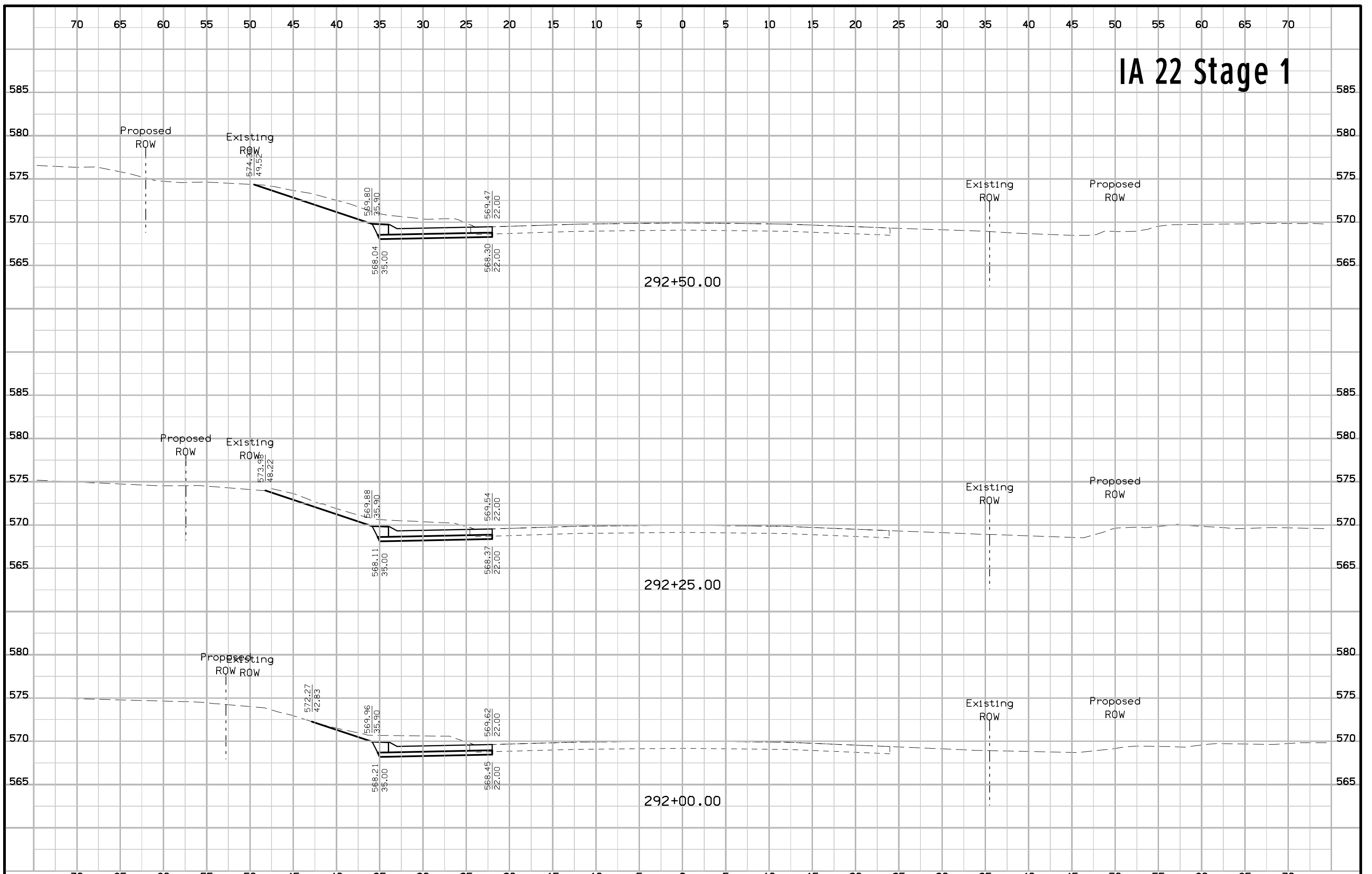
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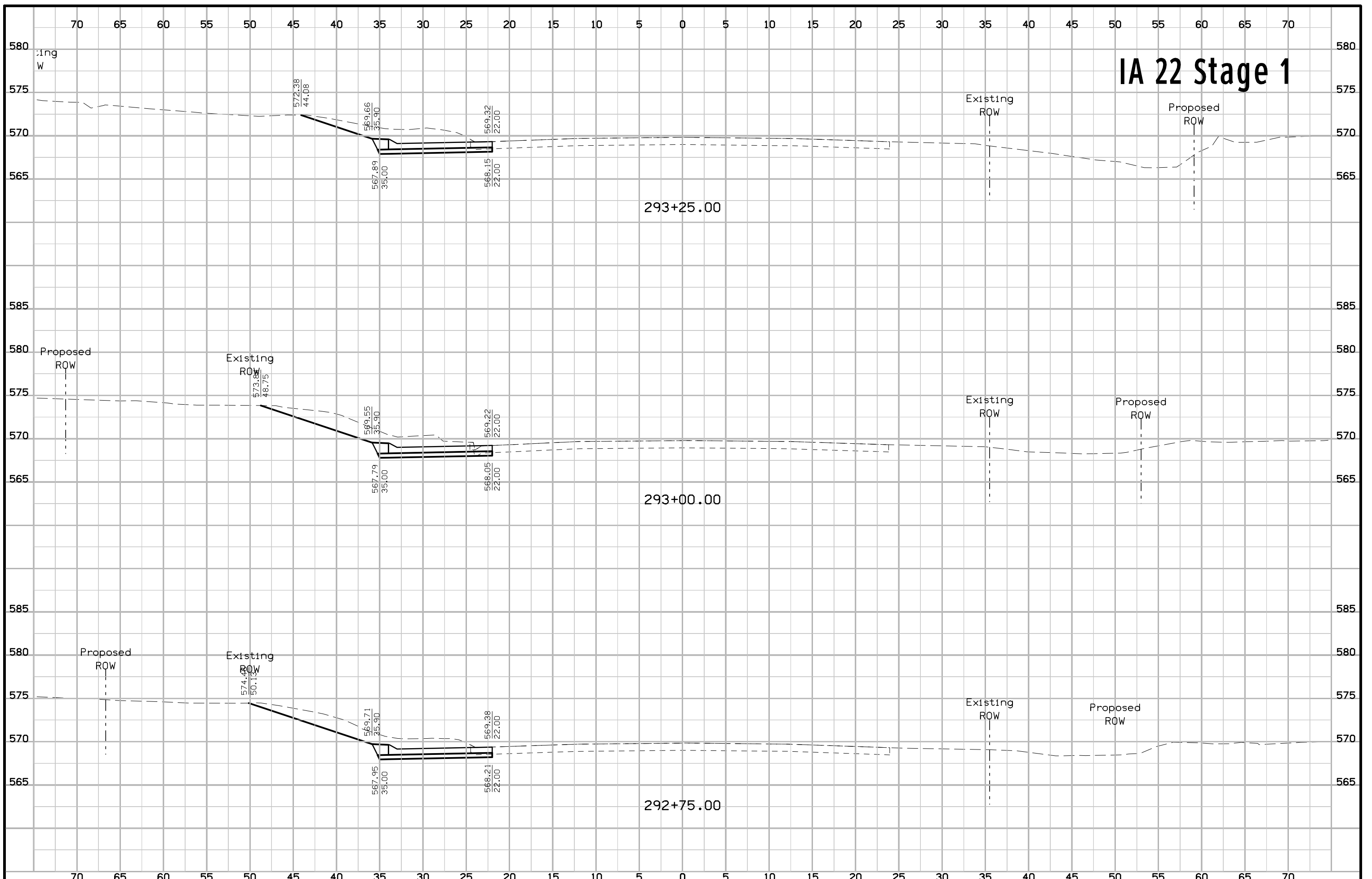
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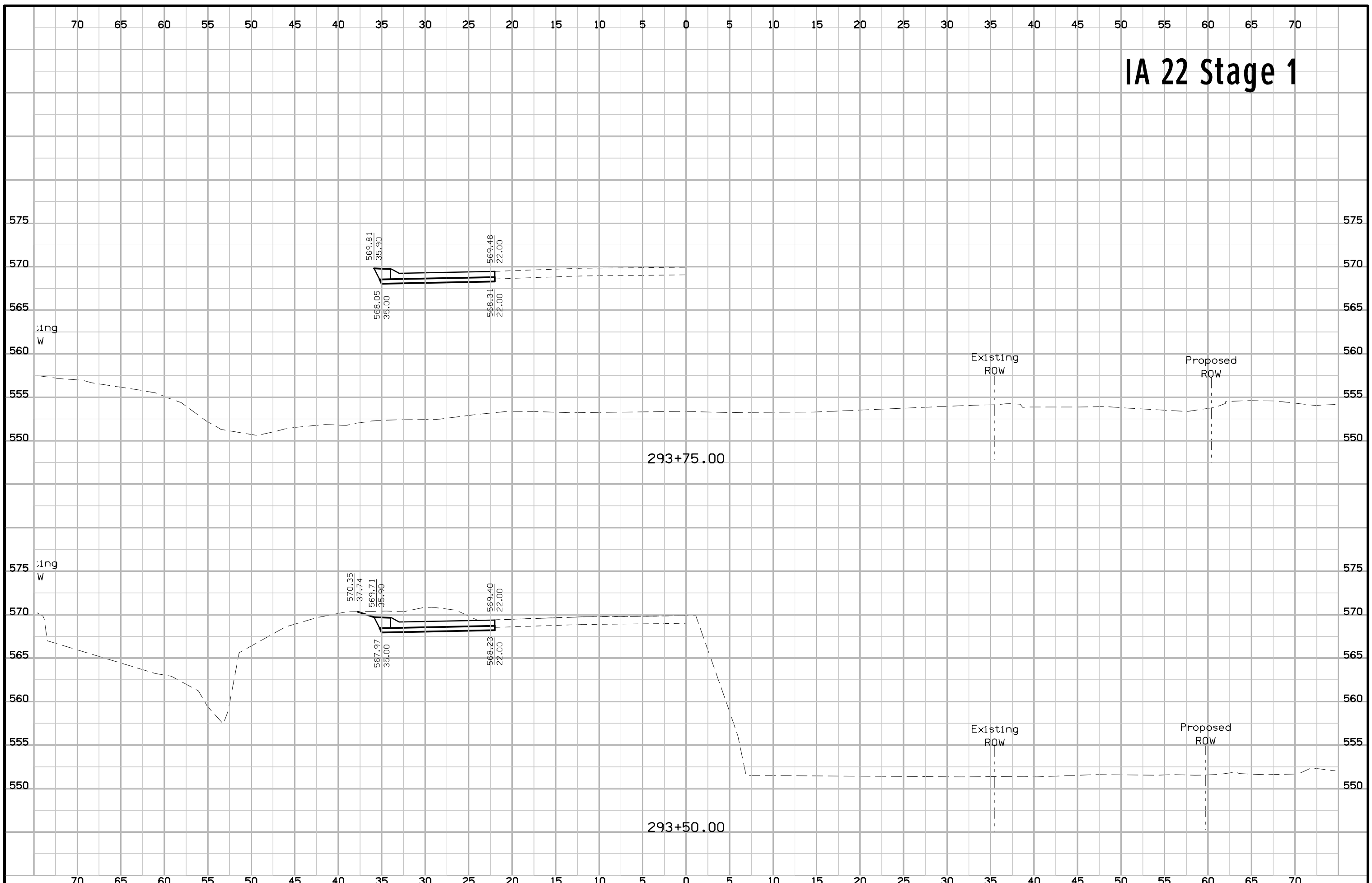
# IA 22 Stage 1



# IA 22 Stage 1

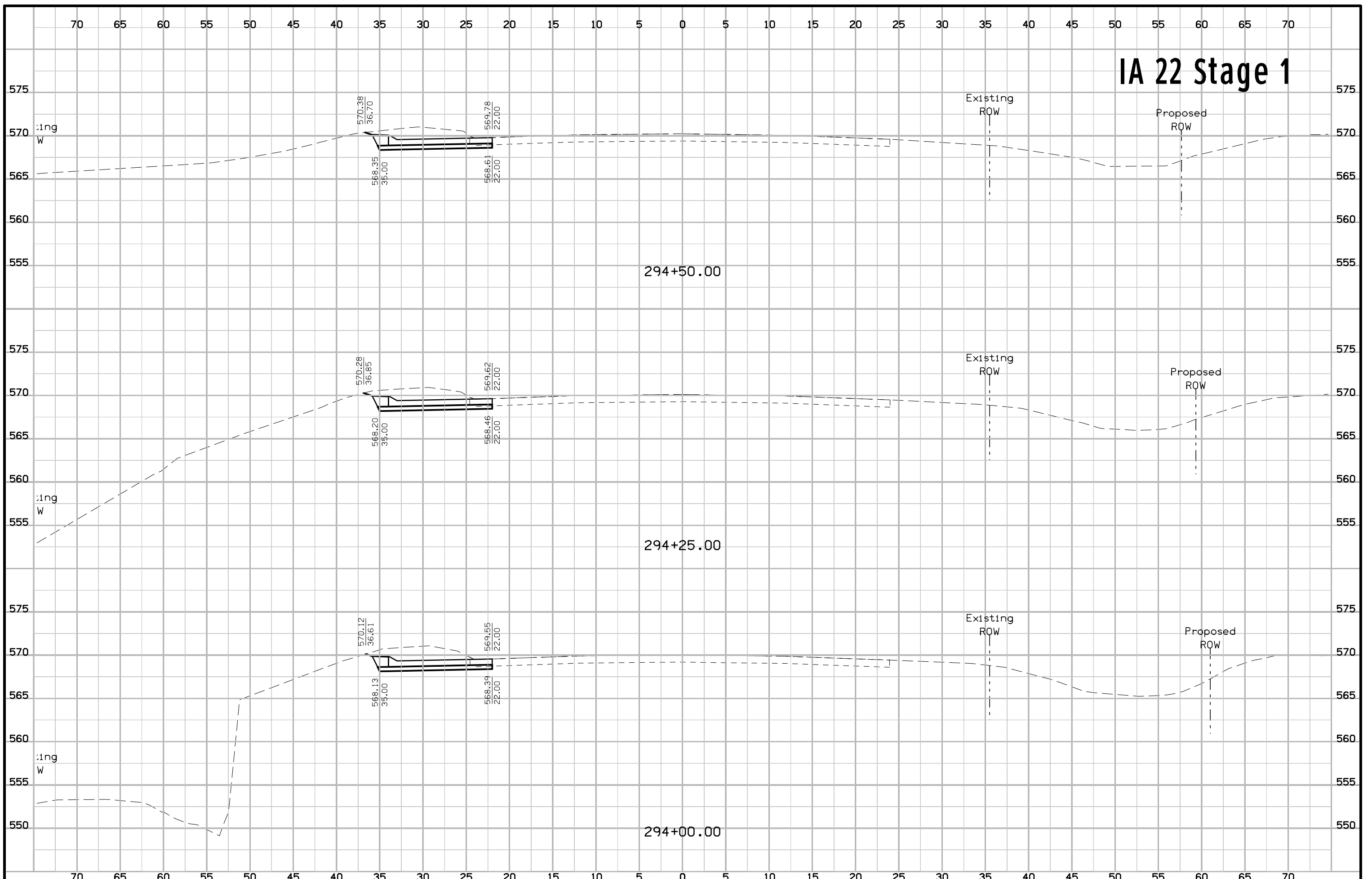


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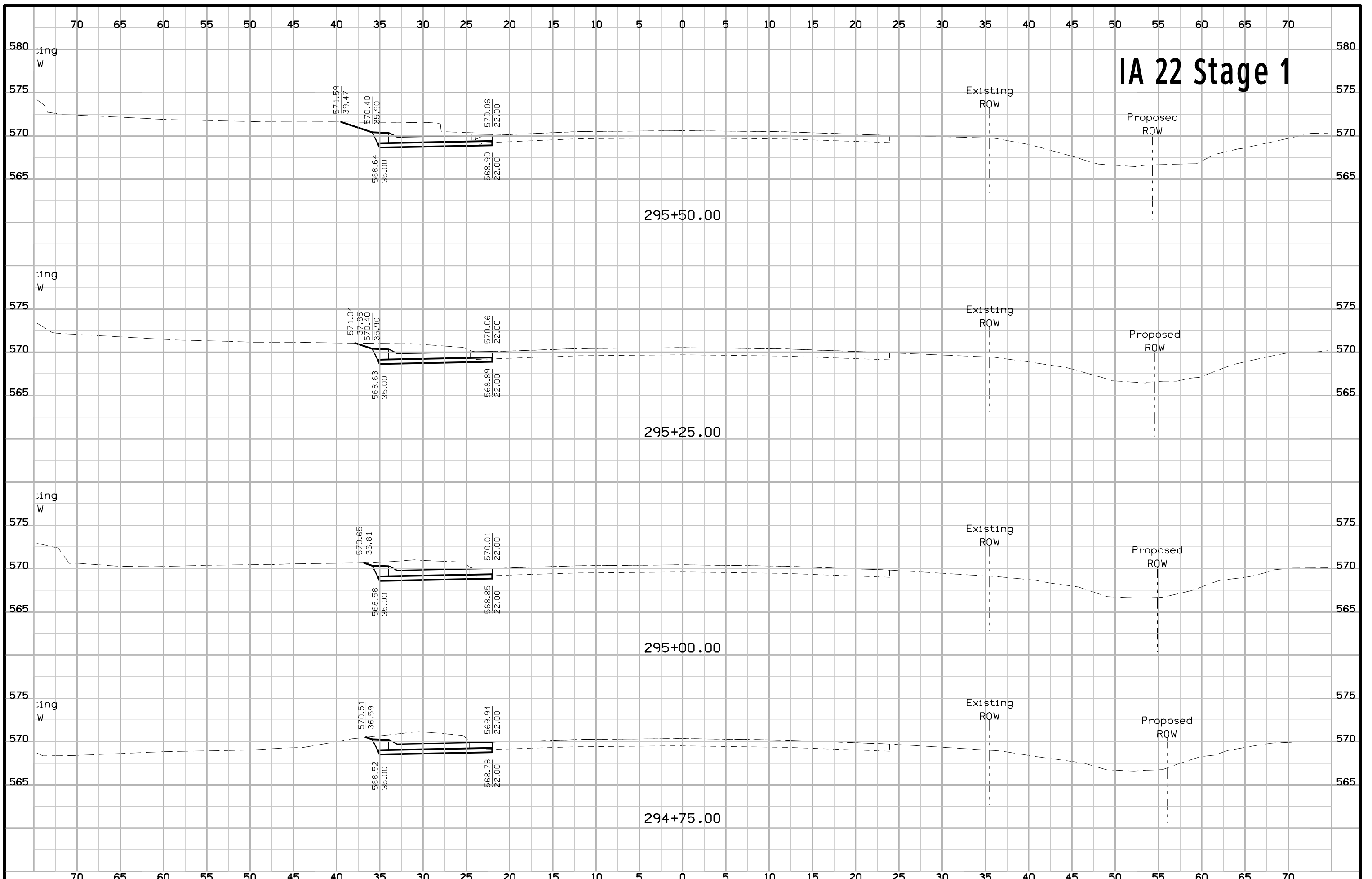




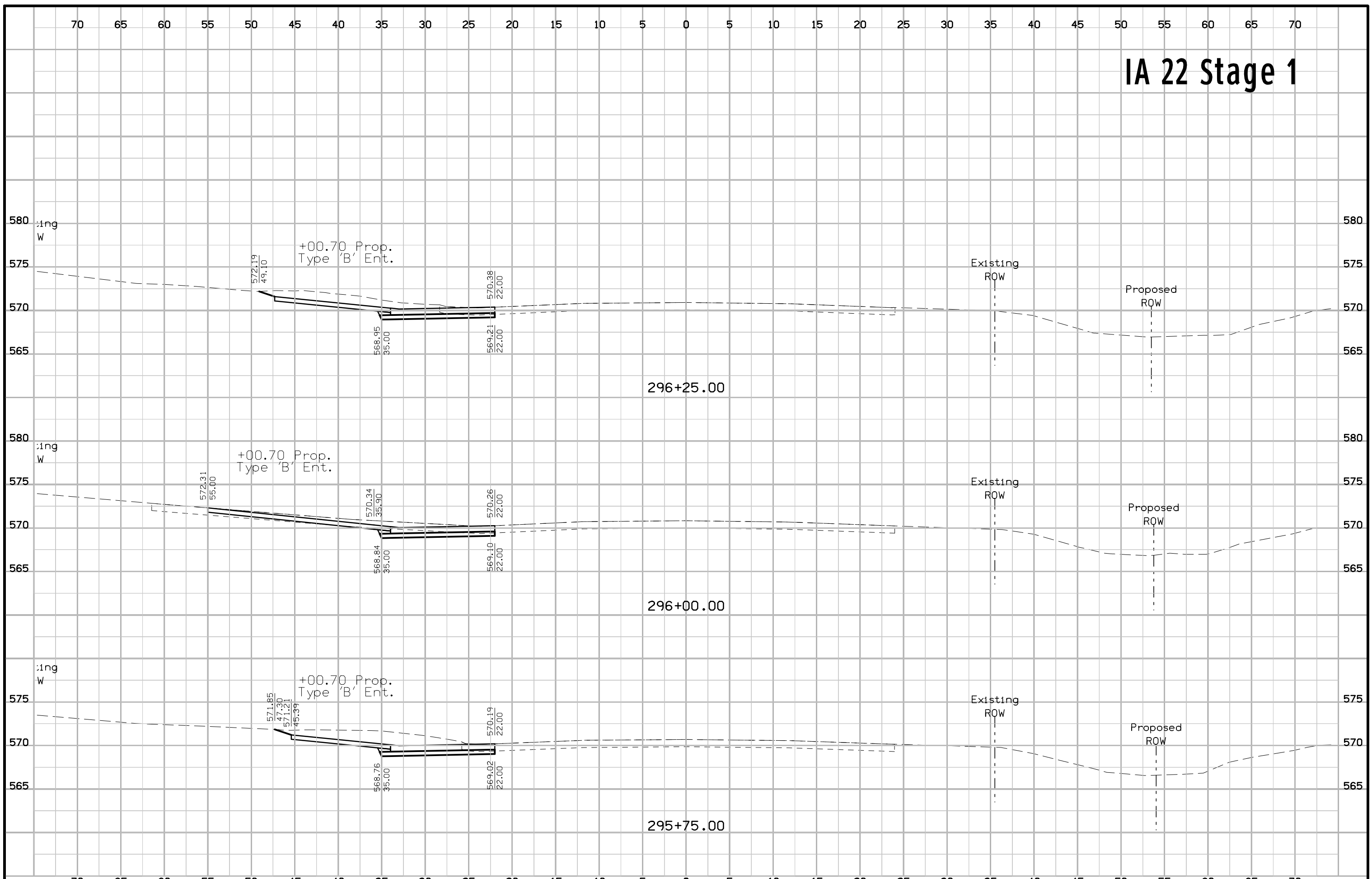
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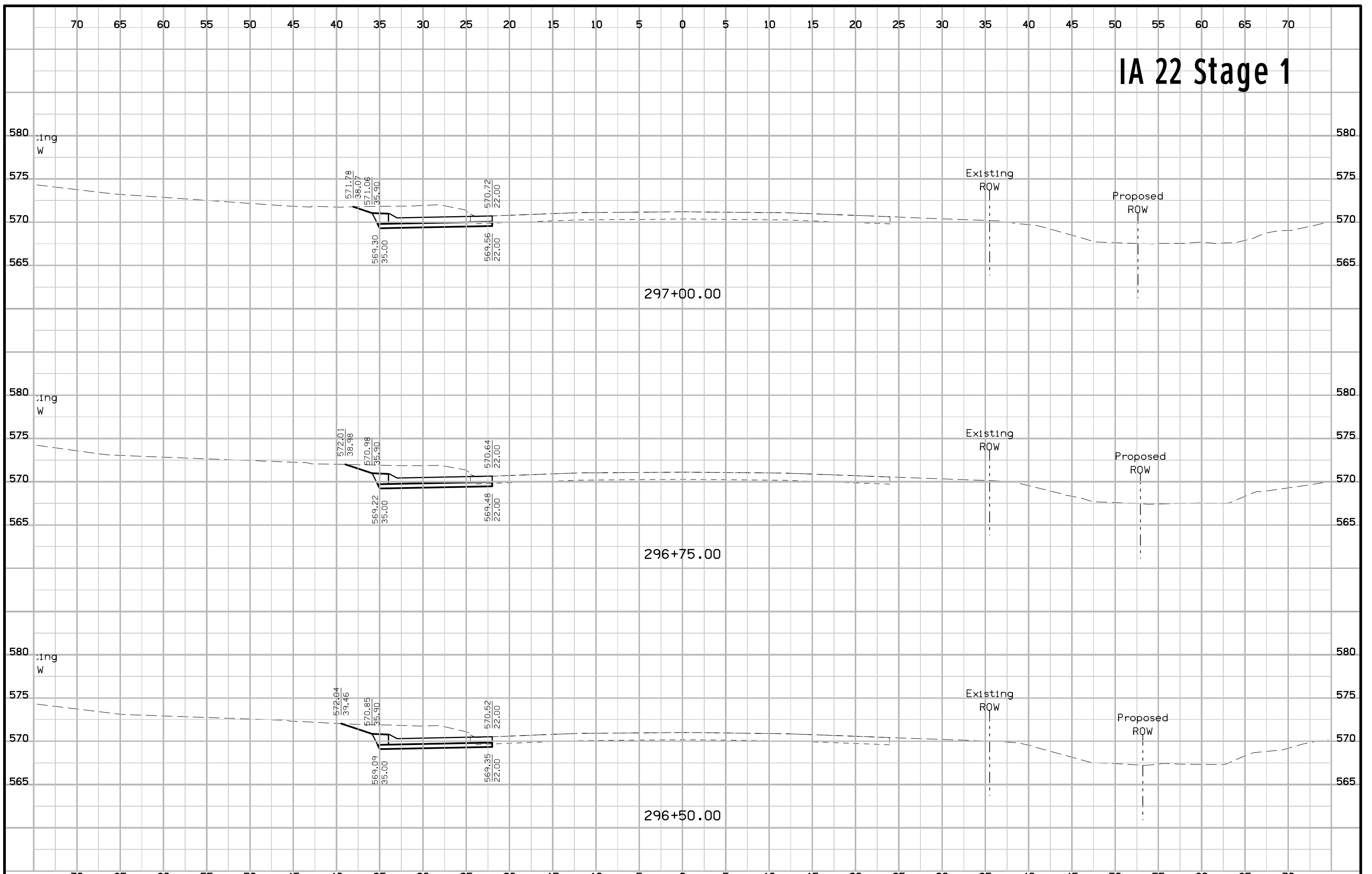
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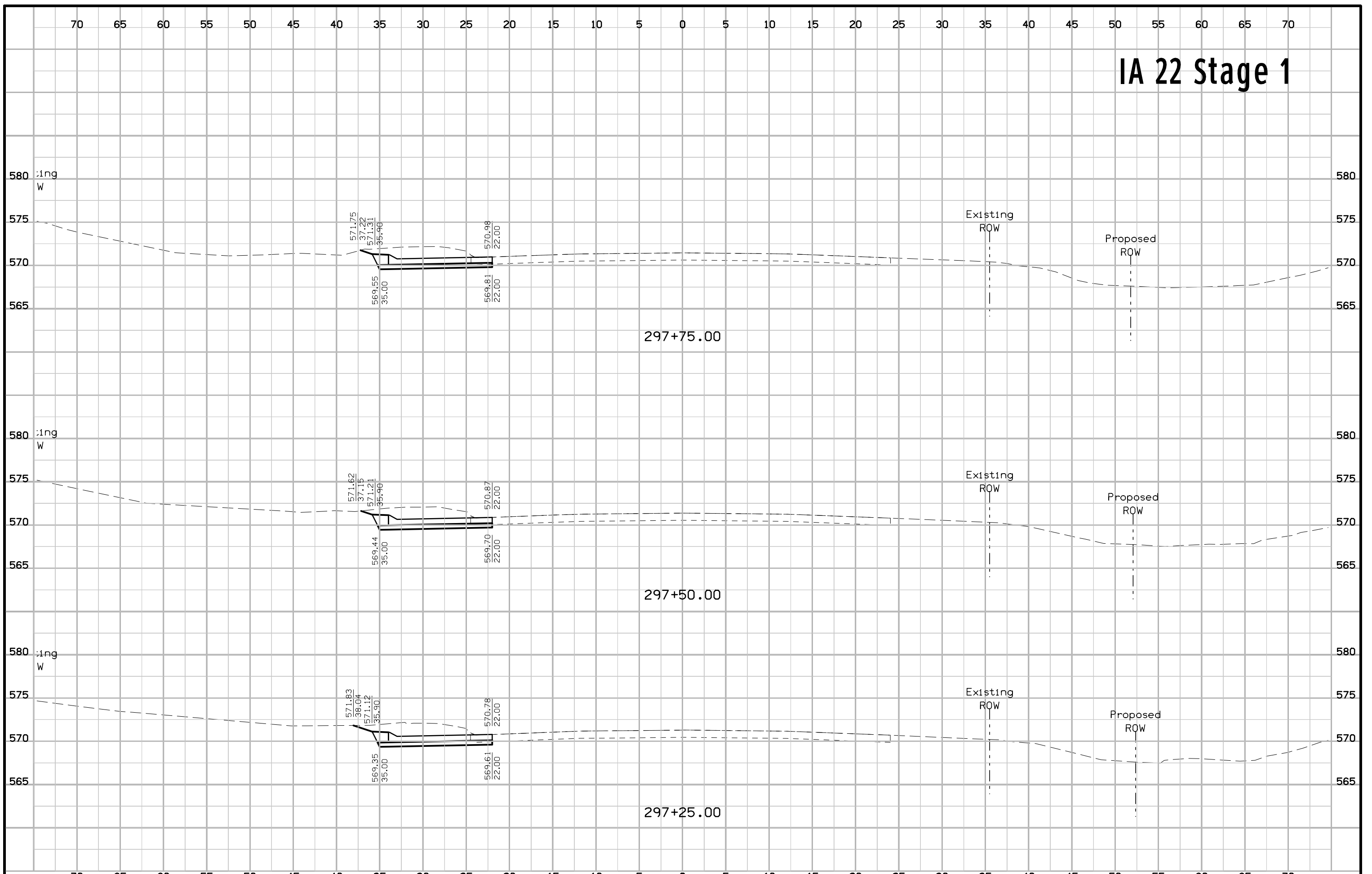
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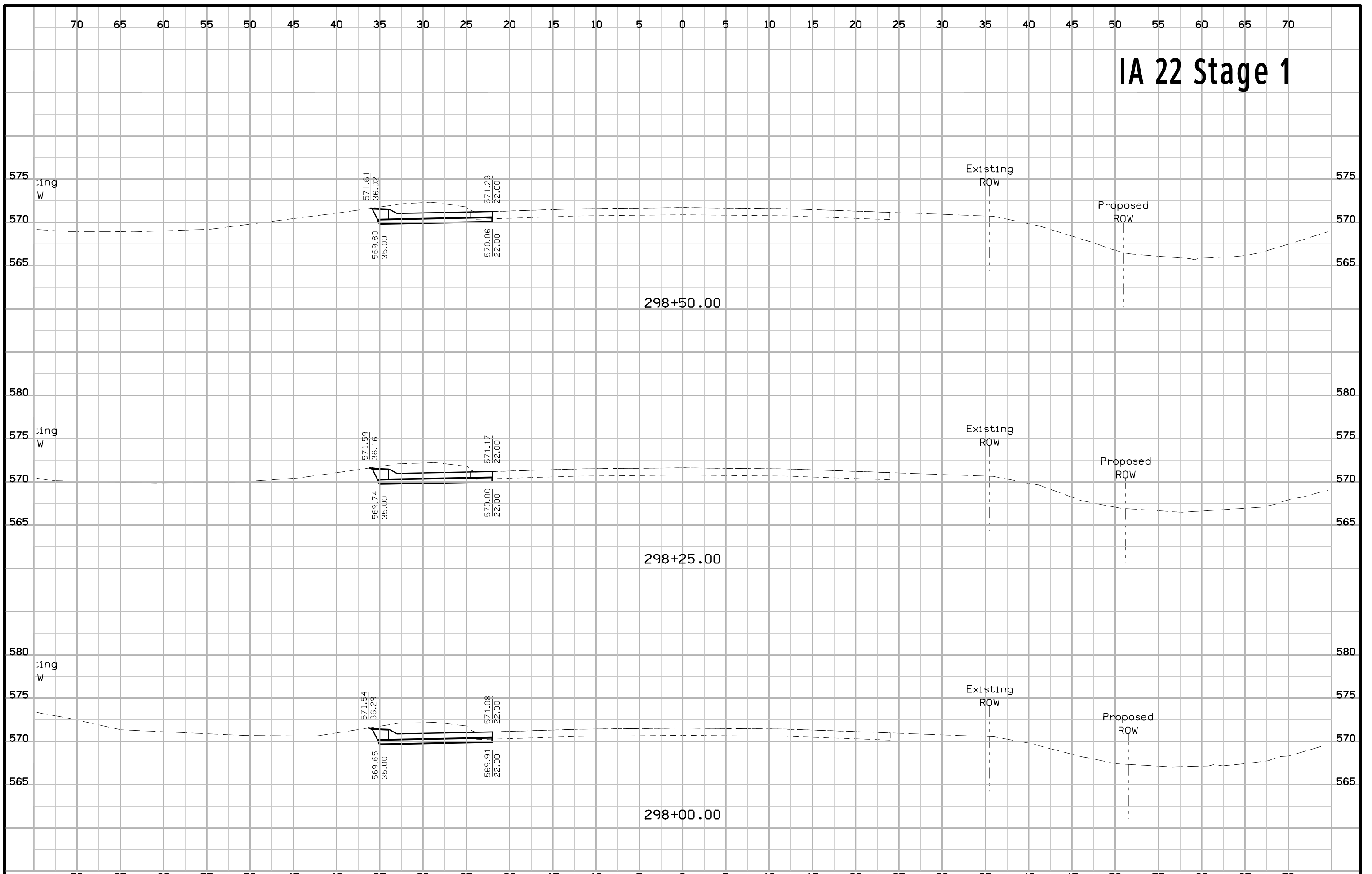
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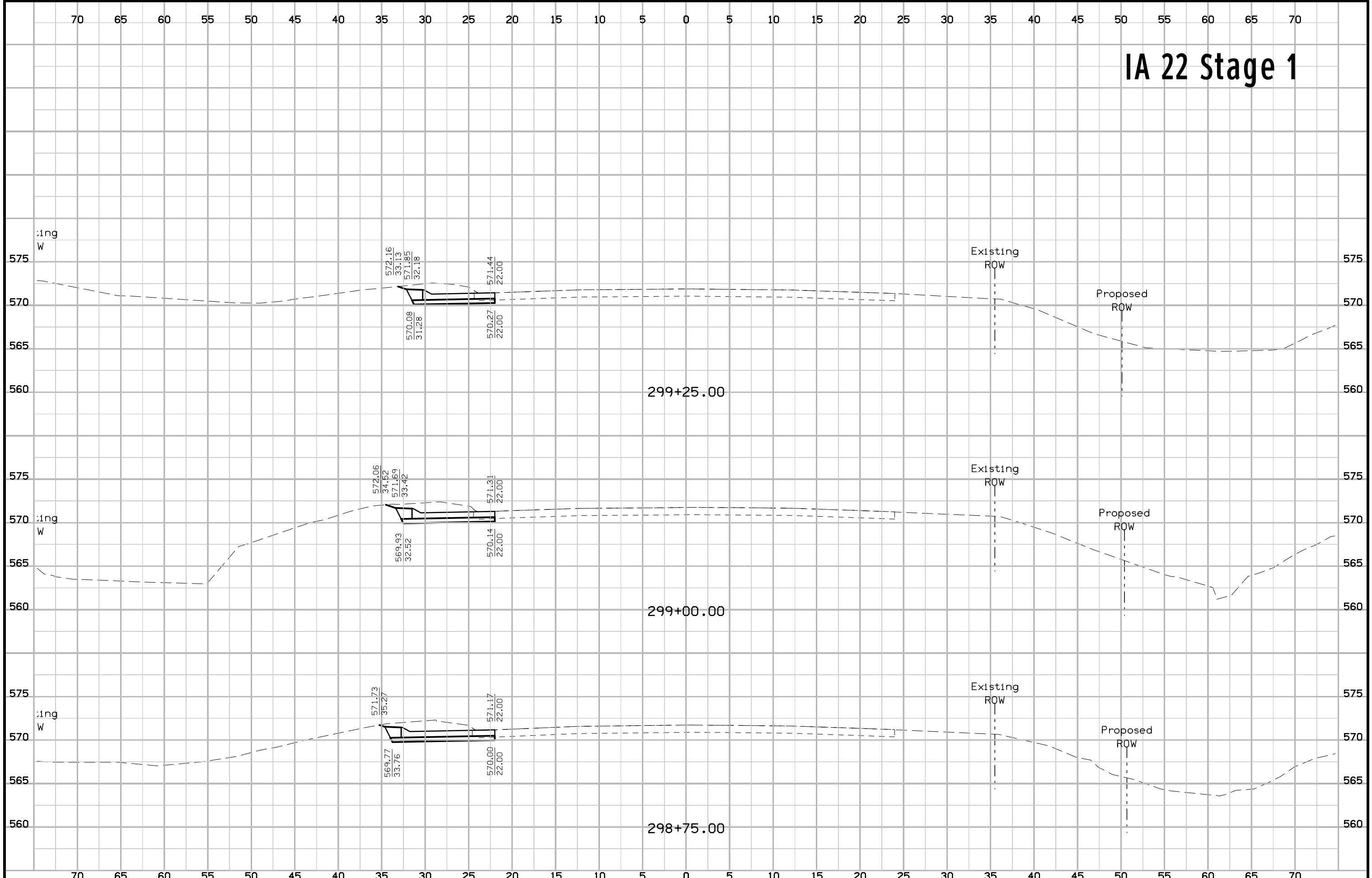
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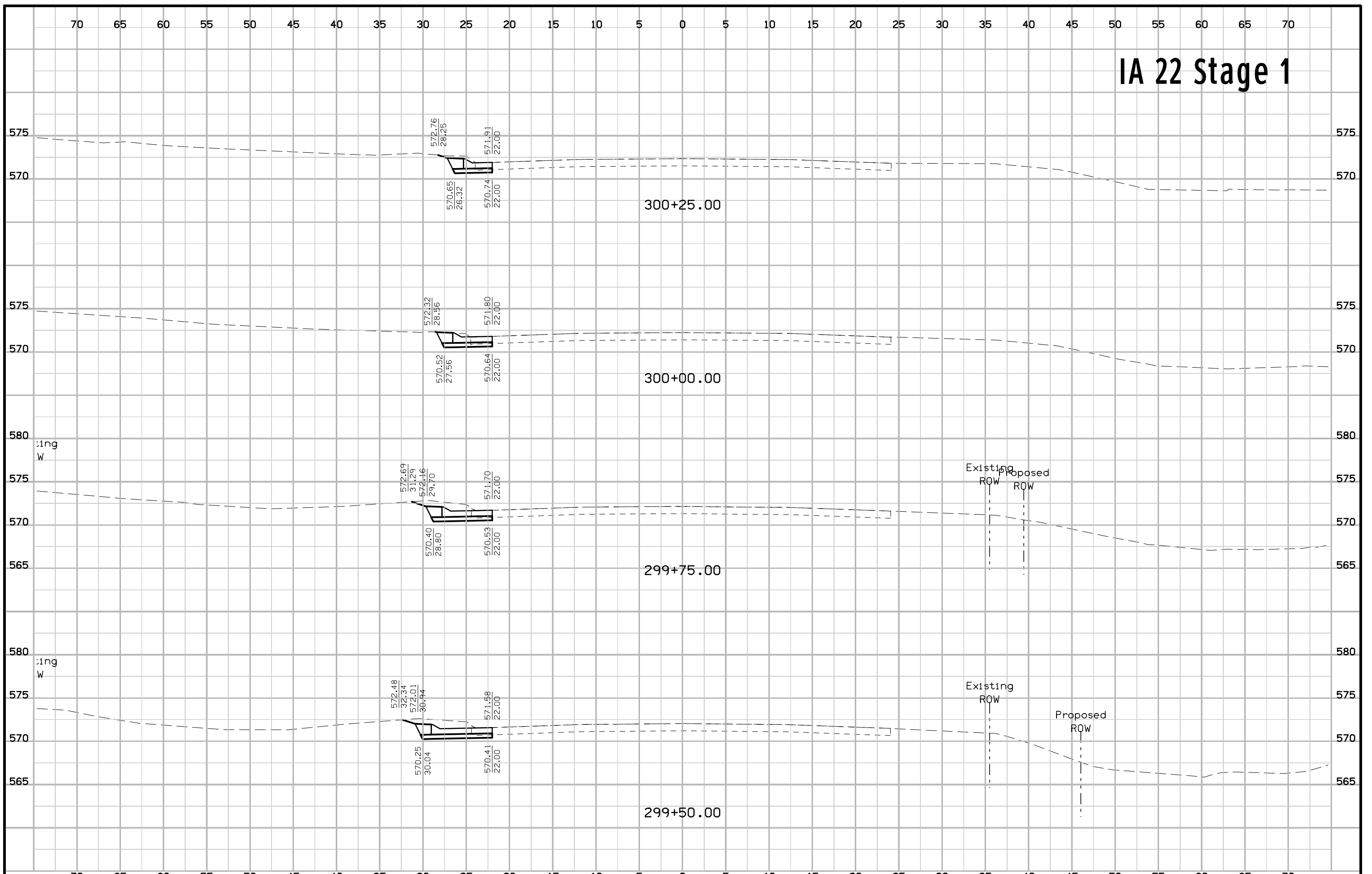
# IA 22 Stage 1



# IA 22 Stage 1

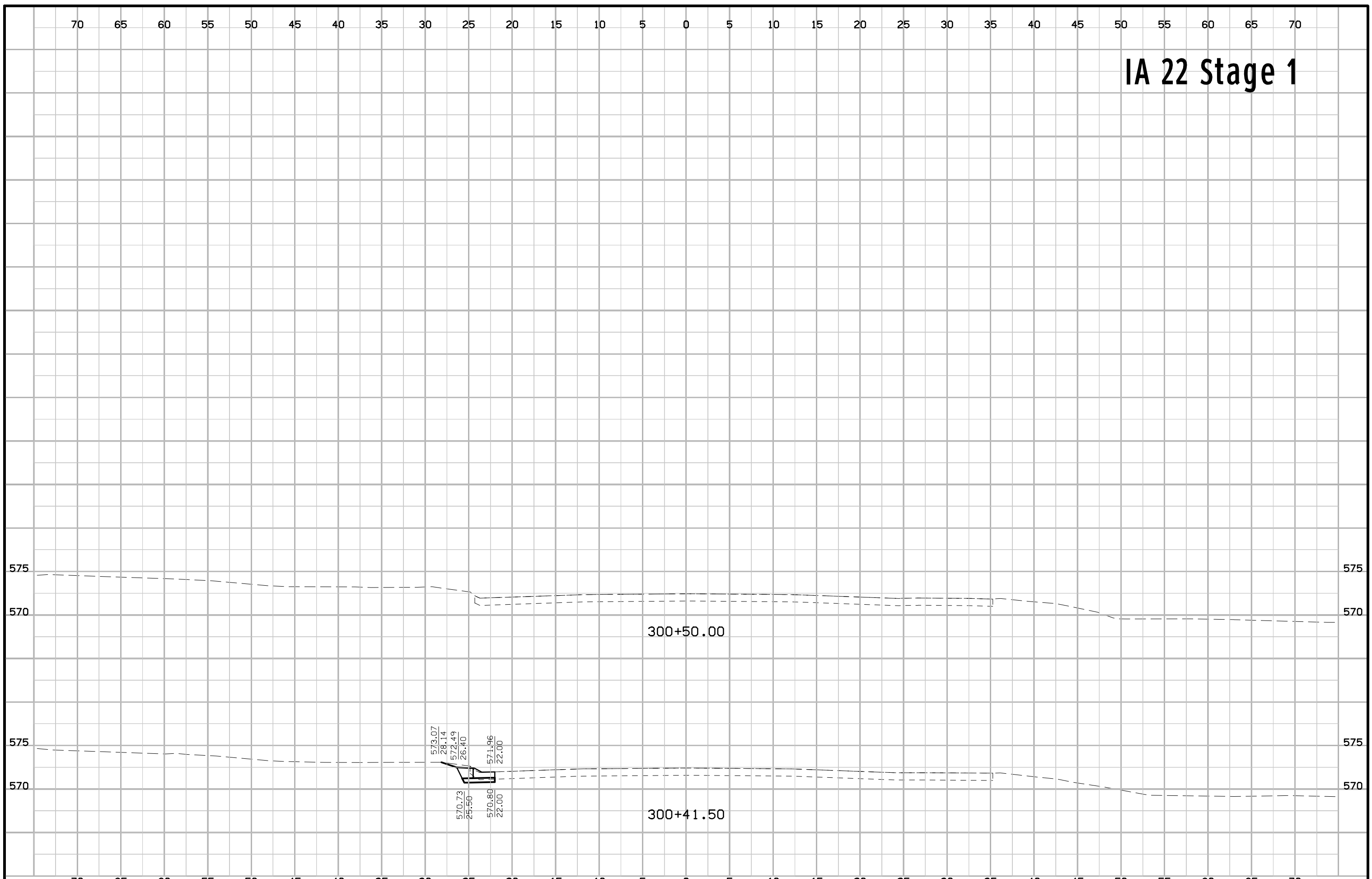


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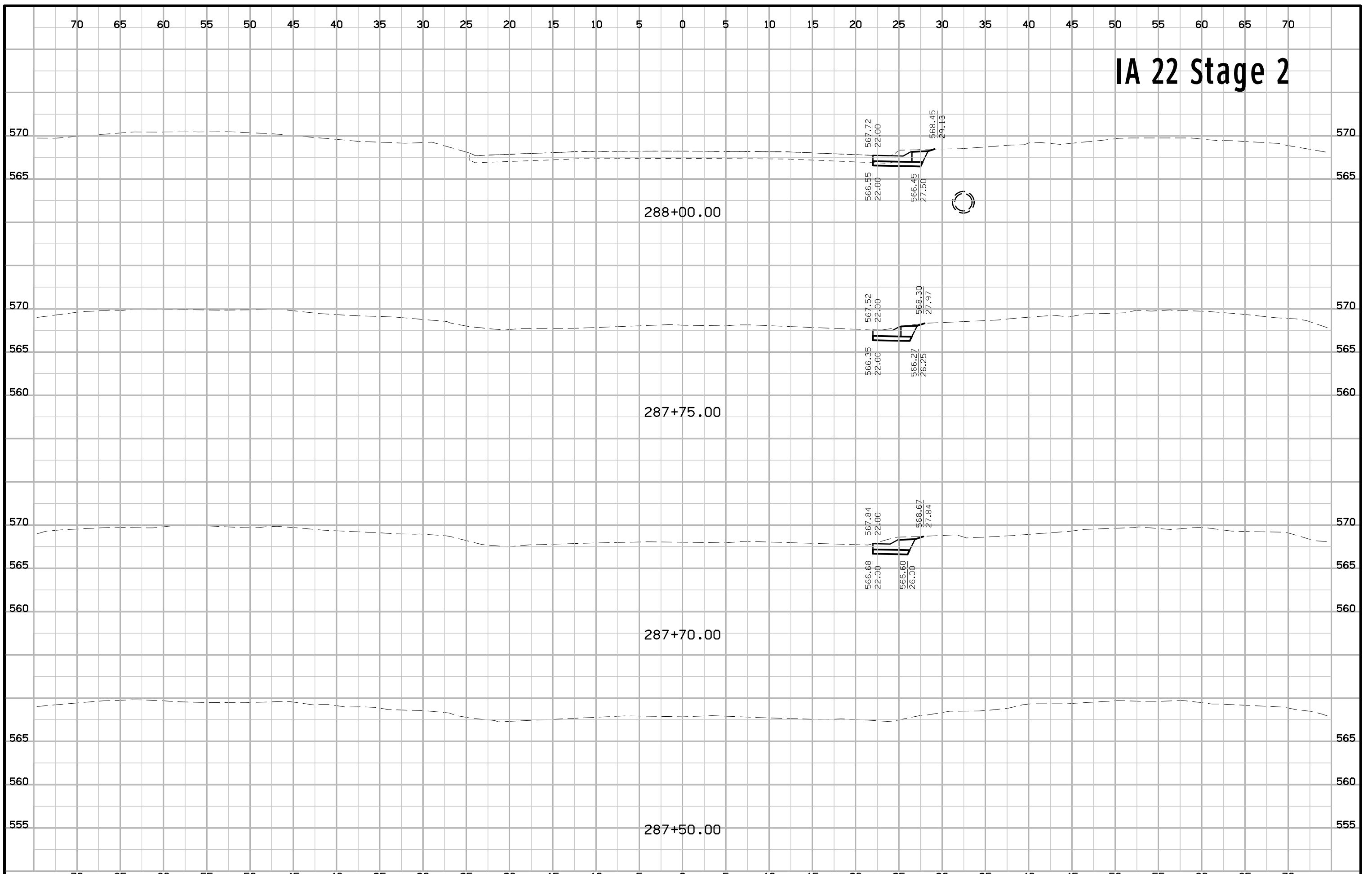


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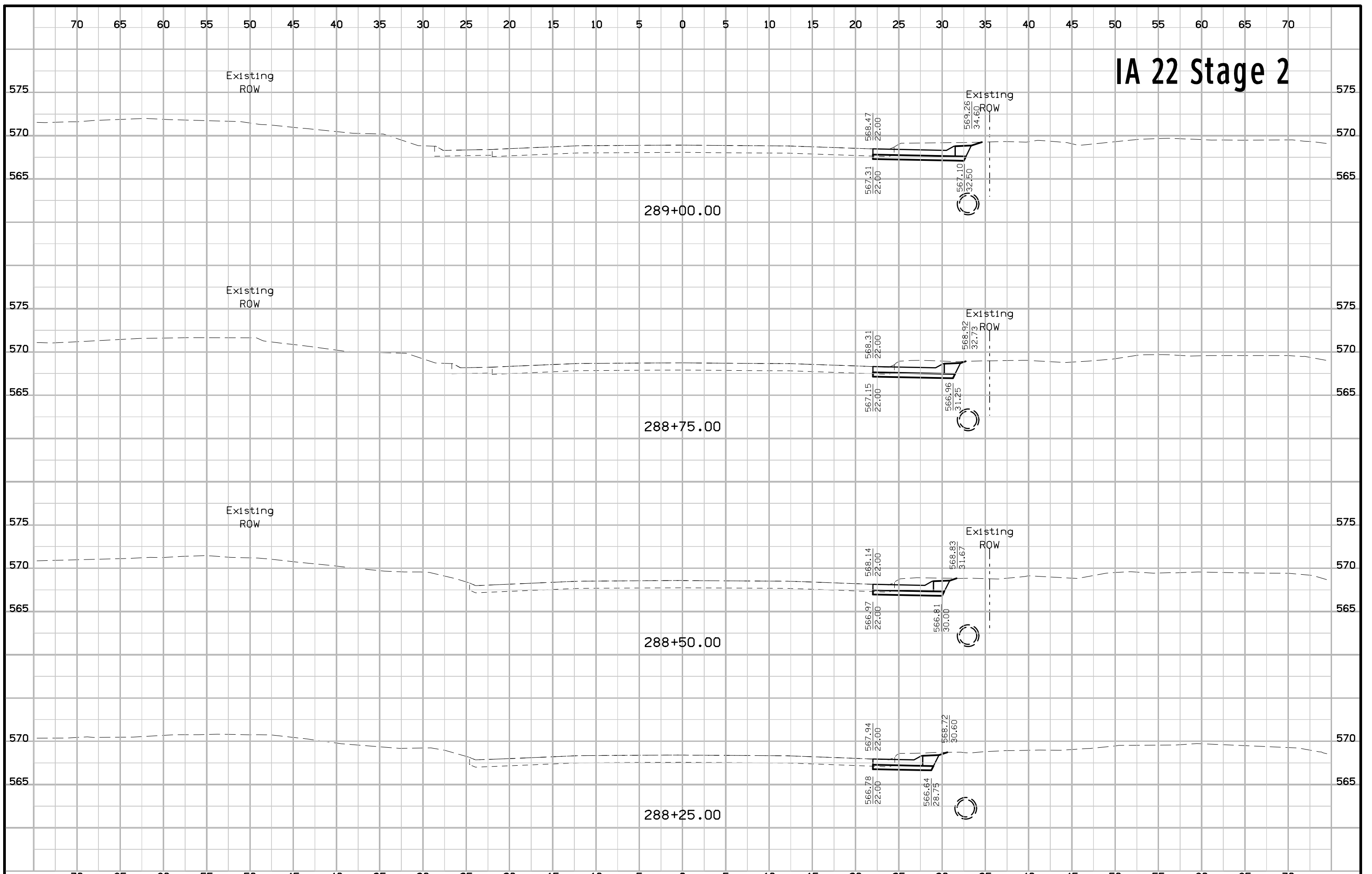


573.07  
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572.49  
26.40  
571.96  
22.00  
570.73  
25.50  
570.80  
22.00

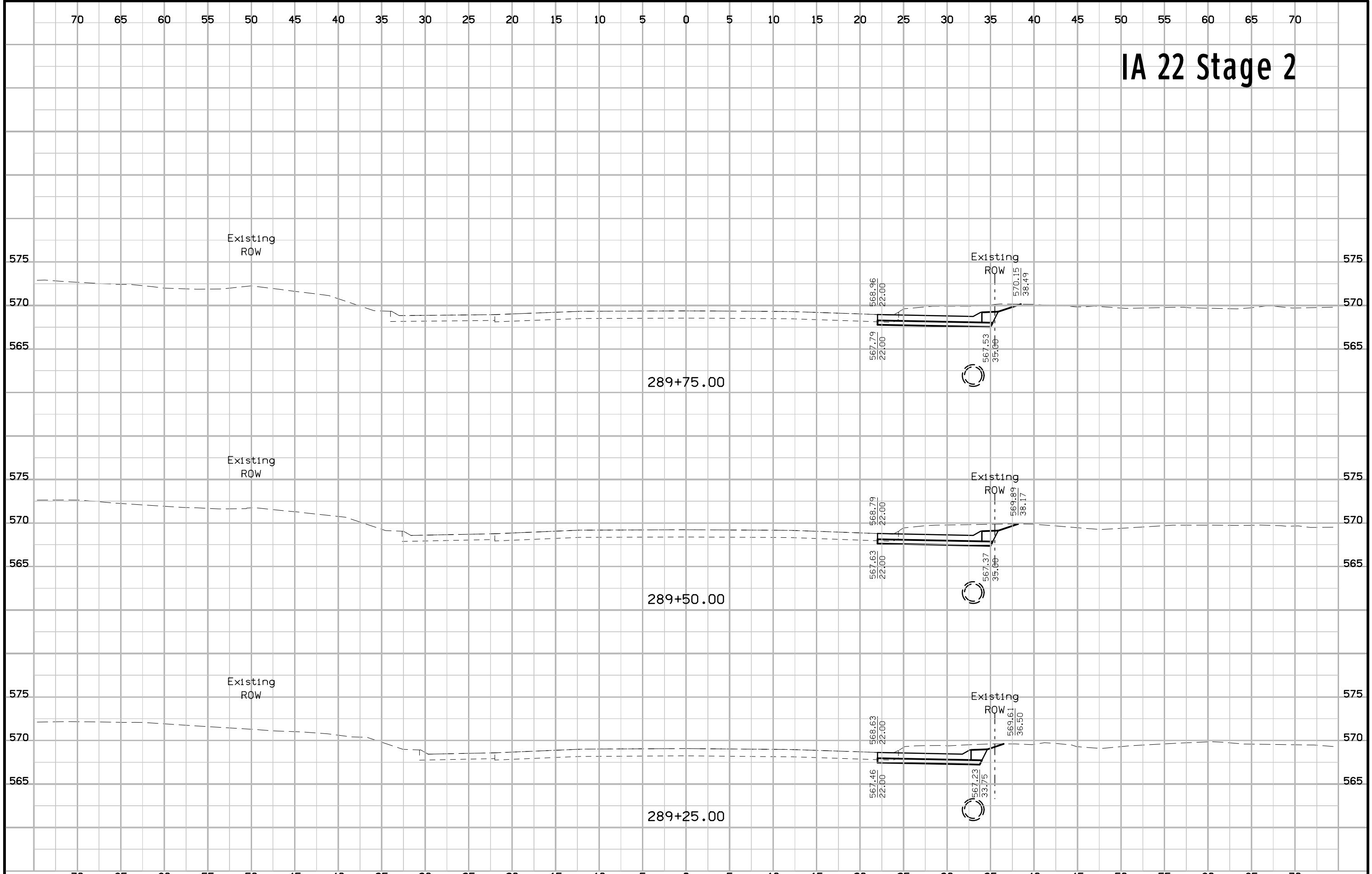
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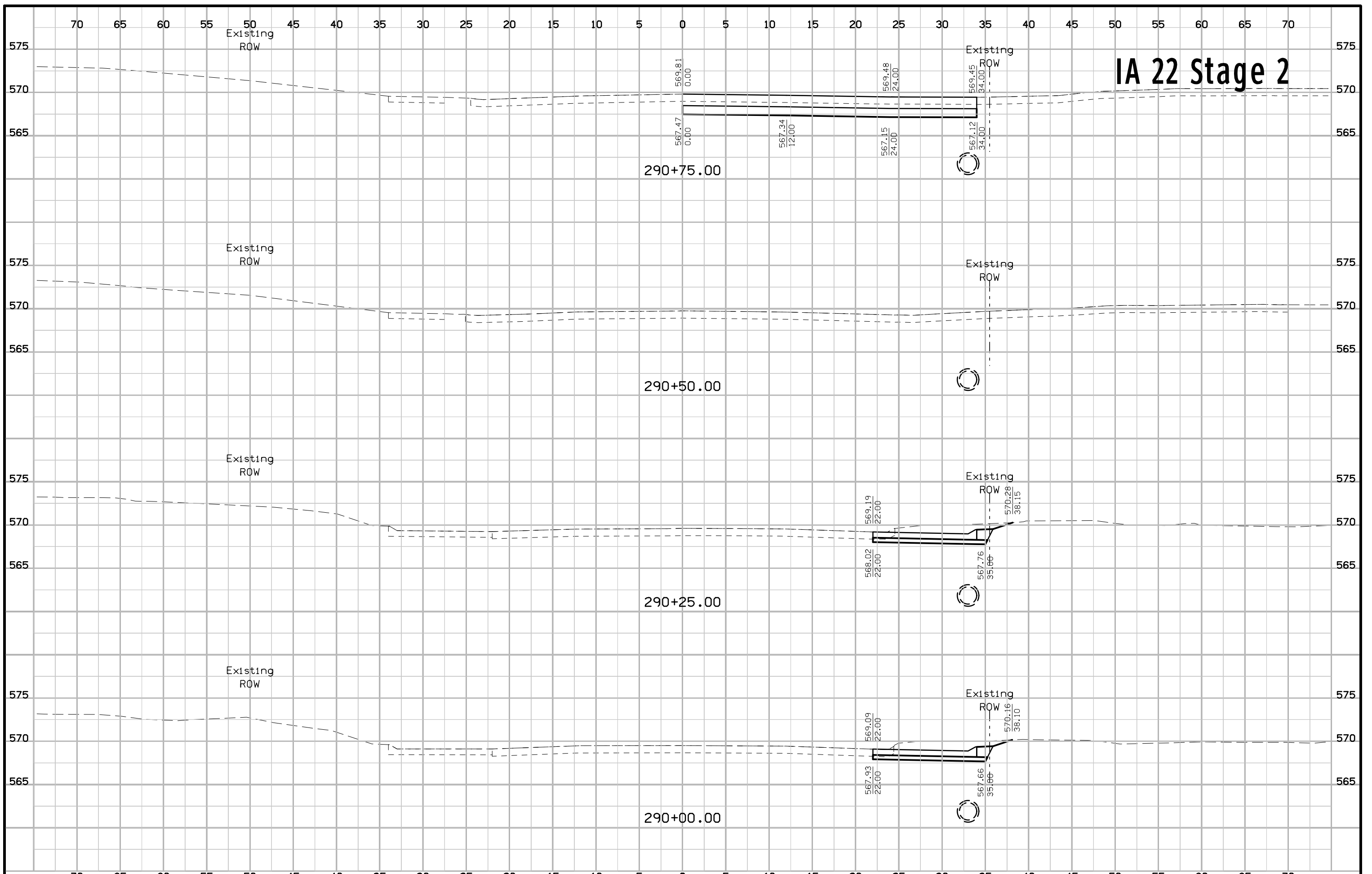
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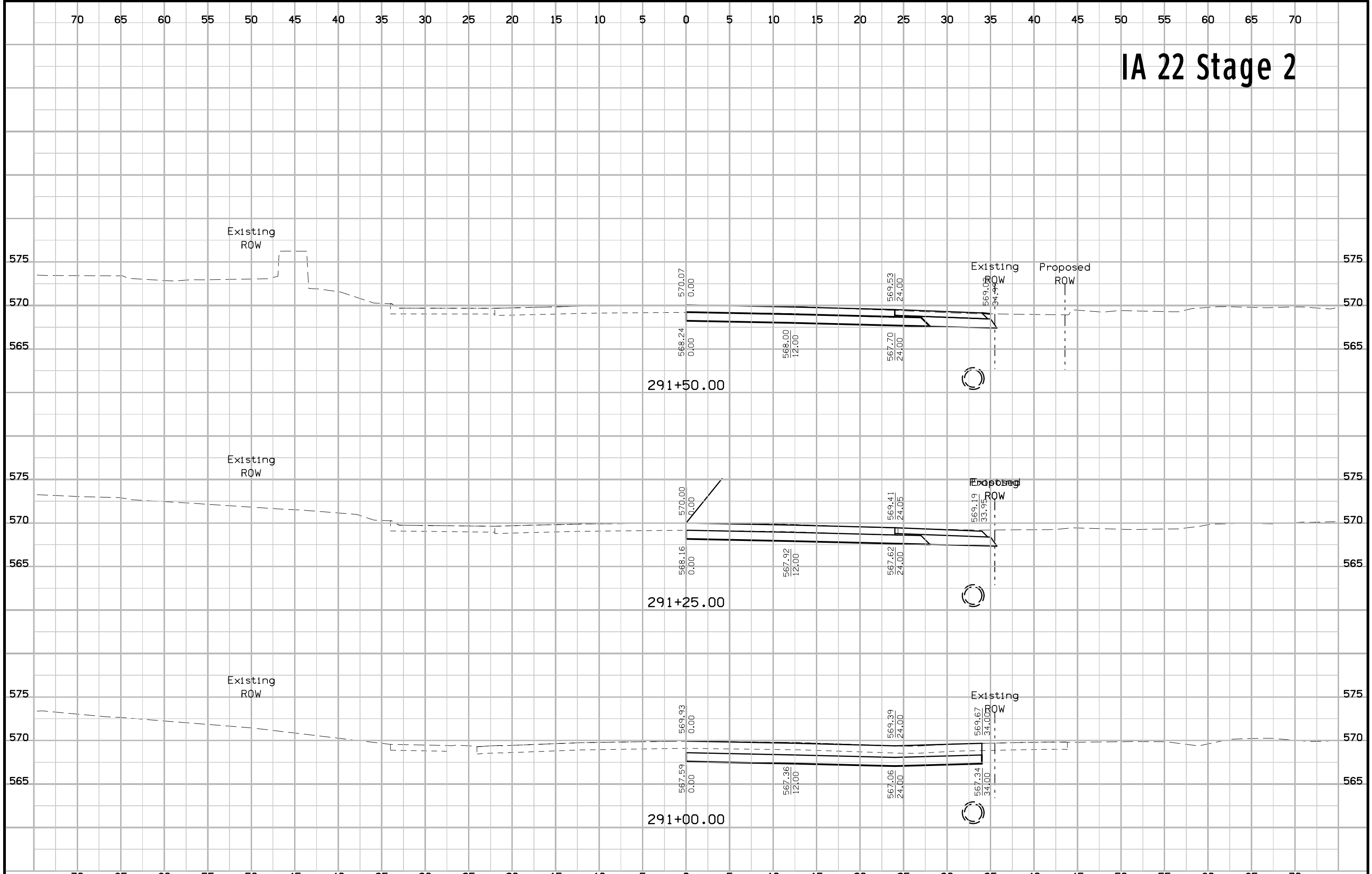
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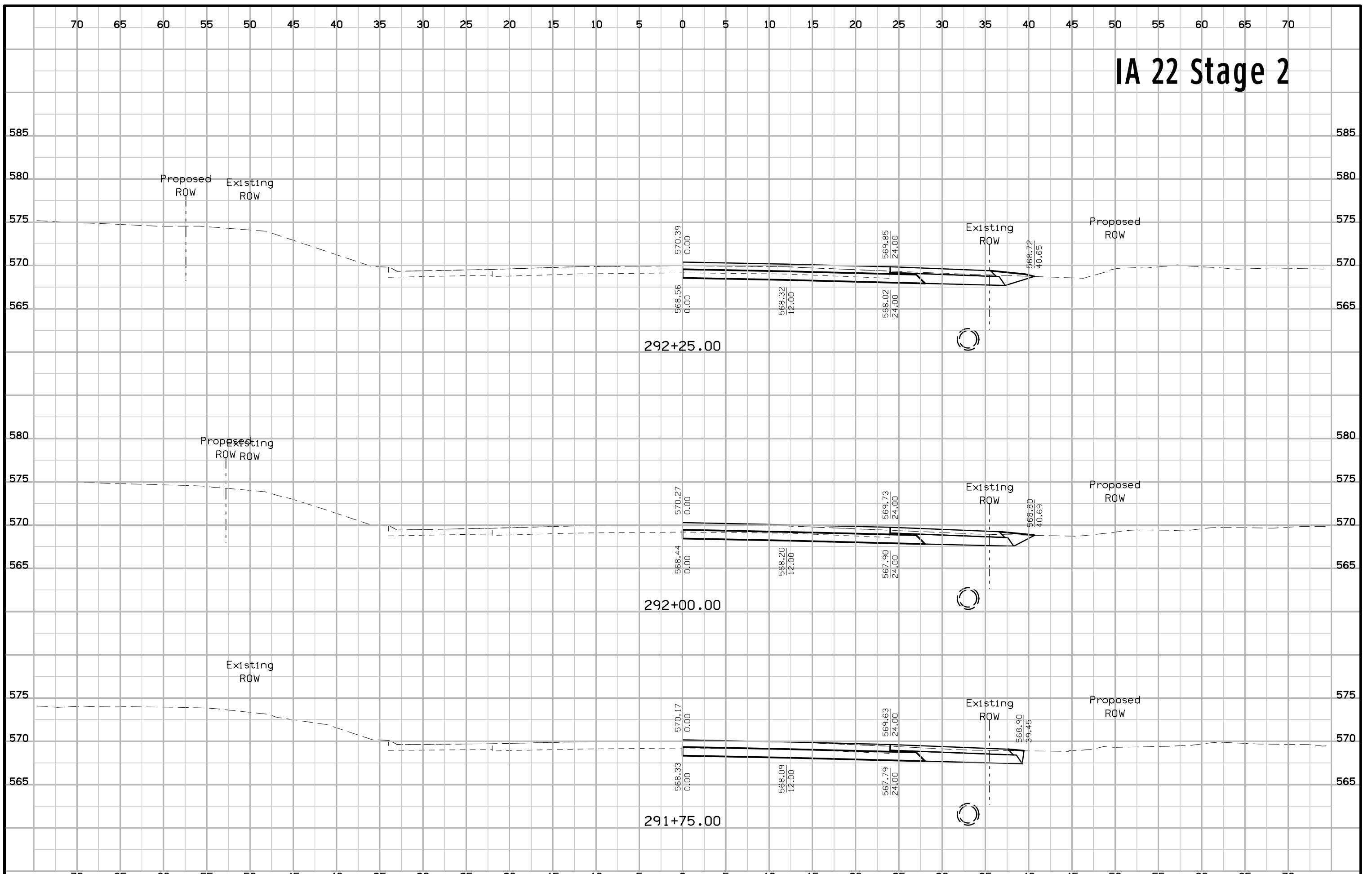
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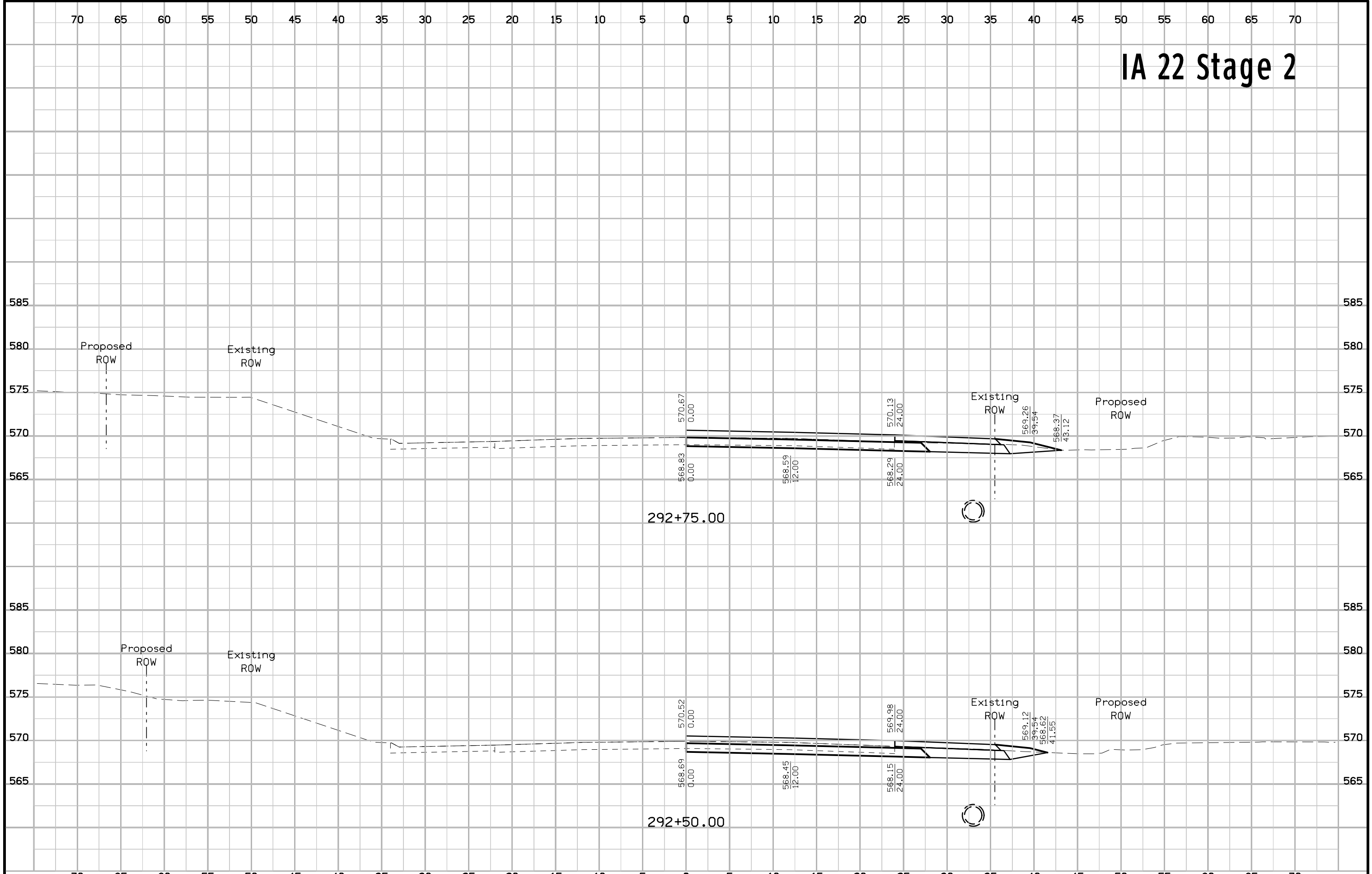
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# IA 22 Stage 2

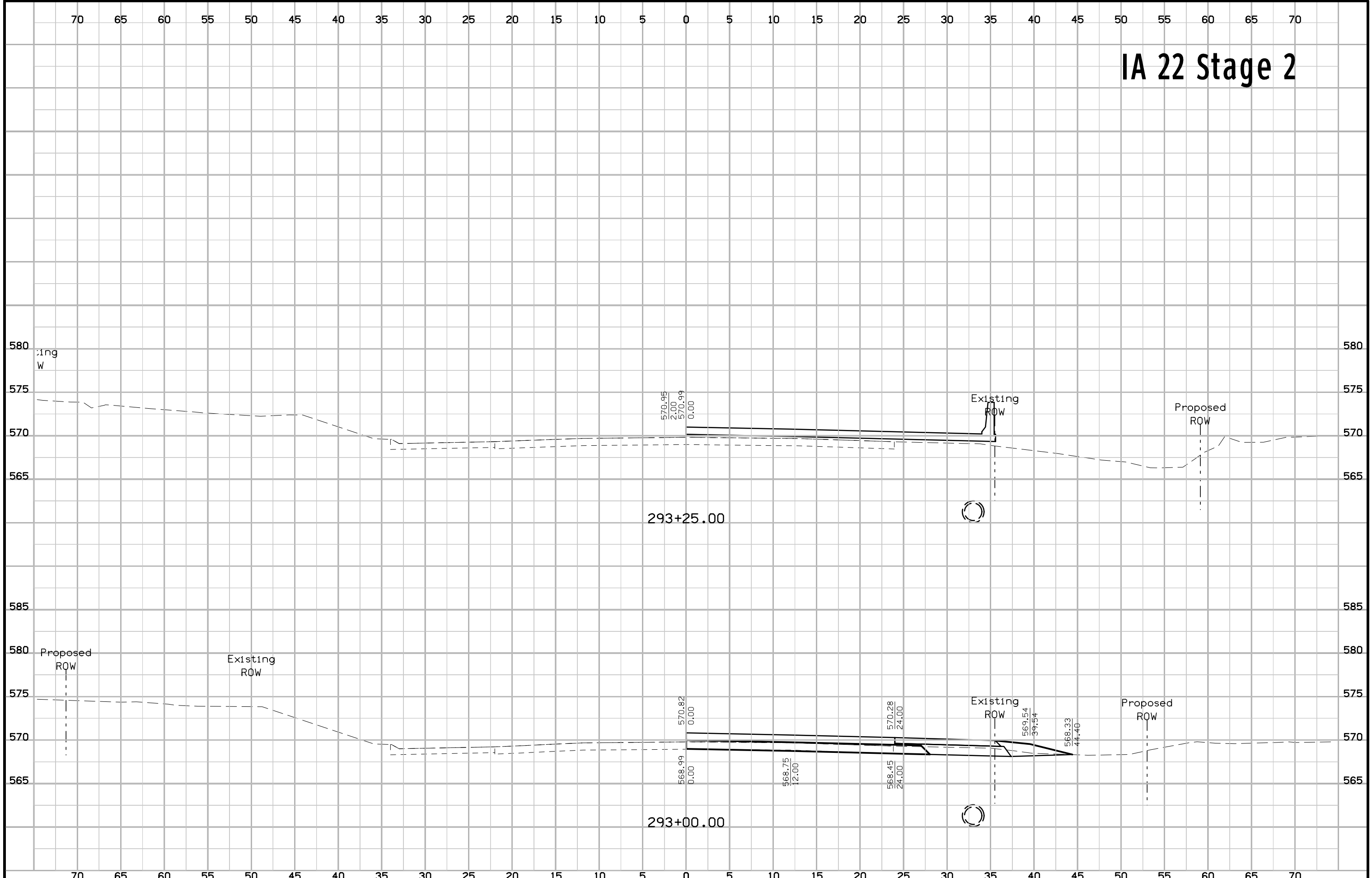


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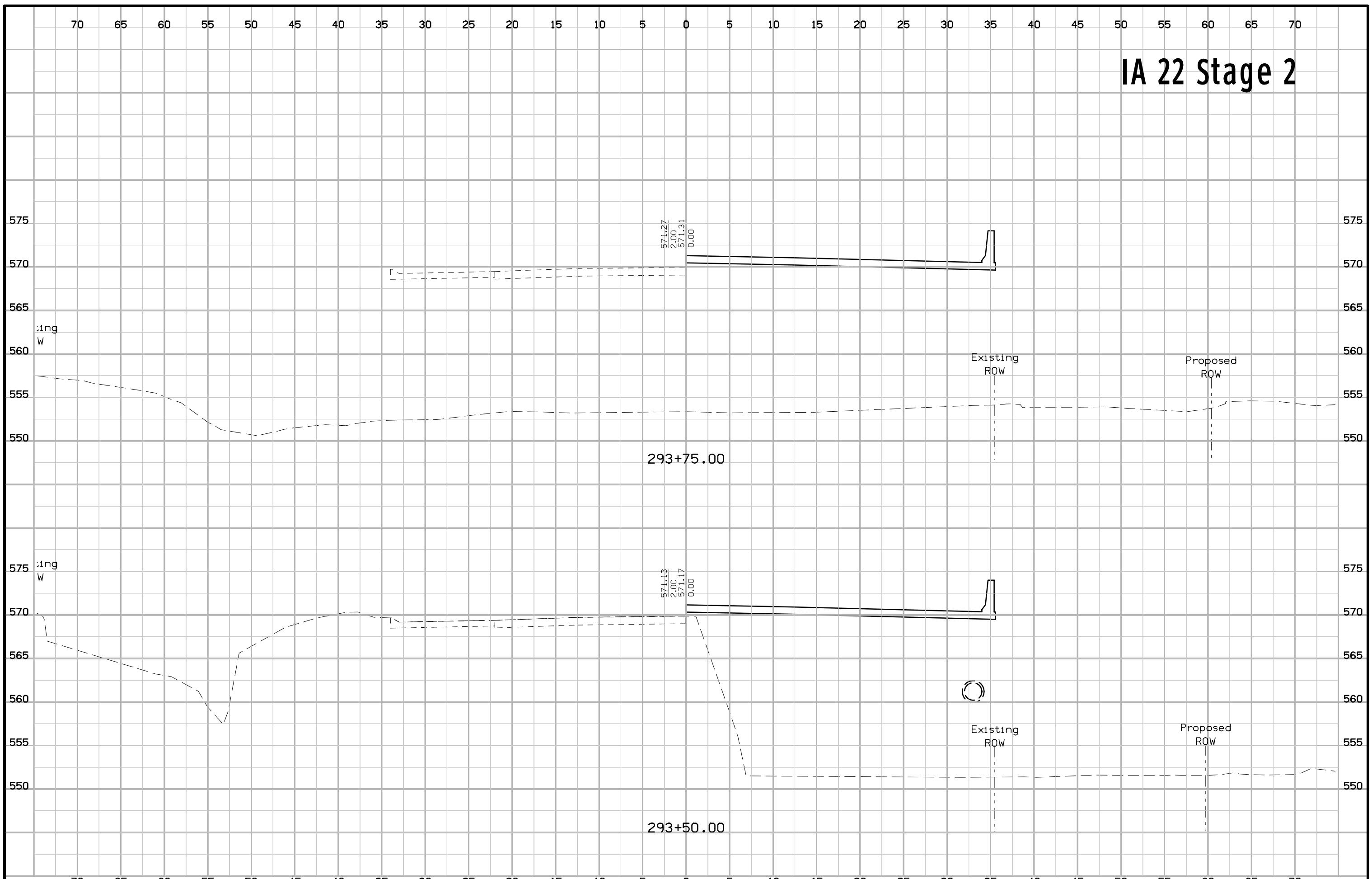




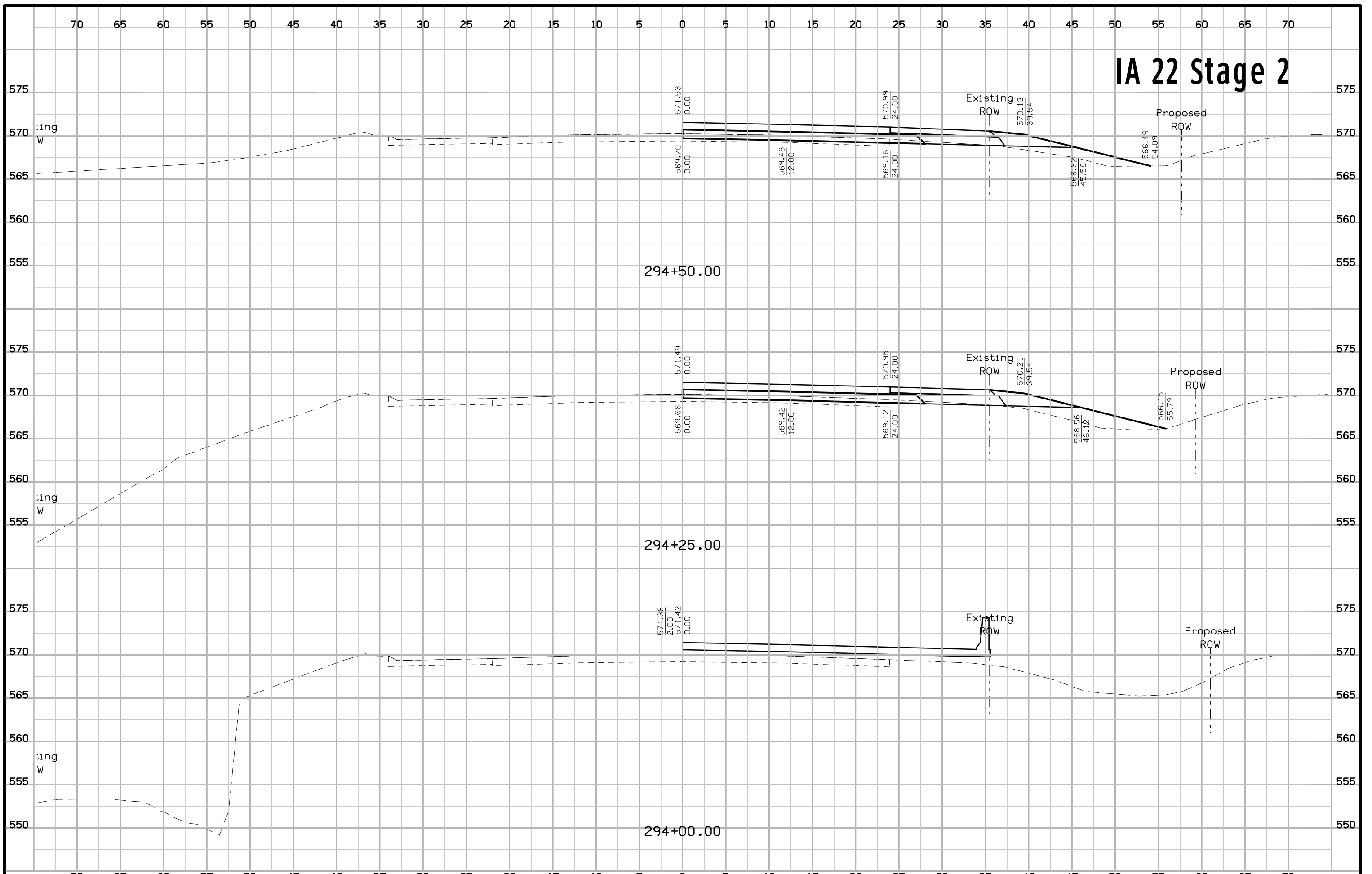
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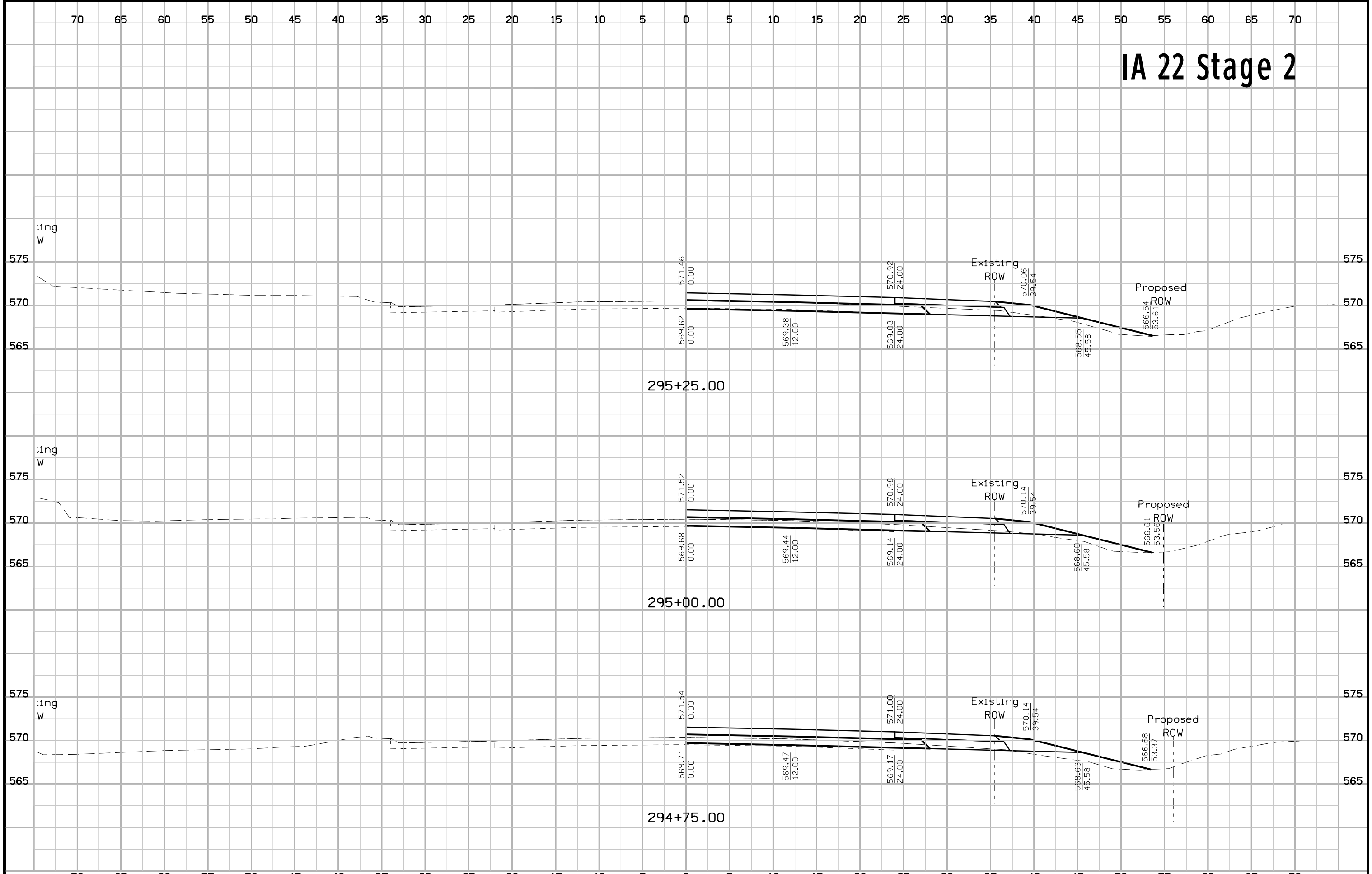
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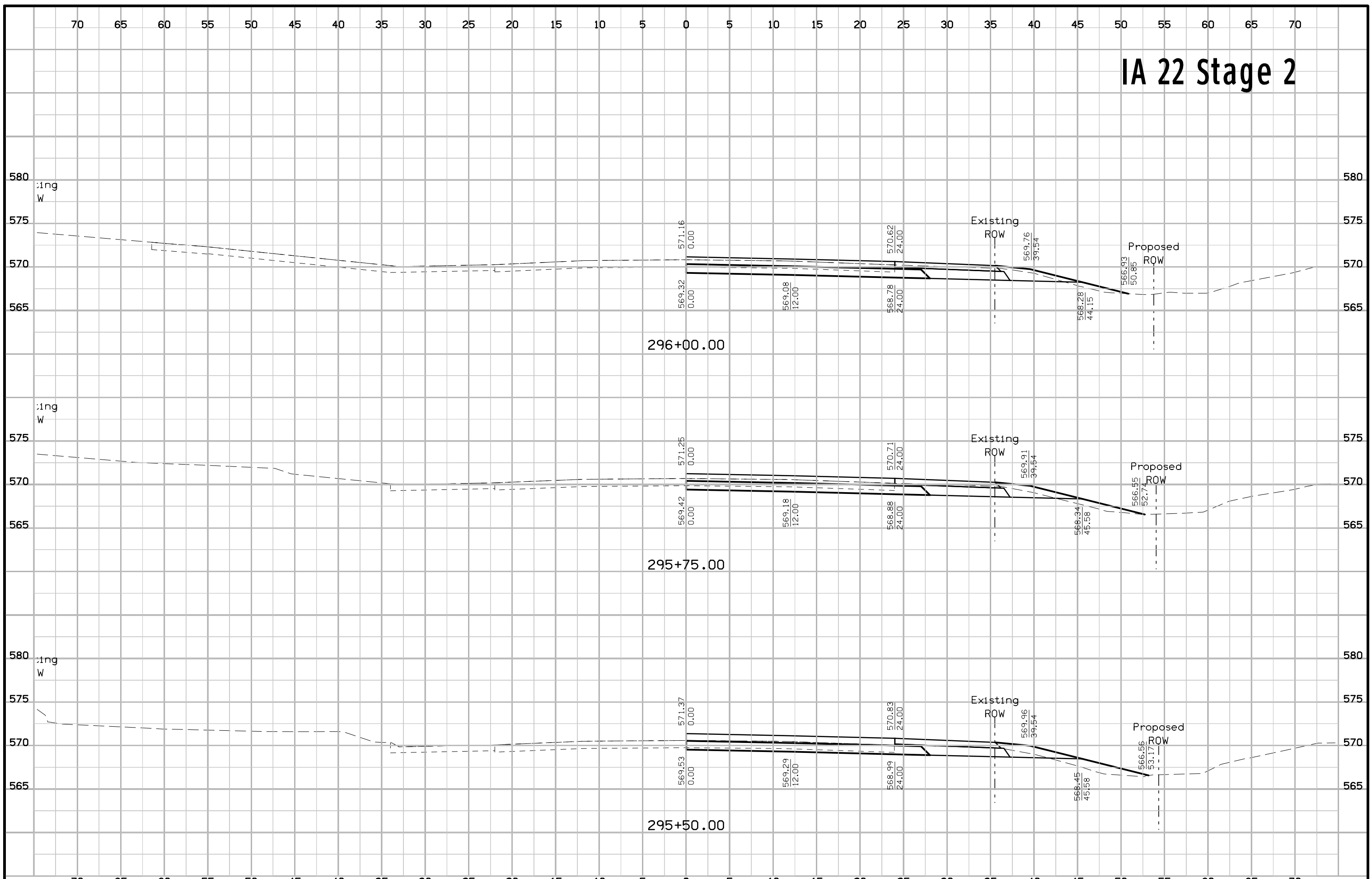
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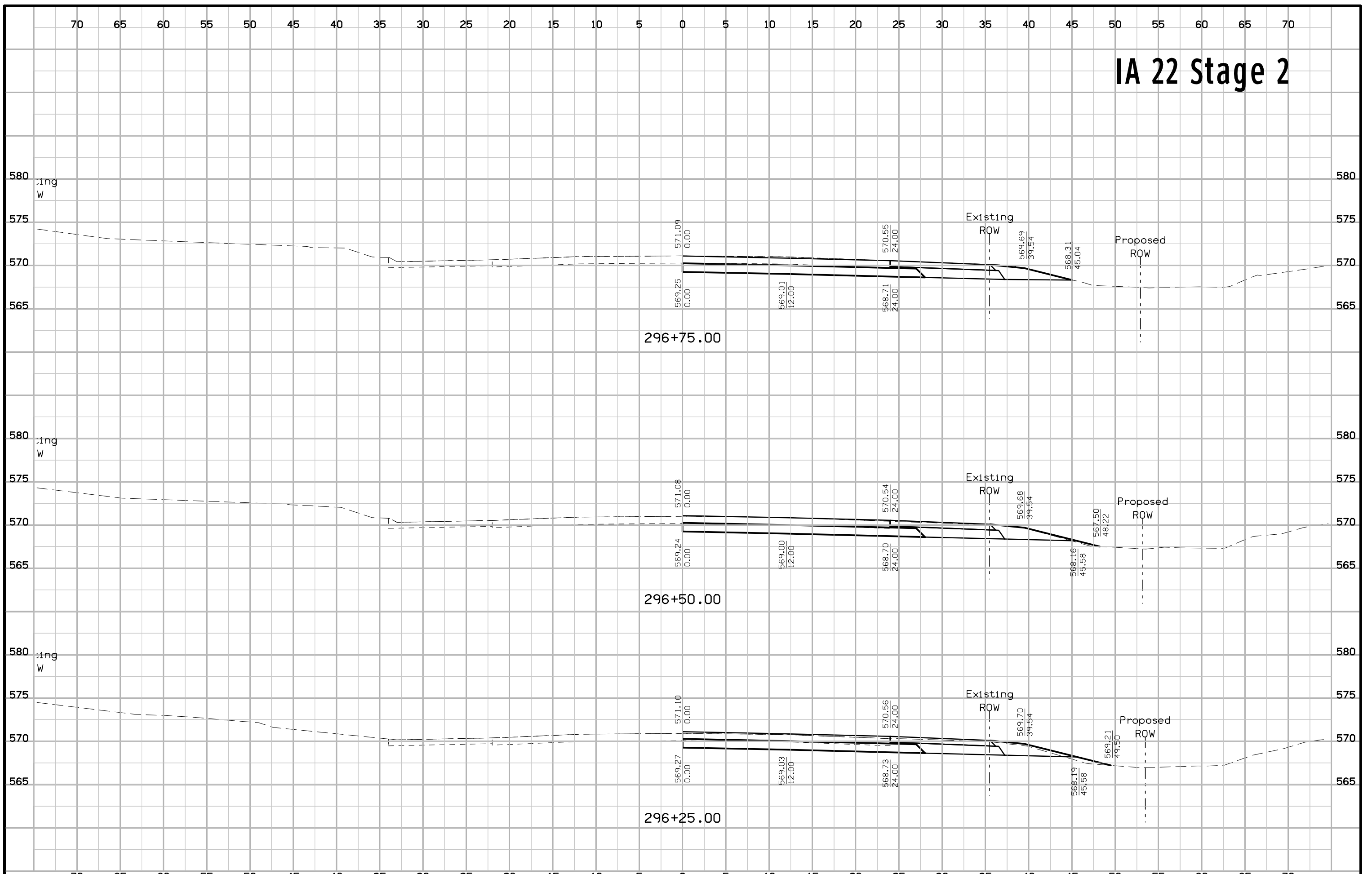
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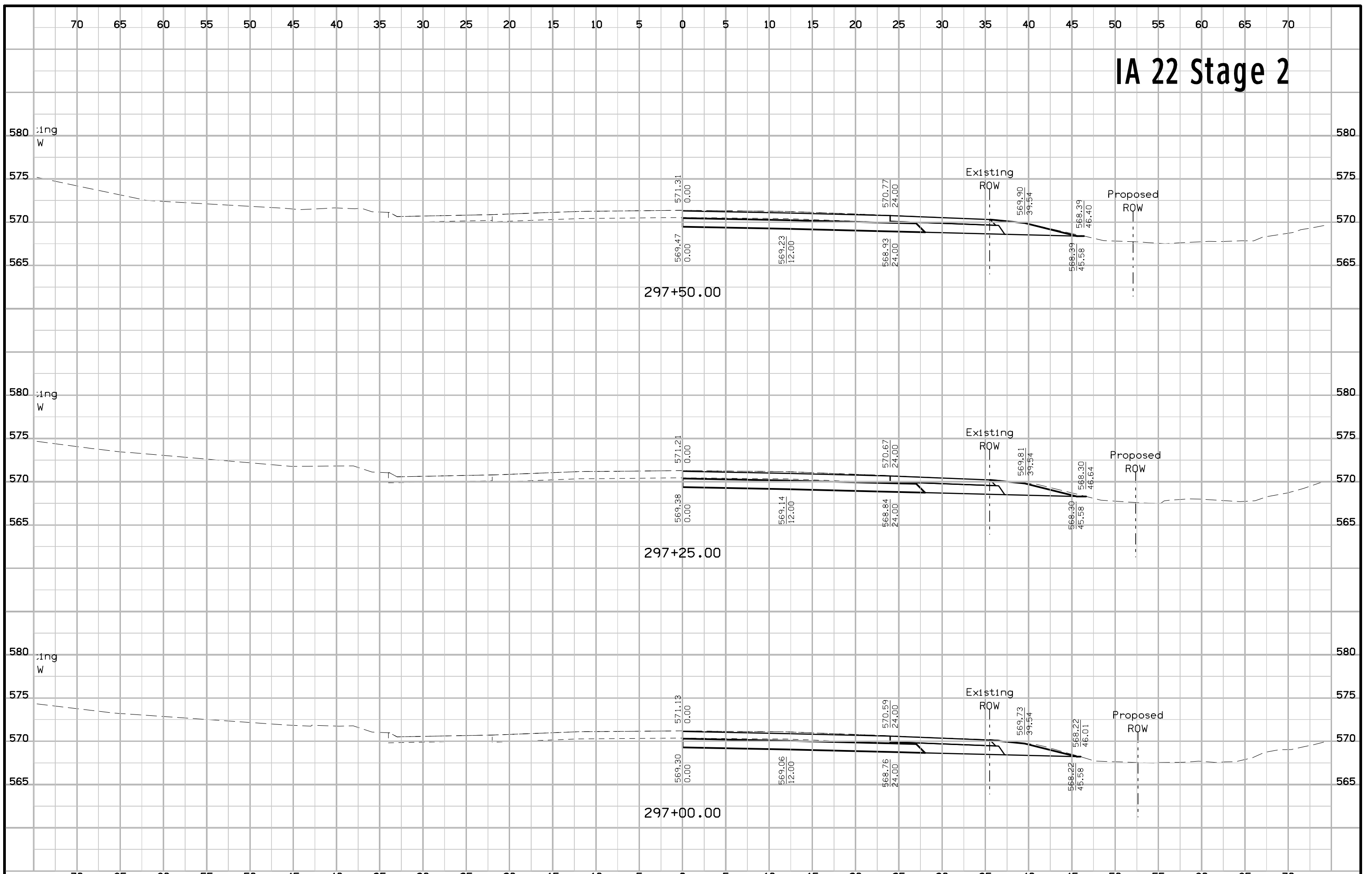
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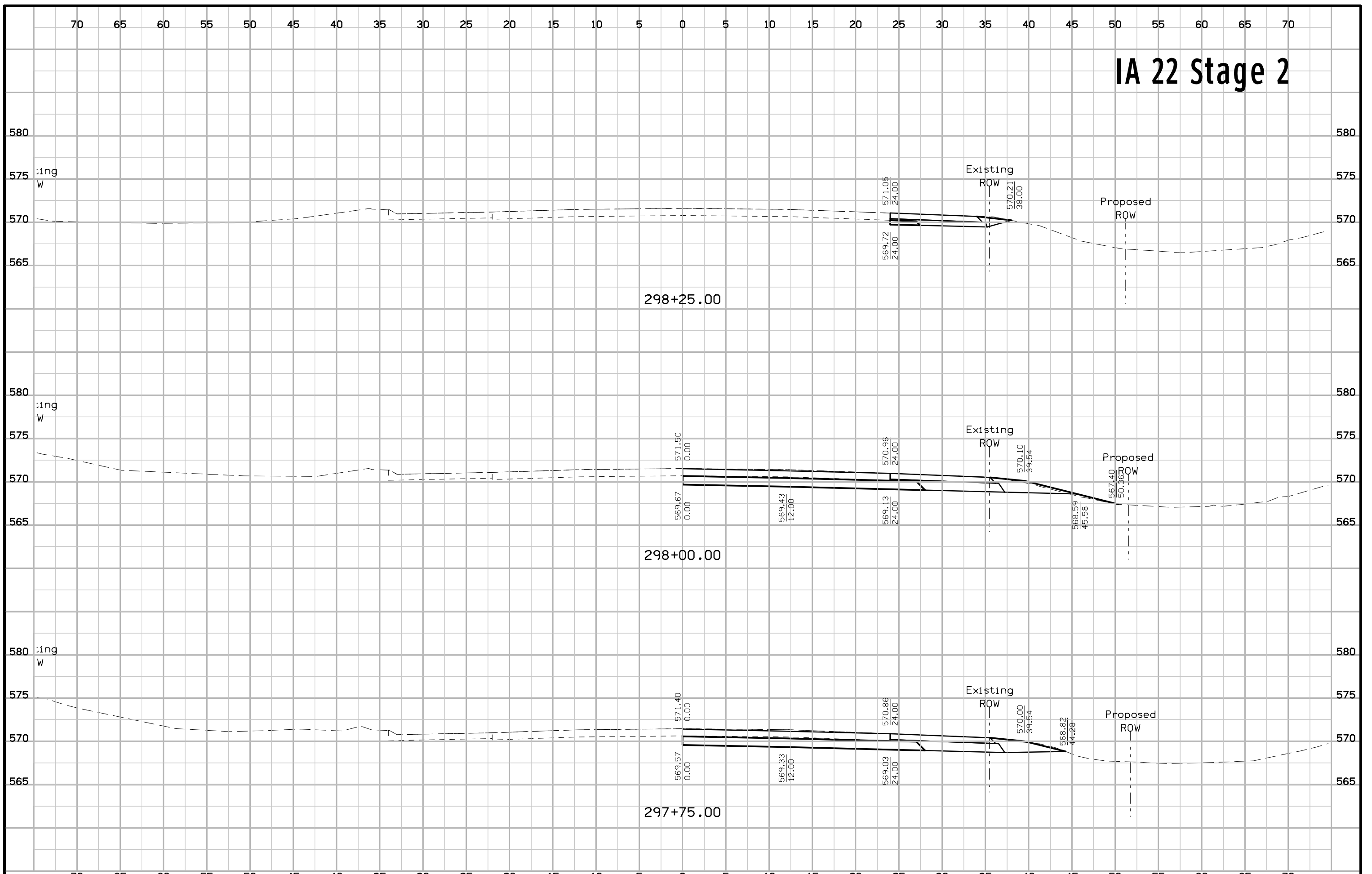
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# IA 22 Stage 2

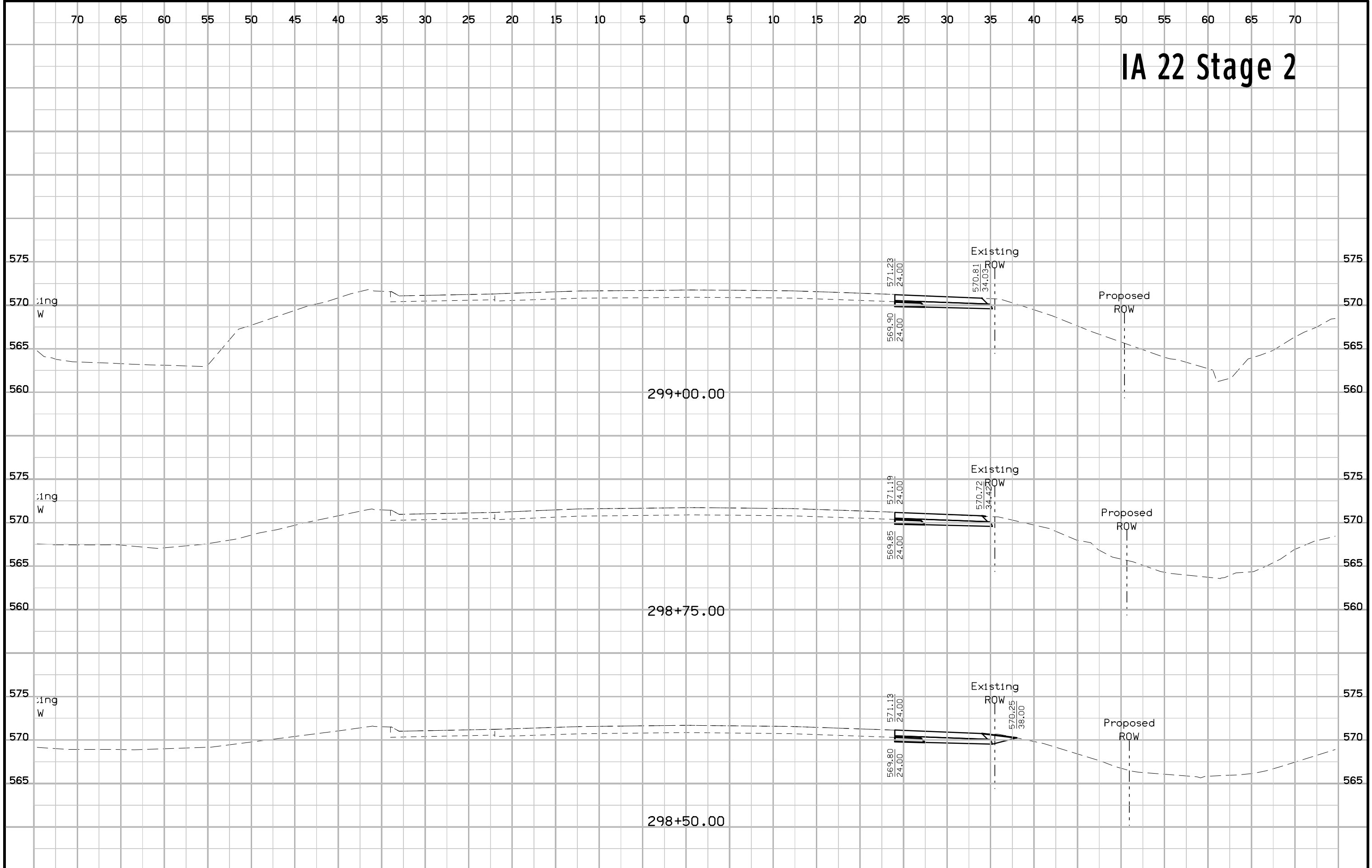


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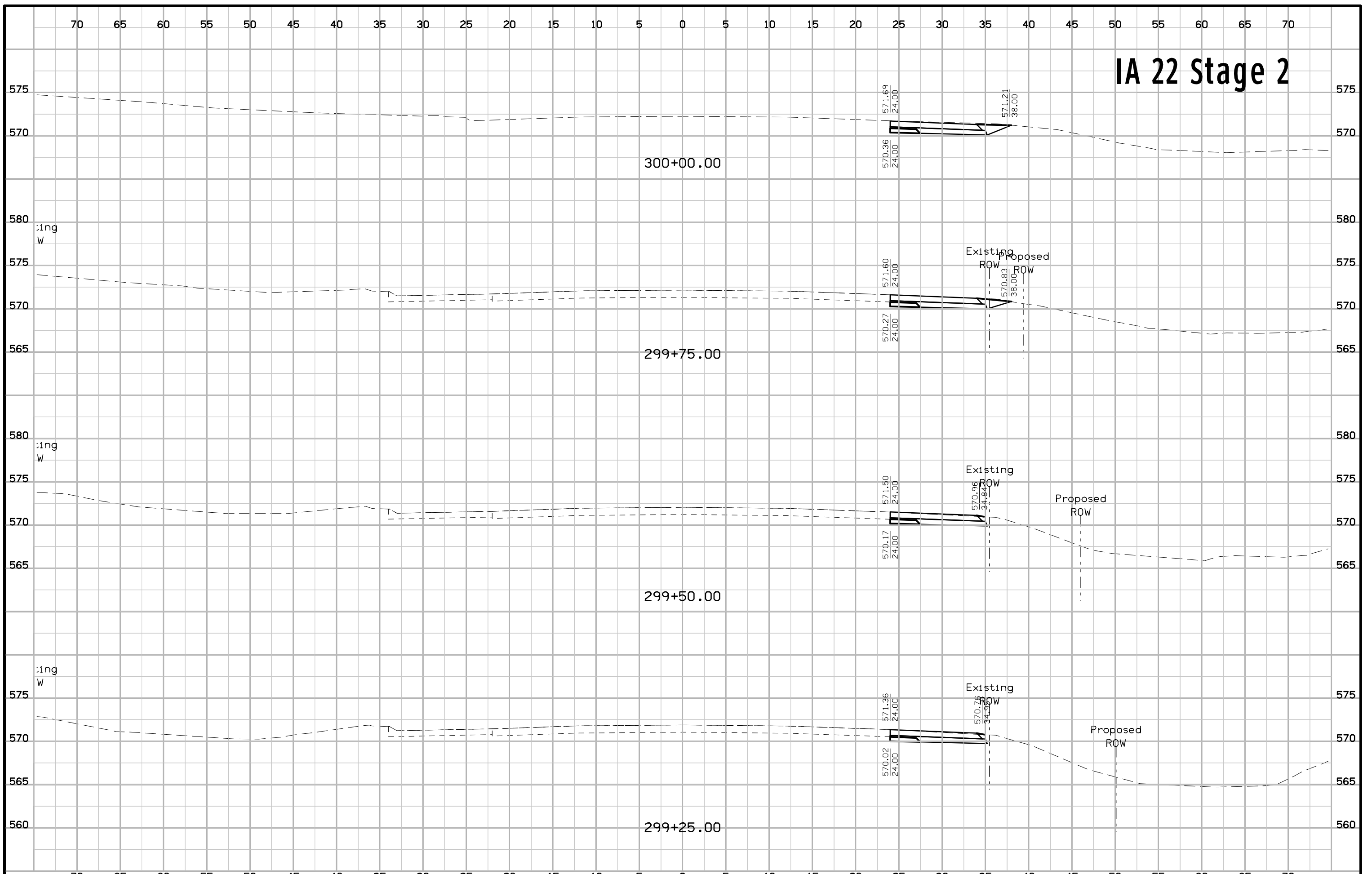




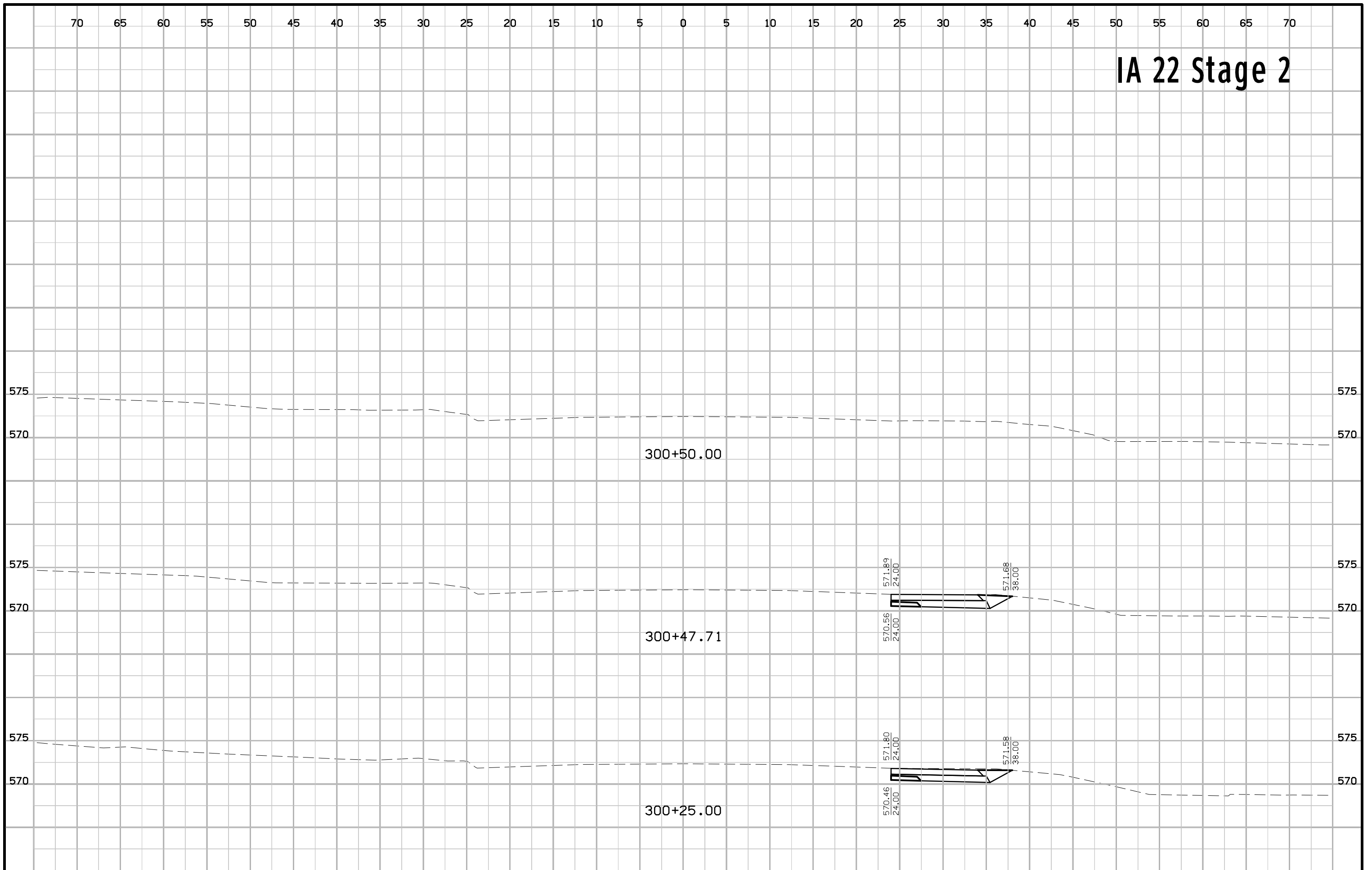
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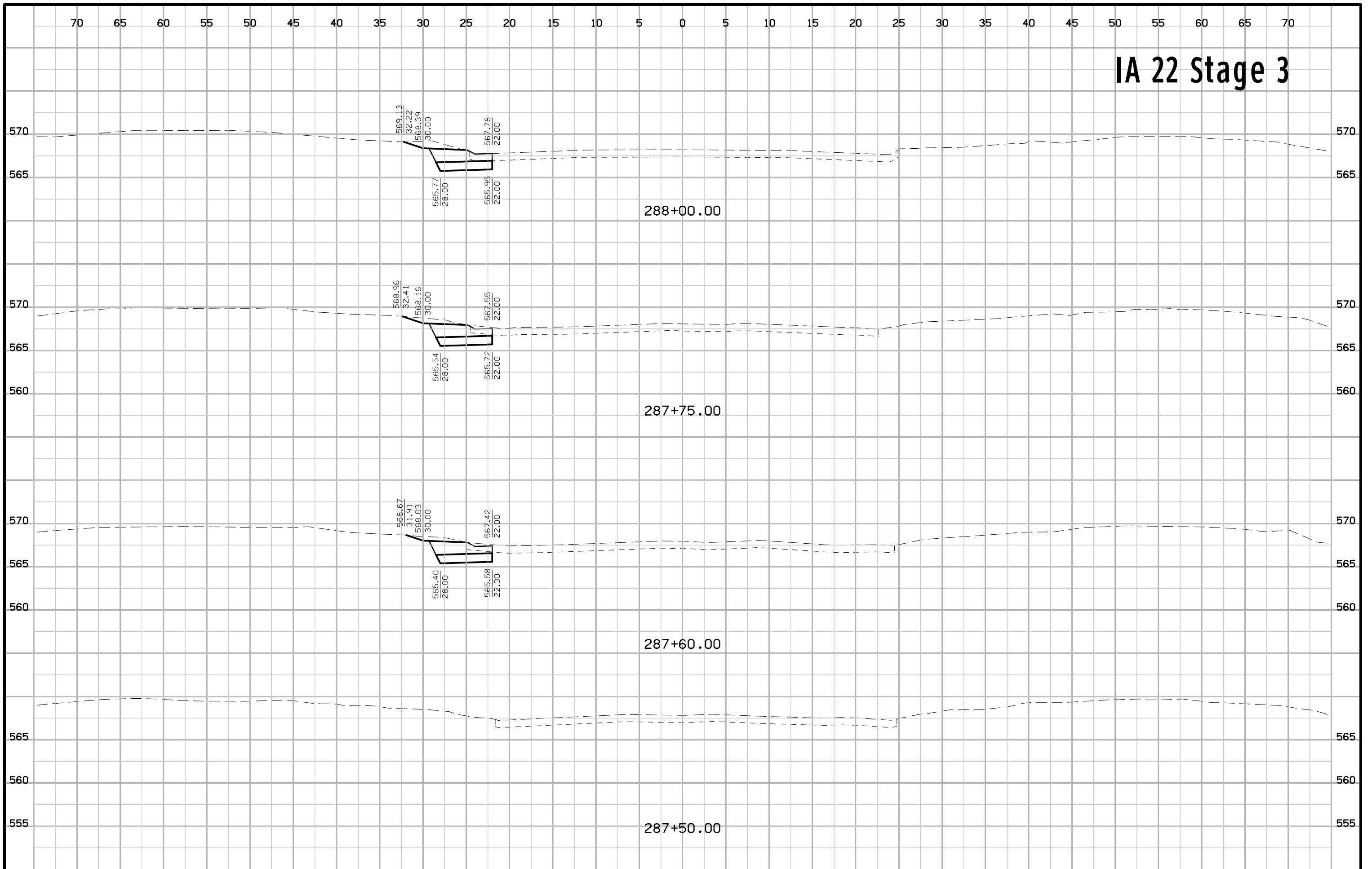
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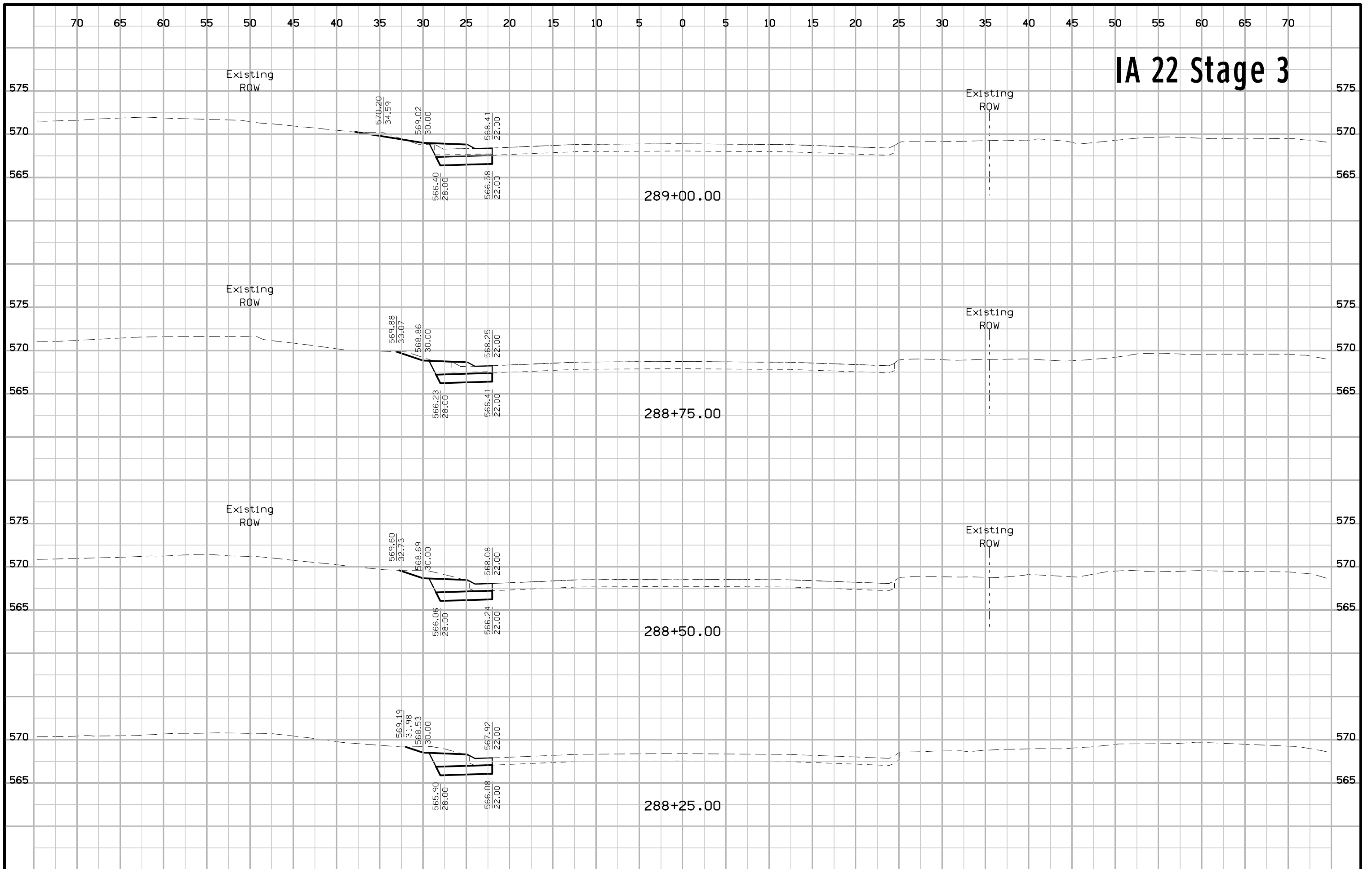
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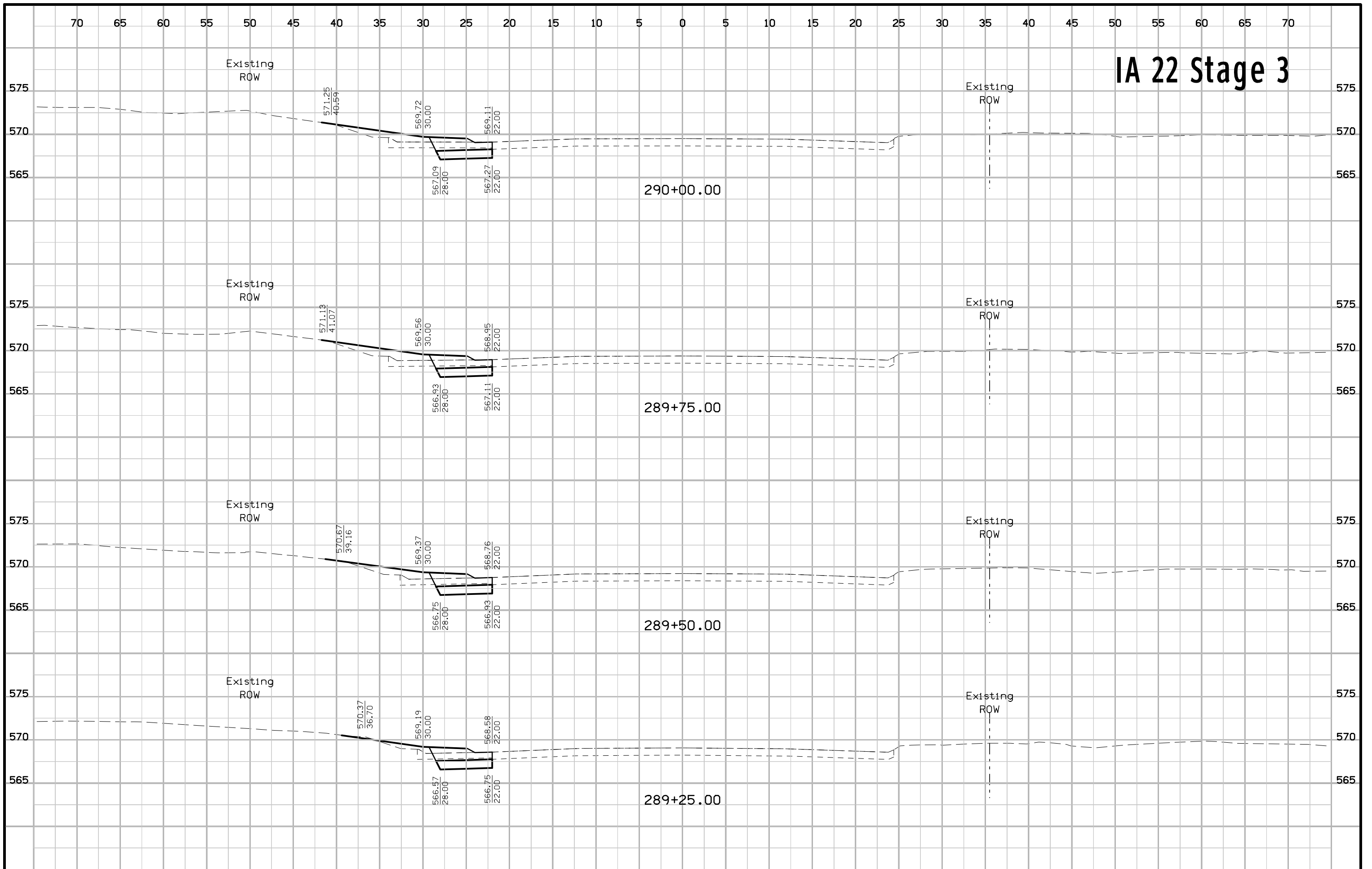
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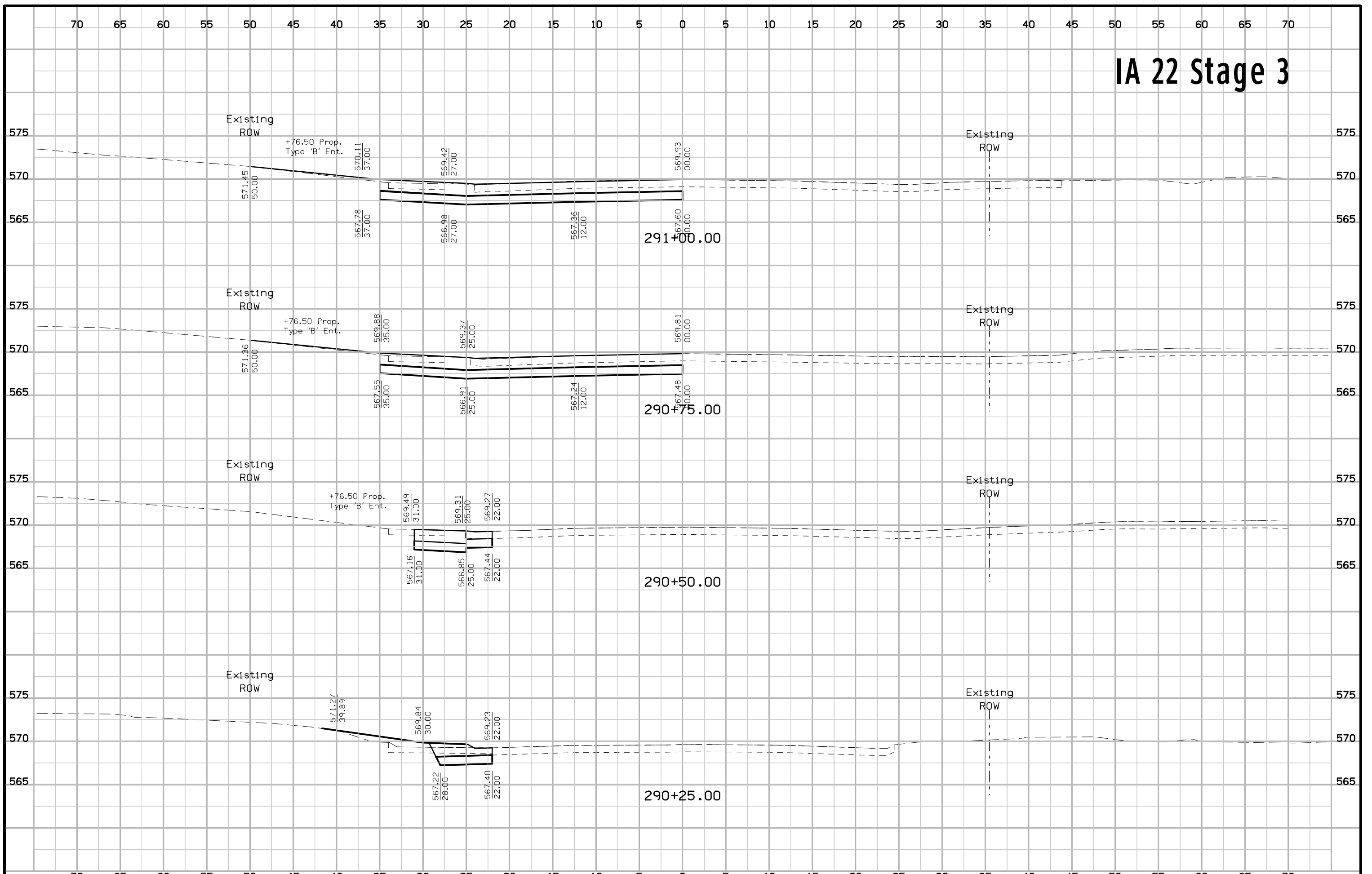
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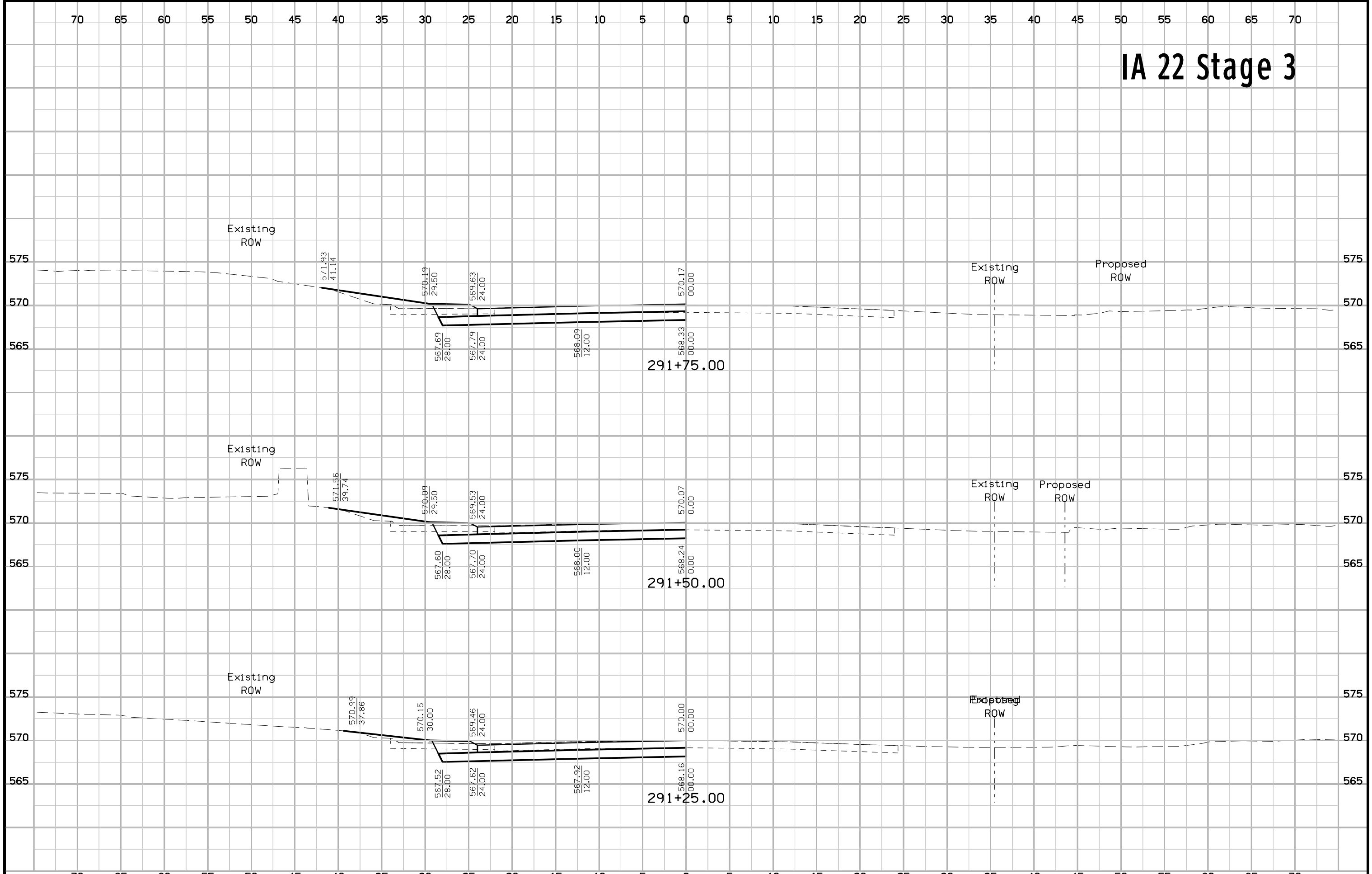
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# IA 22 Stage 3

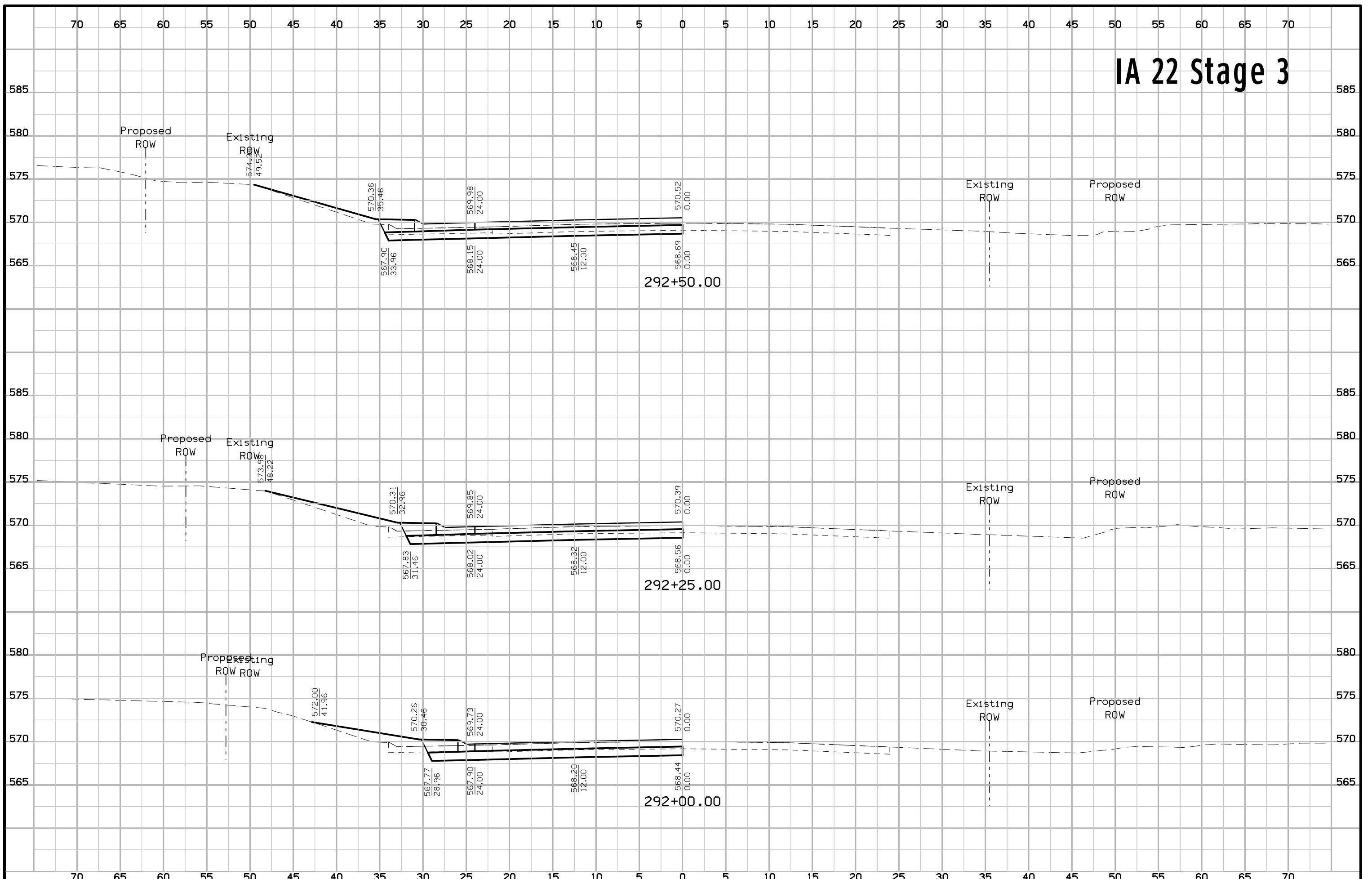


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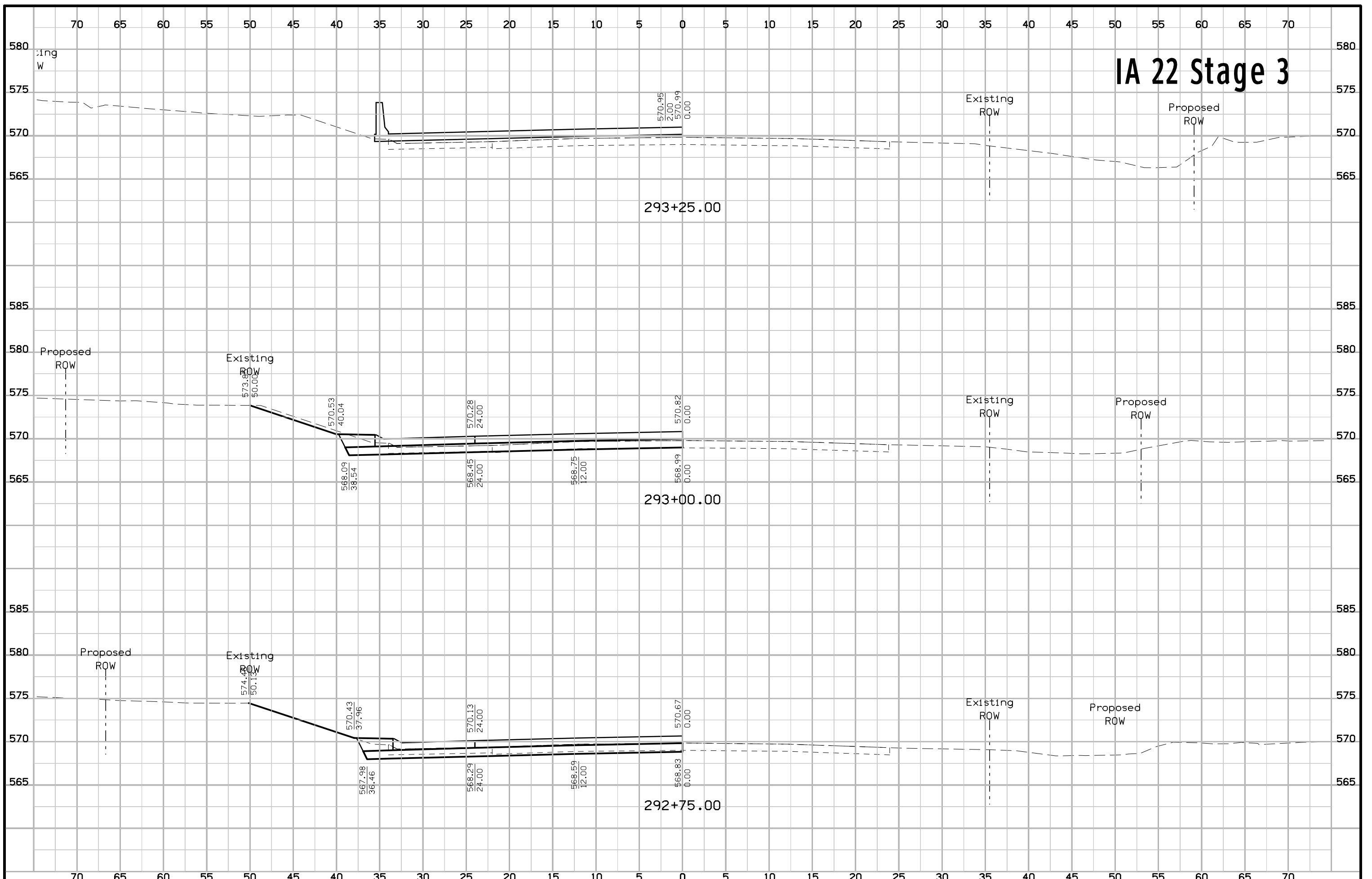




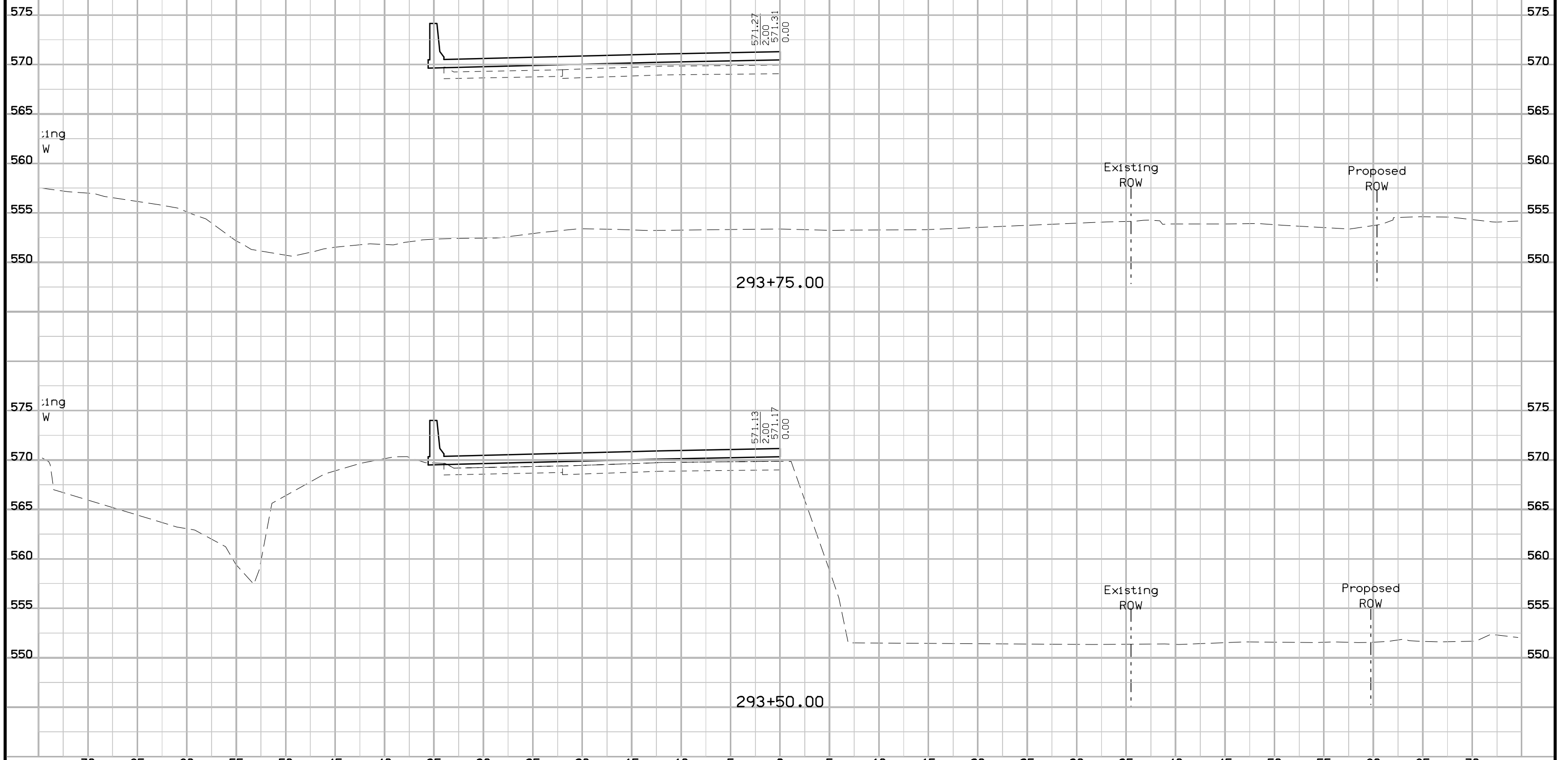
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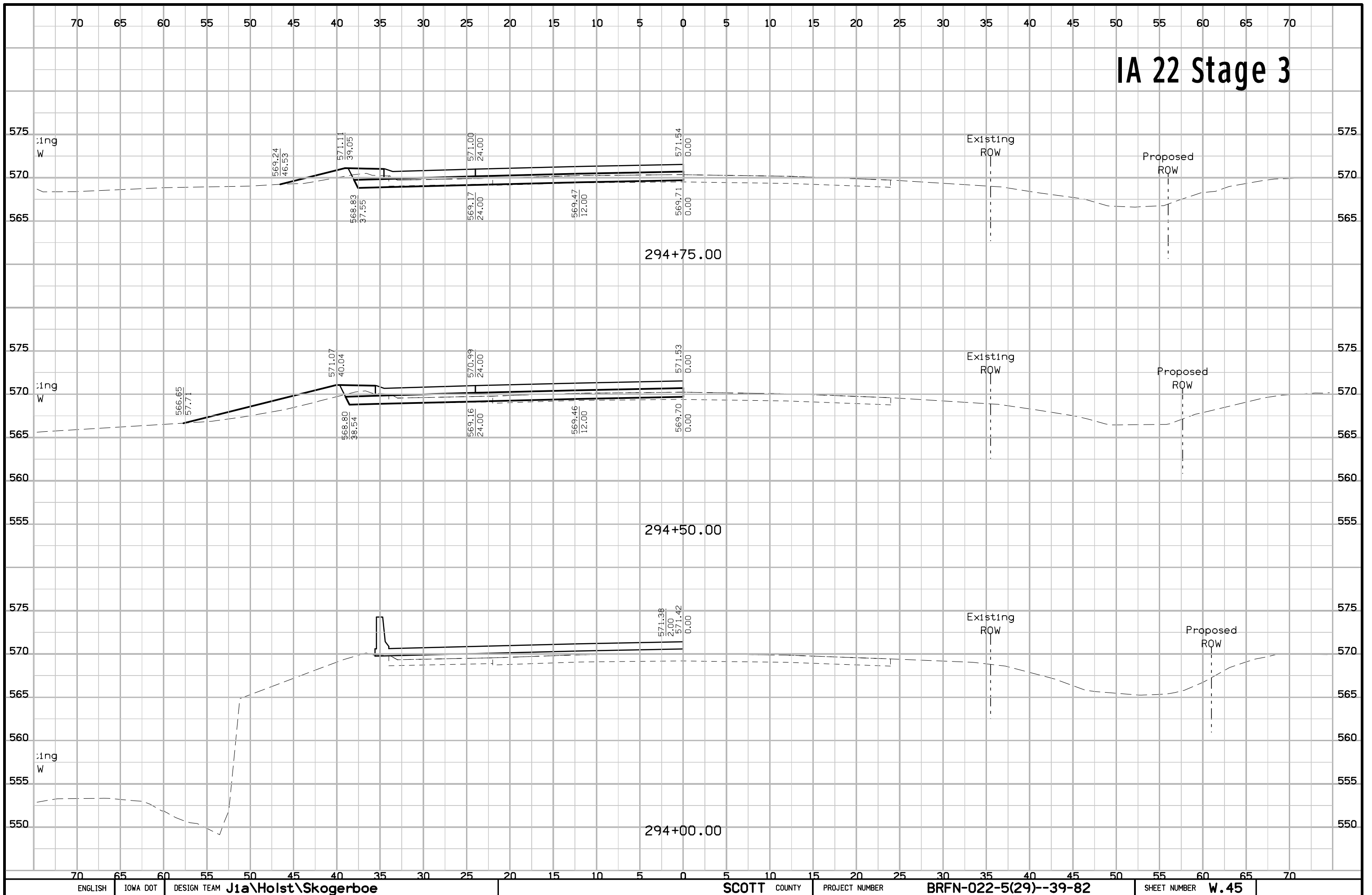
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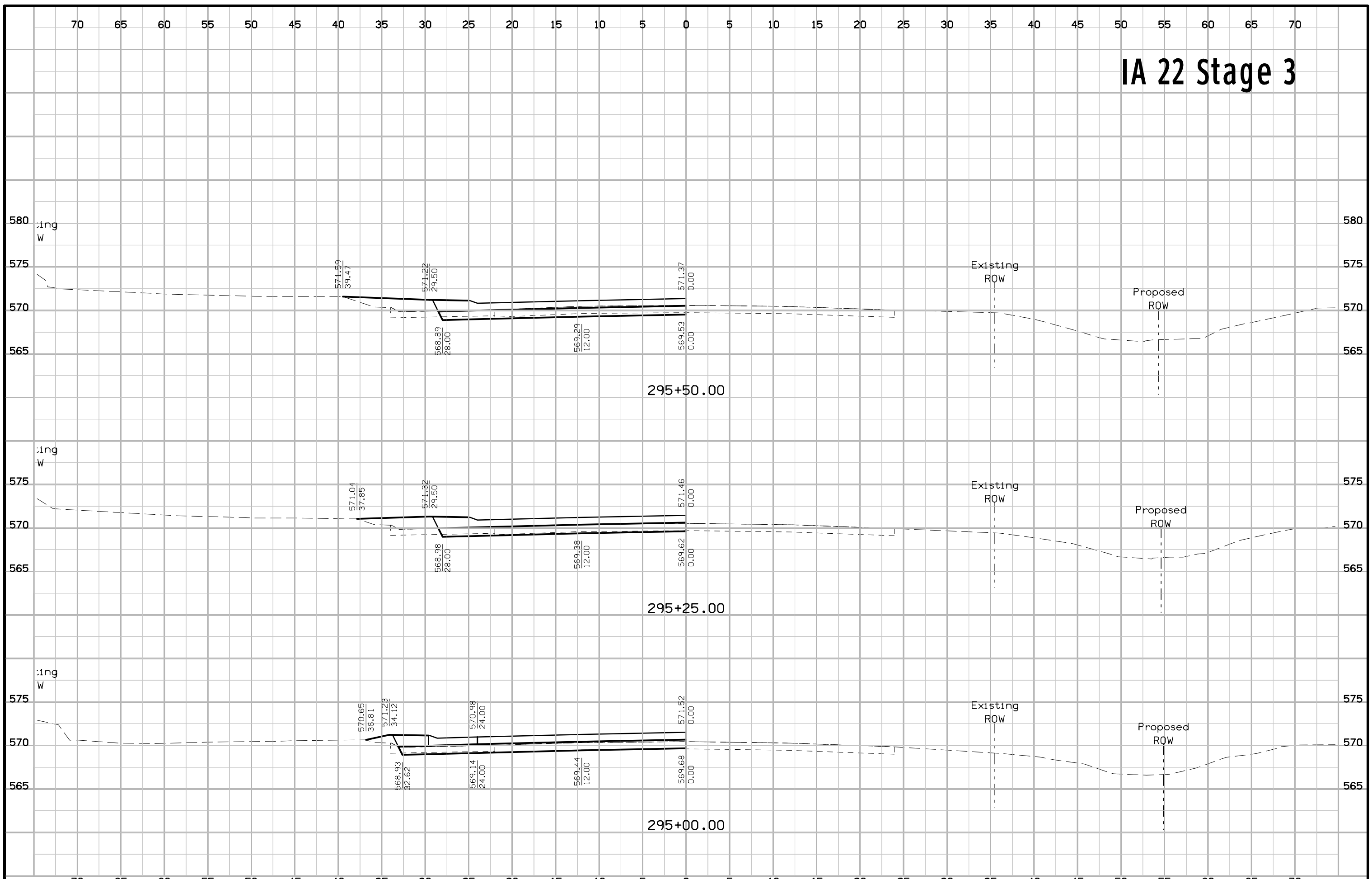
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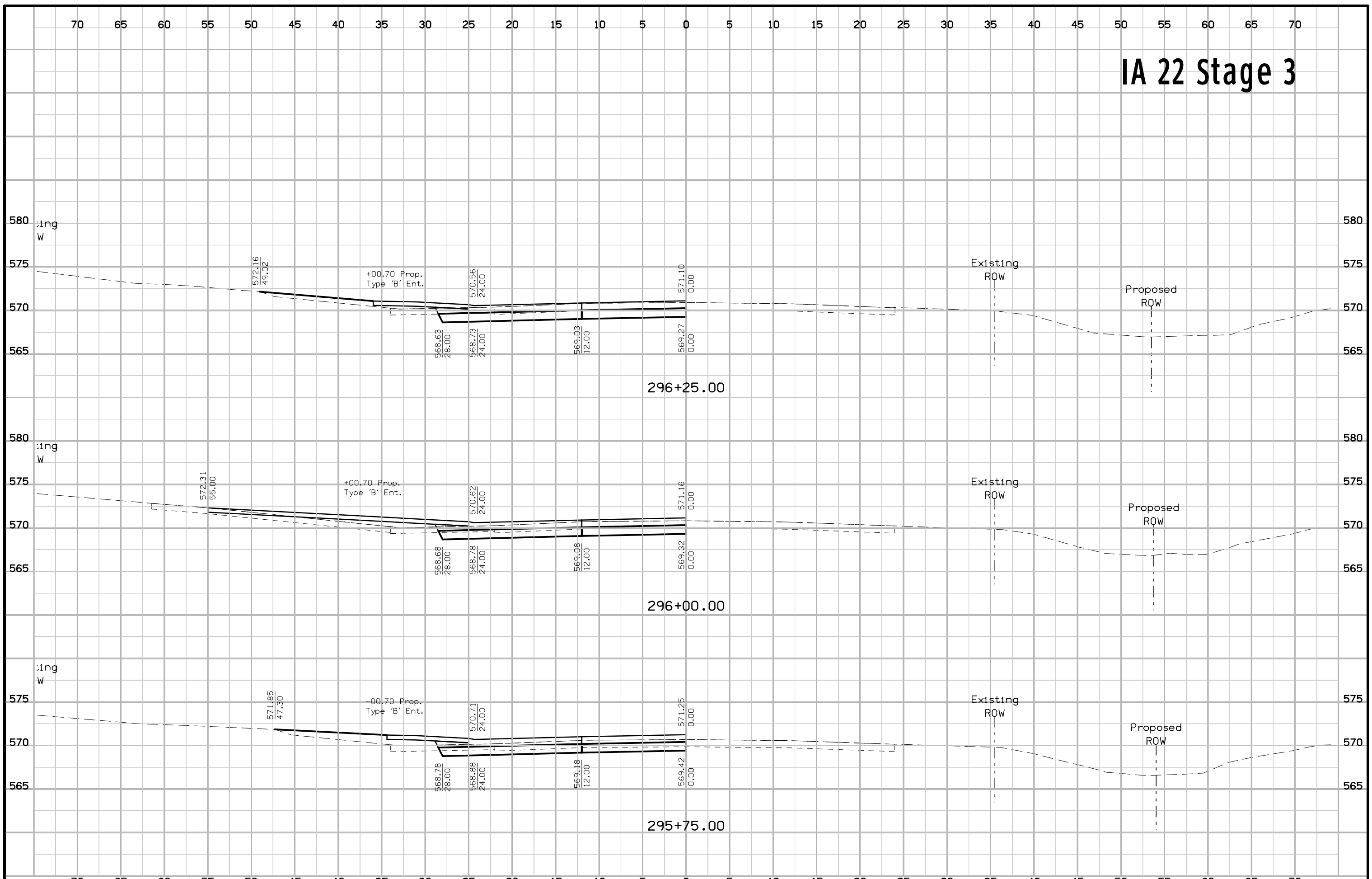
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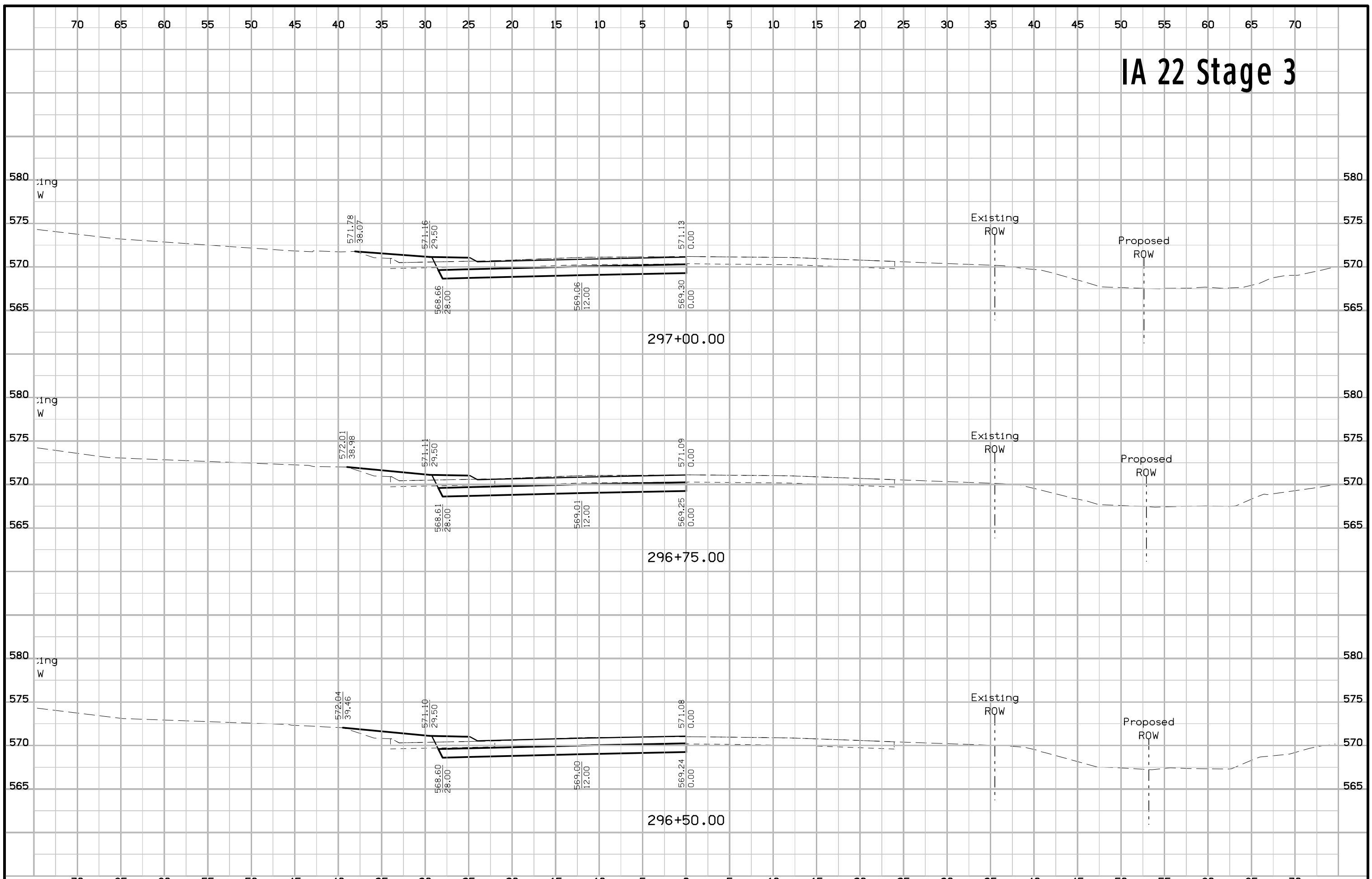
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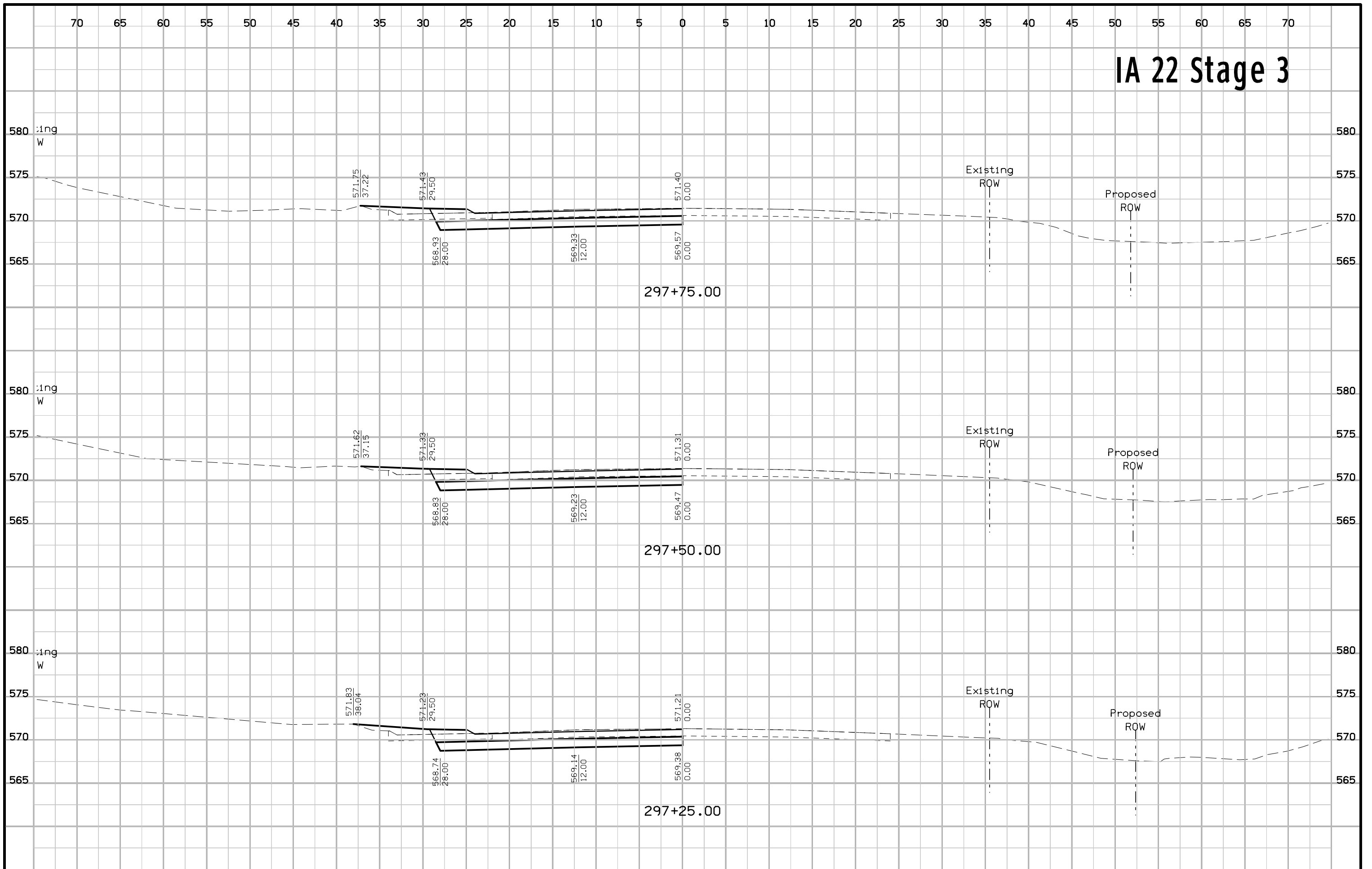
# IA 22 Stage 3



# IA 22 Stage 3

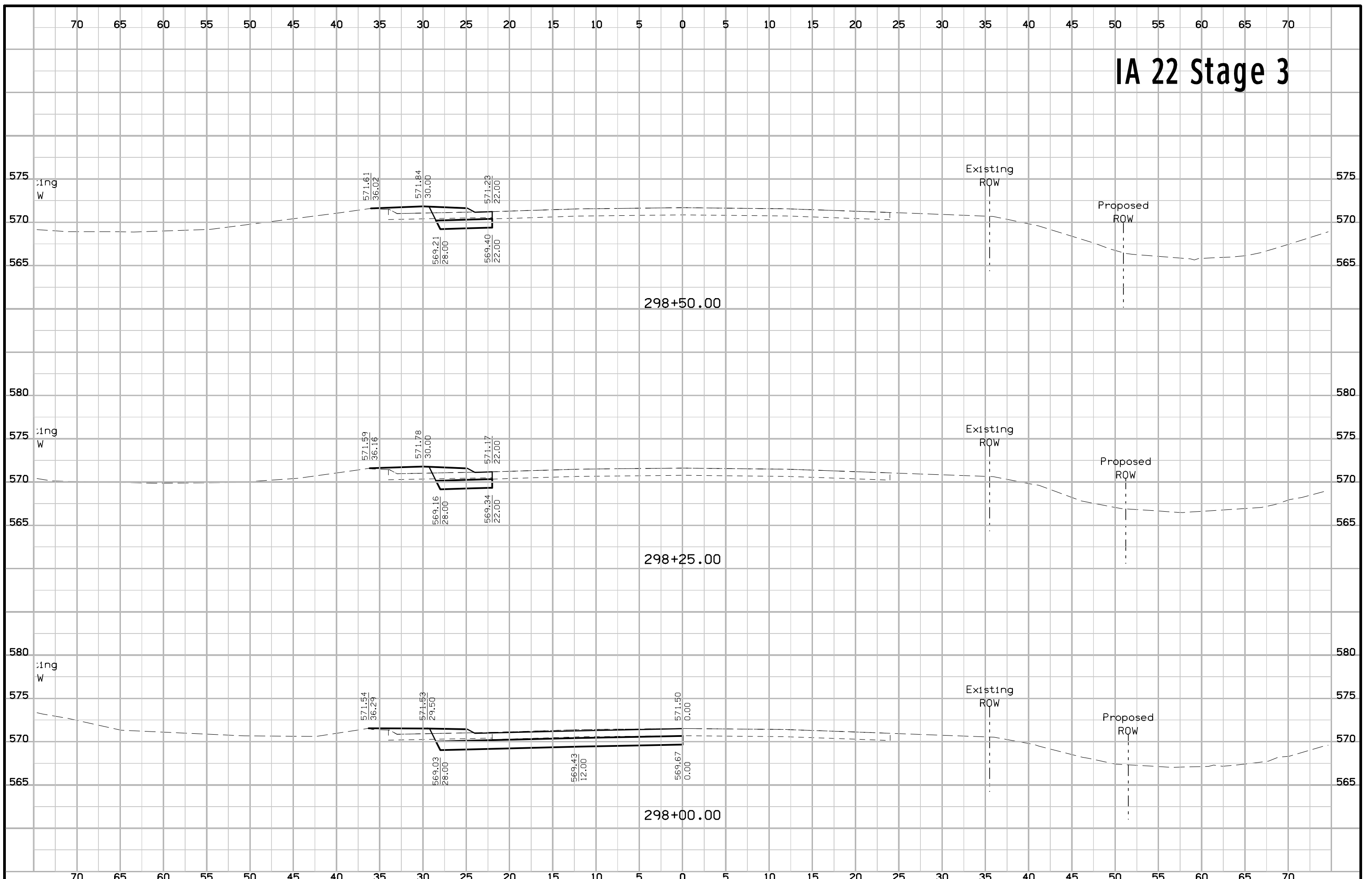


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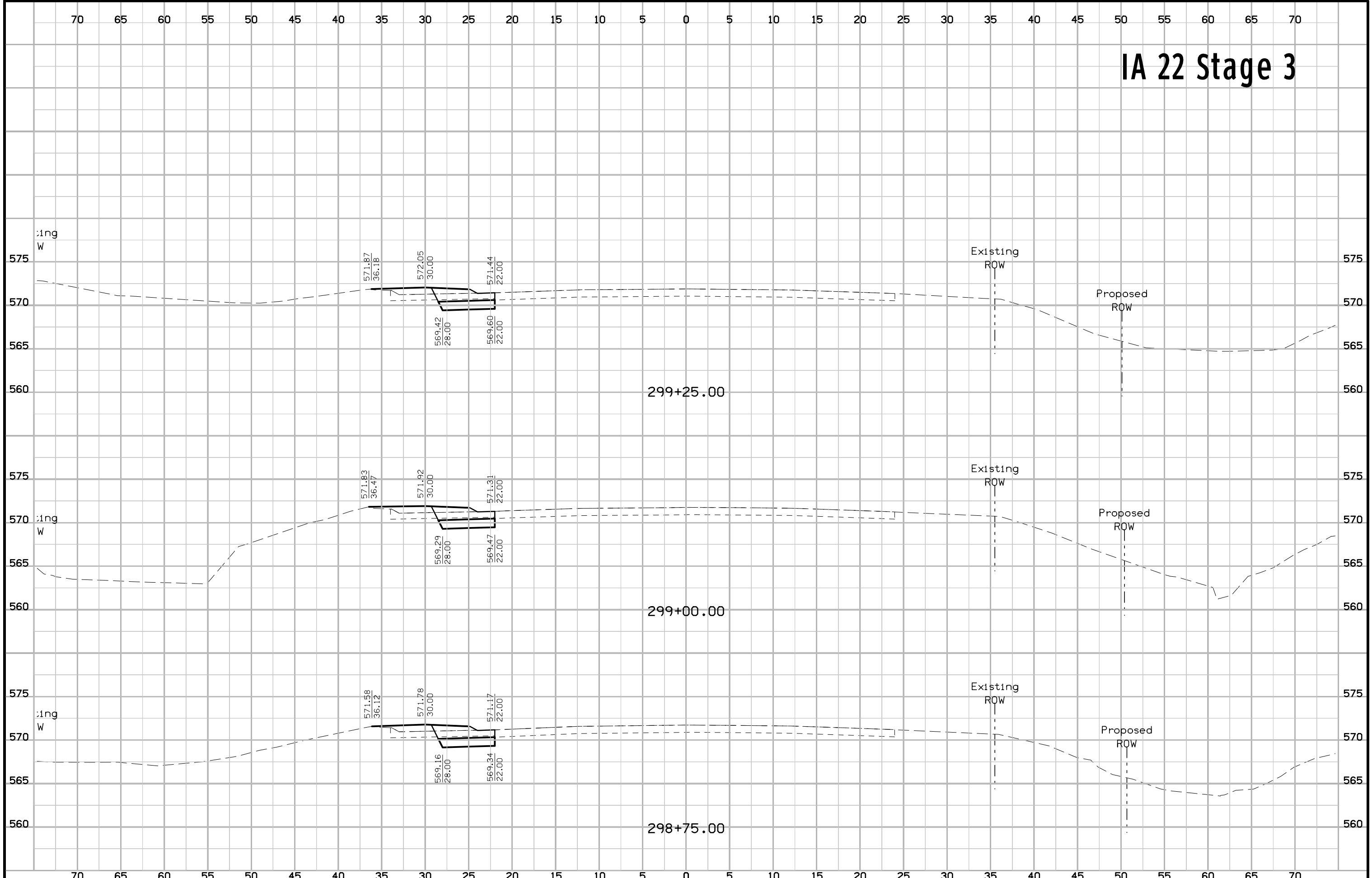




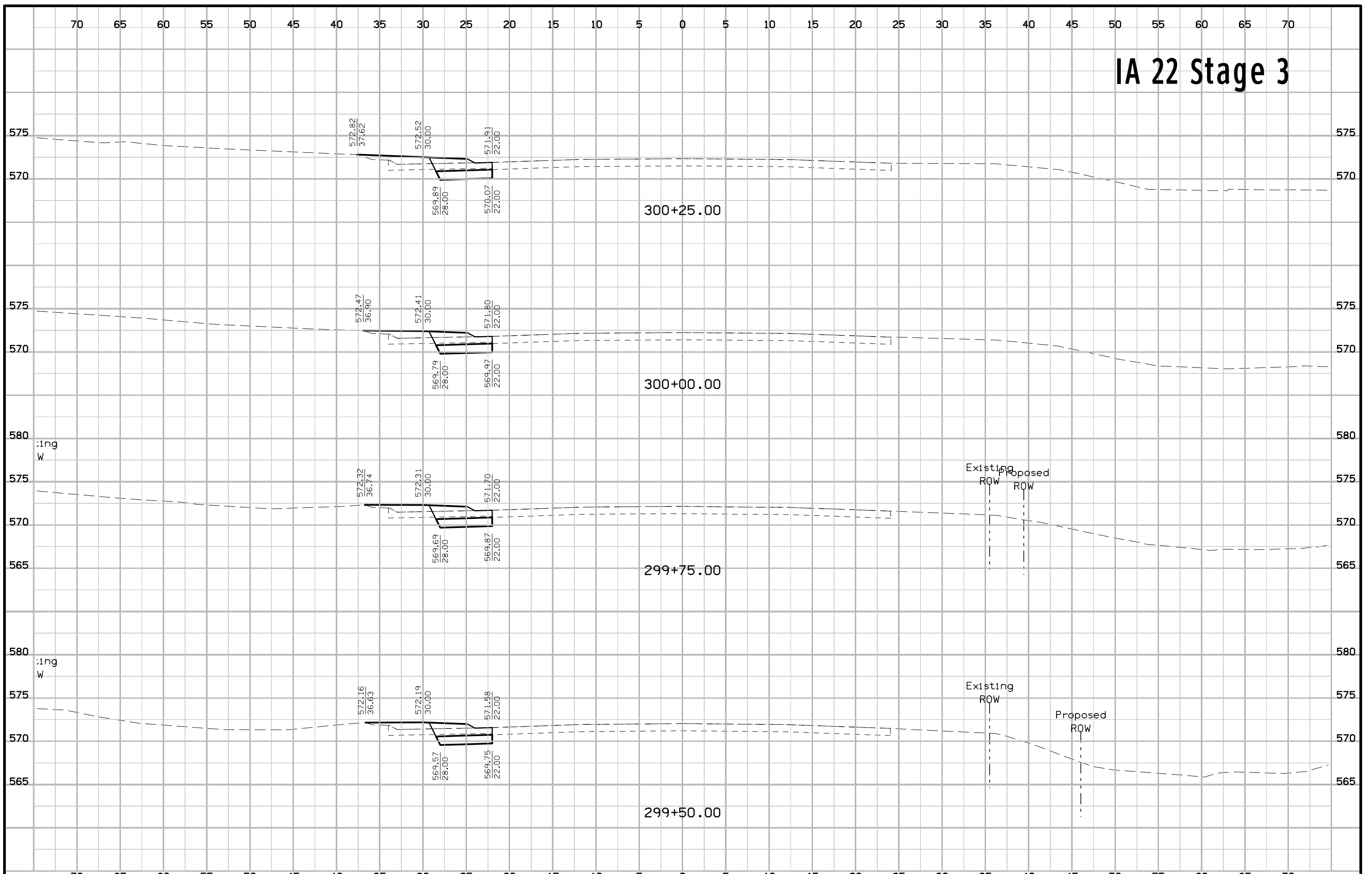
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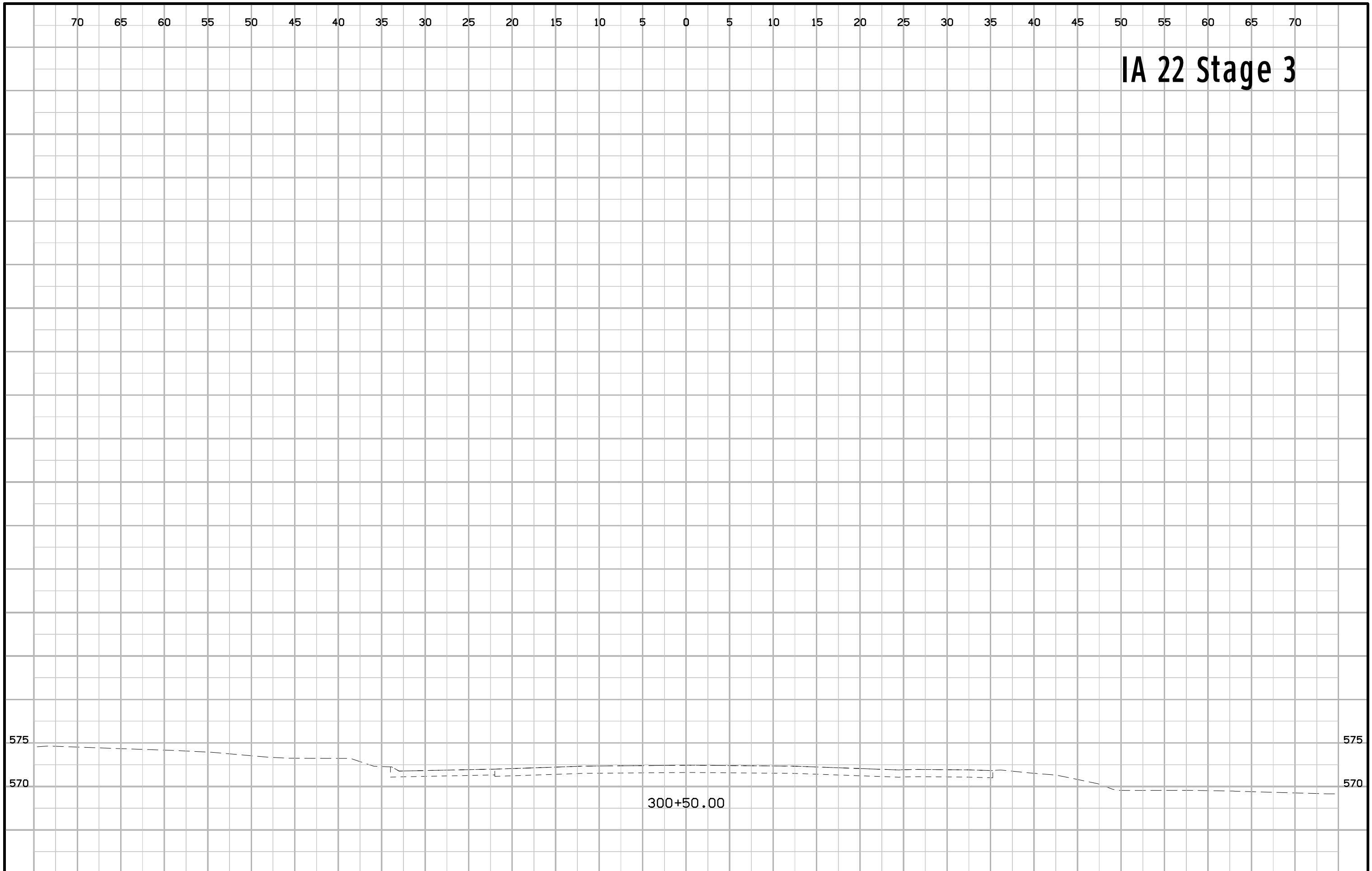
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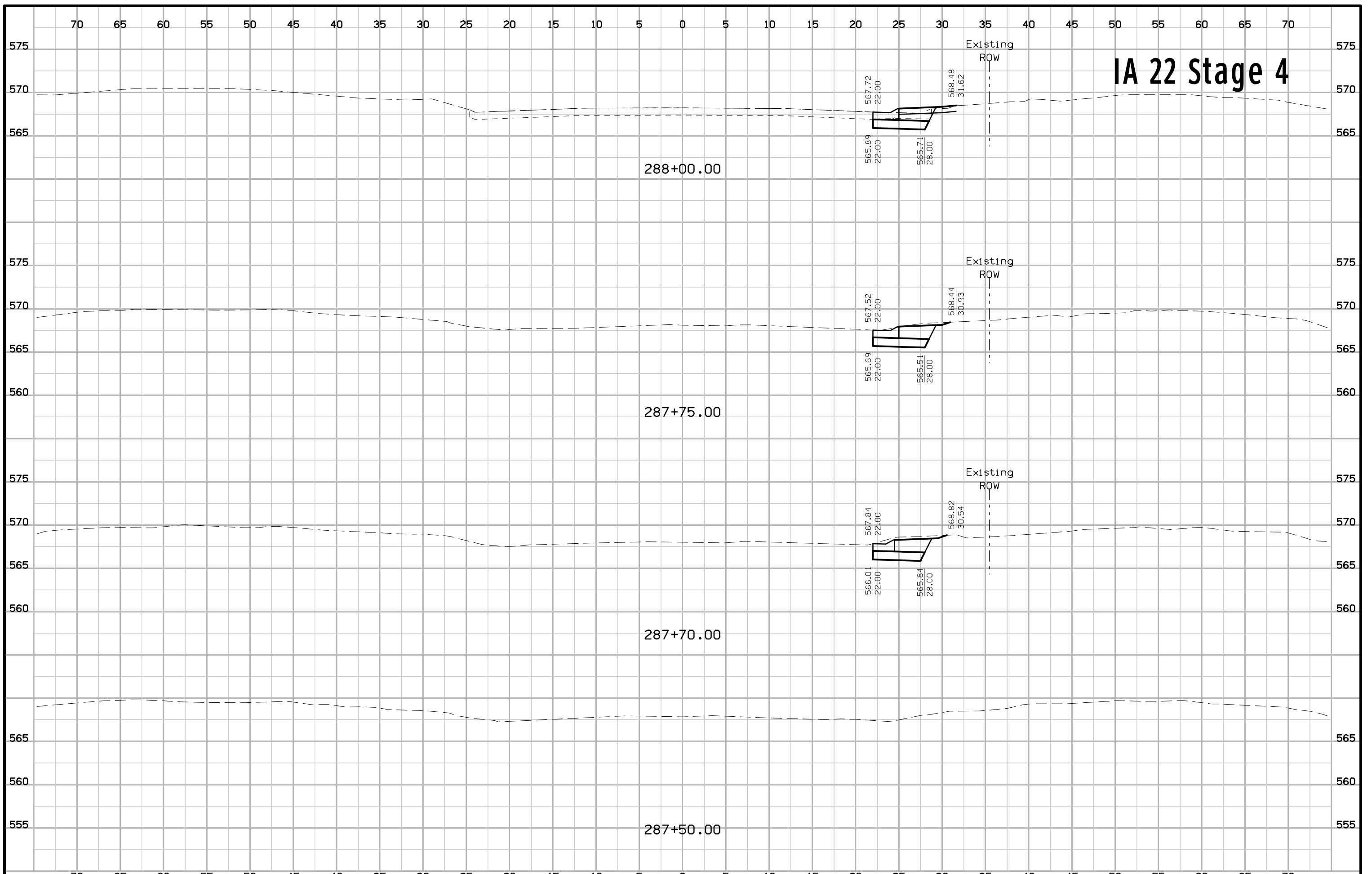
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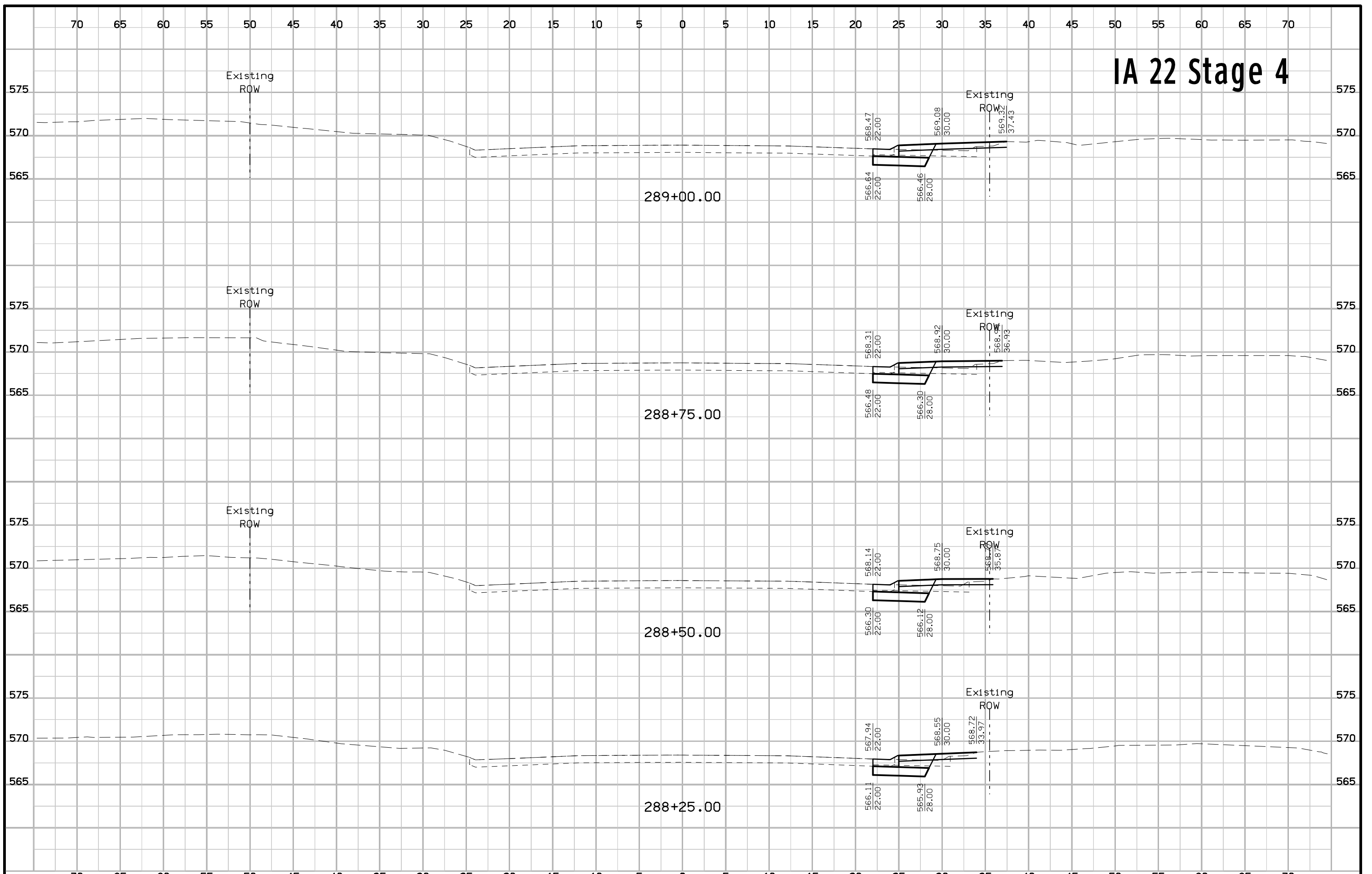
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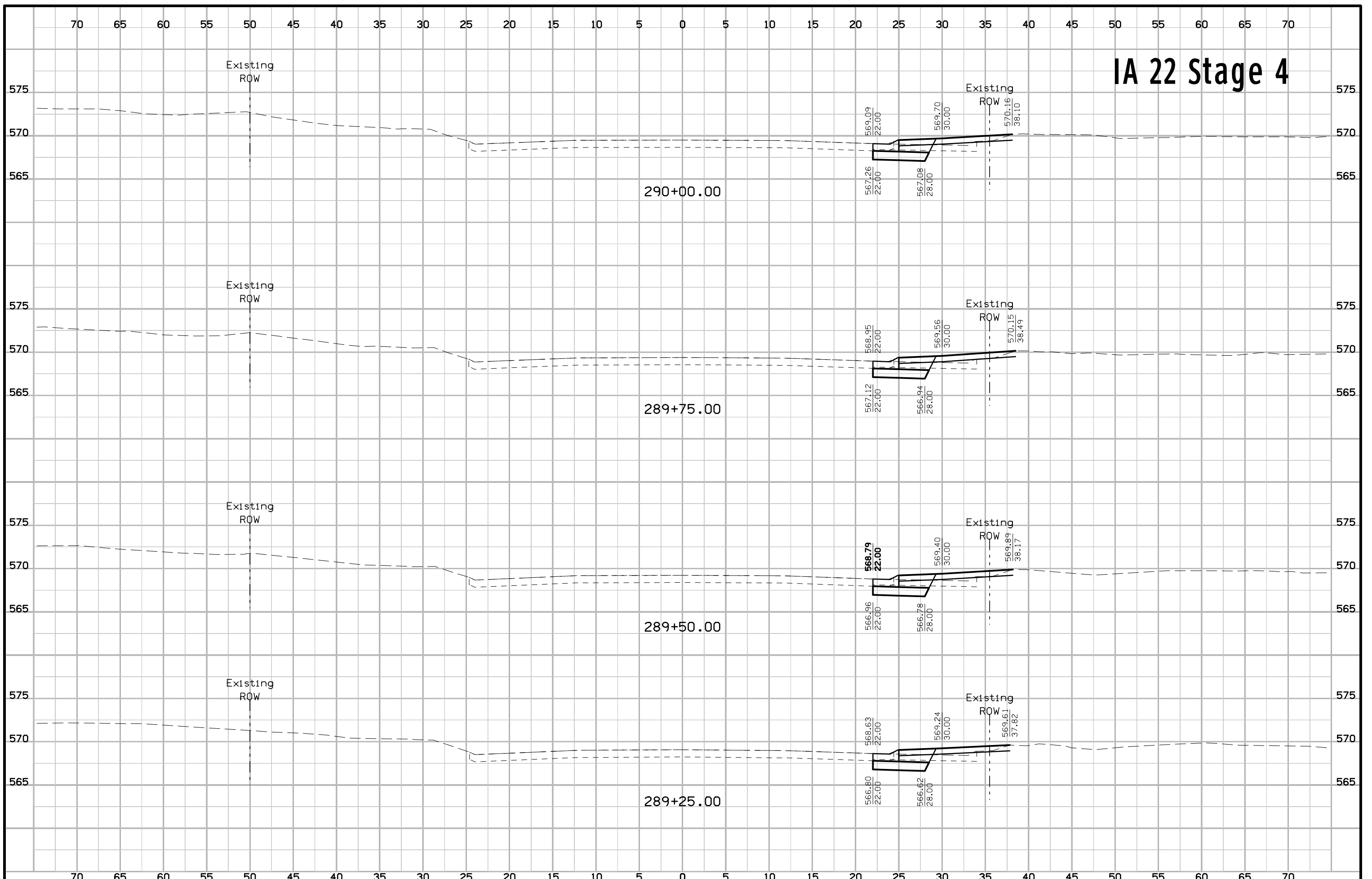
# IA 22 Stage 4



# IA 22 Stage 4



# IA 22 Stage 4



# IA 22 Stage 4

