

## PROJECT CONCEPT STATEMENT

Abutment Scour Repair on IA 92 MP41.5 over North Fork Long Creek  
Located approximately 0.7 mi. W of Jct. US 218  
FHWA No. 51841, Maint. No. 9241.5S092  
T75N, R6W, Section 21  
Washington County, MB-092-5(501)242--77-92

Highway Division  
Bridges and Structures Bureau

Matt Erickson  
matthew.erickson@iowadot.us  
May 15<sup>th</sup>, 2020

### Project Description

The abutment berms have started to erode. The project site is located approximately 0.7 mi. W of Jct. US 218.

### Need for Project

If left unchecked the abutments will continue to be undermined and material behind the abutment will be lost.

The current scheduled letting date is 7-20-2021.

### Concept

#### Abutment Scour Repair with Revetment (See Attachment)

Install 2 foot thick class E revetment along the abutment berms as shown on attachment.

#### Revetment Estimate:

ITEM	QTY.	UNIT	UNIT COST	COST EST.
Revetment, Class E	850	TON	\$50	\$42,500
Engineering Fabric	750	SY	\$5	\$3,750
Traffic Control			5%	\$2,313
Mobilization			10%	\$4,856
Contingency			15%	\$8,013
Total Cost				\$61,432

NOTE: Easement and/or right of way costs are not included in the above cost estimate.

**Recommendation**

Install revetment as described above and on attached drawing.

**Special Considerations**

We are anticipating temporary easements may be needed during construction. Permanent easements and/or right of way acquisition will not be required for future maintenance needs.

The project is not located within a FEMA flood hazard area. Base flood elevations have not been established in this zone.

Since the basin is greater than 100 square miles in area a DNR Floodplain Development Permit will be required.

The river is not a protected stream.

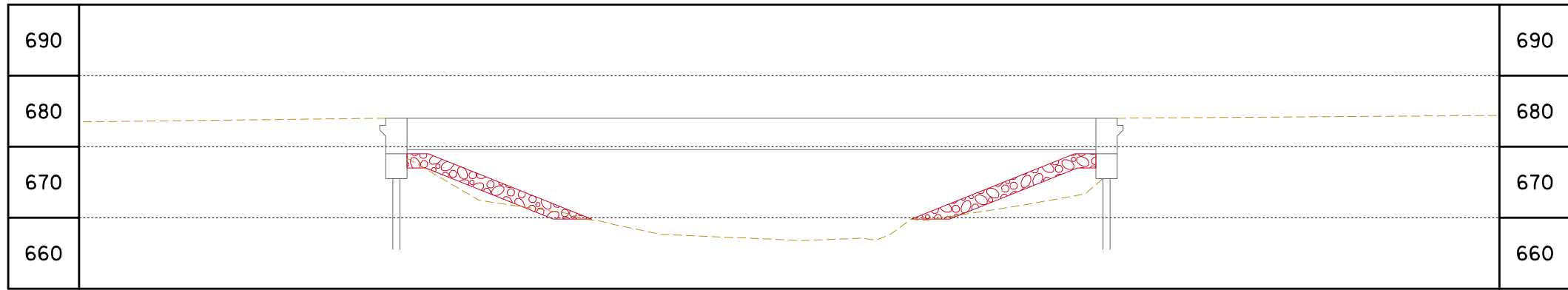
A ground survey will be required. A drawing that shows the limits of survey will be provided.

Extant of existing utilities unknown.

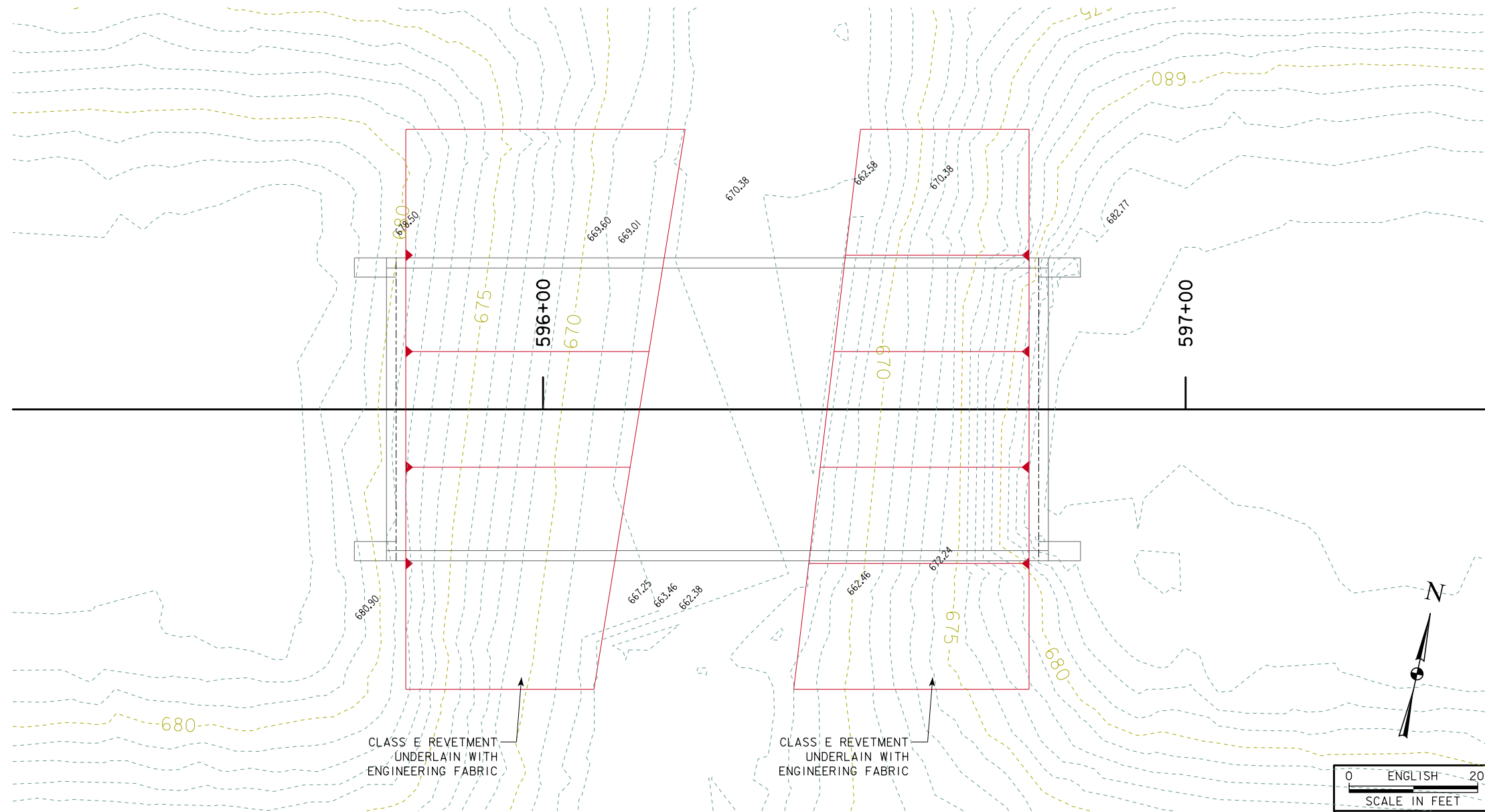
Railroads are not present in the project area and railroad expansion is not expected.

It is anticipated the work for the primary project would be awarded to one prime contractor. The Bridges and Structures Bureau will coordinate the plan preparation with assistance from the Design Bureau.

Trails are not present within the immediate project area and trail expansion is not expected in this area.



LONGITUDINAL SECTION ALONG  $\phi$  APPROACH ROADWAY



**LOCATION**

IA 92 OVER NORTH FORK LONG CREEK  
T-75N R-06W  
SECTION 21  
OREGON TOWNSHIP  
WASHINGTON COUNTY  
FHWA NO. 51841  
BRIDGE MAINT. NO. 9241.5S092  
LATITUDE 41.28833855°  
LONGITUDE -91.54708414°

PRELIMINARY - CONCEPT

DESIGN FOR REPAIRS TO A 0° SKEW  
**100'-0 X 44'-0 PRETENSIONED  
PRESTRESSED CONCRETE BEAM BRIDGE**  
100'-0 CENTER SPAN  
**SITUATION PLAN - CONCEPT**  
STATION 596+27.13 ( $\phi$  IA 92) MAY, 2020  
**WASHINGTON COUNTY**  
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION  
DESIGN SHEET NO.      OF   ? FILE NO.   31927   DESIGN NO.   ?

BRF-92-9(78)--38-92  
BRIDGE-REPLACEMENT

CADD FILE: H920299.S00  
10-26-99 WDT 299 WASHINGTON (CADWELL)

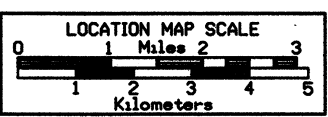
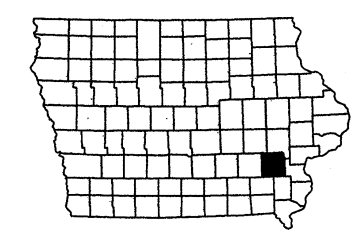
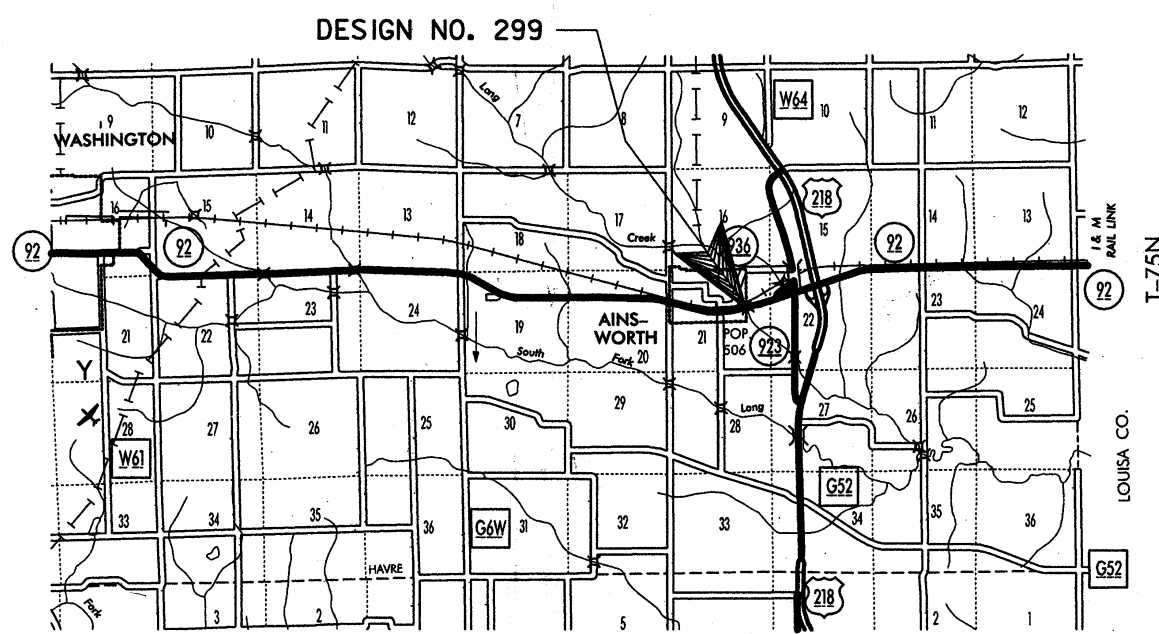
WASHINGTON COUNTY

WASHINGTON 299

CONVENTIONAL SIGNS	
	DIVIDED HIGHWAY
	PAVED ROAD
	BITUMINOUS ROAD
	GRAVEL ROAD
	EARTH ROAD
	INTERSTATE HIGHWAY
	UNITED STATES HIGHWAY
	STATE HIGHWAY
	COUNTY HIGHWAY
	RAILROAD
	PIPELINE
	AIRPORT
	HYDROLOGY
	BRIDGE
	STATE BOUNDARY
	COUNTY BOUNDARY
	CORPORATE LIMIT LINE
	TOWNSHIP LINE
	SECTION LINE

IOWA  
DEPARTMENT OF TRANSPORTATION  
Project Development Division  
PLANS OF PROPOSED IMPROVEMENTS ON THE  
**PRIMARY ROAD SYSTEM**  
WASHINGTON COUNTY  
**BRIDGE-REPLACEMENT**  
ON IOWA 92 OVER NORTH FORK LONG CREEK  
SCALES: As Noted

The standard specifications, series of 1997 of the Iowa Department of Transportation, shall apply to construction work on this project.  
(PLUS CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS)



I hereby certify that this project was constructed in accordance with the contract documents, the "as-built" plans were prepared under my supervision, and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

*David B. Taylor* 1-12-05  
Project Engineer Date

My license renewal date is December 31, 2004

12-11-2004 Year *Taylor* Contractor *Dean Thomas* Project Inspector

15807

30  
Bgc

\*150\*

INDEX OF SEALS	
SHEET NO.	NAME
1	WILLIAM D. TUCKER - STRUCTURAL DESIGN
16	KEITH ALLEN CADWELL - ROAD DESIGN

DESIGN DATA RURAL			
1991	AADT	3490	V.P.D.
2011	AADT	4290	V.P.D.
201X	DHV	X	V.P.H.
	TRUCKS	16	%
	ESALs per day	X	

STATE	FHWA REGION	F.Y.	SHEET	TOTAL
IOWA	7		1	16
PROJECT NUMBER				
BRF-92-9(78)--38-92				
R.O.W. PROJECT NUMBER				
PIN				
86-92-040-2				

INDEX OF SHEETS	
NO.	DESCRIPTION
1	TITLE SHEET
2	BRIDGE ESTIMATE SHEET
2-15	BRIDGE DESIGN NO. 299
16	ROAD ESTIMATE SHEET

VALUE ENGINEERING SAVES  
REFER TO THE GENERAL NOTES ON SHEET 2 OF 16

STANDARD ROAD PLANS				STANDARD BRIDGE PLANS		
NUMBER	DATE	IDENT	DATE	STANDARD	ISSUED	REVISED
RC-16A	10-27-98					
RC-16B	9-21-99					

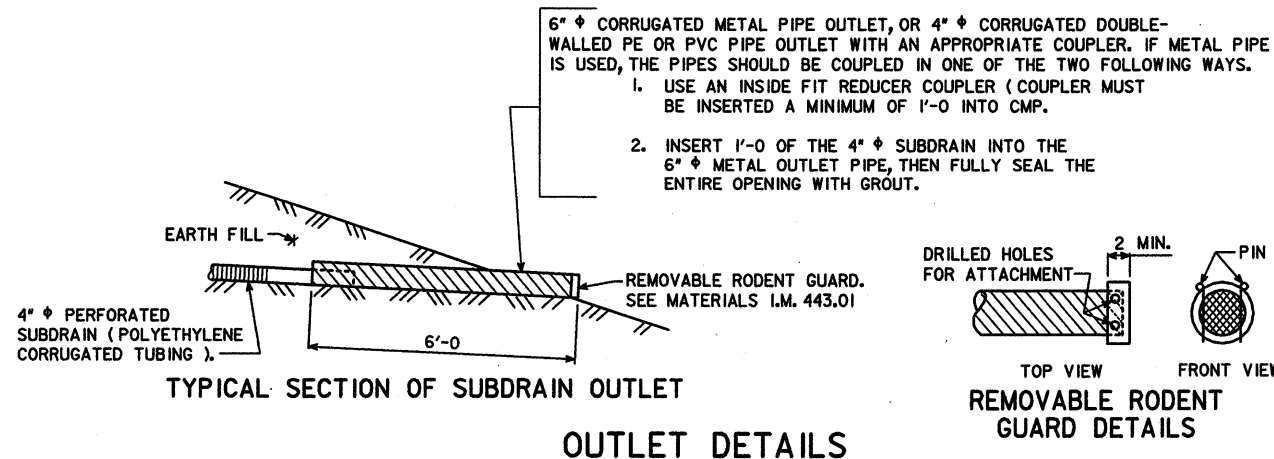
REVISIONS

STRUCTURAL DESIGN	
	I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.
	<i>William D. Tucker</i> 7/21/99 Signature Date William D. Tucker Printed or Typed Name My license renewal date is December 31, 1999 Pages or sheets covered by this seal: 1-15 OF 16

## TOTAL ESTIMATED BRIDGE QUANTITIES

ITEM NUMBER	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUANTITY
1	2401-6745650	REMOVAL OF EXISTING STRUCTURES	LS	1.000	1
2	2402-2720000	EXCAVATION, CLASS 20	CY	137.400	137
3	2403-0100010	STRUCTURAL CONCRETE ( BRIDGE )	CY	241.100	239
4	2404-7775000	REINFORCING STEEL	LB	6054.000	6054
5	2404-7775005	REINFORCING STEEL, EPOXY COATED	LB	41 371.000	41364
6	2407-0550000	BEAMS, PRETENSIONED PRESTRESSED CONCRETE, LXD100	EACH	7.000	7
7	2408-7800000	STRUCTURAL STEEL	LB	1992.000	1992
8	2414-6424110	CONCRETE BARRIER RAILING	LF	254.000	254
9	2501-5425042	PILES, DRIVE STEEL BEARING, HP10 x 42	LF	1710.000	1710
10	2501-5550042	PILES, FURNISH STEEL BEARING, HP10 x 42	LF	1710.000	1710
11	2533-4980005	MOBILIZATION	LS	1.000	1
<b>ENO</b>	<b>2599-999900</b>	<b>RETARDED</b>	<b>CY</b>		<b>194</b>
	<b>2599-999901</b>	<b>10% FOR PRIME CONTRACTOR</b>	<b>LS</b>		<b>1</b>

ITEM NUMBER	ESTIMATE REFERENCE INFORMATION
3	INCLUDES COST OF FURNISHING AND PLACING SUBDRAIN ( INCLUDING EXCAVATION ), GRANULAR BACKFILL AND POROUS BACKFILL AT ABUTMENTS.
6	INCLUDES COST OF BEARING MATERIALS REQUIRED.
7	INCLUDES COST OF 4 DRAINS AT 106 LBS. EACH.



6"  $\phi$  CORRUGATED METAL PIPE OUTLET, OR 4"  $\phi$  CORRUGATED DOUBLE-WALLED PE OR PVC PIPE OUTLET WITH AN APPROPRIATE COUPLER. IF METAL PIPE IS USED, THE PIPES SHOULD BE COUPLED IN ONE OF THE TWO FOLLOWING WAYS.

1. USE AN INSIDE FIT REDUCER COUPLER (COUPLER MUST BE INSERTED A MINIMUM OF 1'-0" INTO CMP.
2. INSERT 1'-0" OF THE 4"  $\phi$  SUBDRAIN INTO THE 6"  $\phi$  METAL OUTLET PIPE, THEN FULLY SEAL THE ENTIRE OPENING WITH GROUT.

**SPECIFICATIONS:**

DESIGN: AASHTO SERIES OF 1996.  
 CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION SPECIFICATION, SERIES OF 1997, PLUS CURRENT SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS.

**DESIGN STRESSES:**

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 1996.  
 REINFORCING STEEL IN ACCORDANCE WITH SECTION 8, GRADE 60.  
 CONCRETE IN ACCORDANCE WITH SECTION 8,  $f'_c = 3,500$  PSI.  
 PRESTRESSED CONCRETE BEAMS, SEE DESIGN SHEET 12.  
 STRUCTURAL STEEL IN ACCORDANCE WITH SECTION 10, ASTM A 36.

**TRAFFIC CONTROL PLAN**

NOTE: THE ROADWAY WILL BE CLOSED TO THRU TRAFFIC. ROAD CLOSURE WILL BE THE RESPONSIBILITY OF THE ROAD CONTRACTOR AS SHOWN ON THE ROAD PLANS.

NOTE: POLLUTION PREVENTION PLAN SHOWN ELSEWHERE IN THESE PLANS.

**GENERAL NOTES:**

THIS DESIGN IS FOR THE REPLACEMENT OF THE EXISTING 70' X 30' R.C. CONTINUOUS SLAB BRIDGE DES. NO. 3335 & 807. PLANS OF THE EXISTING STRUCTURE WILL BE MADE AVAILABLE TO THE CONTRACTOR. CONTACT THE OFFICE OF CONTRACTS - PROJECT DEVELOPMENT DIVISION - IOWA D.O.T. - AMES.

THE CITY AND UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE BRIDGE CONTRACTOR OF THE CONSTRUCTION STARTING DATE.

ALL DIMENSIONS AND DETAILS SHOWN IN THESE PLANS PERTINENT TO NEW CONSTRUCTION IN RELATION TO EXISTING PORTIONS OF THE STRUCTURE SHALL BE VERIFIED IN THE FIELD BY THE BRIDGE CONTRACTOR BEFORE STARTING CONSTRUCTION.

FAINT LINES ON PLANS INDICATE THE EXISTING STRUCTURE.

THE LUMP SUM BID FOR "REMOVAL OF EXISTING STRUCTURES" SHALL INCLUDE REMOVAL OF ABUTMENT SUBSTRUCTURE DOWN TO ELEVATION 669 AND REMOVAL OF PIER TO 1' BELOW STREAM BED ELEVATION.

REMOVALS SHALL BE IN ACCORDANCE WITH SECTION 2401 OF THE STANDARD SPECIFICATIONS.

THE APPROACH FILLS AS SHOWN ARE NOT A PART OF THIS CONTRACT, BUT ARE TO BE IN PLACE BEFORE ABUTMENT PILES ARE DRIVEN. THE BRIDGE CONTRACTOR IS TO LEVEL OFF AND SHAPE THE BERMS TO THE ELEVATIONS AND DIMENSIONS SHOWN. DRESSING OF SLOPES OUTSIDE THE BRIDGE AREA NOT DISTURBED BY THE BRIDGE CONTRACTOR SHALL BE PAID FOR AS EXTRA WORK.

CLASS 20 EXCAVATION QUANTITIES ARE BASED ON THE ASSUMPTION THAT THE CHANNEL EXCAVATION AND APPROACH FILLS ARE COMPLETED PRIOR TO STARTING CONSTRUCTION OF THE ABUTMENTS.

THIS BRIDGE IS DESIGNED FOR HS20-44 LOADING, PLUS 20 LBS. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.

ALL REINFORCING SUPPLIED FOR THIS STRUCTURE IS TO BE GRADE 60.

THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS BEHIND THE ABUTMENTS AS DETAILED. THE SUBDRAINS SHALL BE 4" PERFORATED SUBDRAIN (POLYETHYLENE CORRUGATED TUBING). THE SUBDRAIN OUTLET WILL CONSIST OF A 6' LENGTH OF PIPE WITH A REMOVABLE RODENT GUARD AS DETAILED IN THESE PLANS.

GUARDRAIL IS TO BE PLACED BY OTHERS.

THE BRIDGE CONTRACTOR IS ENCOURAGED TO TAKE FULL ADVANTAGE OF SPECIFICATION 1105.15 -- VALUE ENGINEERING INCENTIVE PROPOSAL. A PAMPHLET AND CONCEPTUAL PROPOSAL FORM WILL BE AVAILABLE AT THE PRECONSTRUCTION CONFERENCE.

DURING CONSTRUCTION OF THIS PROJECT THE BRIDGE CONTRACTOR WILL BE REQUIRED TO COORDINATE OPERATIONS WITH THOSE OF OTHER CONTRACTORS WORKING WITHIN THE SAME AREA. OTHER WORK IN PROGRESS DURING THE SAME PERIOD OF TIME WILL INCLUDE, BUT IS NOT LIMITED TO, CONSTRUCTION OF THE FOLLOWING PROJECTS:

STP-92-9 (56) -- 2C-92 GRADING

DESIGN FOR 0° SKEW

**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**

100'-0" CENTER SPAN

**BRIDGE QUANTITIES AND NOTES**

STA. 596+27.13 MARCH 1999

**WASHINGTON COUNTY**

IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION

DESIGN SHEET NO. 1 OF 14 FILE NO. 28216 DESIGN NO. 299

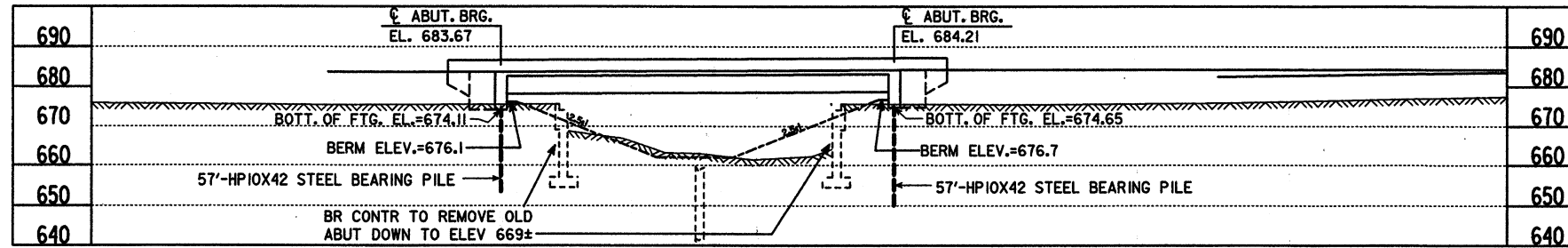
DESIGNED BY J. NELSON CHECKED BY G. HAIG  
 DETAILED BY C.F. RIECKEN CADD FILE

WASHINGTON COUNTY

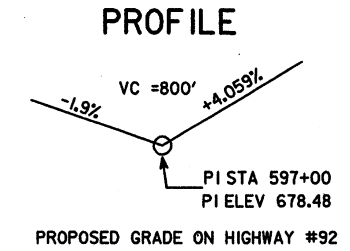
PROJECT NUMBER BRF-92-9(78)--38-92

STATE	FHWA REGION	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
IOWA	7		2	16

NOTE  
ROAD CLOSED TO TRAFFIC  
DURING CONSTRUCTION



LONGITUDINAL SECTION ALONG CL ROADWAY



No. 51A Sta. 596+91.20 23.10 Lt. NEW-IHC-BM NE Wing Wall of Bridge. - EL. 686.60

C.L. CURVE DATA (1992 O.R.)  
 $< 5^{\circ}21'29''$   
 $D = 0^{\circ}22'00''$   
 $T = 731.18$   
 $L = 1461.29$   
 $E = 17.10$   
 $R = 15,626.12$   
 $e = NC$

HYDRAULIC DATA

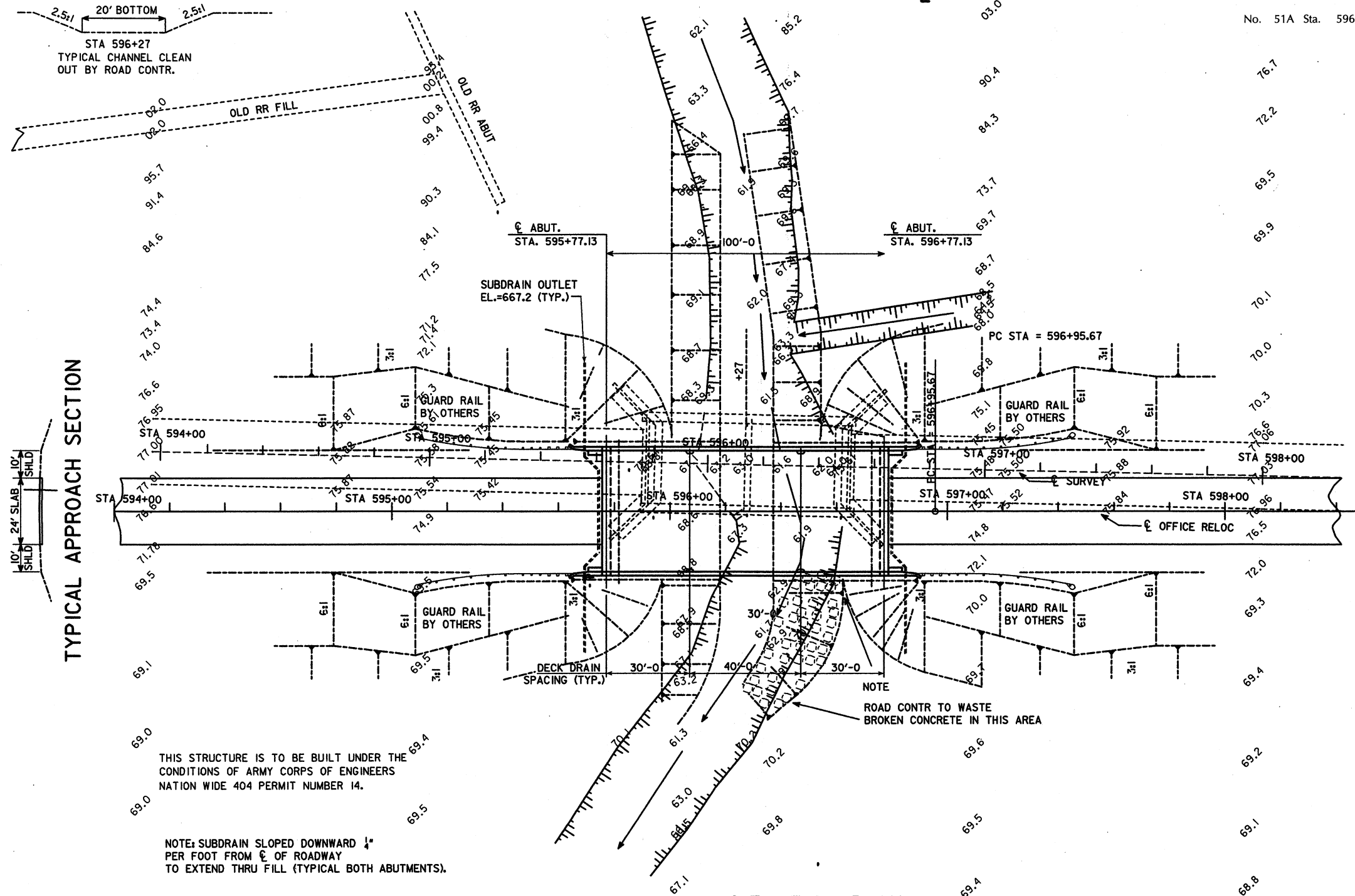
DRAINAGE AREA= 29.6 SQ MILES  
 STREAM SLOPE= 7.54 FT./MI.  
 $Q_{50} = 4330$  CFS  
 NATURAL STAGE= 674.8  
 $Q_{100} = 5125$  CFS  
 NATURAL STAGE= 675.4  
 $Q_{500} = 6750$  CFS  
 STAGE W/BACKWATER= 678.2

TRAFFIC ESTIMATE

1991: 3490 VPD  
 2011: 4290 VPD  
 16% TRUCKS

LOCATION

IA. 92 OVER N. FORK OF LONG CREEK  
 T 75 N R 6 W  
 SECTION 21  
 OREGON TOWNSHIP  
 WASHINGTON COUNTY  
 FHWA NO. 051841



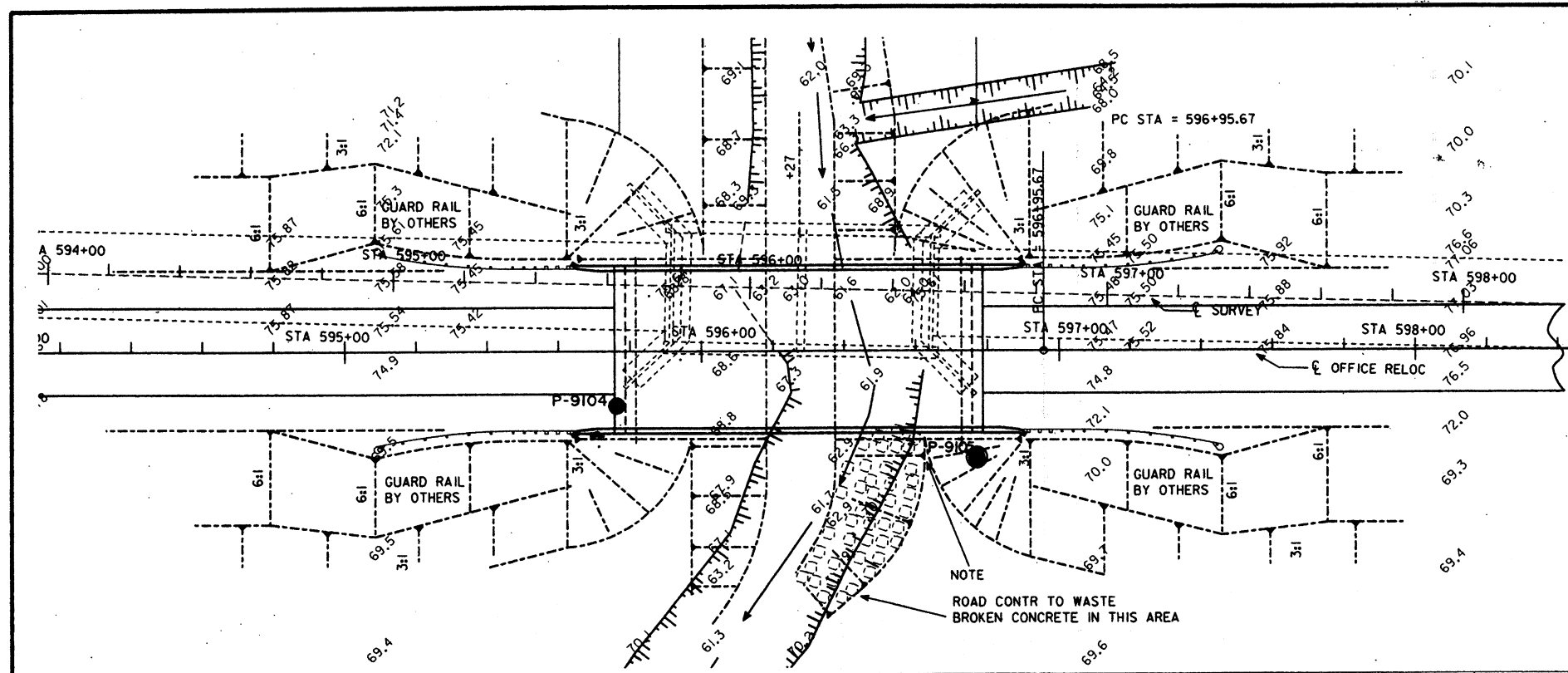
SITUATION PLAN

DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**SITUATION PLAN**  
 STATION: 596+27.13 MARCH 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 2 OF 14 FILE NO. 28216 DESIGN NO. 299

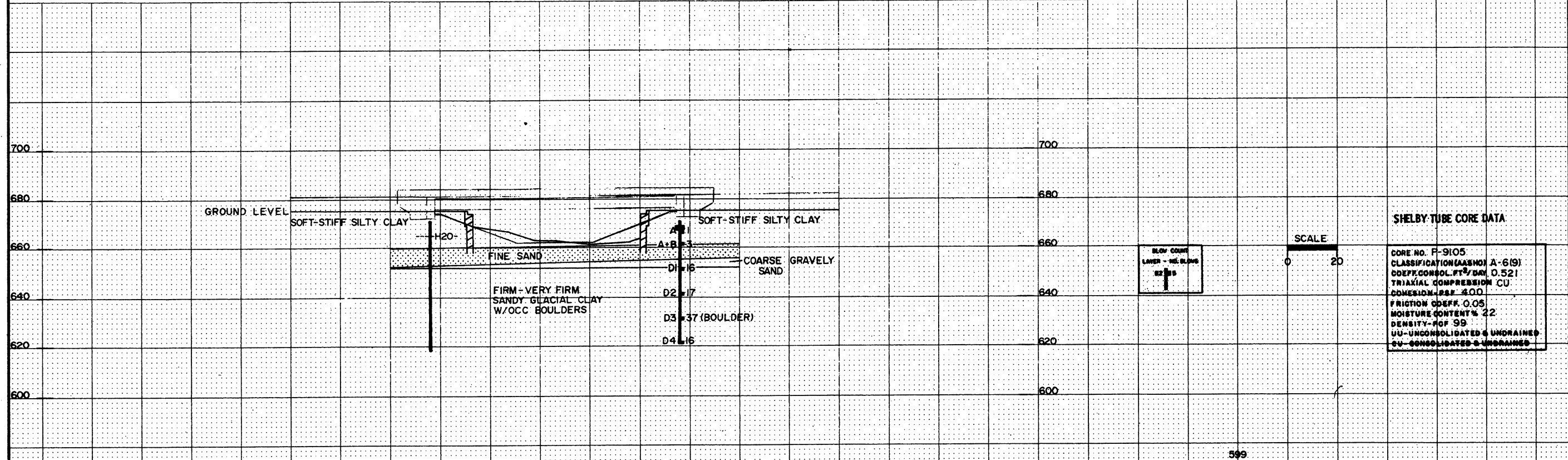
DESIGNED BY J. NELSON CHECKED BY G. HAIG  
 DETAILED BY C. F. RIECKEN CADD FILE

WASHINGTON COUNTY PROJECT NUMBER

STATE	FHWA REGION	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
IOWA	7		3	16

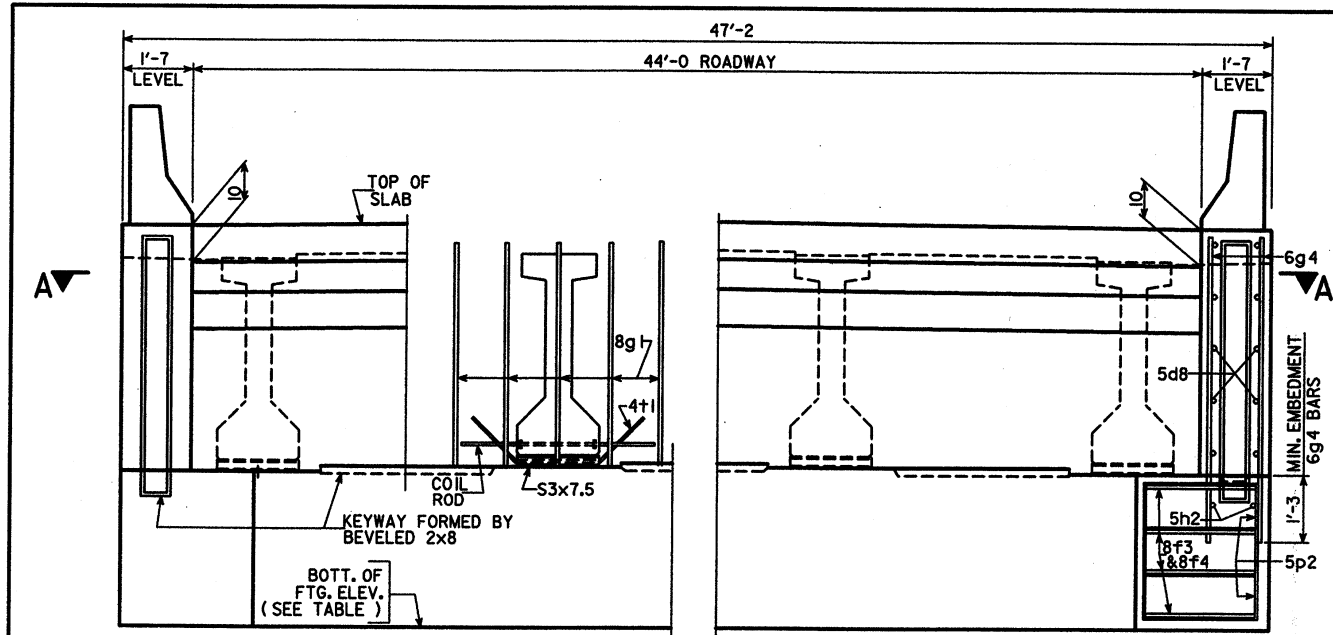


PRELIMINARY  
 DESIGN FOR 0° SKEW  
**100'-0 X 44 PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**  
 100'-0 CENTER SPAN  
**SITATION PLAN**  
 STATION: 596+27.13  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION--HIGHWAY DIVISION  
 DESIGN SHEET NO. 3 OF 14 FILE NO. 28216 DESIGN NO. 299

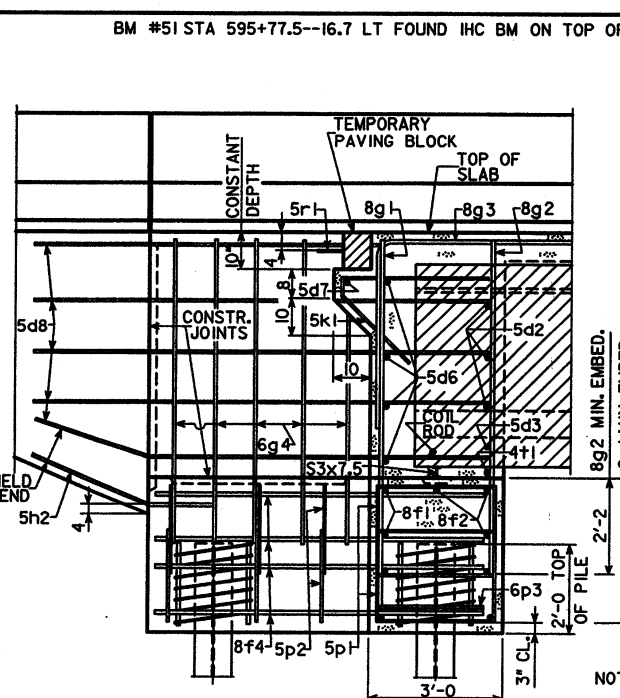


595	P-9104 RT 17	596	P-9105 RT 30	597	598	599
DESIGN NO. 299 FILE NO. 28216 DESIGN SHEET NO. 3 OF 14						DESCRIPTION: REPLACE BRIDGE OVER N FORK LONG CREEK SOIL SURVEYOR: CHESTER DESIGNER/CADD: TRAINUM SOILS BOOK NO.:
ROAD DESIGN • CADD • PRODUCED						DENS. CORE BLOW H2O-WATER SAMPLE SHELBY PLUGGED MOISTURE SELECT SOIL UNSUITABLE SHALE SANDY SOIL ROCK
STATE OF IOWA			FHWA REGION 7		FISCAL YEAR	WASHINGTON COUNTY PROJECT NUMBER
SHEET NUMBER 4 OF 16						

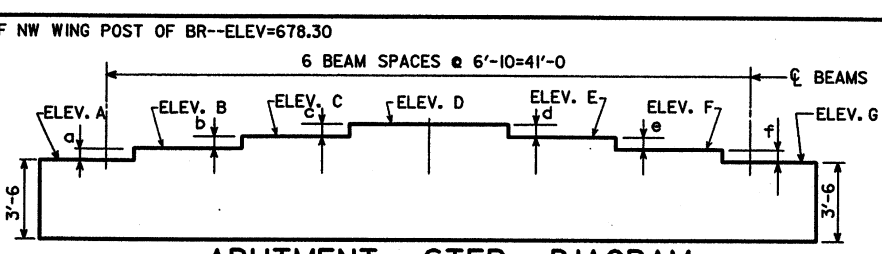
REVISIONS: "ABUTMENT NOTES" CHANGED ON "GRANULAR BACKFILL DETAIL". TAPE NO. 19, DATED 11-9-92.  
 REVISIONS: SPIRAL SPACERS CHANGED. TAPE NO. 17 DATED 10-28-91.  
 REVISIONS: 5-23-91 LAP LENGTHS AND EMBEDMENT LENGTHS CHANGED AND BEARING NOTE ADDED. DEVICE: ZGAIH200,0031ARCH.TAPE NO. 000016 DATE: 5-23-91  
 HE2085.S01 (HSTID02085.S01--WRJ) THIS SHEET REDRAWN, DEVICE: ZGAIH200,0031ARCH.TAPE NO. 000015 DATE: 9-8-88.



**PART REAR ELEVATION AT ABUTMENT**  
(WINGS NOT SHOWN)



**PART SECTION B - B**



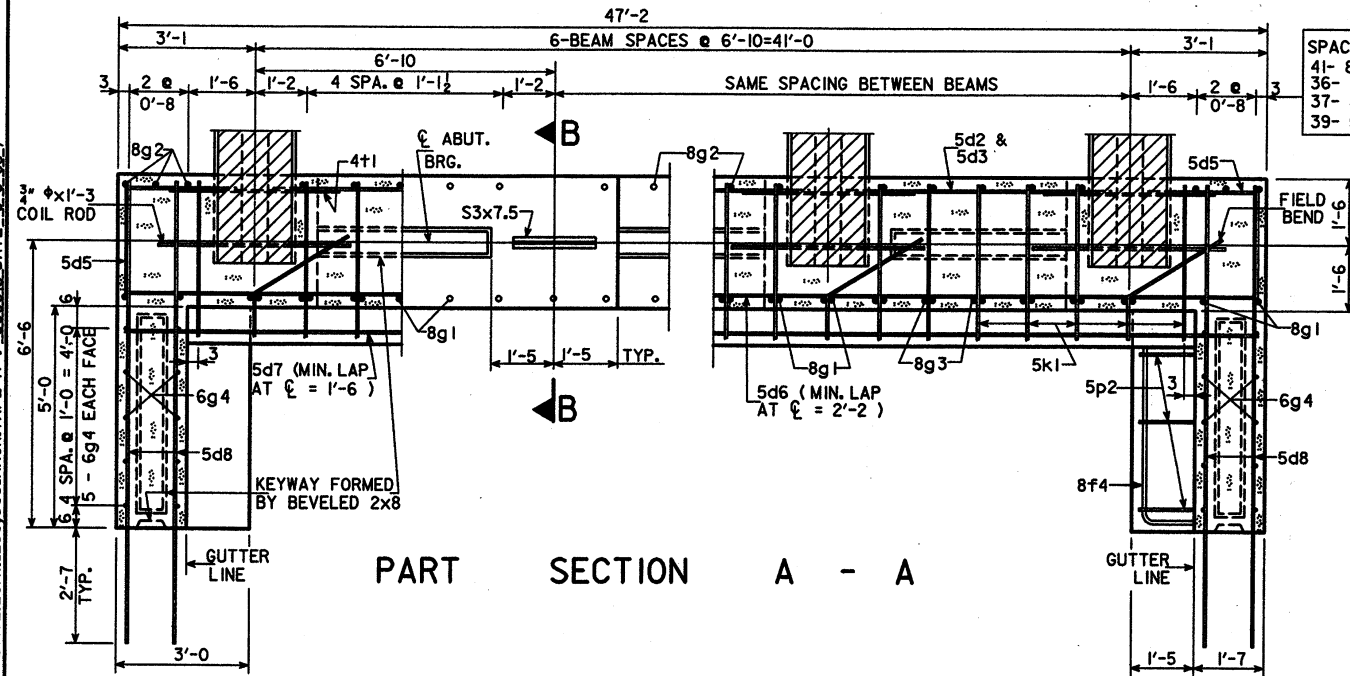
**ABUTMENT STEP DIAGRAM**  
(REAR ELEVATION)

STEP	WEST ABUTMENT	EAST ABUTMENT
a	1 1/8	1 1/8
b	1 1/8	1 1/8
c	1 1/8	1 1/8
d	1 1/8	1 1/8
e	1 1/8	1 1/8
f	1 1/8	1 1/8

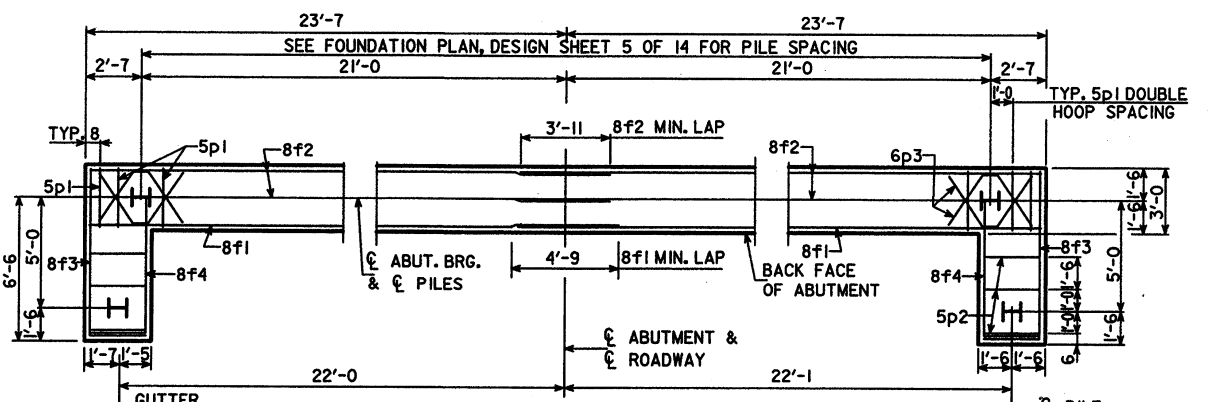
POINT	WEST ABUTMENT	EAST ABUTMENT
ELEV. A	677.61	678.15
ELEV. B	677.74	678.28
ELEV. C	677.89	678.42
ELEV. D	677.99	678.53
ELEV. E	677.89	678.42
ELEV. F	677.74	678.28
ELEV. G	677.61	678.15
BOTTOM FTG. ELEV.	674.11	674.65

NOTE:  
THE SPIRAL AT THE TOP OF EACH PILE TO BE 7 TURNS OF NO. 2 BAR, 21" DIAMETER, 3" PITCH WITH 2 - L 1/2 x 1/2 x 1/8 SPACERS PUNCHED TO HOLD SPIRAL.

SPACING FOR:  
41- 8g1 BACK FACE  
36- 8g2 FRONT FACE  
37- 8g3 BACK FACE  
39- 5k1 BACK FACE



**PART SECTION A - A**

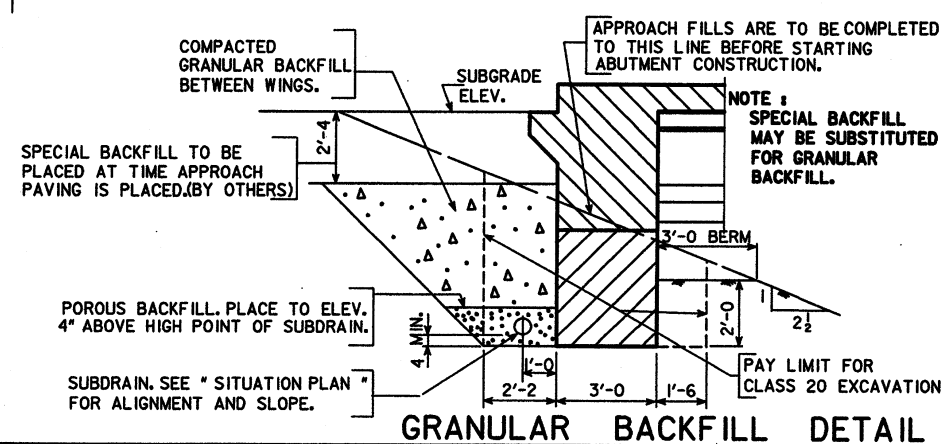


**ABUTMENT PILE PLAN**

NOTE:  
15- HPI0x42 STEEL BEARING PILING REQUIRED AT EACH ABUTMENT.

NOTE:  
SEE FOUNDATION PLAN DESIGN SHEET 5 OF 14 FOR 5p1 DOUBLE HOOP SPACING.

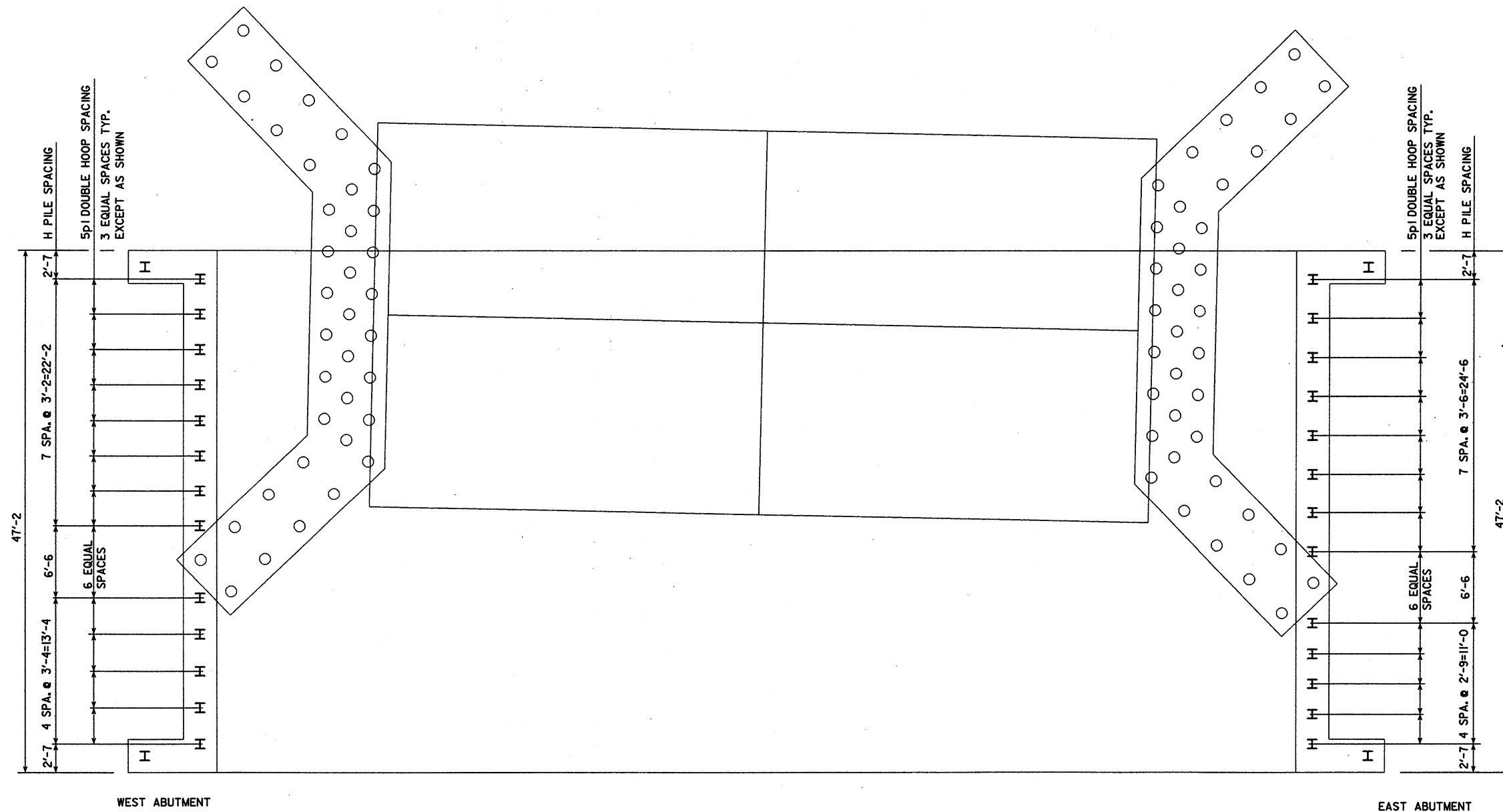
**ABUTMENT NOTES:**  
 ALL EXPOSED CORNERS OF 90° OR SHARPER ARE TO BE FILLETED WITH A 3" DRESSED AND BEVELED STRIP.  
 MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. REINFORCING STEEL IS TO BE SECURELY TIED IN PLACE BEFORE CONCRETE IS PLACED.  
 ALL BACKFILL BEHIND THE ABUTMENT BETWEEN WINGS IS TO BE AS SHOWN. THE REMAINDER OF THE ABUTMENT EXCAVATION IS TO BE BACKFILLED WITH SOIL.  
 THE DESIGN BEARING FOR THE ABUTMENT PILES IS 36 TONS.  
 THE COST OF FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), GRANULAR BACKFILL AND POROUS BACKFILL IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE".



**GRANULAR BACKFILL DETAIL**

DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**ABUTMENT DETAILS**  
 STATION: 596+27.13 MARCH, 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 4 OF 14 FILE NO. 28216 DESIGN NO. 299





**FOUNDATION PLAN**

NOTE: VERIFY LOCATION OF EXISTING ABUTMENT FOOTING BEFORE DRIVING HPI0 X 42 PILING.



DESIGN FOR 0° SKEW  
**100'-0 X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0 CENTER SPAN  
**FOUNDATION PLAN**  
 STATION: 596+27.13 MARCH 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 5 OF 14 FILE NO. 28216 DESIGN NO. 299

DESIGNED BY J. NELSON CHECKED BY G. HAIG  
 DETAILED BY C. F. RIECKEN CADD FILE

WASHINGTON COUNTY

PROJECT NUMBER

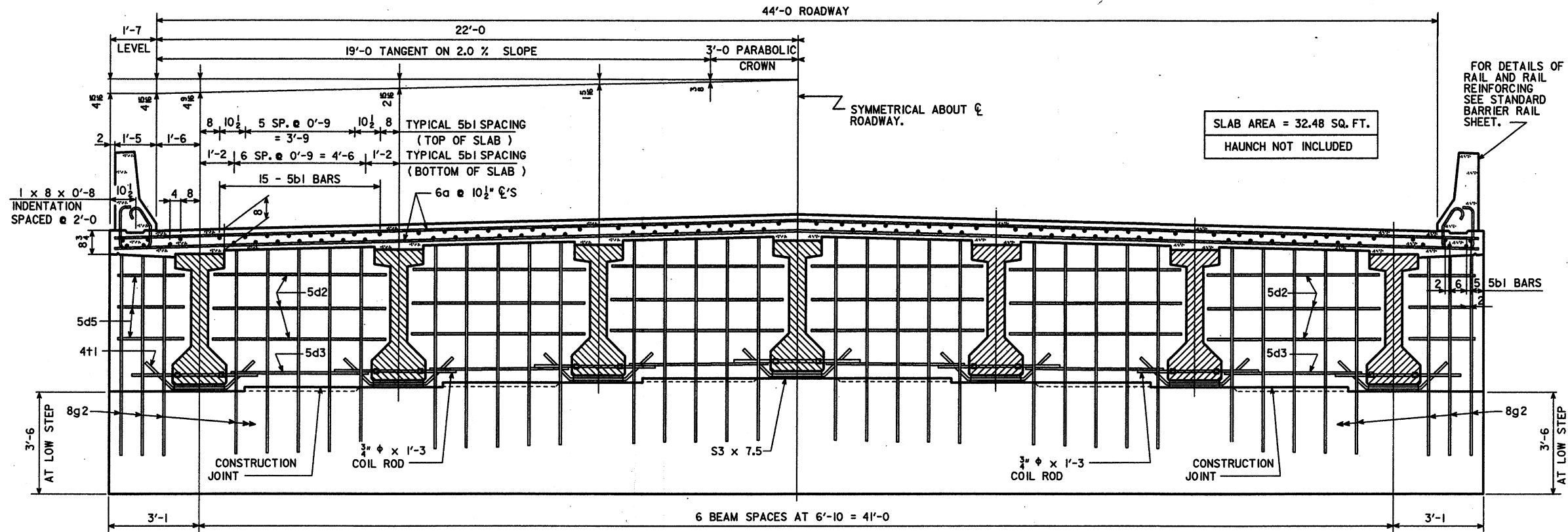
STATE	FHWY REGION	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
IOWA	7		6	16

20-JUL-1999 10:24

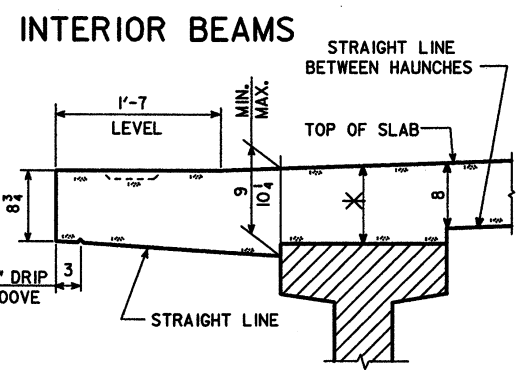
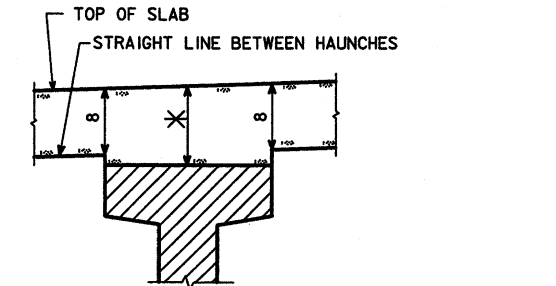
criecke

s:\projects\92092040a86\brf1nal\h920299.s05

REVISED: SPAN LENGTHS ADDED TO "B2" BAR TABLE, PERMISSIBLE LONGIT. CONSTR. JOINT DELETED, AND TANGENT SLOPE CHANGED TO 2%. TAPE NO. 16 DATED 5-23-91  
 HE4384.S01 (HSTD04384.S01--LEP; THIS SHEET REDRAWN, DEVICEZHAQ(200,004) ARCH. TAPE NO. 15, DATE 9-8-88)

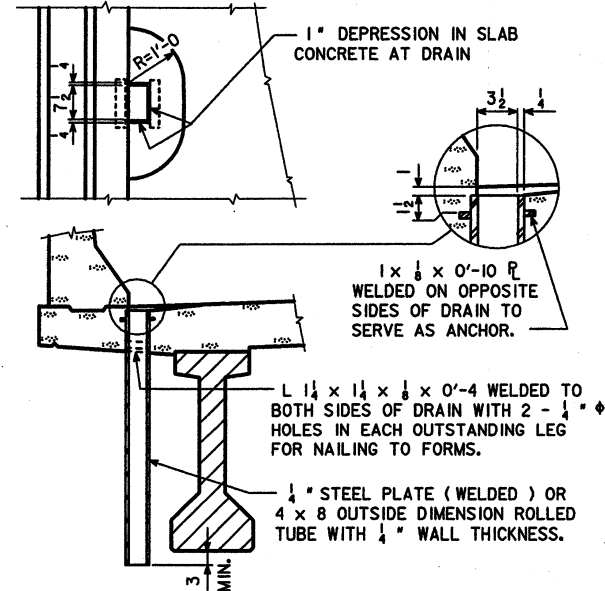


SECTION NEAR ABUTMENT



TYPICAL SLAB AND HAUNCH DETAIL

\* FOR SLAB THICKNESS OVER BEAMS SEE "SLAB THICKNESS DETAILS" ON DESIGN SHEET NO. 7.



NOTE: DRAINS ARE TO BE GALVANIZED. 4 DRAINS REQUIRED. SEE "SITUATION PLAN" ON DESIGN SHEET 2 FOR LOCATION. WEIGHT OF DRAINS IS INCLUDED IN THE QUANTITY FOR "STRUCTURAL STEEL". WEIGHT IS BASED ON ROLLED TUBE.

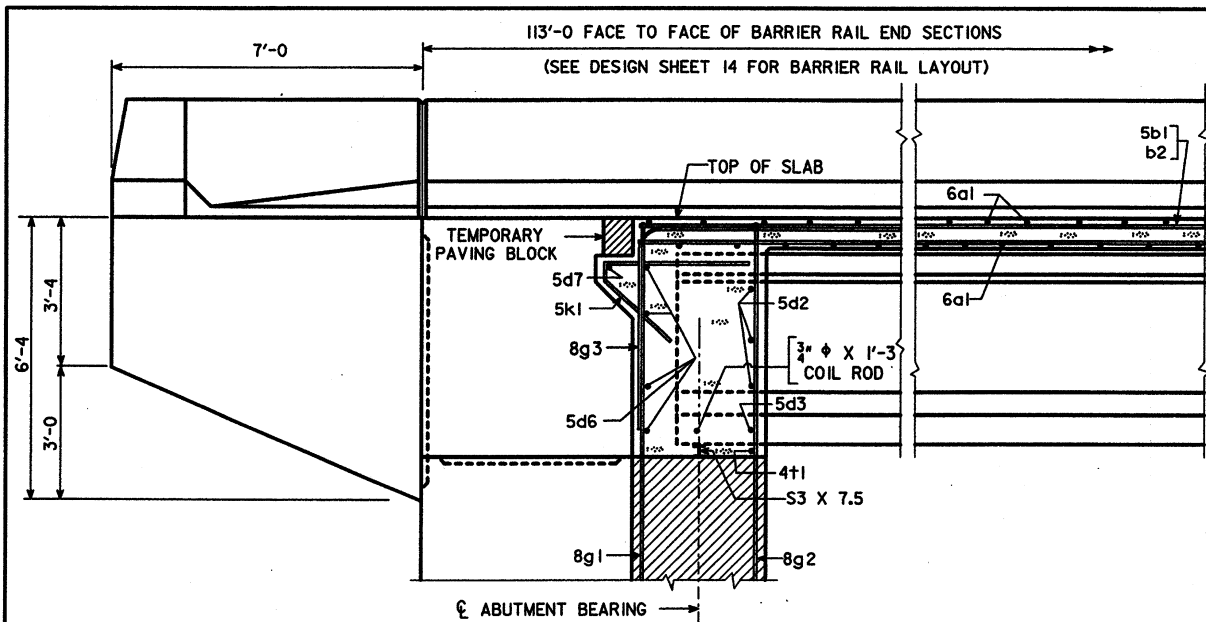
DATA FOR ONE DRAIN				
BEAM SIZE	A	B	C	D
WT. LBS.	4	8	9	106
LENGTH FT.	3'-7 3/4	4'-2 1/2	4'-8 1/2	5'-5 1/2

LENGTH OF S3 x 7.5 (ABUTMENT BEAM SEAT)	
BEAM BOTTOM FLANGE WIDTH	LENGTH OF S3 x 7.5
1'-6	1'-3 1/2
1'-8	1'-6 1/2
1'-10	1'-8 1/2

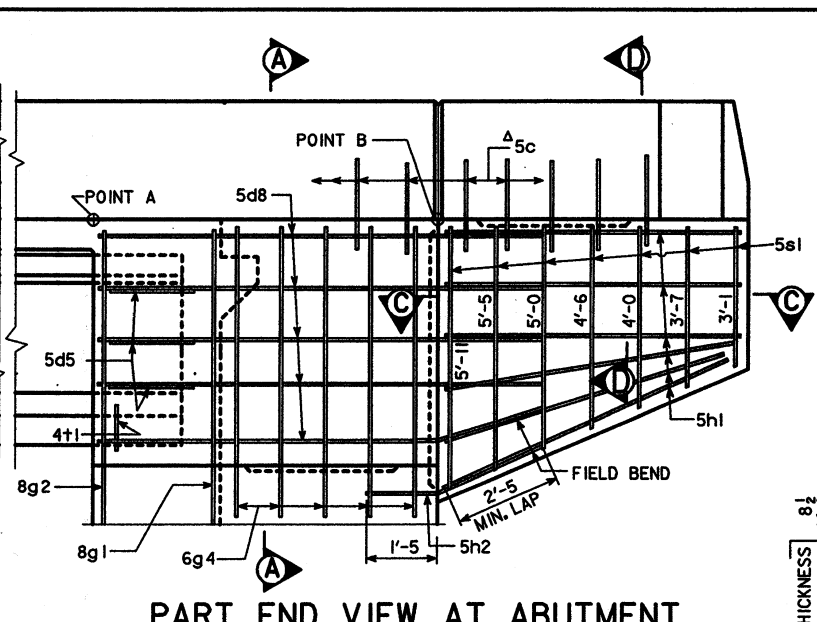
**SUPERSTRUCTURE NOTES:**

THE FLOOR SLAB AS SHOWN INCLUDES 1/2" INTEGRAL WEARING SURFACE.  
 THE PIER AND ABUTMENT DIAPHRAGM CONCRETE IS TO BE PLACED MONOLITHICALLY WITH THE FLOOR SLAB.  
 COST OF ALL PREFORMED EXPANSION JOINT FILLER MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE".  
 ALL BEAMS ARE TO BE SET VERTICAL.  
 FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE PRESTRESSED CONCRETE BEAMS.  
 CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.  
 ALL SLAB AND DIAPHRAGM REINFORCING IS TO BE WIRED IN PLACE AND ADEQUATELY SUPPORTED BEFORE CONCRETE IS PLACED.  
 TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 2 1/2" CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 1" CLEAR ABOVE BOTTOM OF SLAB. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-0" CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4'-0" APART.  
 COST OF BEARING MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "PRETENSIONED PRESTRESSED CONCRETE BEAMS".  
 TRANSVERSE SLAB REINFORCING MAY BE SPLICED WITH ONE LAP LOCATED AS FOLLOWS:  
 TOP BARS - LAP MIDWAY BETWEEN BEAMS (MIN. LAP = 2'-5).  
 BOTTOM BARS - LAP OVER BEAMS (MIN. LAP = 2'-5).  
 PAYMENT FOR REINFORCING BARS SHALL BE BASED ON NO SPLICES, AND NO ALLOWANCE SHALL BE MADE FOR THE ADDITIONAL LENGTH OF BAR REQUIRED FOR THE USE OF SPLICES.

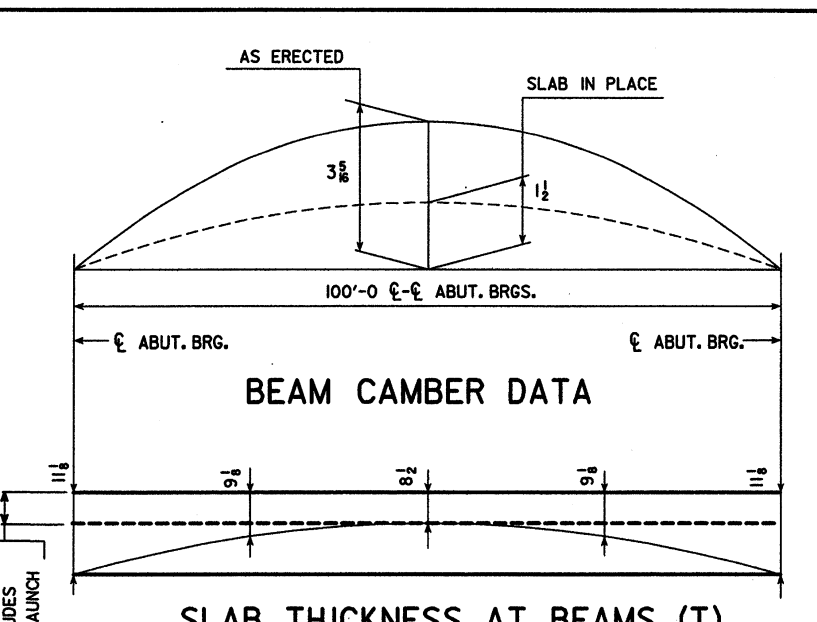
DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**SUPERSTRUCTURE DETAILS**  
 STATION: 596+27.13 MARCH 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 6 OF 14 FILE NO. 28216 DESIGN NO. 299



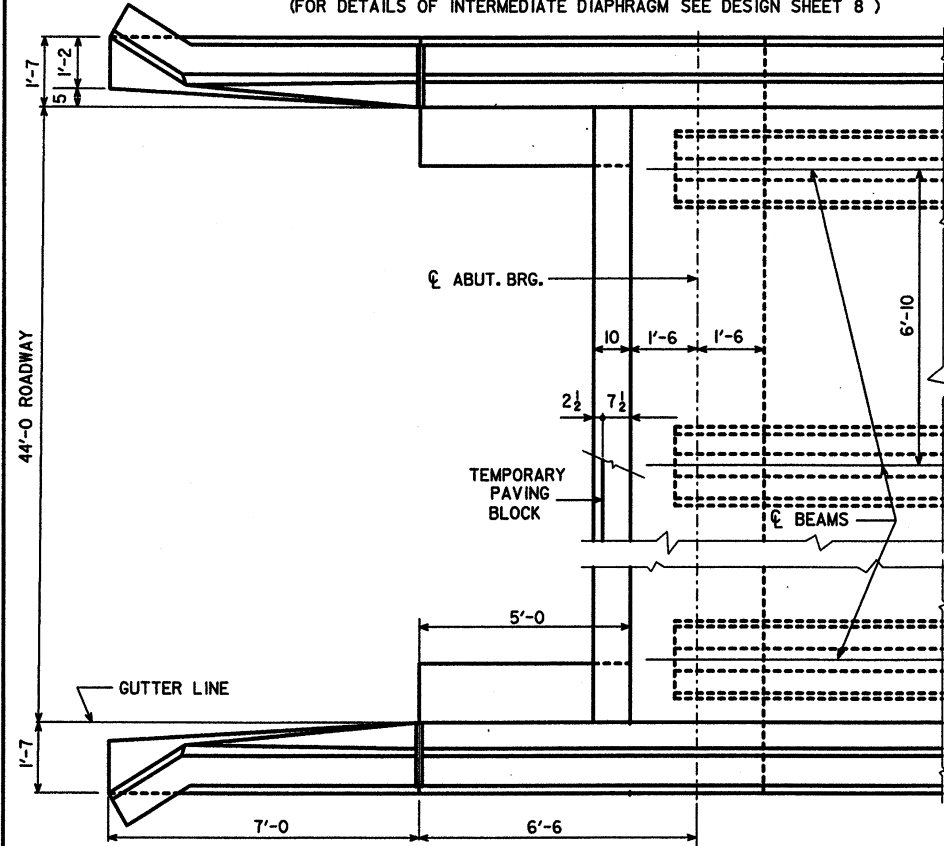
**PART LONGITUDINAL SECTION NEAR GUTTER**  
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE DESIGN SHEET 8)



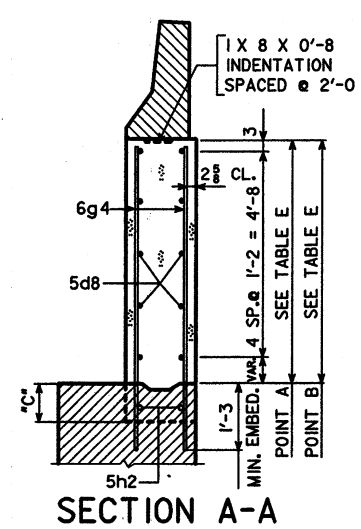
**PART END VIEW AT ABUTMENT**



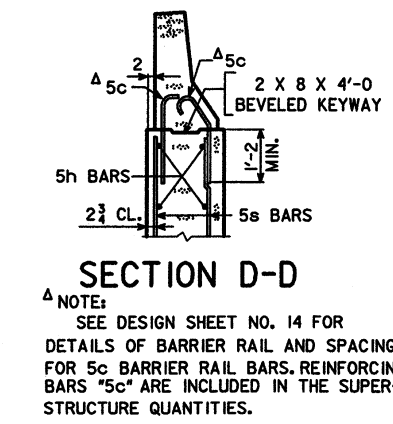
**SLAB THICKNESS AT BEAMS (T)**



**PART PLAN**

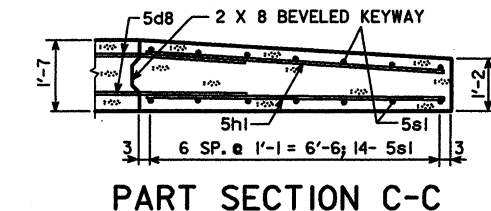


**SECTION A-A**

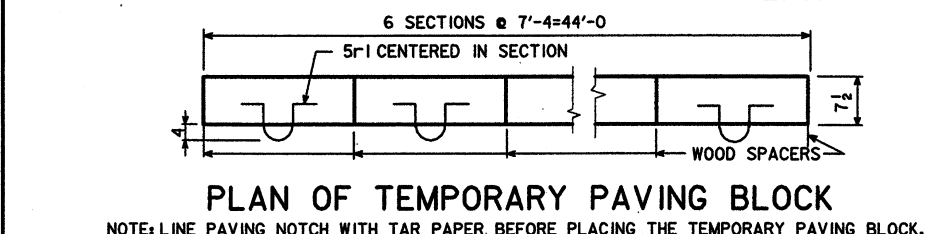


**SECTION D-D**

NOTE: SEE DESIGN SHEET NO. 14 FOR DETAILS OF BARRIER RAIL AND SPACING FOR 5c BARRIER RAIL BARS. REINFORCING BARS "5c" ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.



**PART SECTION C-C**



**PLAN OF TEMPORARY PAVING BLOCK**

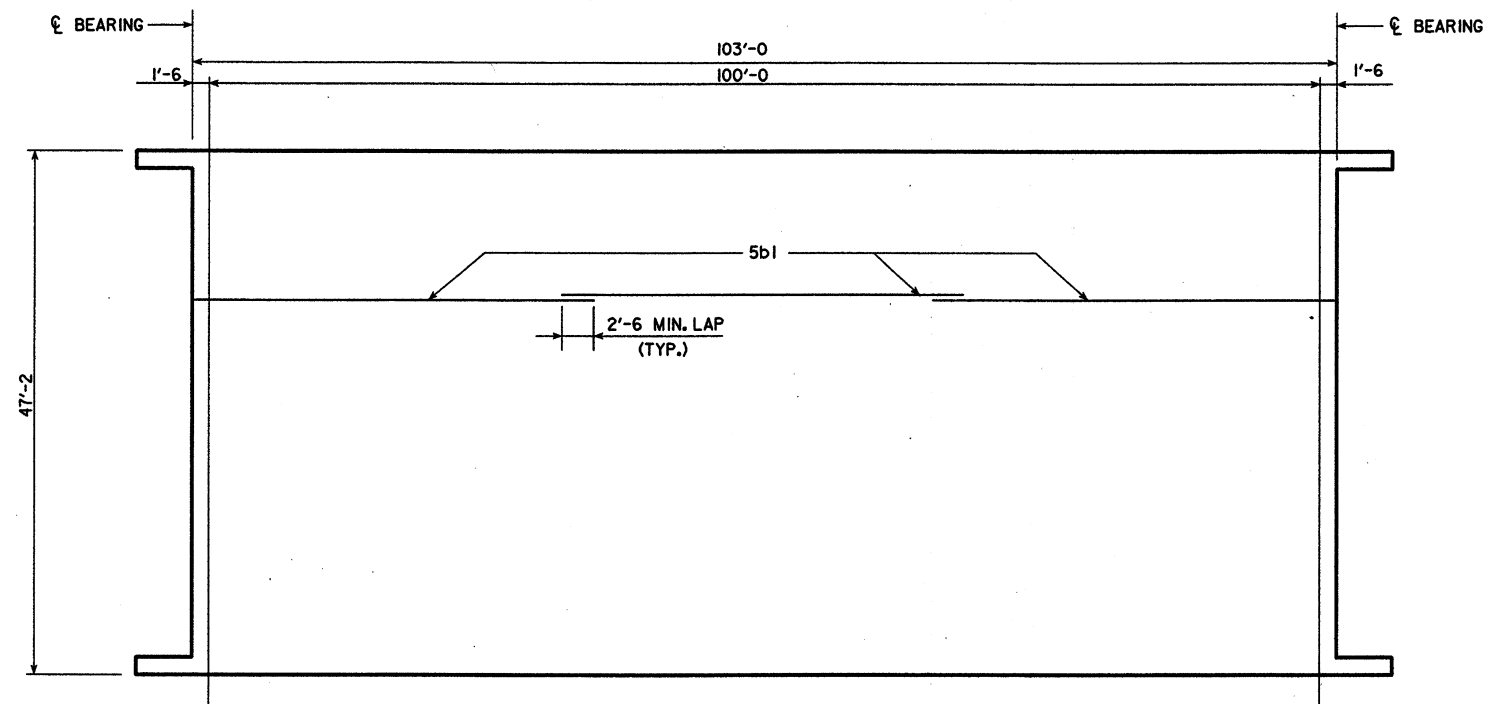
**TABLE E**

LOCATION	"C"	POINT A	POINT B
S.W. CORNER	8 5/8	5'-7 3/8	5'-7 1/8
N.W. CORNER	8 5/8	5'-7 3/8	5'-7 1/8
S.E. CORNER	6 1/8	5'-7 3/8	5'-8 1/2
N.E. CORNER	6 1/8	5'-7 3/8	5'-8 1/2

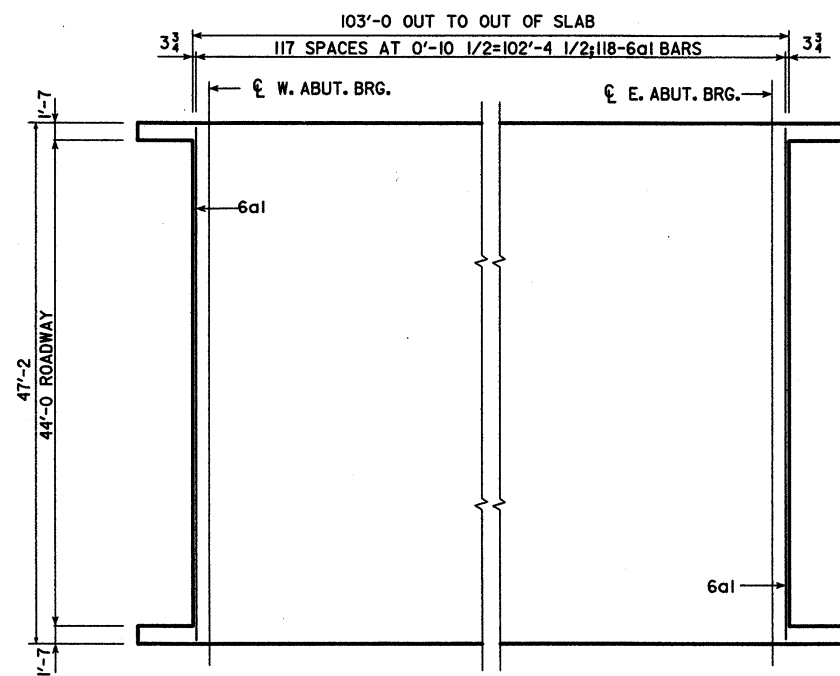
DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**SUPERSTRUCTURE DETAILS**  
 STATION: 596+27.13 MARCH, 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 7 OF 14 FILE NO. 28216 DESIGN NO. 299

REVISED: 5c & 6g4 BAR EMBEDMENTS AND COIL ROD DETAILS CHANGED. TAPE NO. 16, DATED 5-5-91  
 REVISED: 6" MAX. CL. ADDED FOR 5b1 BARS. ARCH. TAPE NO. 14, DATED 5-23-93  
 REVISION: 501 (HSTDOBSOT.SOI) - LEFT THIS SHEET REDRAWN, DEVICEZHAOR200(004) ARCH. TAPE NO. 15, DATE 9-9-98

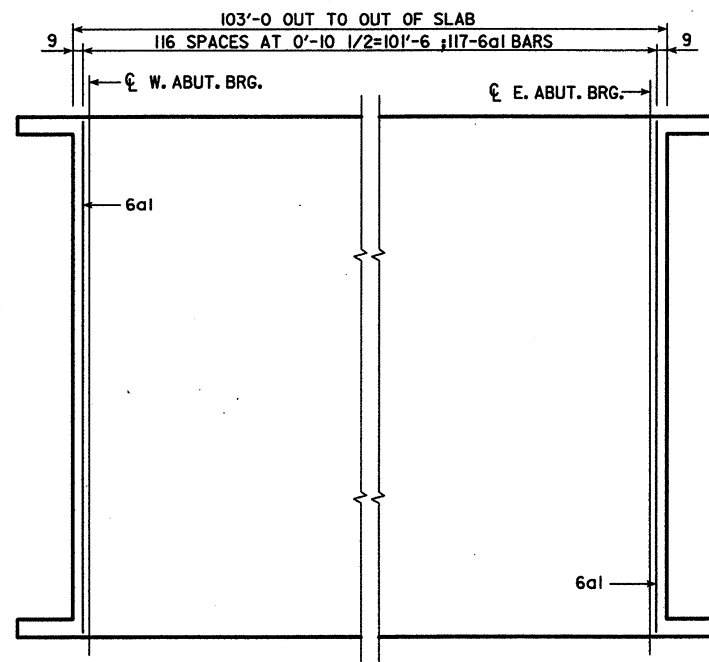




SLAB REINFORCING LAYOUT



TOP SLAB TRANSVERSE REINFORCING



BOTTOM SLAB TRANSVERSE REINFORCING

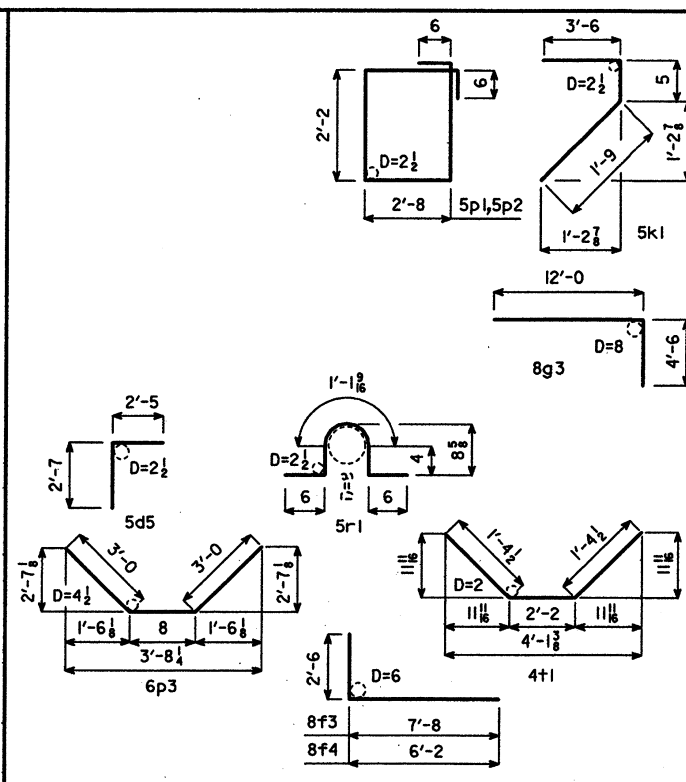
DESIGN FOR 0° SKEW  
**100'-0 X 44' PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**  
 100'-0 CENTER SPAN  
**SUPERSTRUCTURE DETAILS**  
 STATION: 596+27.13 MARCH 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 9 OF 14 FILE NO. 28216 DESIGN NO. 299

DESIGNED BY J. NELSON CHECKED BY G. HAIG  
 DETAILED BY C.F. RIECKEN CADD FILE

WASHINGTON COUNTY PROJECT NUMBER

STATE	FHWA REGION	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
IOWA	7		10	16

REVISIONS: 01-21-95 4" LIN. P.T. COLUMN DELETED FROM REINFORCING BAR LIST. REVISED 6-21-95 4" LIN. P.T. COLUMN DELETED FROM REINFORCING BAR LIST. REVISIONS: 01-21-95 4" LIN. P.T. COLUMN DELETED FROM REINFORCING BAR LIST. REVISIONS: 01-21-95 4" LIN. P.T. COLUMN DELETED FROM REINFORCING BAR LIST. REVISIONS: 01-21-95 4" LIN. P.T. COLUMN DELETED FROM REINFORCING BAR LIST.



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D= PIN DIAMETER.

CONC. PLACEMENT QUANTITIES		ONE SUPER. & TWO ABUTS.
LOCATION	QUANTITY	
SLAB & ABUT. DIAPH.	186.0	
ABUTMENT FOOTINGS	46.6	
ABUTMENT WINGS	4 AT 1.7	6.8
PAVING BLOCKS	2 AT 0.85	1.7
TOTAL (CU. YDS.)		241.1

ESTIMATED QUANTITIES				ONE SUPER. & TWO ABUTS.
ITEM	UNIT	QUANTITY		
STRUCTURAL CONCRETE, CLASS "C"	CU. YD.	241.1		
STRUCTURAL STEEL	LBS.	1,992		
REINFORCING STEEL EPOXY COATED	LBS.	41,371		
REINFORCING STEEL	LBS.	6,054		
PRESTRESSED PRESTRESSED CONCRETE BEAMS	LXD100	NO.	7	
CLASS 20 EXCAVATION	CU. YD.	137.4		
HPIOX42 STEEL	FURNISH	30x57'	L.F.	1710
BEARING PILING	DRIVE	30x57'	L.F.	1710

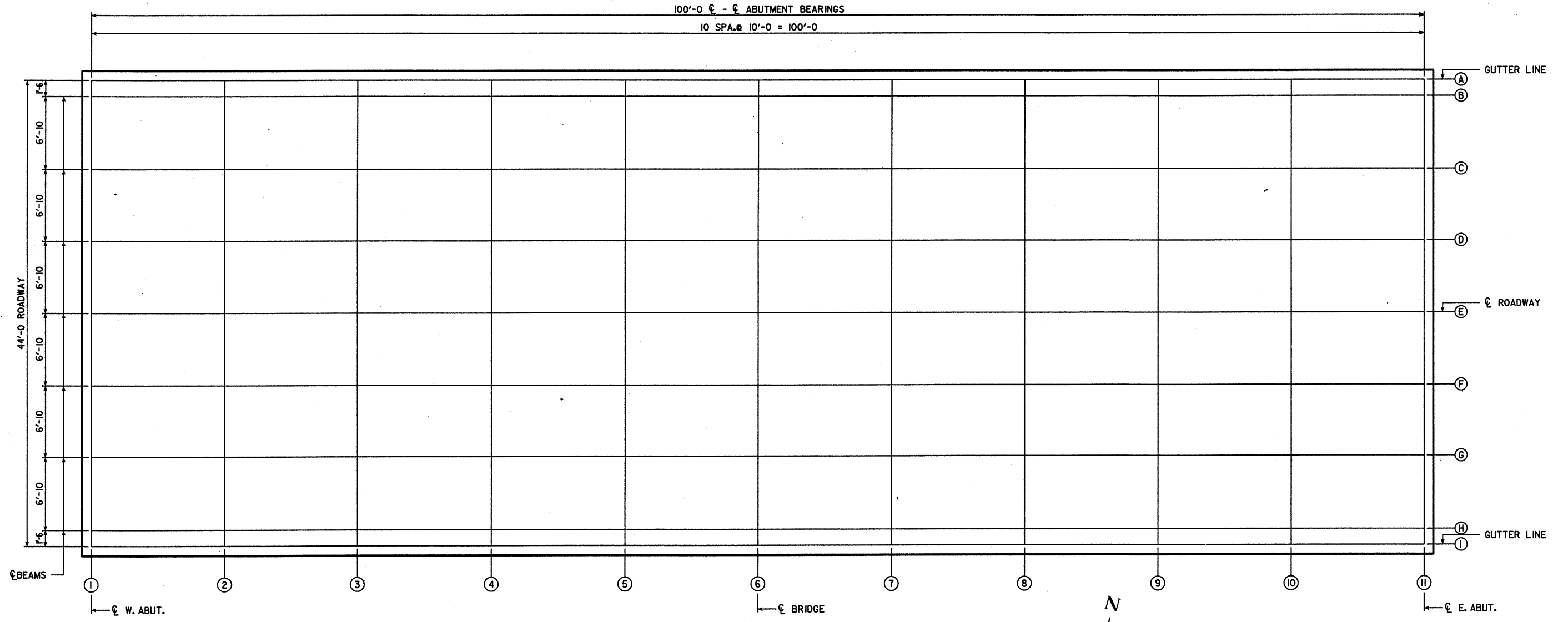
REINFORCING BAR LIST-ONE SUPER. & TWO ABUTS.					
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
6a1	SLAB TRANSV. TOP & BOTT.	—	235	46'-10"	16,531
5b1	SLAB LONGIT. TOP & BOTT.	—	306	36'-0"	11,490
5d6	ABUT. DIAPH. LONGIT. B.F.	—	16	24'-6"	409
5d7	PAVING NOTCH LONGIT.	—	4	24'-2"	101
8f1	ABUT. FOOTING LONGIT. B.F.	—	16	25'-10"	1104
8g1	ABUT. VERT. B.F.	—	82	8'-8"	1897
8g3	ABUT. DIAPH. VERT. B.F.	—	74	16'-6"	3260
5k1	PAVING NOTCH TRANSV.	—	78	5'-8"	461
5p1	ABUT. HOOPS	□	124	10'-8"	1380
6p3	ABUT. BOTT. AT PILES	—	52	6'-8"	521
BARRIER RAIL - SEE DESIGN SHT. NO. 14					4217
REINFORCING STEEL EPOXY COATED - TOTAL (LBS.)					41,371
5d2	ABUT. DIAPH. LONGIT.	—	36	5'-11"	222
5d3	ABUT. DIAPH. LONGIT.	—	12	4'-8"	58
5d5	ABUT. DIAPH. ENDS	—	12	5'-0"	63
5d8	ABUT. DIAPH. WING EXT. LONGIT.	—	40	10'-6"	438
8f2	ABUT. FOOTING LONGIT. F.F.	—	20	25'-5"	1357
8f3	ABUT. EXTENSION LONGIT.	—	16	10'-2"	434
8f4	ABUT. EXTENSION LONGIT.	—	16	8'-8"	370
8g2	ABUT. VERT. F.F.	—	72	7'-7"	1458
6g4	ABUT. DIAPH. WING EXT. VERT.	—	40	6'-9"	406
5h1	ABUT. WING HORIZ.	—	48	6'-8"	334
5h2	ABUT. TO WING ANCHOR	—	8	4'-0"	33
5p2	ABUT. EXTENSION HOOPS	□	24	10'-8"	267
5r1	PAVING BLOCK LIFTING LOOPS	—	12	2'-10"	35
5s1	WING VERT.	—	56	VARIES	263
4+1	UNDER BEAMS AT ABUTMENTS	—	14	4'-11"	46
#2	PILE SPIRAL	—	30	38'-6"	193
	SPIRAL SPACERS, L <sub>1</sub> x L <sub>2</sub> x L <sub>3</sub> x 0.70	—	60	1'-10"	77
REINFORCING STEEL - TOTAL (LBS.)					6054

DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**SUPERSTRUCTURE DETAILS**  
 STATION: 596+27.13 MARCH, 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 10 OF 14 FILE NO. 28216 DESIGN NO. 299

This sheet is drawn by the Bridge Automated Drafting Software (BADS) - OFFICE OF BRIDGE DESIGN

BM #51 STA 595+77.5--16.7 LT FOUND IHC BM ON TOP OF NW WING POST OF BR--ELEV=678.30

TOP OF SLAB ELEVATIONS											
NOTE: ADD 600 TO THE ELEVATIONS SHOWN											
	1	2	3	4	5	6	7	8	9	10	11
A	83.26	83.28	83.31	83.35	83.39	83.44	83.49	83.56	83.63	83.71	83.80
B	83.29	83.31	83.34	83.38	83.42	83.47	83.53	83.59	83.66	83.74	83.83
C	83.43	83.45	83.48	83.51	83.56	83.61	83.66	83.73	83.80	83.88	83.97
D	83.57	83.59	83.61	83.65	83.69	83.74	83.80	83.86	83.94	84.02	84.10
E	83.67	83.70	83.72	83.76	83.80	83.85	83.91	83.97	84.04	84.12	84.21
F	83.57	83.59	83.61	83.65	83.69	83.74	83.80	83.86	83.94	84.02	84.10
G	83.43	83.45	83.48	83.51	83.56	83.61	83.66	83.73	83.80	83.88	83.97
H	83.29	83.31	83.34	83.38	83.42	83.47	83.53	83.59	83.66	83.74	83.83
I	83.26	83.28	83.31	83.34	83.39	83.44	83.50	83.56	83.63	83.71	83.80



TOP OF SLAB ELEVATIONS



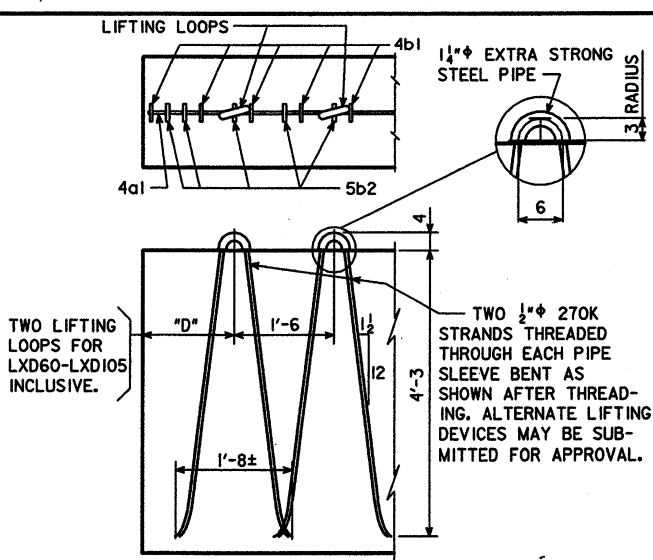
DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED  
 CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**TOP OF SLAB ELEVATIONS**  
 STATION: 596+27.13 MARCH 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 11 OF 14 FILE NO. 28216 DESIGN NO. 299

DESIGNED BY J. NELSON CHECKED BY G. HAIG  
 DETAILED BY C. F. RIECKEN CADD FILE

WASHINGTON COUNTY PROJECT NUMBER

STATE	FHWA REGION	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
IOWA	7		12	16

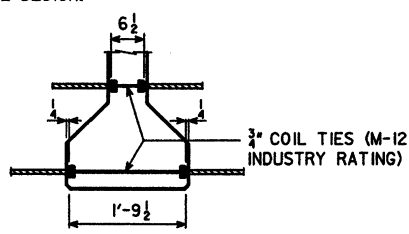
REVISIONS: 12-14-94, SPECIFIC ARTICLES IN CHANGES, DEVICES CHANGED, ARCH. TAPE NO. 20 DATE 12-14-94  
 REVISED: 4-21-95, SHOP BEND OF STRAND PROJECTION AT BEAM ENDS CHANGED.  
 REVISED: 10-97, NOTE ADDED FOR STRAND SUBSTITUTION.  
 REVISED: 11-98, BENDING OF STRAND PROJECTION AND NOTE CHANGED.  
 HLXD04630.S01 (HLXD04630.S01) ARCH. TAPE 9 DATE 05-24-90  
 HLXD04630.S01 (HLXD04630.S01) ARCH. TAPE 9 DATE 05-24-90



**LIFTING LOOP DETAIL**

"D" = 3'-9" FOR LXD100

NUMBER AND EXACT LOCATION OF COIL TIES TO BE AS DETAILED ON SPECIFIC BRIDGE DESIGN.

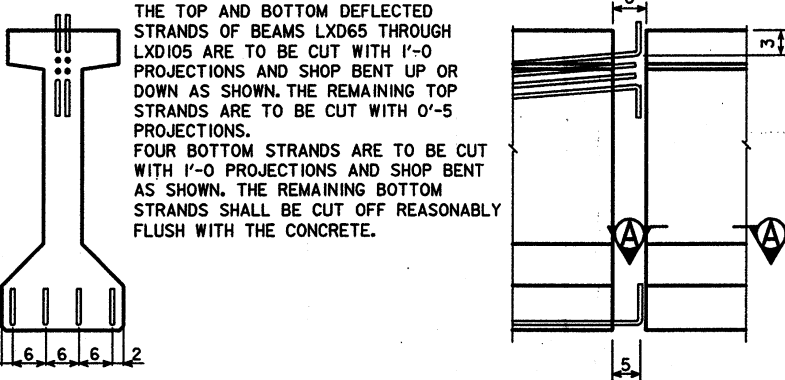


**COIL TIE DETAIL SPECIFICATIONS:**

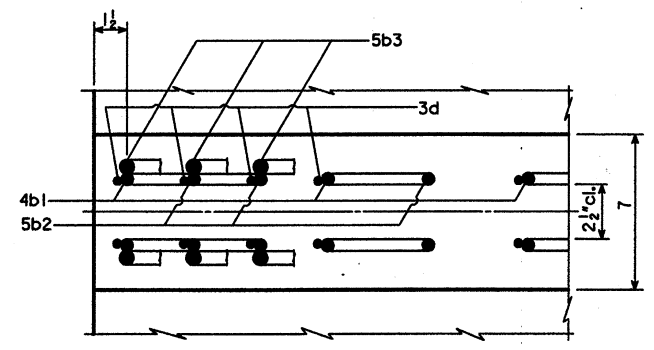
CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.  
 DESIGN: A.A.S.H.T.O., SERIES OF 1989, WITH MINOR MODIFICATIONS.

A = SIZE  
B = NO.

REINFORCING BAR LIST		LXD100	
BEAM	SPAN	NO.	LENGTH
4a1	100'-0"	2	22'-0"
a2	100'-0"	6/4	35'-4"
a3	100'-0"	8/2	36'-0"
4b1	100'-0"	53	10'-4"
5b2	100'-0"	12	8'-8"
5b3	100'-0"	20	4'-4"
4b4	100'-0"	28	10'-7"
3c	100'-0"	81	2'-1"
3d	100'-0"	81	5'-7"
3e	100'-0"	16	2'-3"



**STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS**



**SECTION A-A SHOWING PLACEMENT OF STIRRUPS NEAR END OF BEAM**

**DESIGN STRESSES:**

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 1989:  
 REINFORCING STEEL IN ACCORDANCE WITH SECTION 8, GRADE 60.  
 CONCRETE IN ACCORDANCE WITH SECTION 9,  $f'_c = 5000$  psi. (EXCEPT AS NOTED).  
 PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 9,  $f'_s = 270,000$  psi.

**NOTE:**  
 FOR MODIFIED STIRRUP EXTENSIONS, SEE "BENT BAR DETAILS" AND BEAM DETAIL SHEET FOR DIMENSIONS AND LOCATIONS.

**LXD BEAM DATA**

BEAM	SPAN LENGTH @ BEARING	OVERALL BEAM LENGTH (L)	STRAND SIZE	NO. OF STRANDS		TOTAL INITIAL PRESTRESS KIPS	HOLD DOWN FORCE-KIPS	CAMBER (in.)		DEFLECTION (in.) $\Delta_0$				PERMISSIBLE SPACING		WEIGHT (TONS)	CONCRETE (C.Y.)	REINFORCING STEEL (lbs.)
				STRAIGHT	DEFLECTED			AT RELEASE	AFTER LOSSES	IMMEDIATE (ELASTIC) $\Delta_1$		TIME (PLASTIC) $\Delta_1$		HS20 LOADING				
										CONC. DIAPH.	STEEL DIAPH.	CONC. DIAPH.	STEEL DIAPH.	CONC. DIAPH.	STEEL DIAPH.			
LXD100	100'-0"	101'-0"	1/2" $\phi$	28	8	1115	24.9	1.89	3.33	1.68	1.58	0.42	0.40	7'-6"	7'-6"	33.6	16.6	1439

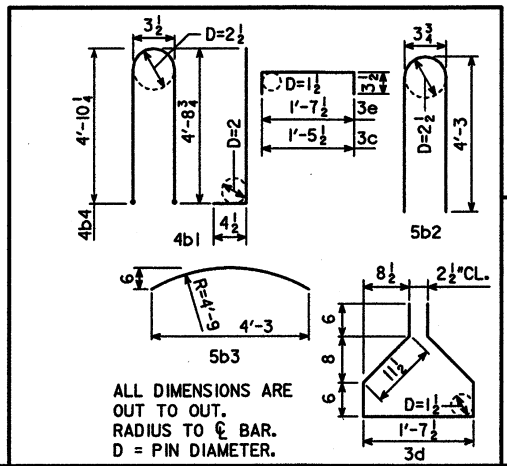
- ① DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM. THE DEFLECTIONS SHOWN ARE FOR A SLAB WEIGHT OF 760 #/FT. (8" SLAB AND 7'-6" BEAM SPACING) AND ONE CONCRETE DIAPHRAGM (3191 #) OR ONE STEEL DIAPHRAGM (285 #) AT  $\frac{1}{2}$  OF SPAN. FOR DIFFERENT SLAB AND DIAPHRAGM WEIGHTS, DEFLECTIONS WILL BE DIRECTLY PROPORTIONAL.
- ② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.  
 TOTAL BEAM DEFLECTIONS AT  $\frac{1}{2}$  OF SPAN,  $\Delta_0$ , DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE:  
 (A)  $\Delta_0 = \Delta_1 + \Delta_2$  FOR SIMPLE SPAN.  
 (B)  $\Delta_0 = \Delta_1 + \frac{3}{4}\Delta_2$  FOR END SPANS OF CONTINUOUS BRIDGE.  
 (C)  $\Delta_0 = \Delta_1 + \frac{1}{2}\Delta_2$  FOR INTERIOR SPANS OF CONTINUOUS BRIDGE.
- ③ TOTAL INITIAL PRESTRESS FOR LXD35 THRU LXD85 INCLUSIVE IS BASED ON 72.664%  $F_{su}$ , AND FOR LXD90 THRU LXD105 ON 75%  $F_{su}$ .  $F_{su} = 270$  ksi AND  $A_s = 0.153$  sq. in.
- \* MINIMUM CONCRETE  $f'_c$  (AT 28 DAYS) SHALL BE 6000 psi. MINIMUM  $f'_c$  AT RELEASE SHALL BE 5000 psi.
- $\Delta$  6'-10" LXD100

**NOTES:**

IF THE STEEL DIAPHRAGM OPTION IS ALLOWED AND USED, HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.  
 IF SOLE PLATE IS REQUIRED FOR BEARING, SOLE PLATE IS TO BE SET IN FORMS WHEN BEAM IS CAST AND FORMED OUT BELOW TO EXCLUDE CONCRETE AS DETAILED ON THE BEARING SHEET.  
 IF STUB ABUTMENTS ARE USED, ALL STRANDS AT THE ENDS OF BEAMS AT STUB ABUTMENTS SHALL BE CUT OFF REASONABLY FLUSH WITH THE CONCRETE.

**NOTES:**

THESE BEAMS ARE DESIGNED FOR AASHTO LIVE LOADS AS INDICATED IN ABOVE TABLE WITH AN ALLOWANCE OF 20 lb. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.  
 HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.  
 ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.  
 TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF APPROXIMATELY 1/4 INCH.  
 BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS.  
 BEAMS TO BE USED IN BRIDGES MADE CONTINUOUS BY THE POURED IN PLACE FLOOR, ARE TO BE AT LEAST 28 DAYS OLD BEFORE THE FLOOR IS PLACED UNLESS A SHORTER CURING TIME IS APPROVED BY THE BRIDGE ENGINEER.  
 THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT AND PIER DIAPHRAGMS SHALL BE ROUGHENED FOR A DISTANCE OF 10' FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.14 OF THE SPECIFICATIONS.  
 UNLESS OTHERWISE NOTED ALL BEAMS ARE TO BE INCREASED IN LENGTH BY .0005L TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.  
 FOR TRANSPORTING, THE OVERHANG SHALL BE IN ACCORDANCE WITH ART. 2407.13 OF STD. SPEC., EXCEPT THE OVERHANG MAY BE INCREASED TO A MAXIMUM OF 12 FEET FOR THE LXD100 BEAM.



THE CONTRACTOR SHALL ASSURE THE LATERAL STABILITY OF THE LXD100 AND LXD105 BEAMS DURING HANDLING, TRANSPORTING AND ERECTION BY PROVIDING TEMPORARY BRACING AS NEEDED.  
 IF THE PRECAST PANEL OPTION IS ALLOWED AND USED FOR BRIDGE DECK FORMATION, TOP FLANGE FINISH SHALL BE MODIFIED AS PER DETAILS ON THE PRECAST PANEL SHEET.  
 1/2" DIAMETER STRANDS STRESSED TO NOT MORE THAN 3,000 LBS. EACH MAY BE USED IN LIEU OF THE  $\alpha$  BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

DESIGN FOR 0° SKEW  
**100'-0" X 44' PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE**  
 100'-0" CENTER SPAN  
**LXD BEAM DETAILS**  
 STATION: 596+27.13 MARCH, 1999  
**WASHINGTON COUNTY**  
 IOWA DEPARTMENT OF TRANSPORTATION - PROJECT DEVELOPMENT DIVISION  
 DESIGN SHEET NO. 12 OF 14 FILE NO. 28216 DESIGN NO. 299







**ESTIMATED ROADWAY QUANTITIES**

100-0A  
10-28-97

ITEM NO.	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUAN.
	2525--2638030	SILT FENCE	FT	200	919

**POLLUTION PREVENTION PLAN**

110-12A  
02-23-93

All contractors/subcontractors shall conduct their operations in a manner that minimizes erosion and prevents sediments from leaving the highway right-of-way. The prime contractor shall be responsible for compliance and implementation of the Pollution Prevention Plan (PPP) for their entire contract. This responsibility shall be further shared with subcontractors whose work is a source of potential pollution as defined in this PPP.

**1. SITE DESCRIPTION**

This Pollution Prevention Plan (PPP) is for the construction of a two lane roadway on IA 92 in Washington County from the east city limits of Washington to east of Ainsworth.

This PPP covers approximately 180 acres with an estimated 120 acres being disturbed.

The PPP is located in an area of one soil association (Otley-Mahaska-Taintor). The estimated average SCS runoff curve number for this PPP after completion will be 68.

Refer to the P.C.C. Pavement Grade and Replace plan (Washington County STP-92-9(56)-2C-92) for locations of typical slopes, ditch grades, and major structural and non-structural controls. A copy of this plan will be on file at the project engineer's office. Runoff from this work will flow into various unnamed ditches which flow into the North Fork Long Creek, the South Fork Long Creek, the Iowa River, and the Mississippi River. North Fork Long Creek and South Fork Long Creek are tributaries of the Iowa River. The Iowa River is a tributary of the Mississippi River.

**POTENTIAL SOURCES OF POLLUTION:**

Site sources of pollution generated as a result of this work relate to silts and sediment which may be transported as a result of a storm event. However, this PPP provides conveyance for other (non-project related) operations. These other operations have storm water runoff, the regulation of which is beyond the control of this PPP. Potentially this runoff can contain various pollutants related to site-specific land

uses. Examples are:

**Rural Agricultural Activities:**

Runoff from agricultural land use can potentially contain chemicals including herbicides, pesticides, fungicides and fertilizers.

**Commercial and Industrial Activities:**

Runoff from commercial, industrial, and commerce land use may contain constituents associated with the specific operation. Such operations are subject to potential leaks and spills which could be commingled with run-off from the facility. Pollutants associated with commercial and industrial activities are not readily available since they are typically proprietary.

**2. CONTROLS**

Prior to beginning grading, excavation or clearing and grubbing operations, silt fence shall be placed along the perimeter of the areas to be disturbed at locations where runoff can move offsite. Vegetation in areas not needed for construction shall be preserved. As areas reach their final grade, additional silt fences, silt basins, intercepting ditches, sod flumes, letdowns, bridge end drains, & earth dikes shall be installed as specified in the plans and/or as required

**POLLUTION PREVENTION PLAN**

110-12A  
02-23-93

by the project engineer. This will include using silt fence as ditch checks and to protect intakes. Temporary stabilizing seeding shall be completed as the disturbed areas are constructed. If construction activity is not planned to occur in a disturbed area for at least 21 days, the area shall be stabilized by temporary seeding or mulching within 14 days. No more than 750,000 square feet of exposed erodible area is allowed in any one grading spread without permission of the project engineer. Other stabilizing methods shall be used outside the seeding period.

This work shall be done in accordance with Section 2525 of the Standard Specification.

If the work involved is not applicable to any contract items, the work shall be paid for according to Article 1109.03 paragraph B.

As the work progresses, additional erosion control items such as rock or sod flumes, rock ditch checks, letdown structures, soil stabilization mats and other appropriate measures shall be installed by the paving or erosion control contractor as determined by the engineer after field investigation. The construction will be completed with the establishment of permanent perennial vegetation of all disturbed areas by the erosion control contractor.

**3. OTHER CONTROLS**

Contractor disposal of unused construction materials and construction material wastes shall comply with applicable state and local waste disposal, sanitary sewer, or septic system regulations. In the event of a conflict with other governmental laws, rules and regulations, the more restrictive laws, rules or regulations shall apply.

**APPROVED STATE OR LOCAL PLANS:**

During the course of this construction, it is possible that situations will arise where unknown materials will be encountered. When such situations are encountered, they will be handled according to all federal, state, and local regulations in effect at the time.

**4. MAINTENANCE**

The contractor is required to maintain all temporary erosion control measures in proper working order, including cleaning, repairing, or replacing them throughout the contract period. Cleaning of silt control devices shall begin when the features have lost 50% of their capacity.

**5. INSPECTIONS**

Inspections shall be made jointly by the contractor and the contracting authority every seven calendar days and after each rain event that is 1/2" or greater. The contractor shall immediately begin corrective action on all deficiencies found. The findings of this inspection shall be recorded in the project diary. This PPP may be revised based on the findings of the inspection. The contractor shall implement all revisions. All corrective actions shall be completed within 3 calendar days of the inspection.

**6. NON-STORM DISCHARGES**

This includes subsurface drains (i.e. longitudinal and standard subdrains), slope drains and bridge end drains. The velocity of the discharge from these features may be controlled by the use of patio blocks, Class A stone or erosion stone.

**ESTIMATE REFERENCE INFORMATION**

100-4  
07-15-97

Data listed below is for informational purpose only and shall not constitute a basis for any extra work orders.

ITEM NO.	ITEM CODE	DESCRIPTION
	2525--2638030	SILT FENCE Item is for temporary erosion control, to be placed as directed by the engineer.

**STANDARD ROAD PLANS**

105-4  
12-03-96

The following Standard Road Plans shall be considered applicable to construction work on this project:

NUMBER	DATE	NUMBER	DATE	NUMBER	DATE
RC-16A	10-27-98	RC-16B	09-21-99		

Design No. 299  
No. 28216  
Design Sheet No. C.01

**ROADWAY DESIGN**



I hereby certify that this plan was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

*Keith A. Cadwell* 7/20/99  
Signature Keith Allen Cadwell Date

Printed or Typed Name

My license renewal date is December 31, 2000.

Pages or sheets covered by this seal: C.01