

IOWA DEPARTMENT OF TRANSPORTATION

TO OFFICE: District 3
DATE: July 19, 2012
ATTENTION: Tony Lazarowicz
REF. : Woodbury County
NHSX-020-1(121)--3H-97
FROM: Yanxiao Jia
PIN: 98-97-020-010-02
OFFICE: Design
STP-031-1(32)--2C-97
PIN: 98-97-020-010-03
SUBJECT: Field Exam – City of Correctionville

A field exam was held on June 21, 2012 to review the proposed plan for the reconstruction of U.S. 20 and relocated IA 31 in the City of Correctionville.

Those present for the field exam included the following: Tony Lazarowicz, Shane Tymkowicz, Darwin Bishop, Dixie Nelson, and Tony Babcock from District 3; Sharon Dumdei from the Office of Right of Way; Dave Claman from the Office of Bridges and Structures; Ryan Miller, Dave Fullarton, Allison Smyth, Tyler Schlueter, and Yanxiao Jia from the Office of Design; and Mike Frahm and Pat Langschwager from the City of Correctionville.

U.S. 20

U.S. 20 is functionally classified as a “commercial and industrial” route and is a service level “B” roadway.

The 2015 and 2035 estimated daily traffic are:

- 5,900 AADT and 7,400 AADT respectively with 24% truck traffic from existing IA 31 to Hackberry Street;
- 6,630 AADT and 8,260 AADT respectively with 26% truck traffic for the remainder of the project.

U.S. 20 will be designed for 50 mph. Reconstruction will start from just west of Aspen Street (relocated IA 31) east to approximately Sta. 10867+60, a total of 4,900 ft. The typical section will provide a 5-lane roadway with a 14 ft. center turn lane throughout the project. From Driftwood Street (existing IA 31) to Hackberry Street, the typical cross section will be 68 ft. (back-to-back) wide curbed roadway, which consists of two 15 ft. outside lanes (includes 3 ft. curb and gutter section), two 12 ft. inside lanes, and a 14 ft. wide center turn lane. The typical cross section will also provide 19 ft. wide shoulders behind the curb on both sides of the roadway which will provide the preferred 22 ft. wide clearzone from the edge of the 12 ft. outside traveled lane. A 6” standard curb will be constructed in order to better control access along both sides of the highway. The typical section for the rest of the project will have two 14 ft. outside lanes, two 12 ft. inside lanes, and a 14 ft. center turn lane with 8 ft. shoulders and 6:1/3.5 foreslopes. Four foot of these shoulders will be paved.

U.S. 20 will be widened to the north side in order to reduce impacts to the businesses on the south side.

An eastbound right turn lane will be provided at the intersection of U.S. 20 and relocated IA 31/Aspen Street.

New storm sewer will be installed in the curbed section on both sides of the roadway to facilitate drainage and accommodate staged construction. Any sanitary sewer or water line relocation will be designed and paid for by the city.

Drainage on the south side, west of Aspen Street/Relocated IA 31, was discussed. Currently water from the south ditch drains north through a crossroad pipe. One suggestion was to eliminate this pipe and keep the drainage on the south side and drain it to the Little Sioux River. A flap gate will need to be used to control water from the river backing into the south ditch. After the field exam, Preliminary Bridge expressed their preference of utilizing a crossroad pipe.

Relocated IA 31

IA 31 is classified as an access route and is a service level “C” roadway. The 2015 and 2035 estimated daily traffic is 1,450 AADT and 1,850 AADT respectively with 21% truck traffic.

Relocated IA 31 will also be designed for 50 mph. It will start from existing IA 31 on the west side of Correctionville north to U.S. 20, a distance of approximately 3,700 ft. The typical section will consist of two 14 ft. lanes with 8 ft. granular shoulders and 6:1/3.5 foreslopes.

The horizontal alignment for relocated IA 31 will be shifted west to reduce impacts to the Mid-American transmission line, which is located on the east side of Aspen Street. It was discovered, after the Field Exam, that this transmission line is on private easement. In order to provide a 15’ permanent highway right of way beyond the top of the backslope, the horizontal alignment will need to be shifted further west to avoid relocating most of the power poles.

Preliminary Bridge proposed to use 36” pipes under IA 31 to accommodate drainage. These pipes were sized to ensure water level on the east side of relocated IA 31 will recede at least at the same rate as the Little Sioux River after a major flooding event. Profile grade from 5th Street to Sioux Avenue will be lowered to improve sideroad connections.

Intersection returns at Sioux Avenue, 5th Street, and 7th Street should be designed to accommodate a WB-67. Side road connections for both 5th and 7th Street will have granular surfacing with a 28 ft. wide top width.

It is likely that access control for IA 31 will be Priority IV. In order to meet the 600 ft. spacing requirement, it is recommended to relocate the park entrance, at Sta. 2178+50 (left side), south to line up with 5th Street, and the field entrance at Sta. 2178+50,

right side, to a location along the north side of 5th Street.

It was also recommended that stop sign rumble strips be installed on the west approach of the existing IA 31 at the intersection of existing IA 31/relocated IA 31. This will need to be reviewed with Traffic and Safety according to the Design Manual.

Staging

These two projects will be tied and built using staged construction. Traffic on U.S. 20 and IA 31 shall be maintained at all times. The following is a summary of what was discussed:

- Hackberry Street from U.S. 20 north to IA 31 will be resurfaced with 2” HMA material to accommodate IA 31 traffic when the intersection of existing IA 31/U.S. 20 is under construction.
- Temporary pavement will be utilized on the south side of U.S. 20.
- During construction, two-way traffic will be maintained on existing U.S. 20 and the temporary pavement while the westbound lanes are constructed. Traffic will then be switched over to the newly completed westbound lanes while the eastbound lanes are constructed.
- The city requested that Hackberry Street and existing IA 31, north of U.S. 20, not be closed at the same time.
- It is recommended to maintain traffic on Sioux Avenue, if possible, during the fall harvest season in order to accommodate delivery trucks to the grain elevator, which is located between 5th Street and 7th Street.
- The district will discuss with the county to see if County Road L36/Osceola Avenue can be closed to speed up construction.
- Coordinate staging with the projects west (east of Merville to west of Correctionville) and east (east of Correctionville to west of the west junction with U.S. 59) of this project.
- Use of 35 mph as the regulatory speed limit during construction was also discussed. There were some concerns about reducing speed limit from 55 mph to 35 mph after a 6% grade for EB traffic on the west end of the project. District and Design will work together to resolve this issue.

Additional Survey Needs

It was identified that the following additional survey will be required:

- Entrance for DOT maintenance garage/nursing home
- 5th Street, 7th Street, and the relocated park entrance

Action Items for the District

- Obtain survey for the DOT maintenance garage entrance
- Work with the city to locate water line along Fir Street and U.S. 20 from Fir Street to Hackberry Street
- Work with the city on possible sanitary sewer and water main relocations

- Work with the county on possible closure of County Road L36/Osceola Avenue during construction

A bid item for Field Lab should be included in the plans. In order to accommodate temporary access for entrances and reduce extra work orders during construction, the District requested that the plans include a bid item for Granular Surfacing on Road and additional quantity for Class 10 fill material.

Permanent and temporary right of way will be required.

The Class 10 borrow needs are approximately 10,000 cubic yards for U.S. 20 and 33,300 cubic yards for IA 31. The borrow material will be from the west project (east of Merville to west of Correctionville).

No plans are included in this submittal; however, plan and cross section sheets may be viewed as pdf files in folder

<\\ntdfs\W\DataStor\Projects2\97020050A98\Design\Section2\Design Events\D2.>

These projects are currently scheduled for a February 2015 letting. The construction costs are estimated to be \$6,874,700 for U.S. 20 and \$1,479,500 for IA 31 respectively. There was no concept for these projects.

YJ:mk

cc: M. J. Dillavou

M. J. Kennerly

D. L. Maifield

K. K. Patel

R. L. Stanley

T. Crouch

M. D. Masteller

C. B. Brakke

D. A. Popp

D. R. Claman

B. Hofer

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Dixie Nelson

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K. D. Nicholson

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M. A. Swenson

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G. A. Novey

J. Vortherms

Tyler Schlueter

Darwin Bishop

Ryan Miller

WOODBURY COUNTY

UNKNOWN PAVEMENT - GRADE AND NEW
STP-031-1(32)--2C-97

LETTING DATE
2-17-2015

| INDEX OF SHEETS | |
|---------------------|---|
| No. | DESCRIPTION |
| A Sheets | Title Sheets |
| A.1 | Title Sheet |
| A.2 | Location Map Sheet |
| B Sheets | Typical Cross Sections and Details |
| B.1 - 2 | Typical Cross Sections and Details |
| D Sheets | Mainline Plan and Profile Sheets |
| * D.1 | Plan & Profile Legend & Symbol Information Sheet |
| * D.2 - 4 | "Mainline Name" |
| E Sheets | Side Road Plan and Profile Sheets |
| * E.1 - 3 | "Side Road Name" |
| G Sheets | Survey Sheets |
| G.1 - 3 | Reference Ties and Bench Marks |
| G.4 - 10 | Horizontal Control Tab. & Super for all Alignments |
| T Sheets | Earthwork Quantity Sheets |
| T.1 | Earthwork Quantity Sheets |
| W Sheets | Mainline Cross Sections |
| W.1 | Cross Sections Legend & Symbol Information Sheet |
| W.2 - 45 | Mainline Cross Sections |
| X Sheets | Side Road Cross Sections |
| X.1 - 4 | Side Road Cross Sections - 5th St. |
| X.5 - 9 | Side Road Cross Sections - Park Ent., Lt. Sta. 2178+ |
| X.10 - 11 | Side Road Cross Sections - Field Ent., Rt. Sta. 2178+ |
| X.12 - 14 | Side Road Cross Sections - Field Ent., Lt. Sta. 2183+ |
| X.15 - 18 | Side Road Cross Sections - 7th St. |
| X.19 - 20 | Side Road Cross Sections - Sioux Ave. |
| * Color Plan Sheets | |



Iowa Department of Transportation

Highway Division

PLANS OF PROPOSED IMPROVEMENT ON THE

PRIMARY ROAD SYSTEM WOODBURY COUNTY

UNKNOWN PAVEMENT - GRADE AND NEW

Ia. 31 From SW Of Correctionville N. To US 20

SCALES: As Noted

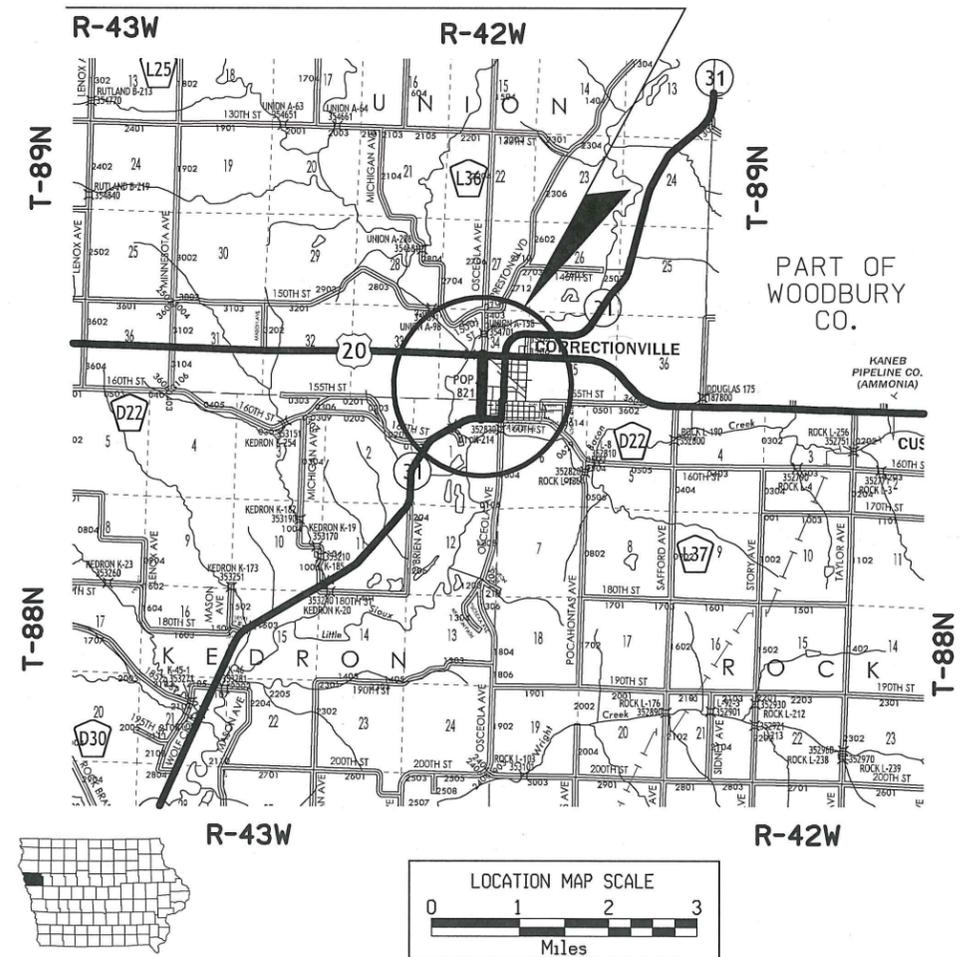
Refer to the Proposal Form for list of applicable specifications.

Value Engineering Saves. Refer to Article 1105.15 of the Specifications.

For Mileage Summary
Refer to Sheet A.2



PROJECT LOCATION



For Project Location Map
Refer to Sheet A.2

| DESIGN DATA URBAN | | | |
|-------------------|--------------|-------|--------|
| 2015 | AADT | 1,450 | V.P.D. |
| 2035 | AADT | 1,850 | V.P.D. |
| 2035 | DHV | -- | V.P.H. |
| | TRUCKS | 21 | % |
| | Total | | |
| | Design ESALs | -- | |

| INDEX OF SEALS | | |
|----------------|------|-------------------------|
| SHEET NO. | NAME | TYPE |
| A.1 | X | Primary Signature Block |
| X | X | X |
| | | |
| | | |
| | | |

| REVISIONS | | TOTAL |
|-------------------------------|--|-------|
| PROJECT IDENTIFICATION NUMBER | | |
| 98-97-020-010-03 | | |
| PROJECT NUMBER | | |
| STP-031-1(32)--2C-97 | | |
| R.O.W. PROJECT NUMBER | | |
| STPN-031-1(33)--2J-97 | | |

Field Exam Plans

- Tony Lazarewicz
 - Shane Tymkrowicz
 - Darwin Bishop
 - Dixie Nelson
 - Tony Babcock
- } Dist. 3
- Sharon Dundei - ROW
 - Dave Claman - Preliminary Bridge
- Ryan Miller
 - Dave Fullarton
 - Allison Smyth
 - Tyler Schlueter
 - Yanxiao Jia
 - Mike Frahm
 - Pat Langschwager
- } Design
- } City of Correctionville

- D3 PLAN - Date: 09-07-2012
- D5 PLAN - Date: 02-01-2013
- D8 PLAN - Date: 12-02-2014

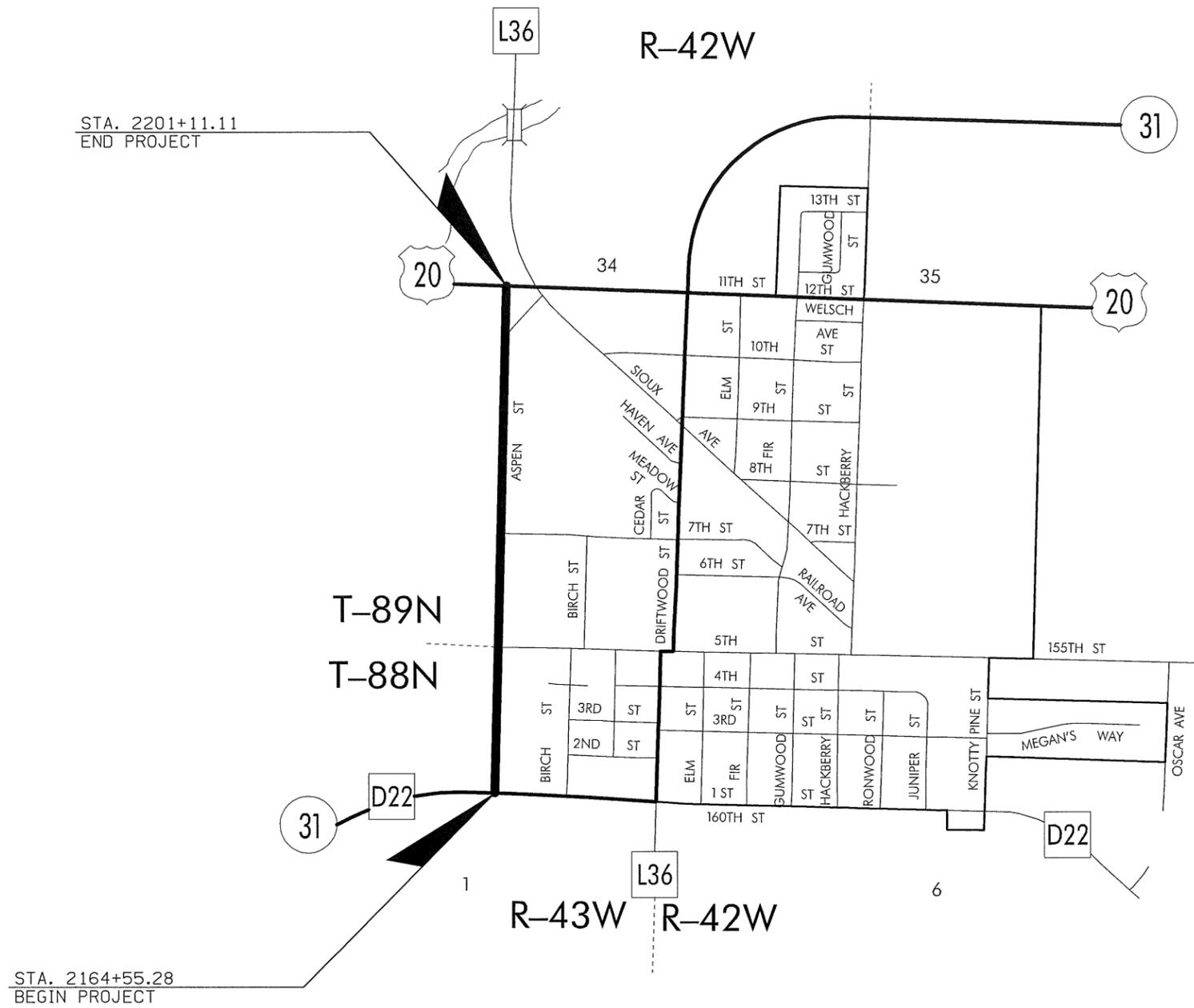
Estimated Borrow: CY

PRELIMINARY PLANS

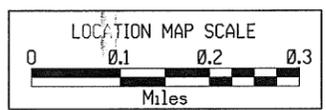
Subject to change by final design.

D2 PLAN - Date: 06-21-2012

| MILEAGE SUMMARY | | | |
|-----------------|---|----------|-------|
| Div. | Location | Lin. Ft. | Miles |
| 1 | IA 31 Sta. 2164+64.58 to Sta. 2201+11.11 | 3646.53 | |
| | Total Length of Roadway | 3646.53 | 0.691 |
| | Total Length of Project | 3646.53 | 0.691 |



HIGHWAY AND STREET MAP
OF
CORRECTIONVILLE
IOWA



FIELD EXAM NOTES

FIELD EXAM QUESTIONS

DISTRICT TO PROVIDE

- 1. Names and Addresses of Affected Utility Companies (Y) (N) _____
- 2. List of special events (Y) (N) _____
- 3. Clearing and Grubbing (Y) (N) _____
- 4. District to remove/replace signs or Contractor? (Y) (N) _____

DISCUSS/REVIEW WITH DISTRICT

50 mph

- 1. Furnished borrow? (Y) (N) _____
- 2. Field Lab? (Y) (N) _____
- 3. Field Office? (Y) (N) _____
- 4. Construction Survey? (Y) (N) _____
- 5. UAC ML crossroad culvert? UAC SR/entrance culverts? (Y) (N) _____
- 6. Relocate field entrance at approx. Sta. 2182+50? (Y) (N) _____
- 7. Relocate park entrance across from sideroad? (relocate field ent.?) (Y) (N) _____
- 8. Side road closures during construction? (Stage/alternate?) (Y) (N) _____
- 9. Design Speed/Speed Limit (Y) (N) _____
- 10. Sioux Ave. Turning Path – Truck Type? (Y) (N) WB-67 _____
- 11. Park Ent. Turning Path – Truck Type? (Y) (N) _____
- 12. Park Ent. Pave? Shoulder, Shldr + 20', or to End of Returns? (Y) (N) _____
- 13. Rock Field Ent's? To ROW, or Tie-In? (Y) (N) _____
- 14. Staging at BOP? Alternating one-way traffic on existing IA 31? (Y) (N) _____
- 15. (Y) (N) _____
- 16. (Y) (N) _____

CHECK/VERIFY ON FIELD EXAM

- 1. WB-67 (N+S Ends, 5th St., 7th St.?) (Y) (N) _____
- 2. (Y) (N) _____
- 3. (Y) (N) _____
- 4. (Y) (N) _____

Preferred Clear-zone Distances (feet).

| design speed | design ADT | FORESLOPES | | | BACKSLOPES | | |
|----------------|-------------------|----------------|---|------------------|-------------------|---------------------------|----------------|
| | | 6:1 or flatter | Steeper than 6:1, up to and including 4:1 | Steeper than 4:1 | Steeper than 4:1* | 4:1 or flatter, up to 6:1 | 6:1 or flatter |
| 40 mph or less | ADT < 750 | 10 | 10 | ** | 10 | 10 | 10 |
| | 750 ≤ ADT < 1500 | 12 | 14 | ** | 12 | 12 | 12 |
| | 1500 ≤ ADT < 6000 | 14 | 16 | ** | 14 | 14 | 14 |
| | ADT ≥ 6000 | 16 | 18 | ** | 16 | 16 | 16 |
| 45 – 50 mph | ADT < 750 | 12 | 14 | ** | 10 | 10 | 12 |
| | 750 ≤ ADT < 1500 | 16 | 20 | ** | 12 | 14 | 16 |
| | 1500 ≤ ADT < 6000 | 18 | 26 | ** | 14 | 16 | 18 |
| | ADT ≥ 6000 | 22 | 28 | ** | 16 | 20 | 22 |
| 55 mph | ADT < 750 | 14 | 18 | ** | 10 | 12 | 12 |
| | 750 ≤ ADT < 1500 | 18 | 24 | ** | 12 | 16 | 18 |
| | 1500 ≤ ADT < 6000 | 22 | 30 | ** | 16 | 18 | 22 |
| | ADT ≥ 6000 | 24 | 32 | ** | 18 | 22 | 24 |
| 60 mph | ADT < 750 | 18 | 24 | ** | 12 | 14 | 16 |
| | 750 ≤ ADT < 1500 | 24 | 32 | ** | 14 | 18 | 22 |
| | 1500 ≤ ADT < 6000 | 30 | 40 | ** | 18 | 22 | 26 |
| | ADT ≥ 6000 | 32 | 44 | ** | 22 | 26 | 28 |
| 65 – 70 mph | ADT < 750 | 20 | 26 | ** | 12 | 16 | 16 |
| | 750 ≤ ADT < 1500 | 26 | 36 | ** | 16 | 20 | 22 |
| | 1500 ≤ ADT < 6000 | 32 | 42 | ** | 20 | 24 | 28 |
| | ADT ≥ 6000 | 34 | 46 | ** | 24 | 30 | 30 |

* Backslopes as steep as 2.5:1 can be considered as part of the clear zone, as long as they are relatively smooth and do not contain any fixed objects. Refer to Section 8A-4 of the Design Manual for information regarding backslopes steeper than 2.5:1.

** Since a vehicle traveling on a slope steeper than 4:1 is likely to be diverted to the bottom of the slope, the width of any slope steeper than 4:1 cannot be counted in the clear zone determination. Refer to Section 8A-2 of the Design Manual for information on providing clear recovery areas at the base of steep slopes.

Acceptable Clear-zone Distances (feet).

| design speed | design ADT | FORESLOPES | | | BACKSLOPES | | |
|----------------|-------------------|----------------|---|------------------|-------------------|---------------------------|----------------|
| | | 6:1 or flatter | Steeper than 6:1, up to and including 4:1 | Steeper than 4:1 | Steeper than 4:1* | 4:1 or flatter, up to 6:1 | 6:1 or flatter |
| 40 mph or less | ADT < 750 | 7 | 7 | ** | 7 | 7 | 7 |
| | 750 ≤ ADT < 1500 | 10 | 12 | ** | 10 | 10 | 10 |
| | 1500 ≤ ADT < 6000 | 12 | 14 | ** | 12 | 12 | 12 |
| | ADT ≥ 6000 | 14 | 16 | ** | 14 | 14 | 14 |
| 45 – 50 mph | ADT < 750 | 10 | 12 | ** | 8 | 8 | 10 |
| | 750 ≤ ADT < 1500 | 14 | 16 | ** | 10 | 12 | 14 |
| | 1500 ≤ ADT < 6000 | 16 | 20 | ** | 12 | 14 | 16 |
| | ADT ≥ 6000 | 20 | 24 | ** | 14 | 18 | 20 |
| 55 mph | ADT < 750 | 12 | 14 | ** | 8 | 10 | 10 |
| | 750 ≤ ADT < 1500 | 16 | 20 | ** | 10 | 14 | 16 |
| | 1500 ≤ ADT < 6000 | 20 | 24 | ** | 14 | 16 | 20 |
| | ADT ≥ 6000 | 22 | 26 | ** | 16 | 20 | 22 |
| 60 mph | ADT < 750 | 16 | 20 | ** | 10 | 12 | 14 |
| | 750 ≤ ADT < 1500 | 20 | 26 | ** | 12 | 16 | 20 |
| | 1500 ≤ ADT < 6000 | 26 | 30 | ** | 14 | 18 | 24 |
| | ADT ≥ 6000 | 30 | 30 | ** | 20 | 24 | 26 |
| 65 – 70 mph | ADT < 750 | 18 | 20 | ** | 10 | 14 | 14 |
| | 750 ≤ ADT < 1500 | 24 | 28 | ** | 12 | 18 | 20 |
| | 1500 ≤ ADT < 6000 | 28 | 30 | ** | 16 | 22 | 26 |
| | ADT ≥ 6000 | 30 | 30 | ** | 22 | 26 | 28 |

* Backslopes as steep as 2.5:1 can be considered as part of the clear zone, as long as they are relatively smooth and do not contain any fixed objects. Refer to Section 8A-4 of the Design Manual for information regarding backslopes steeper than 2.5:1.

** Since a vehicle traveling on a slope steeper than 4:1 is likely to be diverted to the bottom of the slope, the width of any slope steeper than 4:1 cannot be counted in the clear zone determination. Refer to Section 8A-2 of the Design Manual for information on providing clear recovery areas at the base of steep slopes.

Preferred Effective Shoulder widths for Two-Lane Highways (values shown in feet)

| Auxiliary Lanes (includes turn lanes) | 6 | |
|---|--------------------|------------|
| Two-Lane Highways | Outside | |
| | Effective Shoulder | Paved |
| Any route with a designated bike trail | 10 | full width |
| On all curves with a radius of 954.93' or less | 10 | full width |
| On roadways approaching urban areas (due to increased bike traffic) | 10 | full width |
| On roadways with an existing ADT > 5000 | 10 | 6 |
| On all other NHS | 10 | 4 |
| On non-NHS routes with an existing ADT > 3000 | 10 | 4 |

Acceptable Shoulder widths for Two-Lane Highways (values shown in feet)

| Auxiliary Lanes (includes turn lanes) | 4 | |
|---------------------------------------|---------------------------------------|-----------|
| Two-Lane Highways | Effective Shoulder | Paved |
| | Design Year Traffic in Vehicles / Day | under 400 |
| 400-2000 | | 2 |
| over 2000 | | 2 |

| Design Element | Acceptable Values | | Preferred Values | Project Values | Comments |
|---|---|-------------------------|--|----------------------|--|
| | Rural Two-Lane Highways | Rural Two-Lane Highways | Rural Two-Lane Highways | | |
| design speed (mph) | Cannot be less than the posted speed limit | | 60 | X 50 | Verify Design Speed; 8100' R curve at N. end = 55mph |
| full depth paved width (ft) | 12 | 14 | 14 | | |
| design lane width (ft) | 11 | 12 | 12 | | |
| pavement cross-slope (%) | 1.5% minimum, 3% maximum | | 2%, However, when adjacent lanes slope in the same direction, increase slope by 0.5% per lane up to 3% | 2 | |
| effective shoulder width and type (see Section 3C-4) | See Shoulder Tables | | See Shoulder Tables | 10 | |
| shoulder cross-slope (%) | not less than the adjacent lane, 2 to 6% for paved, 4 to 6% for granular, 6 to 8% for earth | | 4 | 4 | |
| foreslope (see Roadway Typical Cross Sections) | adjacent to shoulder | 3:1 | 10:1 for 4' then 6:1 | 10:1 for 4' then 6:1 | |
| | beyond standard ditch depth and design clearzone | 3:1 | 3.5:1 | | |
| normal outside ditch (depth x width) (ft) | -- | | 5 x 10 | 5 x 10 | Verify Ditch Depth |
| Backslope (For cut areas greater than 25 feet, contact the Soils Design Section for assistance with backslope benches.) | 2.5:1 | | 3:1 | 3:1 | |
| transverse slopes | w/ drainage structures | 6:1 | 8:1 | 8:1 | |
| | w/o drainage structures | 6:1 | 10:1 | 10:1 | |
| | at sideroads | 6:1 | 10:1 | 10:1 | |
| Level of Service | B | | -- | | |

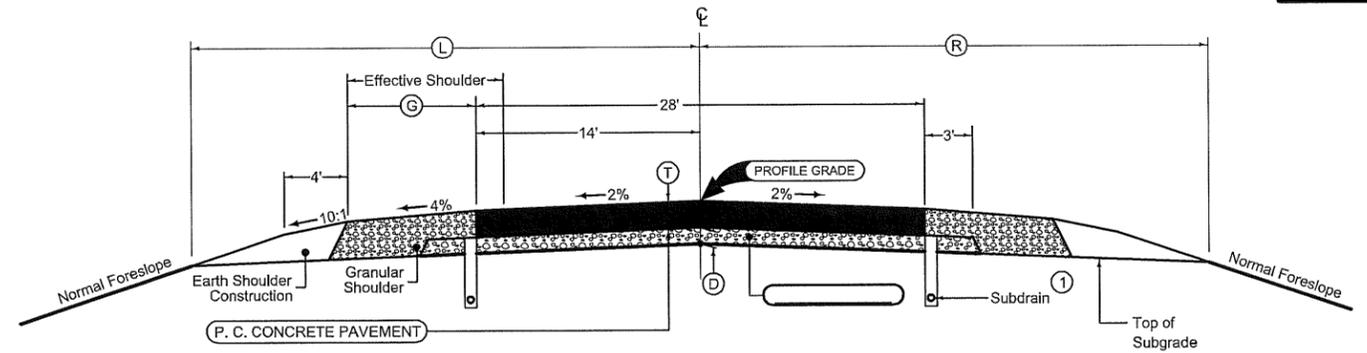
| LOCATION | | | MAINLINE | | | | | SHLDR |
|---------------------|--------------------|------------|----------|--------|------|------|------|-------|
| ROAD IDENTIFICATION | STATION TO STATION | | Ⓡ | Ⓛ | Ⓛ | Ⓡ | Ⓢ | |
| | | | Inches | Inches | Feet | Feet | Feet | |
| IA 31 | 2164+55.28 | 2201+11.11 | X | X | 14 | 14 | 8 | |
| 5th St. | 75+14.00 | 76+86.56 | X | X | 14 | 14 | 8 | |
| 7th St. | 83+14.00 | 84+87.36 | X | X | 14 | 14 | 8 | |
| Stoux Ave. | 99+14.07 | 100+00.00 | X | X | 14 | 14 | 8 | |

Dimensions are symmetrical about the centerline.
 Normal sections shown may be appropriately modified for areas specifically designated by the Engineer such as intersections or superelevated curves.

① Refer to RF-19C.

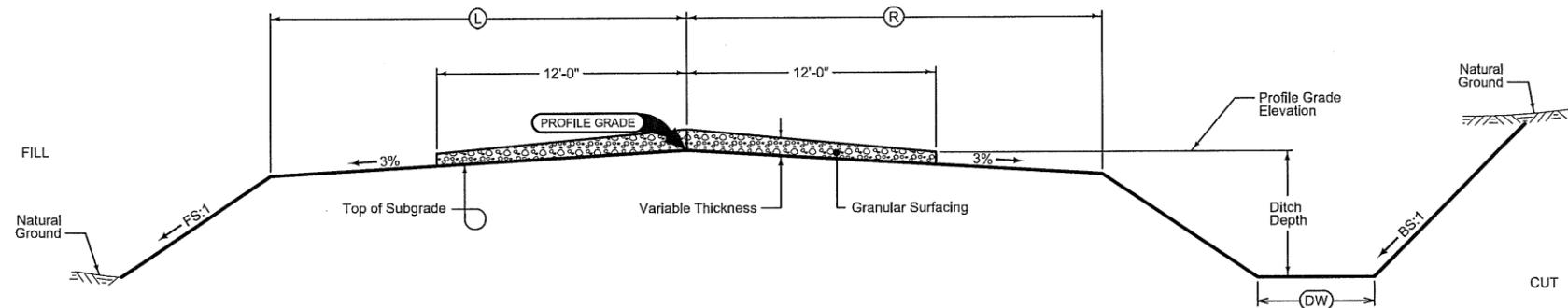
Mainline Pavement:
 Transverse joints: CD at 20' spacing
 Longitudinal joint: L-2

2P_G--G
 04-20-10



2-LANE PCC PAVING (Granular Shoulders)

| LOCATION | | | DIMENSIONS | | | | |
|---------------------|--------------------|----------|------------|------|----|----|------|
| ROAD IDENTIFICATION | STATION TO STATION | | Ⓛ | Ⓡ | FS | BS | Ⓢ |
| | | | Feet | Feet | | | Feet |
| 5th St. | 76+86.56 | 78+35.00 | 14 | 14 | 3 | 3 | X |
| Park Entrance | 77+00.00 | 80+02.53 | 14 | 14 | 3 | 3 | X |
| 7th St. | 84+87.36 | 86+00.00 | 14 | 14 | 3 | 3 | X |



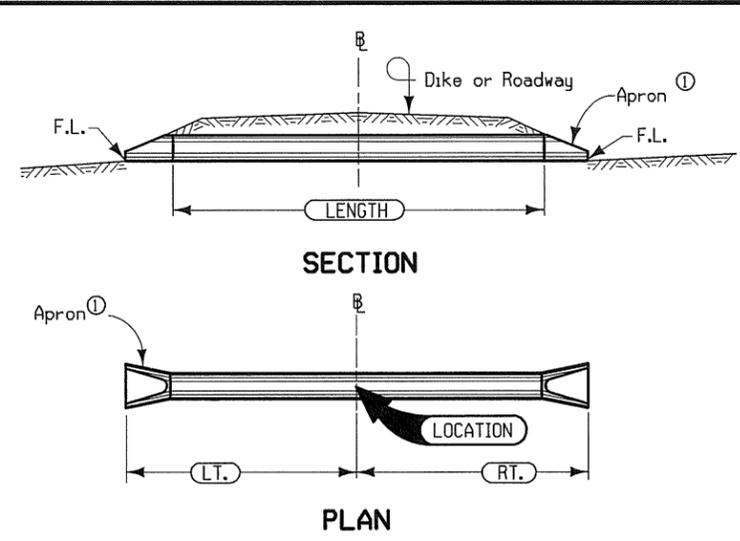
GRADING AND GRANULAR SURFACING

Normal section shown may be modified appropriately in areas of superelevated curves or other locations specifically designated by the Engineer.

See plan & profile sheets and cross sections for additional details of ditches and backslopes.

Place Granular Surfacing as follows:
 Grading design application rate is ___ tons per mile.
 Paving design application rate is ___ tons per mile.

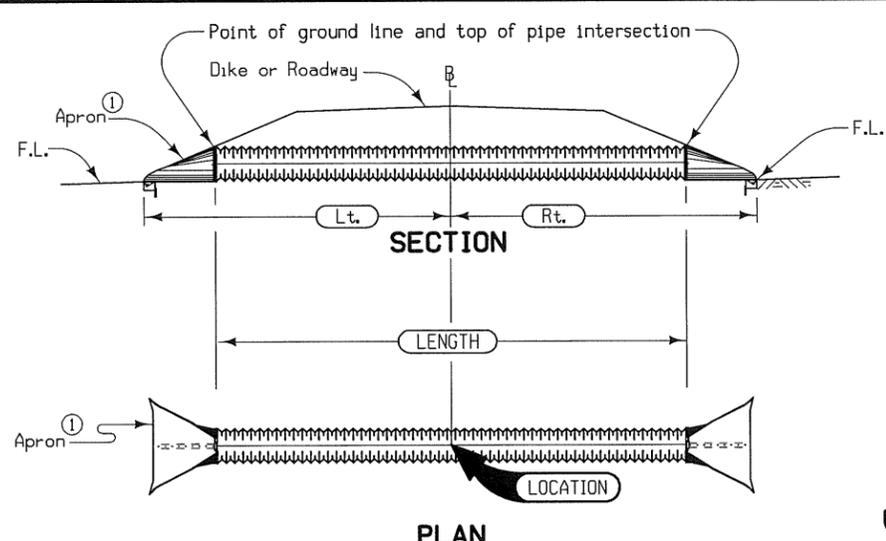
2_GradeGran
 04-17-12



1101
04-30-02

Notes:
 B shall be C of roadway, dike, survey, or other; as detailed on plans.
 Skew angle is the angle which one end of the pipe is ahead (by stationing) of line perpendicular to the B (example skew Rt. ahead 30%).
 Refer to tabular listing and other plans for additional information.
 ① See Standard Road Plan RF-3 For Conc. or RF-5 for Metal.

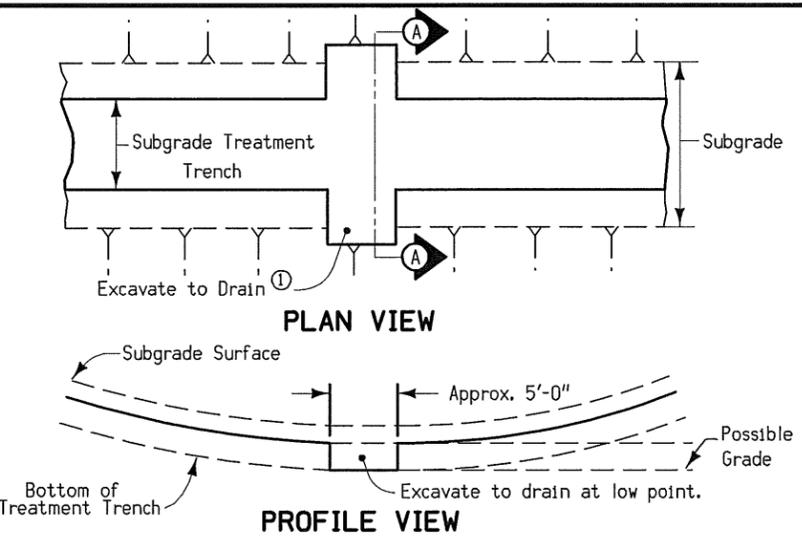
PIPE CULVERT



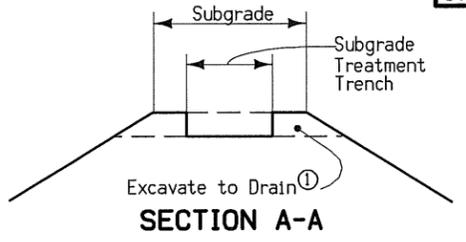
1601
10-16-12

B shall be C of roadway, dike, survey, or other; as detailed on plans.
 Skew angle is the angle which one end of the pipe is ahead (by stationing) of a line perpendicular to the B (example skew Rt. ahead 30 degrees).
 ① See Standard Road Plan RF-3 for Concrete or RF-5 for Metal and Polyethylene.

UNCLASSIFIED PIPE CULVERT

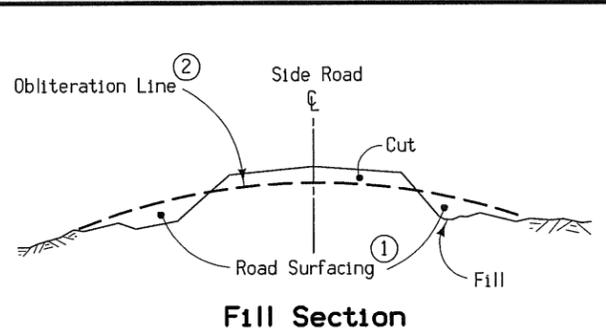


8301
04-20-04

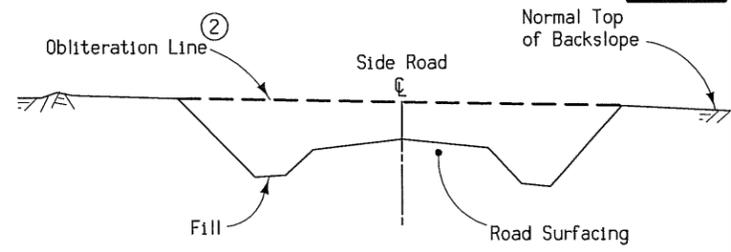


① The contractor shall excavate a portion of subgrade as necessary to provide drainage for the treatment trench. When the subgrade treatment is granular soils, the contractor shall excavate a portion of the subgrade and backfill it with the granular soils every 300 feet.
 This excavation shall be considered incidental.

TYPICAL DETAILS OF DRAIN FOR SUBGRADE TREATMENT TRENCH

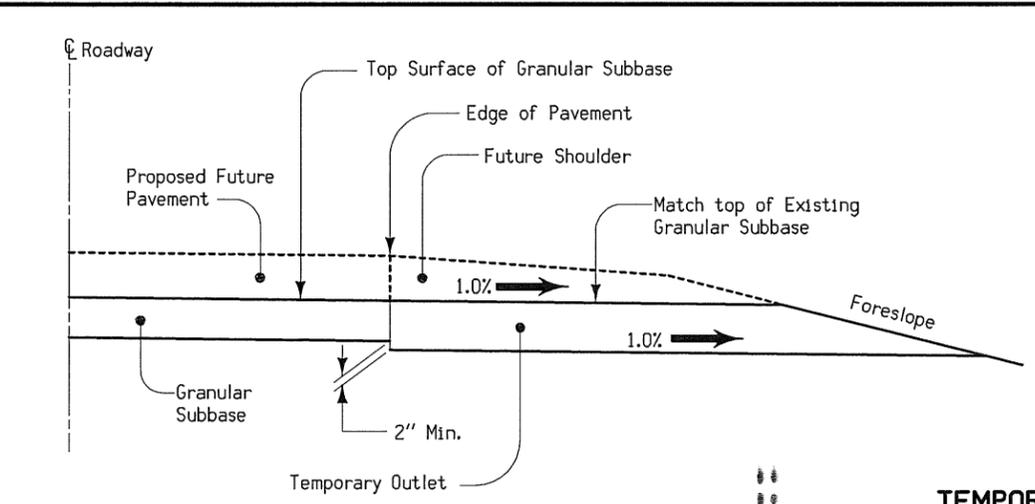


4302
04-03-01



① Existing road surfacing (granular material) shall be placed as shown unless otherwise directed by the Engineer or provided for in the detail project plans.
 ② When specified, the upper 1' to be suitable for vegetation (grass or crops).
 Note:
 The work of obliterating or reshaping old roadbeds shall be done at the direction of the Engineer.

TYPICAL DETAILS FOR OBLITERATION EXISTING ROADBED

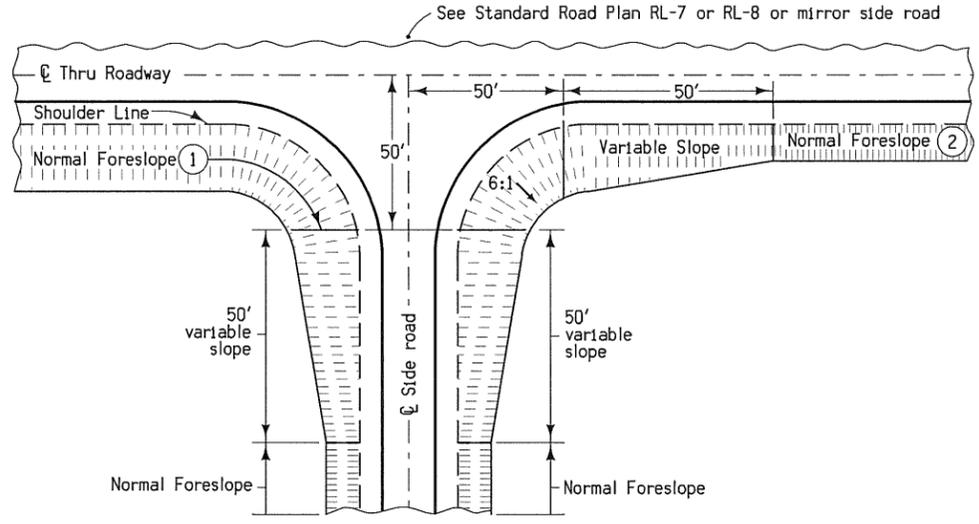


8303
10-22-93

NOTES:
 When granular subbase is placed under conditions where positive drainage is not present, a temporary outlet shall be constructed by placing a granular subbase 'French' drain a minimum 18" wide at the location tabulated for a future longitudinal subdrain outlet. This work shall be considered incidental to "Granular Subbase".

TEMPORARY OUTLET FOR GRANULAR SUBBASE DRAINAGE

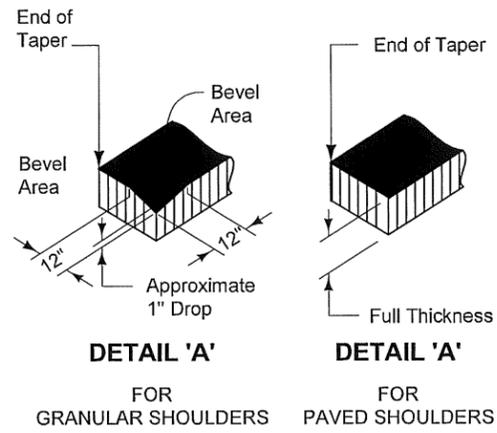
2116
04-15-08



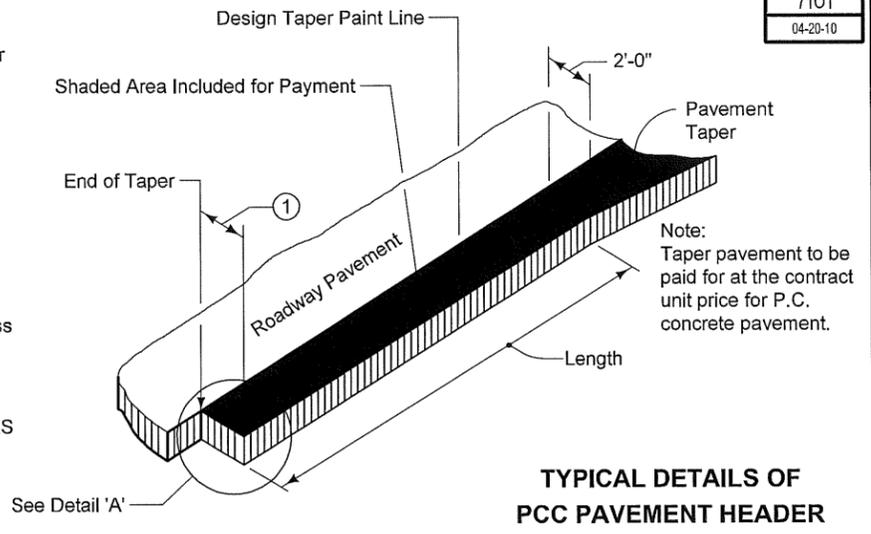
- ① For normal foreslopes 6:1 or flatter.
- ② For normal foreslopes steeper than 6:1.

SIDE ROAD GRADING

7101
04-20-10

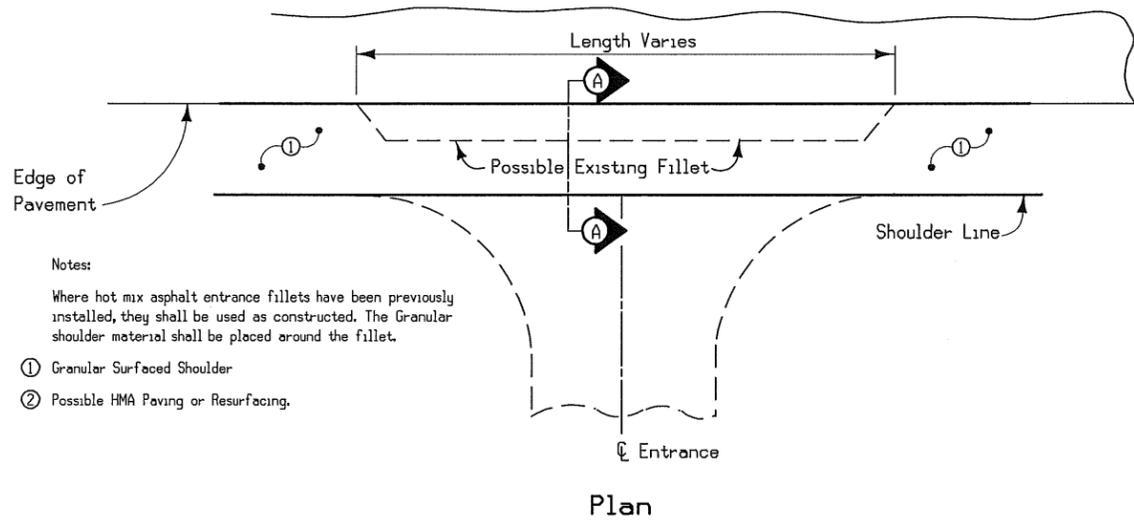


- ① Normal width is 2'-0". Construct 4'-0" width when butting into 4' wide HMA shoulders (See Typical 7154A).



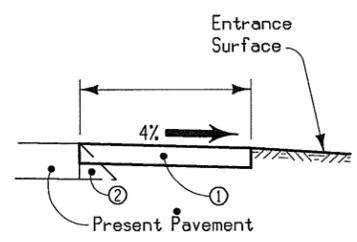
TYPICAL DETAILS OF PCC PAVEMENT HEADER

7117
10-02-01

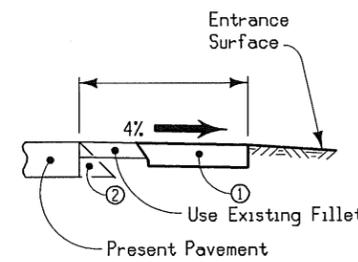


- Notes:
- Where hot mix asphalt entrance fillets have been previously installed, they shall be used as constructed. The Granular shoulder material shall be placed around the fillet.
- ① Granular Surfaced Shoulder
 - ② Possible HMA Paving or Resurfacing.

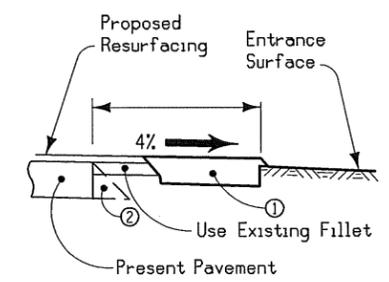
Plan



Section A-A Without Fillet

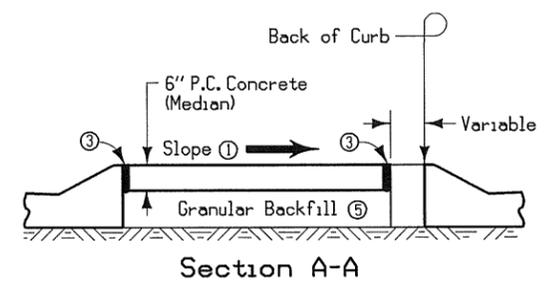
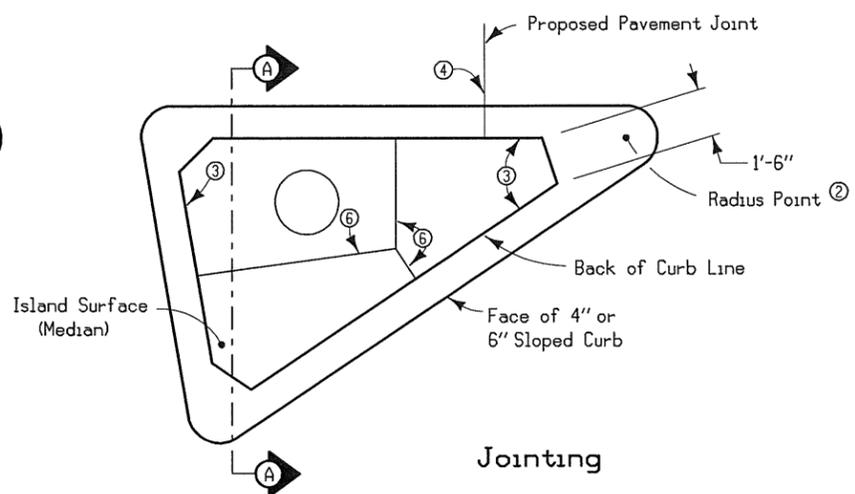
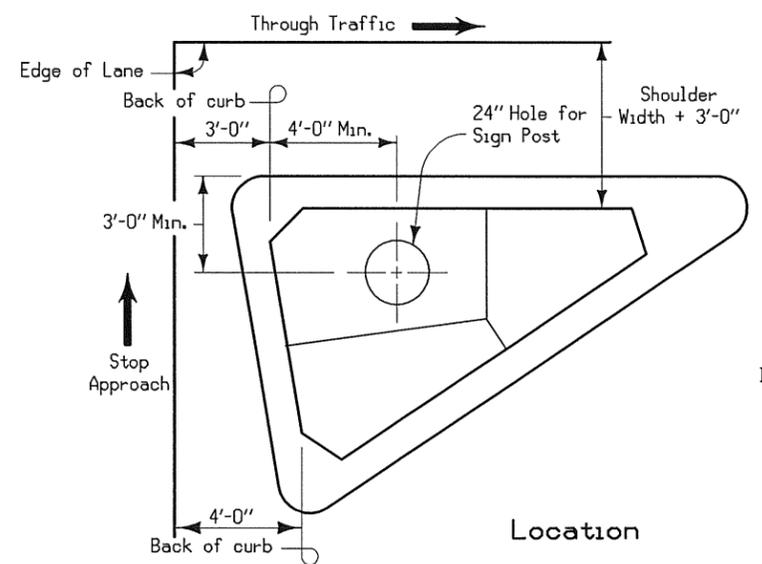


Section A-A With Previous Fillet



Section A-A With Previous Fillet And Resurfacing Less than 1 1/2"

GRANULAR SHOULDER CONSTRUCTION THRU ENTRANCES



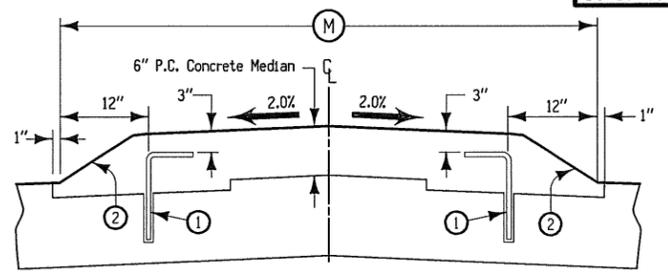
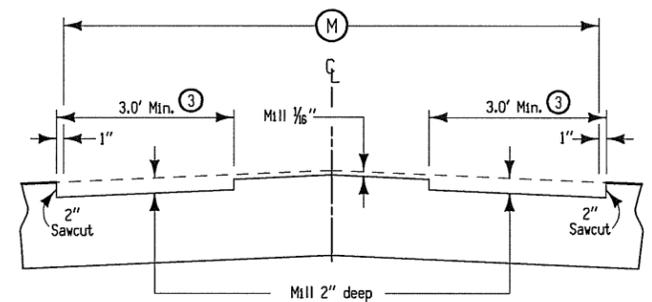
- ① Surface of island shall be shaped as necessary to drain.
- ② Radius point is located at the back of curb line. Contractor shall pave across and between curbs on a straight line as indicated.
- ③ 'E' Joint, see PV-101.
- ④ Construct 'C' Joint In Curb as needed to continue intersection pavement joints. See PV-101.
- ⑤ The furnishing and placing of granular backfill shall be incidental to the price bid for Portland Cement Concrete Median.
- ⑥ 'C' Joints as required. See PV-101.

Location

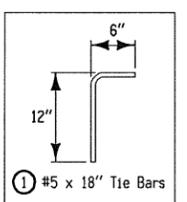
Jointing

Section A-A

TRAFFIC ISLAND WITH SLOPED CURB



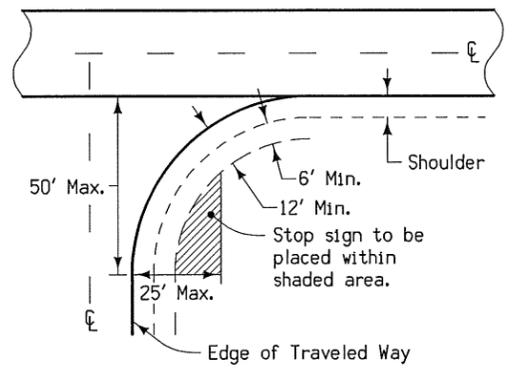
Details of Median Placement



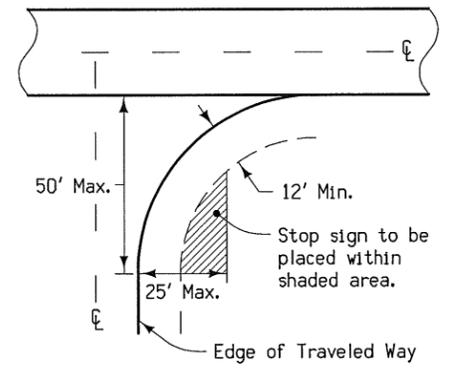
- ② 6" Sloped Curb
- ③ The contractor has the option to mill 2" across the total width of 'M'.

DOWELED MEDIAN WITH SLOPED CURB

Notes:
This section may be appropriately modified in areas specifically designated by the Engineer.
Use 'C' joints in the doveled median and match the location of all transverse and longitudinal joints to the joints in the existing pavement.
Place tie bars at 24" C-C longitudinal spacing between joints in existing pavement. Drill 3/4" holes for tie bars and epoxy to new pavement. See Tabulation 112-5 for additional details. Epoxy material shall be as specified in Materials IM491.11, appendix C.



CASE 'A' WITH SHOULDER



CASE 'B' WITHOUT SHOULDER

NOTES:
Stop signs should be confined to the shaded areas, but as close to the approach roadway as possible to provide the motorist with the best visual impact.
If possible, stop signs should be placed at the point where vehicles are to stop or as near as practical.
In rural areas, the lateral clearance should not be closer than 6' from the edge of a usable shoulder, or if none, 12' from edge of the traveled way.
In urban areas, stop signs should be placed a minimum of 6' from the near edge of the intersected street or a minimum of 4' in advance of the near edge of a marked crosswalk. Lateral clearance may be reduced to a minimum of 2' from the face of a curb.
Where the approach roadway consists of two lanes of traffic, a second stop sign should be placed where it is visible to traffic in the inner lane.
At channelized intersections, the additional stop sign may be placed on a channelized island or median.

STOP SIGN PLACEMENT

SURVEY SYMBOLS

- SIGN SI Sign
- ☐ TDC Tree Deciduous
- * TEV Evergreen Tree
- D Centerline Draw or Stream (Down)
- BLD Building or Foundation
- ⊕ PPA Power Pole Co. 1
- EP Edge of Paved Roads (ML or SR)
- ENP Edge Paved Entrance & Park Lot
- SWK Sidewalk
- - - ENU Edge Unpaved Entrance & Parking
- ⊕ LP L.P. Tank
- FWD Wood Fence
- # FCL Chain Link and Security Fence
- ⊕ LUM Luminaire
- ⊕ MH Utility Access (Manhole)
- TER Terrace
- EW Edge of Water
- SH Paved Shoulder
- CUL Culvert
- ⊕ FLG FLG Flag Poles
- ⊕ SHR Shrub
- RET Retaining Walls
- - - SNP Unpaved Shoulder
- - - BNK Stream Bank
- SIGN SI Sign
- - - EG Edge of Gravel Road
- SIGN SI Sign
- DIK Centerline of Dike or Dam
- BRG Bridge
- GDL Guard Rail Steel
- HDG Hedge Row
- ⊕ WM Wind Mill
- ⊕ BIN Grain Bin
- ⊕ TV Satellite TV Dish
- ⊕ FHD Fire Hydrants
- RIP Rip-Rap

UTILITY LEGEND

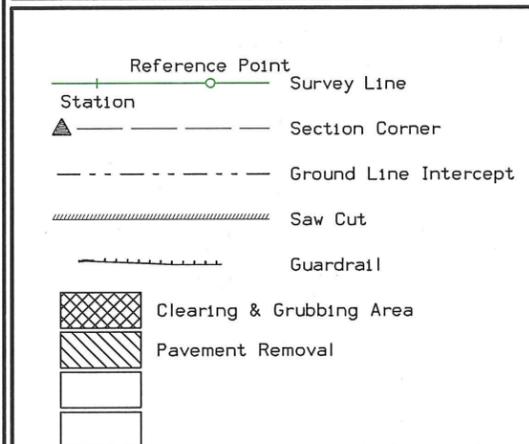
- ⊕ MidAmerican Energy
- ⊕ Mid American(A)
- St.S. — City of Holstein
- San. — City of Holstein
- T1 — Qwest Local Network
- T2 — Frontier
- T3 — Frontier formerly Western Iowa Telephone
- T4 — Schaller Telephone
- T5 — Knology
- W — City of Holstein
- G — MidAmerican Energy
- G2 — Kaneb Pipeline Co.(B)
- E2 — Northwest REC
- E3 — Iowa D.O.T.
- F0 — INS
- F02 — Qwest
- F04 — Mcleod
- F05 — Knology
- F06 — Prairie Wave formerly Knology
- F07 — ICN
- F08 — Schaller Telephone

PLAN VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

| LINEWORK | Design Color No. | Description |
|--------------------|--|---|
| Green | (2) | Existing Topographic Features and Labels |
| Blue | (1) | Proposed Alignment, Stationing, Tic Marks, and Alignment Annotation |
| Magenta | (5) | Existing Utilities |
| SHADING | | |
| Design Color No. | Description | |
| Yellow (4) | Highlight for Critical Notes or Features | |
| Red (3) | Delineates Restricted Areas | |
| Lavender (9) | Temporary Pavement Shading | |
| Gray, Light (48) | Proposed Pavement Shading | |
| Gray, Med (80) | Proposed Granular Shading | |
| Gray, Dark (112) | Proposed Grade and Pave Shading | |
| Brown, Light (236) | Grading Shading | |
| Tan (8) | Proposed Sidewalk Shading | |
| Blue, Light (230) | Proposed Sidewalk Landing Shading | |
| Pink (11) | Proposed Sidewalk Ramp Shading | |

PROFILE VIEW COLOR LEGEND OF PLAN AND PROFILE SHEETS

| LINEWORK | Design Color No. | Description |
|-------------------|------------------|---------------------------------|
| Green | (2) | Existing Ground Line Profile |
| Blue | (1) | Proposed Profile and Annotation |
| Magenta | (5) | Existing Utilities |
| Blue, Light (230) | | Proposed Ditch Grades, Left |
| Black (0) | | Proposed Ditch Grades, Median |
| Rust (14) | | Proposed Ditch Grades, Right |



RIGHT-OF-WAY LEGEND

- ▲ Proposed Right-of-Way
- ▲ Existing and Proposed Right-of-Way
- ▲ Easement and Existing Right-of-Way
- Borrow
- Easement (Temporary)
- ⊕ Easement
- X Excess
- A/C Access Control

PLAN AND PROFILE LEGEND AND SYMBOL INFORMATION SHEET

(COVERS SHEET SERIES D, E, F, & K)

SCS PI Sta 1558+49.03
 $\Delta = 32^\circ 14' 26.39''$ (RT)
 Theta = $4^\circ 29' 59.99''$
 Ls = 300.00
 Ts = 702.52
 Es = 702.52
 P = 80.21
 K = 1.96
 K = 149.97
 Xc = 299.81
 Yc = 7.85
 LT = 200.06
 ST = 100.06
 LC = 299.92

Curve Data
 $\Delta = 23^\circ 14' 26.40''$ (RT)
 T = 392.74
 L = 774.69
 R = 1,909.86
 E = 39.96

Install rumble strip
Review w/ Traffic and Safety

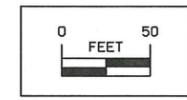
KEDRON TWP.
 T-88N R-43W
 SEC. 1

Sta. 2164+55.28 ML
 = 1564+55.33 SR, 11.97' Lt.
 Begin Project

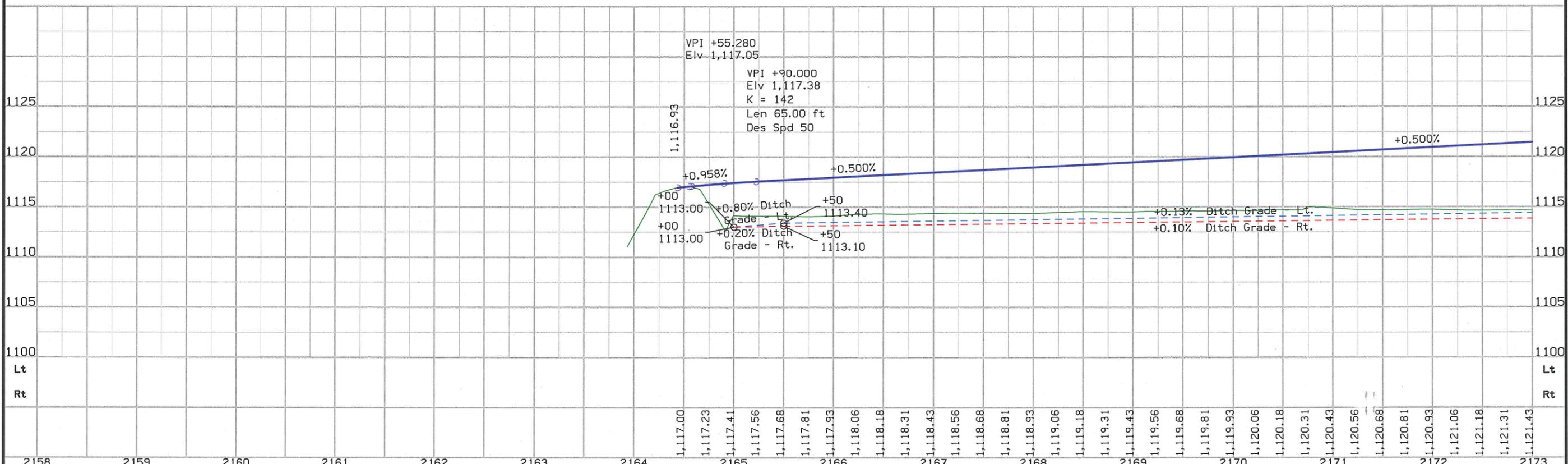


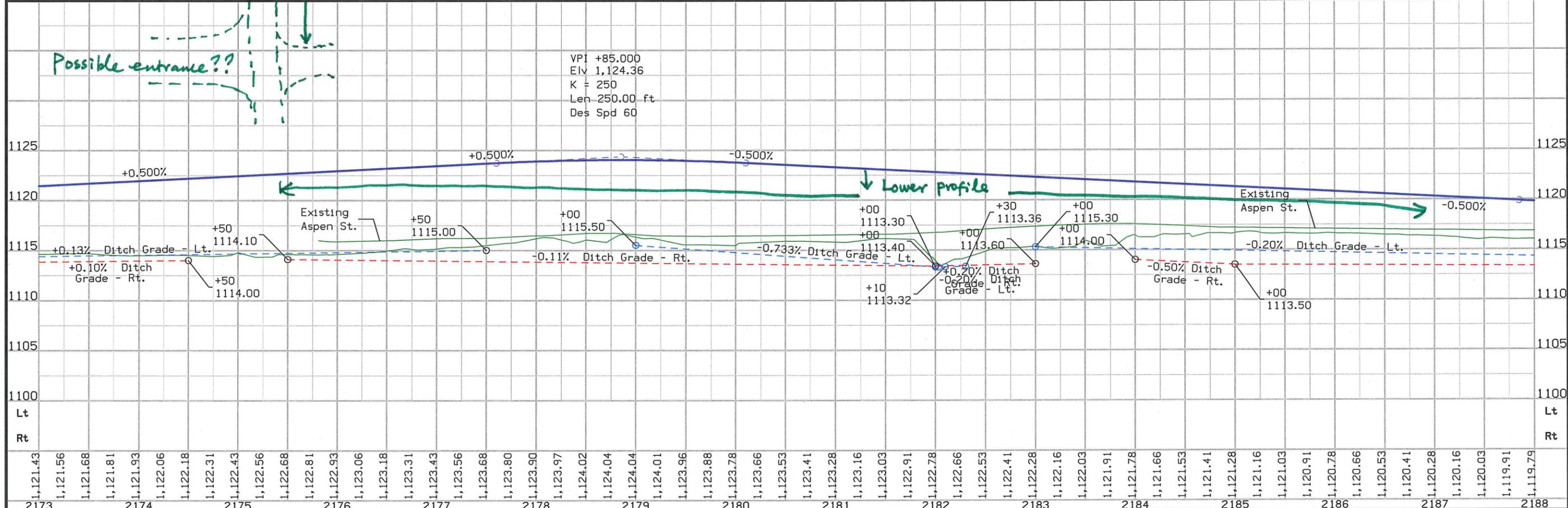
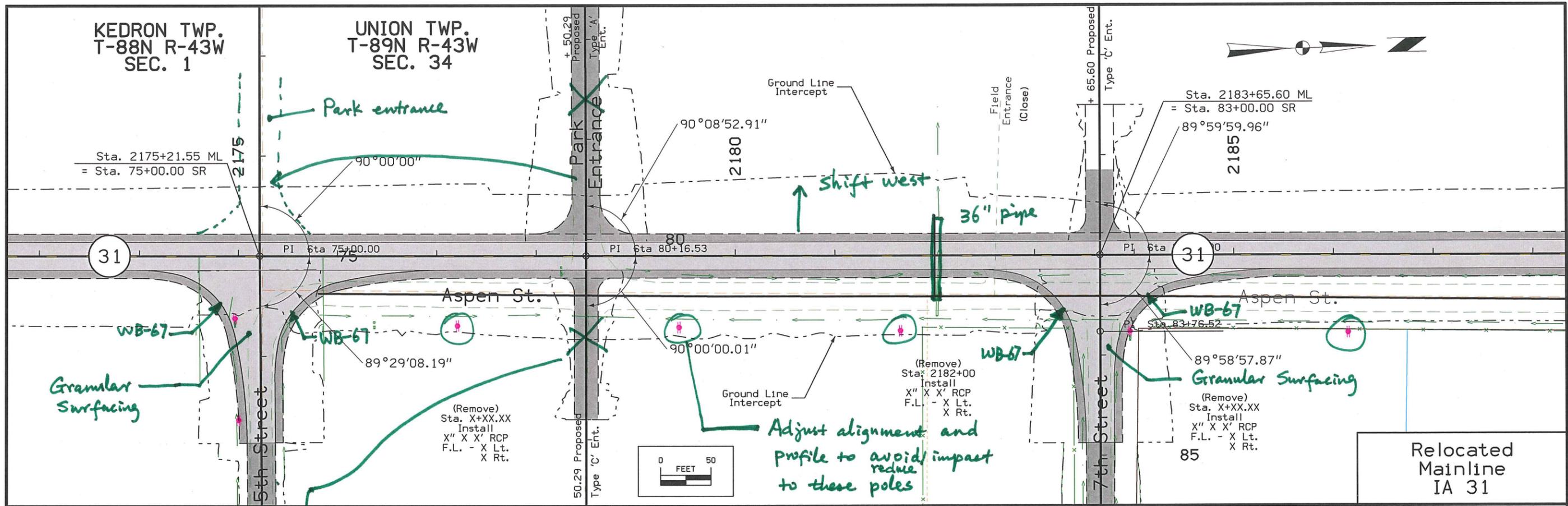
2160

2170

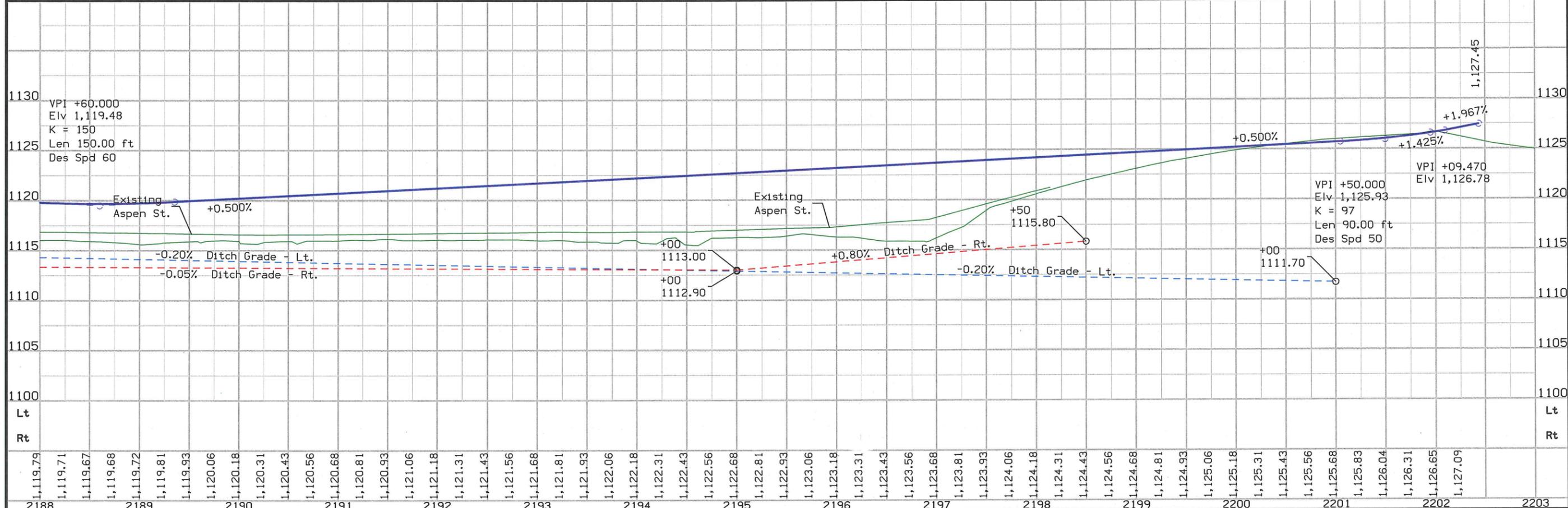
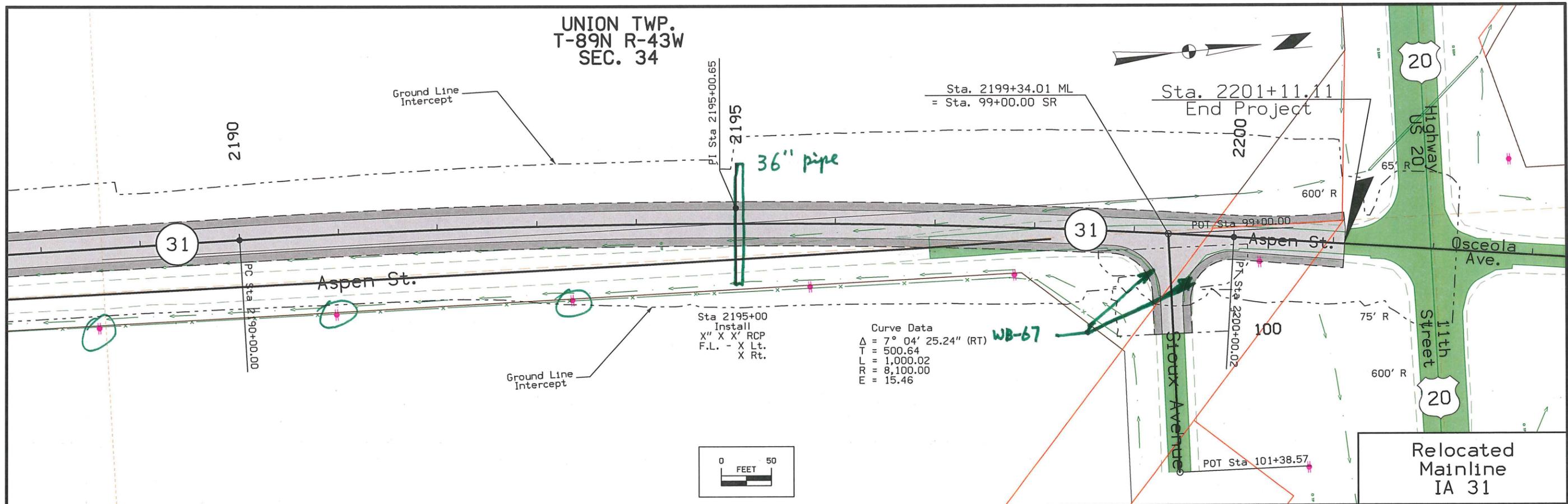


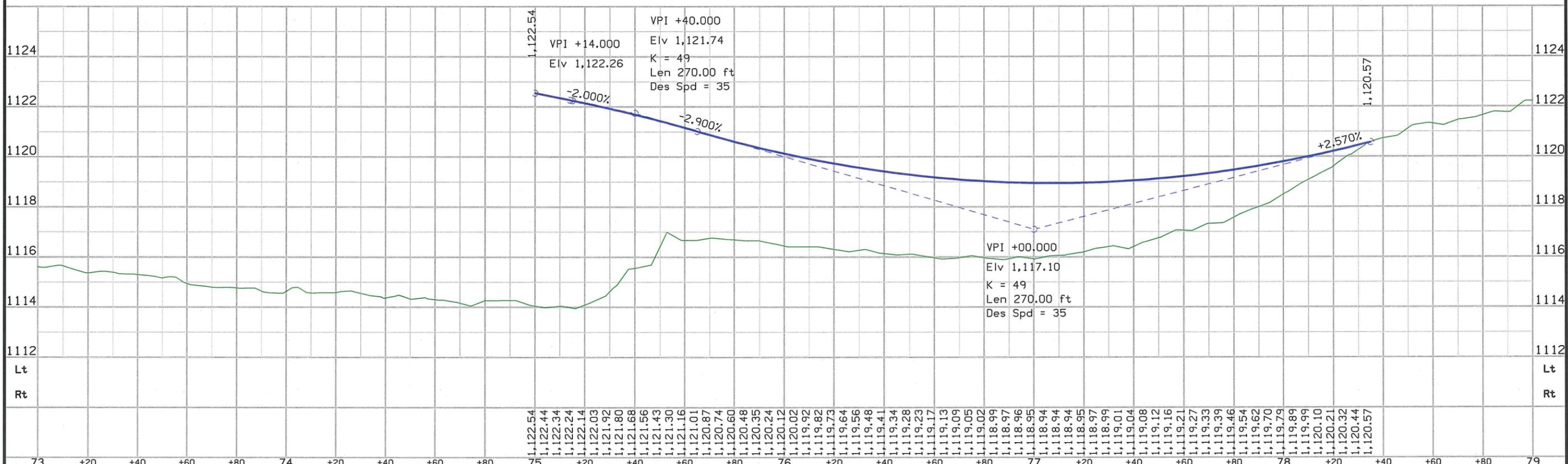
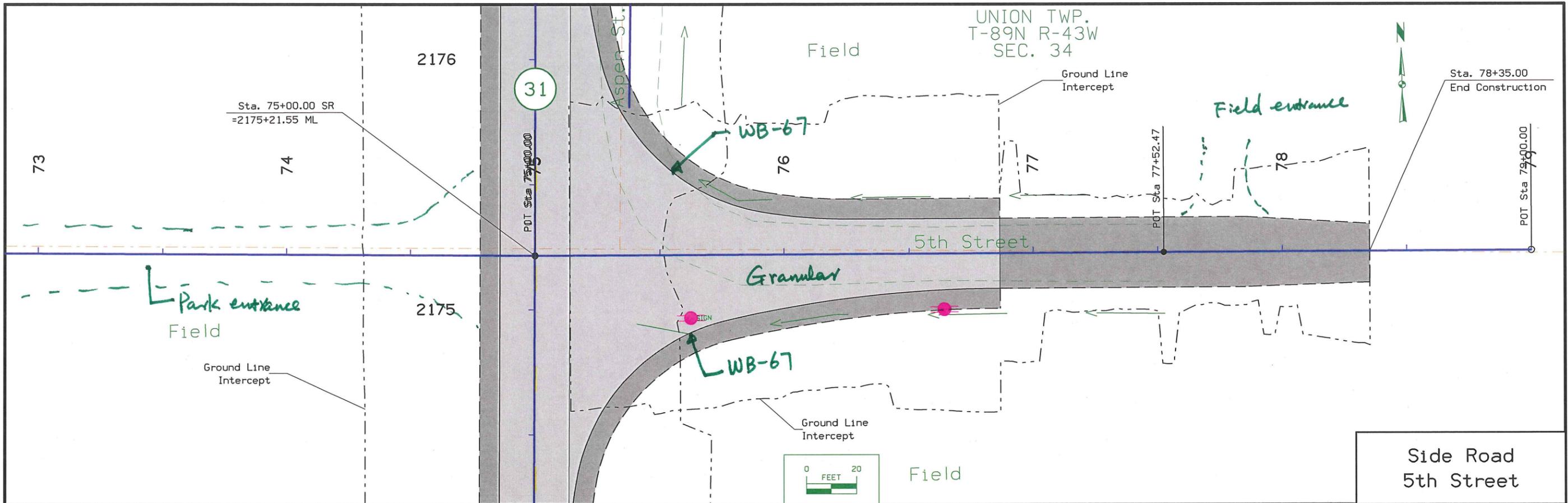
Relocated
 Mainline
 IA 31





UNION TWP.
T-89N R-43W
SEC. 34





UNION TWP.
T-89N R-43W
SEC. 34

PI Sta. 77+00.00 SR
Begin Construction

PI Sta. 80+16.53 SR
=POT Sta. 2178+50.29 ML

PI Sta. 81+80.00 SR
End Construction

31

2179

80

81

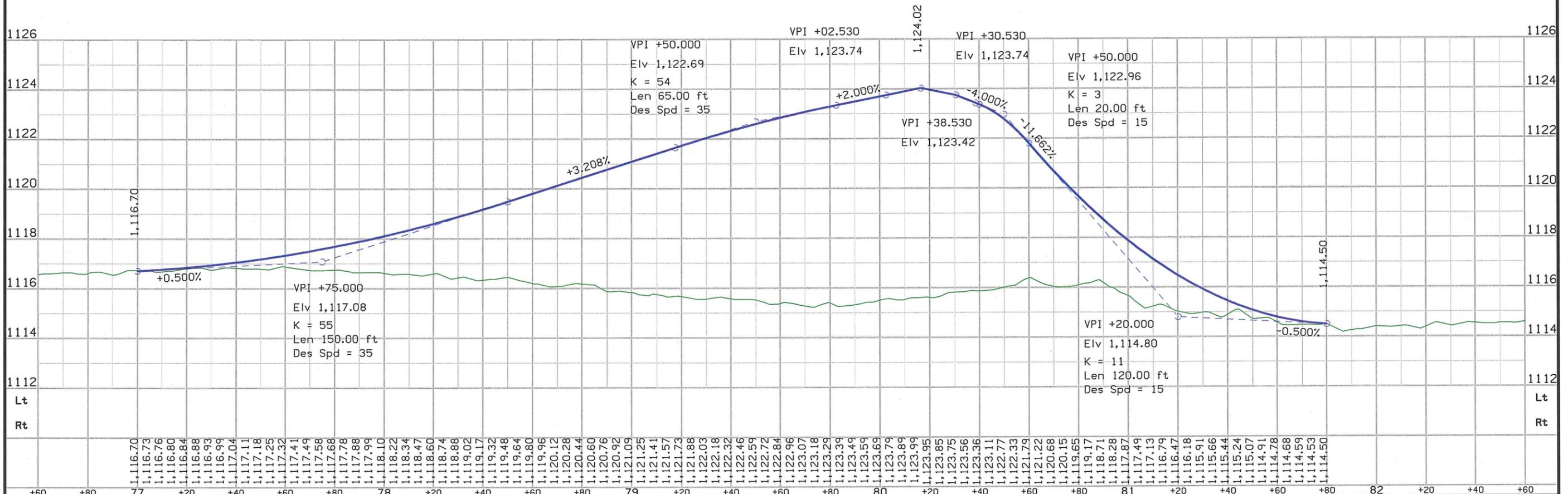
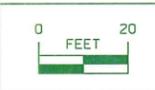
82

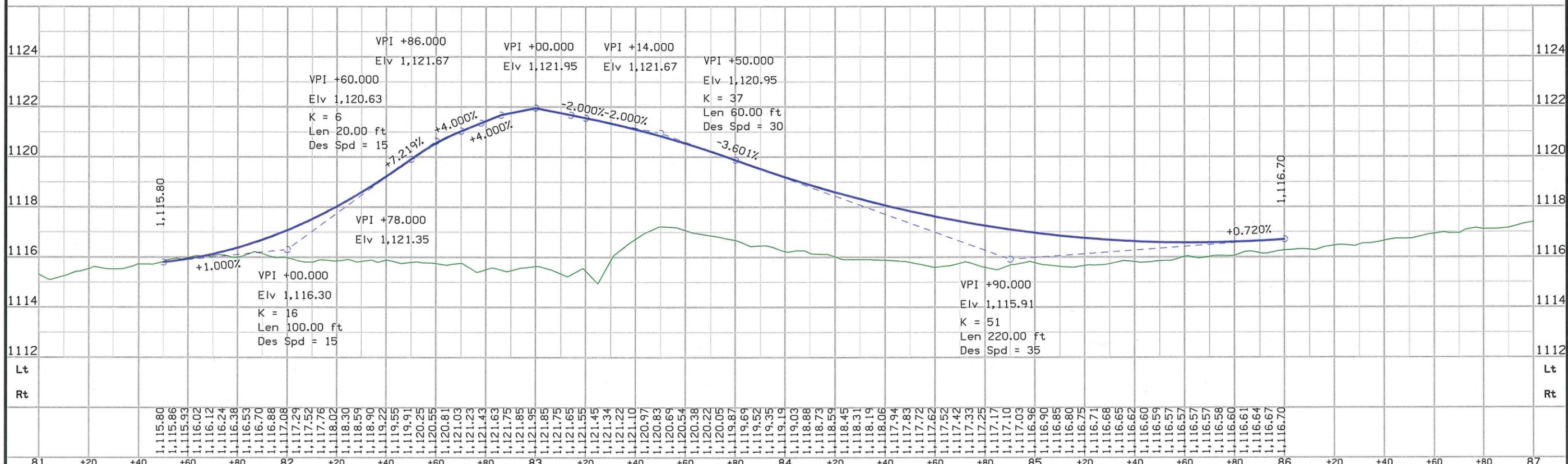
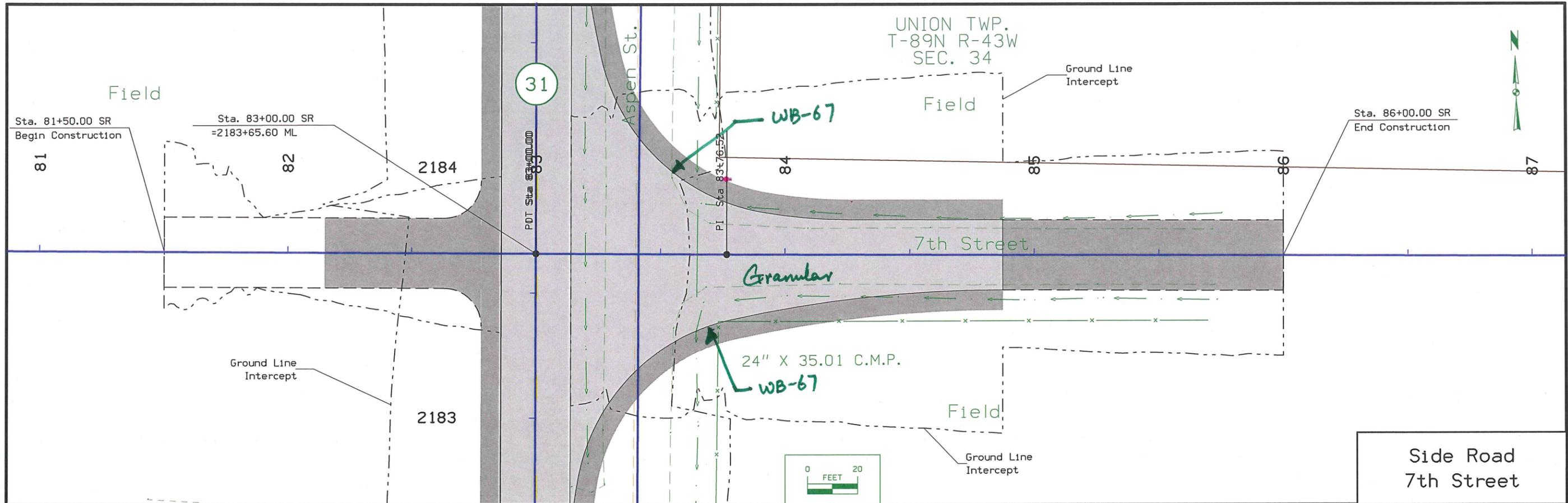
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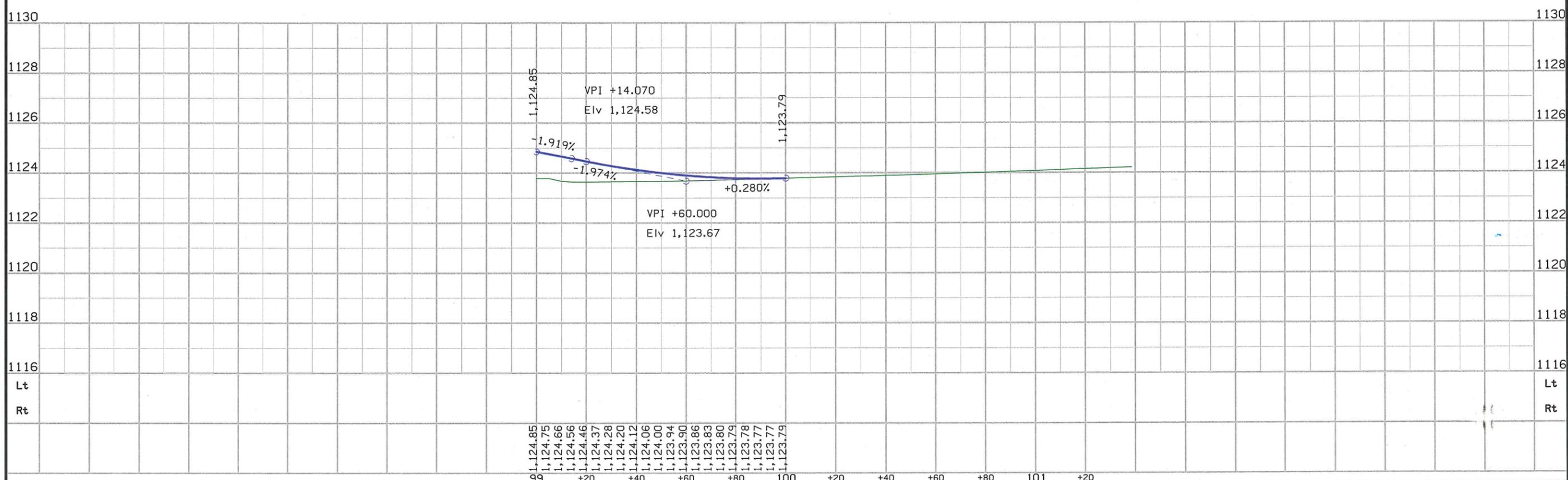
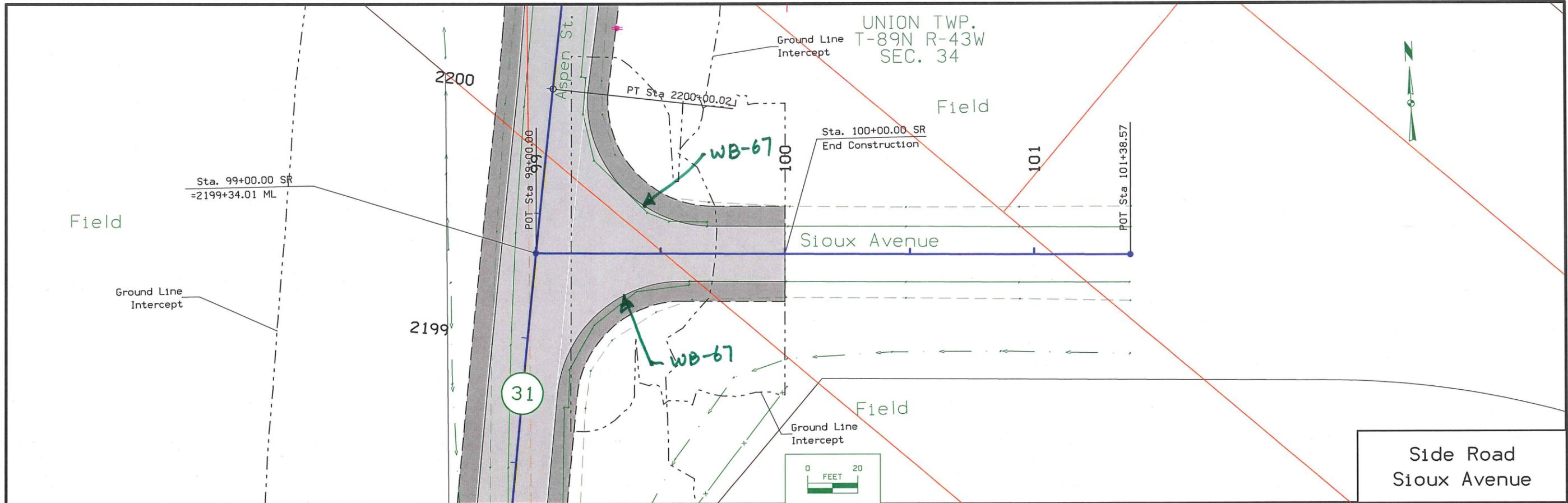
24" X 34.79 C.M.P.

See sheet D.3

Side Road
Park Entrance







Survey Information

General Information

Measurement units for this survey are US survey feet. This survey is for proposed four lane for US 20 between just east of Merville to Early. Project control information is from a 1998 planning study survey (SAP's 130-133). This field survey was to obtain roadway and drainage features and break lines. The entire digital terrain model and topography in the corridor will be completed by supplemental aerial photography.

Vertical Control

Vertical control for this survey is relative to a 1998 digital bench level survey. The 1998 survey is reported to be relative to NAVD88 datum. A level loop was started at BM 997 (USGS BM 29RS) and headed west along US 20 then back to BM 997 (USGS BM 29RS) where adjustments were made. Another run was started at BM 508 then continued east to BM 998 (USGS BM G14) where adjustments were made. NAVD88 elevations were determined at USGS BM 29RS and USGS BM G 14 by converting published NGVD29 elevations using transformation software. The unadjusted bench level run from BM 29RS had a misclosure at BM G 14 of 0.31'. This misclosure is verified by 1998 GPS observations. Even though the misclosure between the two marks is relatively high the error was distributed proportionately along the route.

Vertical equations are as follows:

Datum Benchmark
BM # 997 this survey = (USGS BM 29RS) Elevation = 1378.29 NAVD
1988
= NGVD29 Elevation 1377.761

Datum Benchmark
BM # 998 this survey = (USGS BM G14) Elevation = 1143.26 NAVD
1988
= NGVD29 Elevation 1142.816

Wolf Creek Bridge West Bridge Seat this survey Elevation = 1287.28
= Wolf Creek Bridge West Bridge Elevation = 1287.30
Project # FN-201(58)--21-97

Horizontal Control

Horizontal control for this survey is from 1998 GPS network survey as follows:

GENERAL INFORMATION FOR G.P.S. PROJECT :
SAP 130, NHS-20-1(77)--19-97

SAP 131, NHS-20-2(53)--19-47

SAP 132, NHS-20-2(54)--19-47

SAP 133, NHS-20-2(55)--19-81

STATE PLANE COORDINATE ZONE 1401 (IOWA NORTH LAMBERT)

STATE PLANE COORDINATES HELD AT POINT G046

AVERAGE PROJECT LATITUDE = 42 28 31.38335

RESULTING RADIUS = 6364559.741

MEAN PROJECT ELEVATION = 415.000

SEA LEVEL FACTOR = 0.999934799

AVERAGE PROJECT SCALE FACTOR = 0.999950986

COMBINED FACTOR (GRID) = 0.999885789

1 / GRID = 1.000114224

VERTICAL DATUM = NAVD 88 <> HORIZONTAL DATUM = NAD 83

Local Project Plane Coordinate Conversion Equation :

- a. Local Project Coord y = [(State Plane y - hold point y) / grid factor] + hold point y
- b. Local Project Coord x = [(State Plane x - hold point x) / grid factor] + hold point x

Coordinates are metric units. Metric units were converted to US Survey feet units for project coordinates.

NOTE: GPS DERIVED HEIGHTS ARE NOT RELATIVE TO ADJUSTED PROJECT HEIGHTS.

Survey Information

Alignment Information

The horizontal alignment for this survey is a retrace of As-built Plans, Fed. Project # 2, Fed. Project # 2 (15), Fed. Project # 2(19), and Fed. Project # 2(11). Stationing was established at PI Sta. 226+64.5. Stationing was then backed westerly to the BOP and carried forward to the EOP. The followings PI's were set by the District 3 Office.

PI Sta.. 226+64.5 (Fed. Project # 2 (19))
 PI Sta. 274+54.6 (Fed. Project # 2)
 PI Sta. 390+20.4 (Fed. Project # 2)
 PI Sta. 422+35.2 (Fed. Project # 2)
 PI Sta. 491+85.9 (Fed. Project # 2)
 PI Sta. 533+02.9 (Fed. Project # 2 (15))
 PI Sta. 580+47.1 (Fed. Project # 2 (15))
 PI Sta. 690+48.1 (Fed. Project # 2 (15))
 PI Sta. 742+55.0 (Fed. Project # 2 (15))
 PI Sta. 776+98.7 (Fed. Project # 2 (15))
 PC Sta. 882+75.6 (Fed. Project # 2 (11))
 PT Sta. 905+83.1 (Fed. Project # 2 (11))
 PC Sta. 916+76.4 (Fed. Project # 2 (11))
 PT Sta. 940+20.5 (Fed. Project # 2 (11))
 PI Sta. 966+07.1 (Fed. Project # 2 (11))
 PI Sta. 1003+59.8 (Fed. Project # 2 (11))
 PI Sta. 1035+20.3 (Fed. Project # 2 (11))
 PI Sta. 1045+51.7 (Fed. Project # 2 (11))
 PI Sta. 1057+00.1 (Fed. Project # 2 (11))
 PI Sta. 1148+65.5 (Fed. Project # 2 (11))

Equations are as follows:

PI Sta. 226+64.5 This Survey
 = PI Sta. 226+64.5 (Fed. Project # 2 (19))

PI Sta. 274+54.22 this survey
 = PI Sta. 274+54.6 (Fed. Project # 2)

PI Sta. 390+21.71 this survey
 = PI Sta. 390+20.4 (Fed. Project # 2)

PI Sta. 422+36.66 this survey
 = PI Sta. 422+35.2 (Fed. Project # 2)

PI Sta. 491+88.23 this survey
 = PI Sta. 491+85.9 (Fed. Project # 2)

PI Sta. 533+05.21 this survey
 = PI Sta. 533+02.9 (Fed. Project # 2 (15))

PI Sta. 580+49.39 this survey
 = PI Sta. 580+47.1 (Fed. Project # 2 (15))

PI Sta. 690+49.96 this survey
 = PI Sta. 690+48.1 (Fed. Project # 2 (15))

PI Sta. 742+55.60 this survey
 = PI Sta. 742+55.0 (Fed. Project # 2 (15))

PI Sta. 776+98.28 this survey
 = PI Sta. 776+98.7 (Fed. Project # 2 (15))

PC Sta. 882+75.6
 = PC Sta. 882+75.6 (Fed. Project # 2 (11))

PT Sta. 905+83.1
 = PT Sta. 905+83.1 (Fed. Project # 2 (11))

PC Sta. 916+76.4
 = PC Sta. 916+76.4 (Fed. Project # 2 (11))

PT Sta. 940+20.5
 = PT Sta. 940+20.5 (Fed. Project # 2 (11))

PI Sta. 966+07.1
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PI Sta. 1003+59.8
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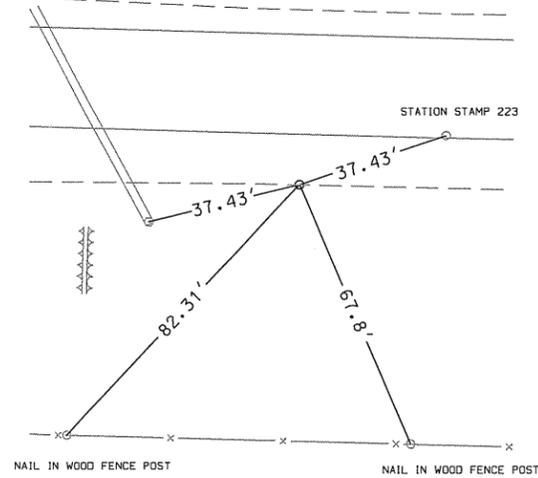
PI Sta. 1035+20.3
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PI Sta. 1045+51.7
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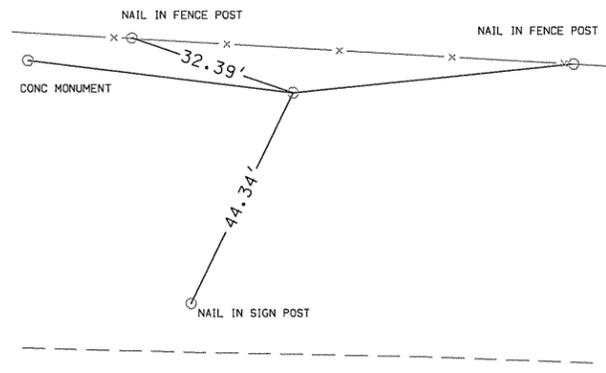
PI Sta. 1057+00.1
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PI Sta. 1148+65.5
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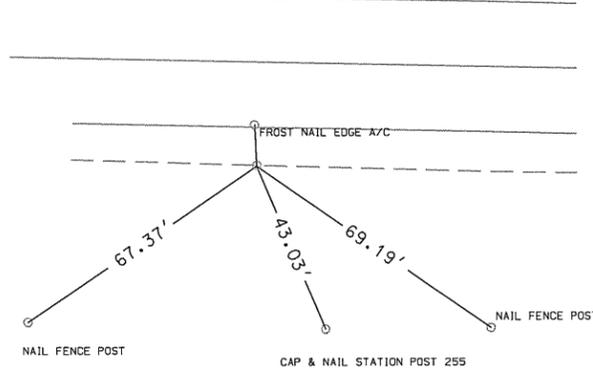
CP STA. 222+64.43, 107.80 Rt.
 CP No. 102, FOUND 5/8 REBAR
 N=3648748.078, E=4243594.245



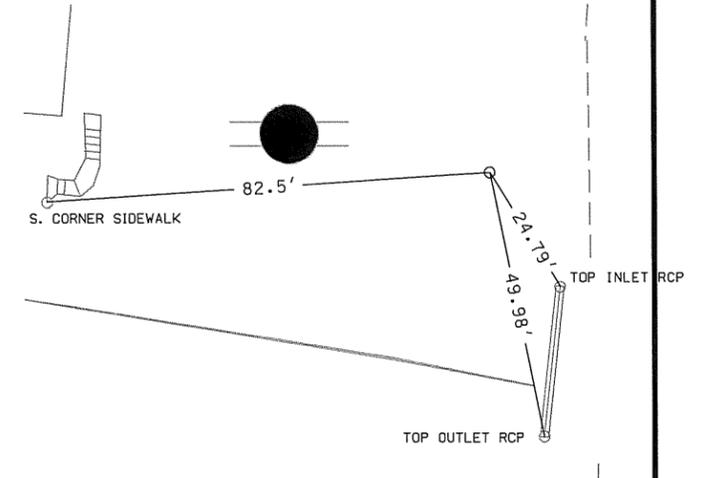
CP STA. 244+71.37, 74.05 Lt.
 CP No. 11 Conc Monument
 N=3648880.543, E=4245805.058



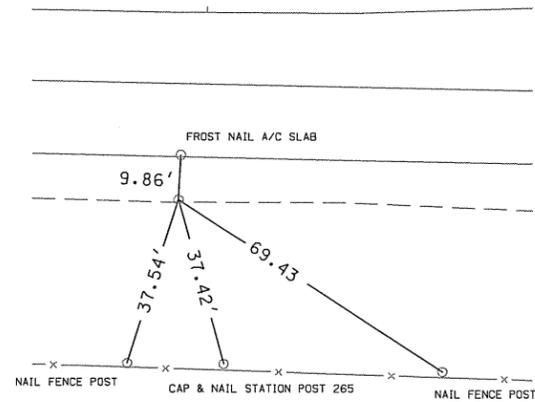
CP STA. 254+81.60, 25.04 Rt.
 CP No. 104 Found Rebar
 N=3648759.776, E=4246812.925



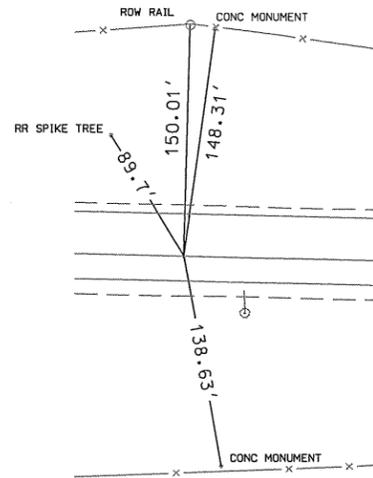
CP STA. 262+61.0, 1341.85 Rt.
 CP No. 997 Found Disk
 N=3647426.521, E=4247564.15



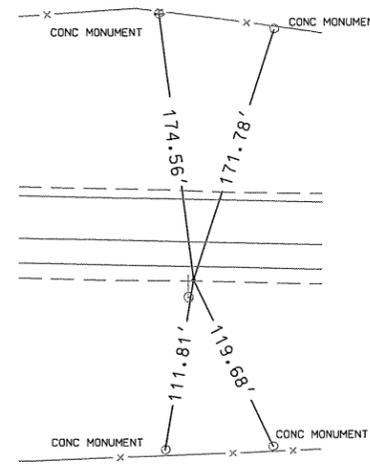
CP STA. 264+89.96, 25.53 Rt.
 CP 105, Found 5/8 Rebar
 N=3648737.623, E=4247821.041



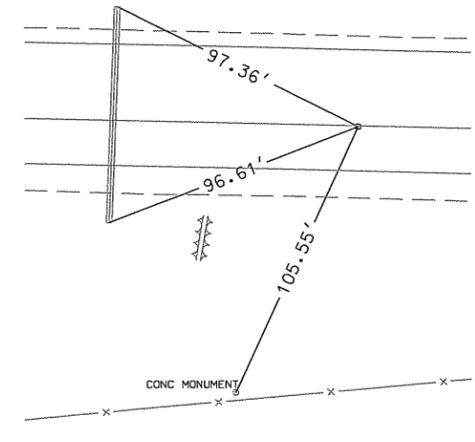
PI STA. 274+54.22
 PI 413, Found PK Nail
 N=3648742.4349, E=4248785.6266



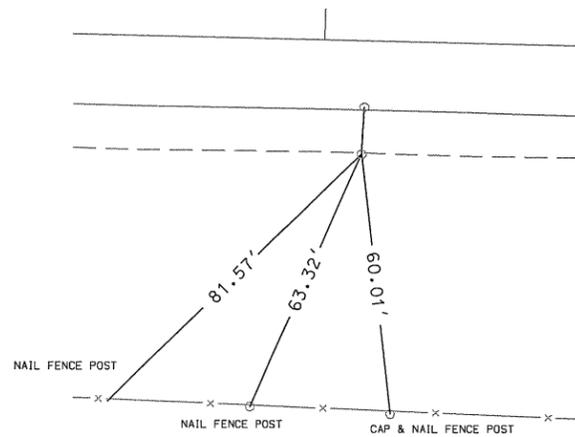
CP STA. 274+98.03, 25.01 Rt.
 CP 12 Found 5/8 Rebar
 N=3648716.2360, E=4248828.8450



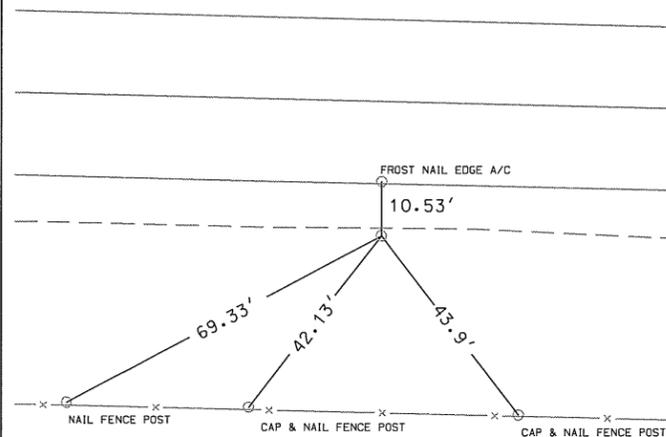
CP STA. 279+99.82, 0.35 Rt.
 CP 403 Found Hinge Nail
 N=3648728.9800, E= 4249331.0580



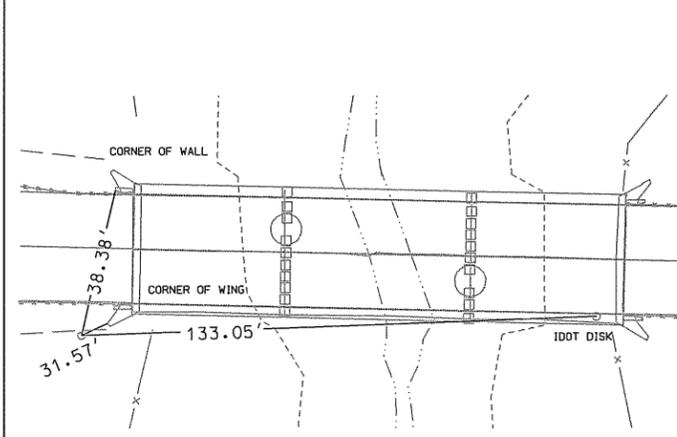
CP STA. 298+09.00, 26.06 Rt.
 CP 107 Found 5/8 Rebar
 N=3648659.8010, E= 4251139.0960



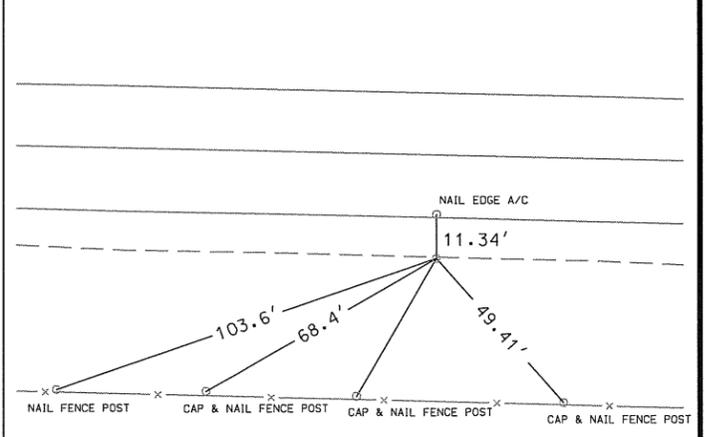
CP STA. 311+15.83, 26.09 Rt.
 CP 108 Found 5/8 Rebar
 N=3648628.3670, E=4252445.5520



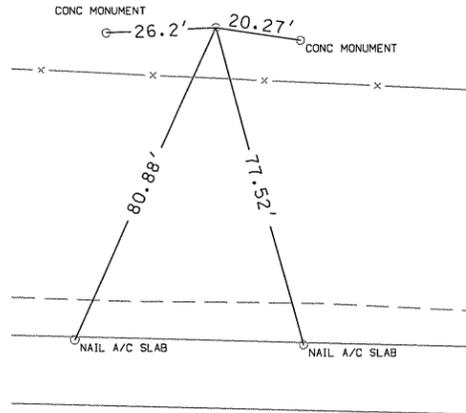
CP STA. 324+23.74, 22.70 Rt.
 CP 13 Found 5/8 Rebar
 N=3648600.3370, E=4253753.1660



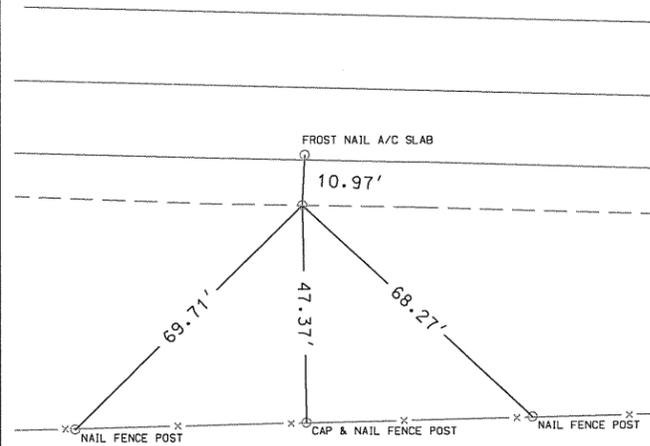
CP STA. 334+65.85, 26.59 Rt.
 CP 109 Found 5/8 Rebar
 N=3648571.4010, E=4254794.8840



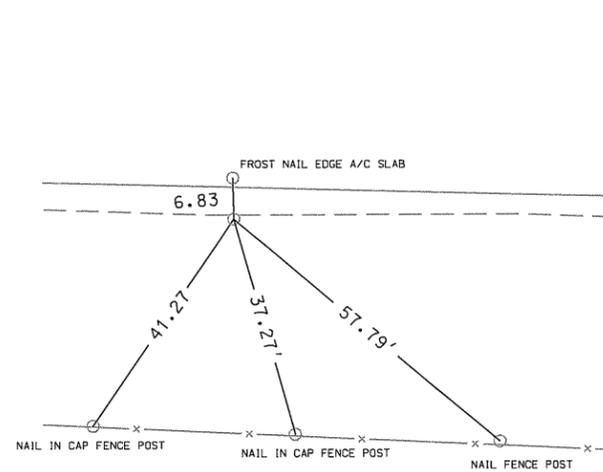
CP STA.345+05.75, 89.60 Lt.
 CP 14 Found Right Of Way Rail
 N=3648662.5780, E= 4255837.2690



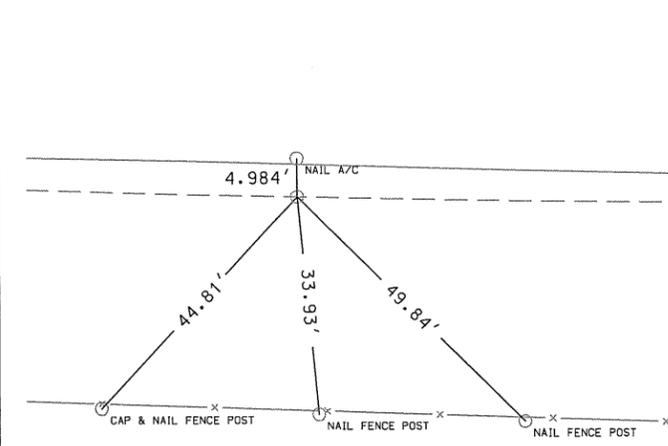
CP STA.356+33.49, 25.61 Rt.
 CP 110 Found 5/8 Rebar
 N=3648520.2950, E=4256961.9160



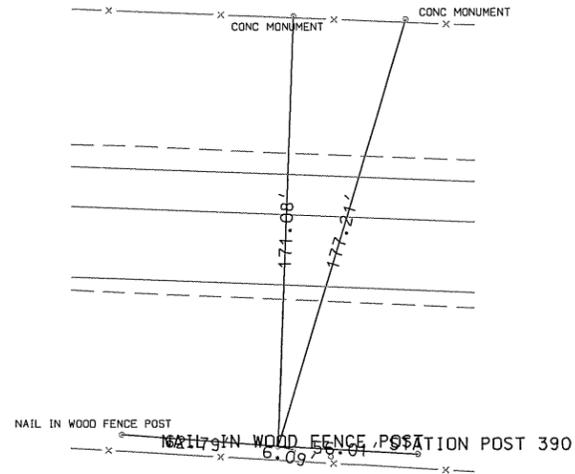
CP STA.367+56.46, 33.28 Rt.
 CP 111 Found 5/8 Rebar
 N=3648485.6490, E=4258084.3820



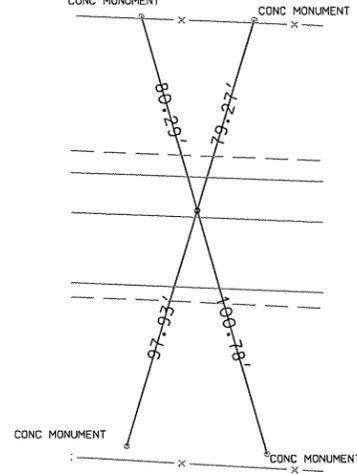
CP STA.378+79.17, 33.02 Rt.
 CP 112 Found 5/8 Rebar
 N=3648458.9350, E=4259206.7690



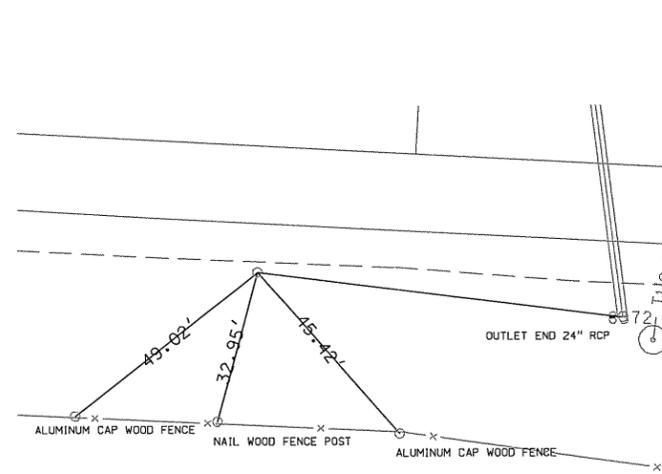
CP STA.389+97.13, 92.18 Rt.
 CP 15 Found 5/8 Rebar
 N=3648370.6890, E=4260321.6200



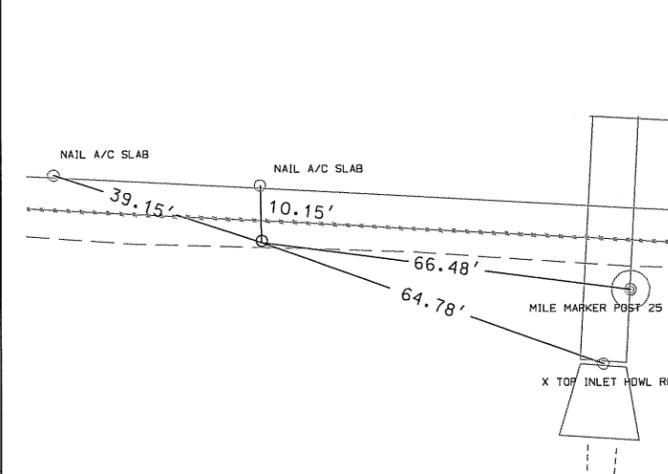
PI STA. 390+21.72
 PI 404 Found PK Nail
 N=3648464.4870, E=4260349.7820



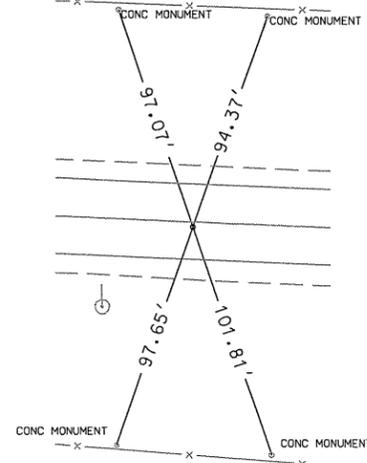
CP STA. 401+67.75 26.68 Rt.
 CP 414 Found 5/8 Rebar
 N=3648375.5989, E=4261492.7266



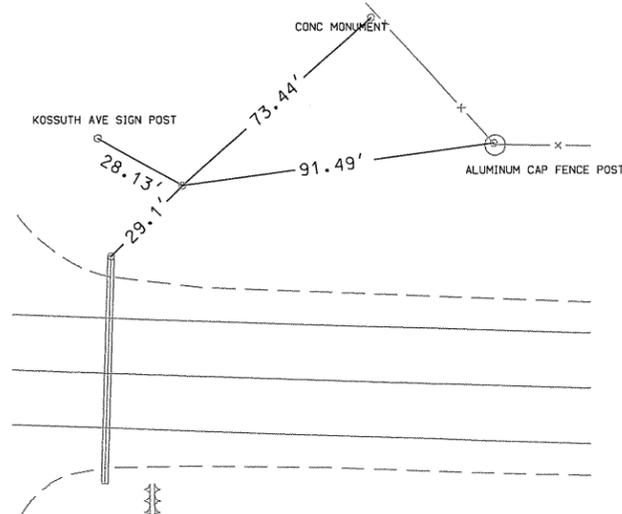
CP STA. 413+39.58, 25.58 Rt.
 CP 415 Found 5/8 Rebar
 N=3648313.0436, E=4262662.8897



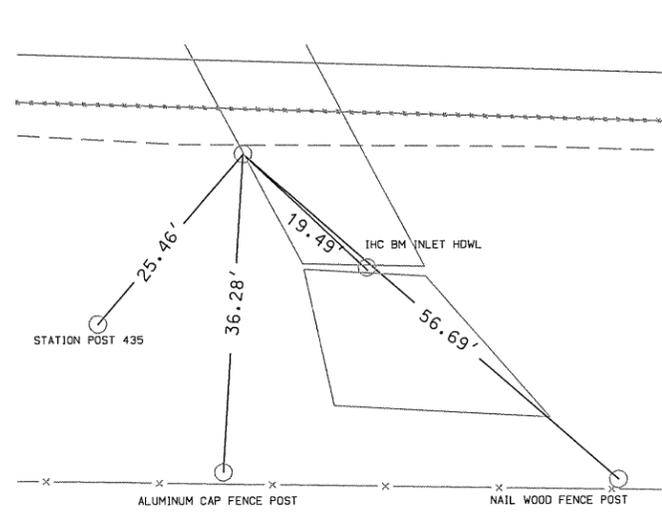
PI STA. 422+36.66
 PI 405 Not Set
 N=3648289.8581, E=4263560.0330



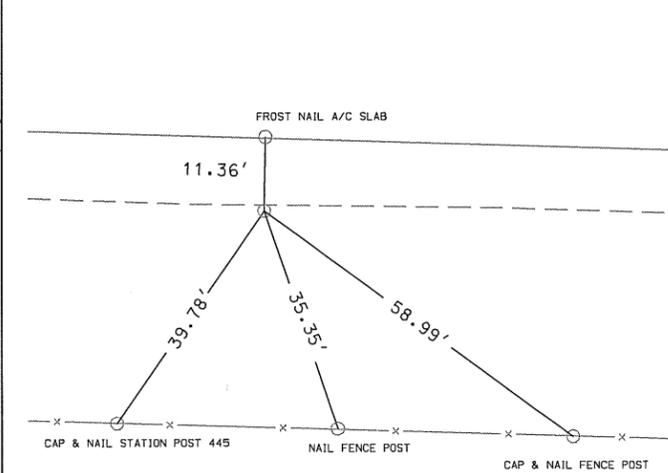
CP STA. 425+10.61, 55.15 Lt.
 CP 16 Found 5/8 Rebar
 N=3648336.9320, E=4263835.6950

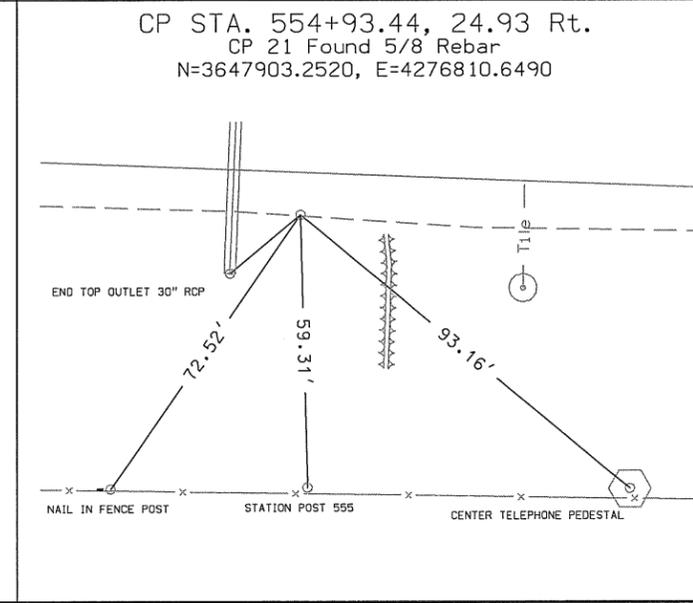
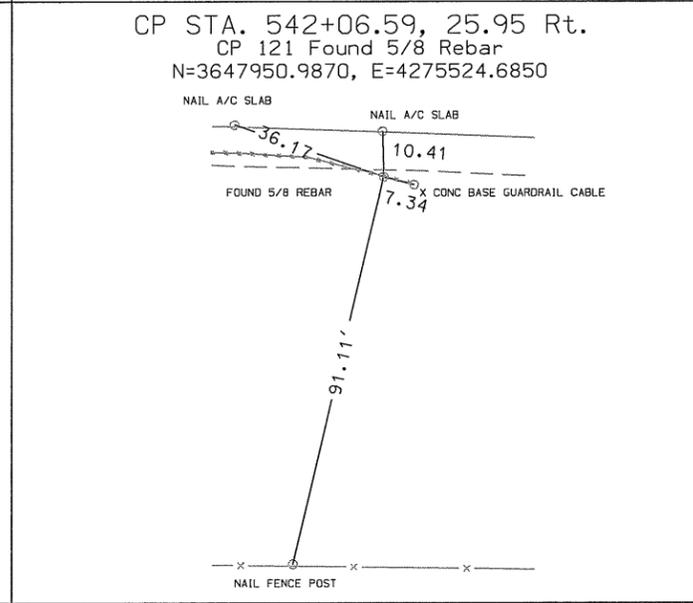
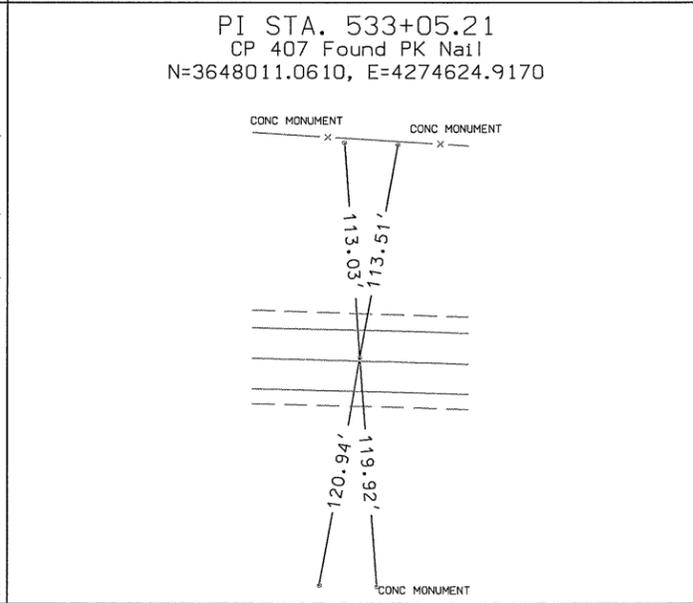
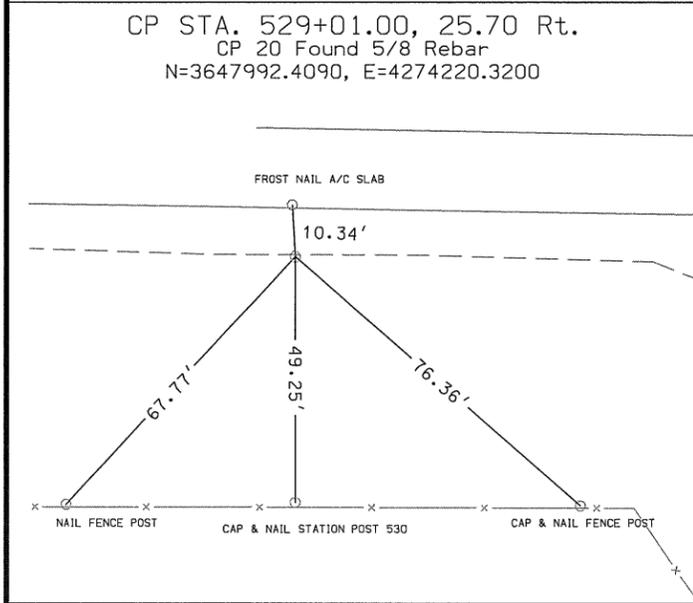
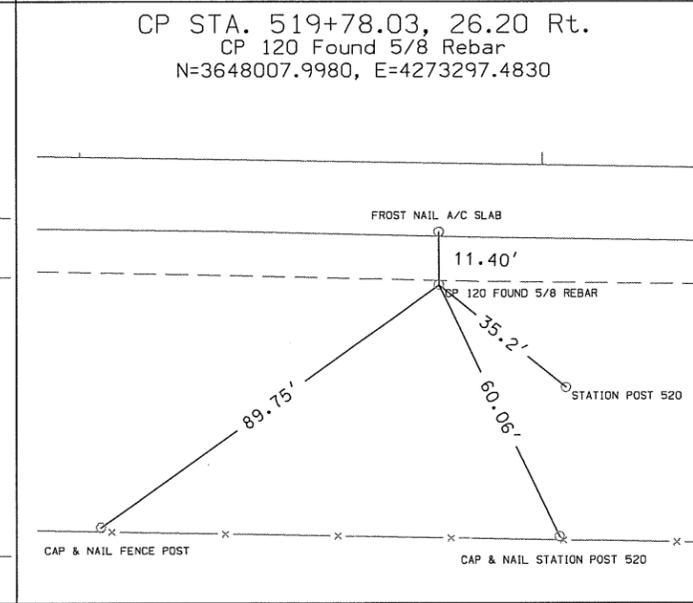
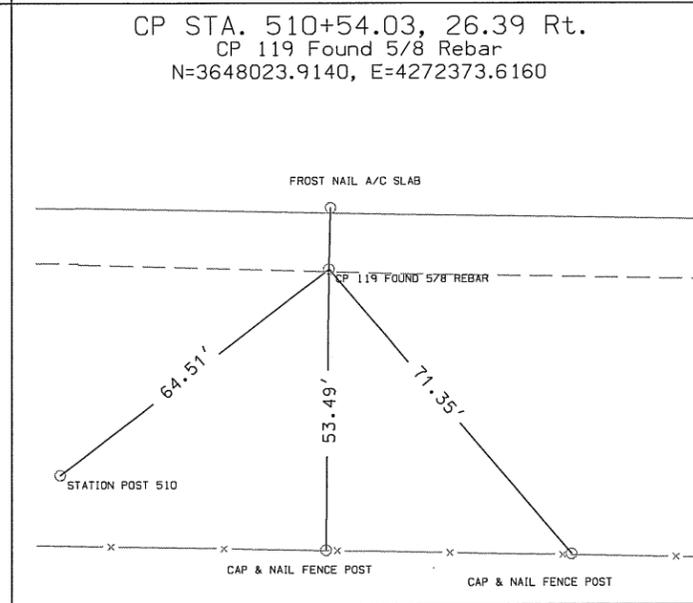
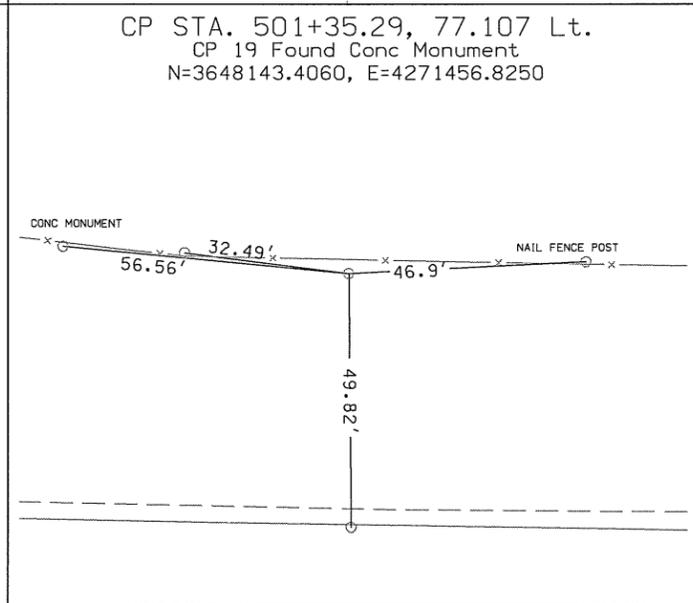
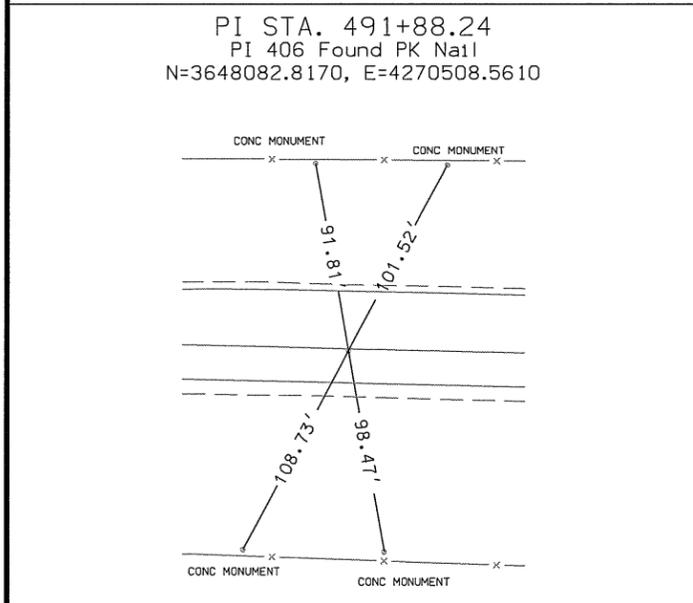
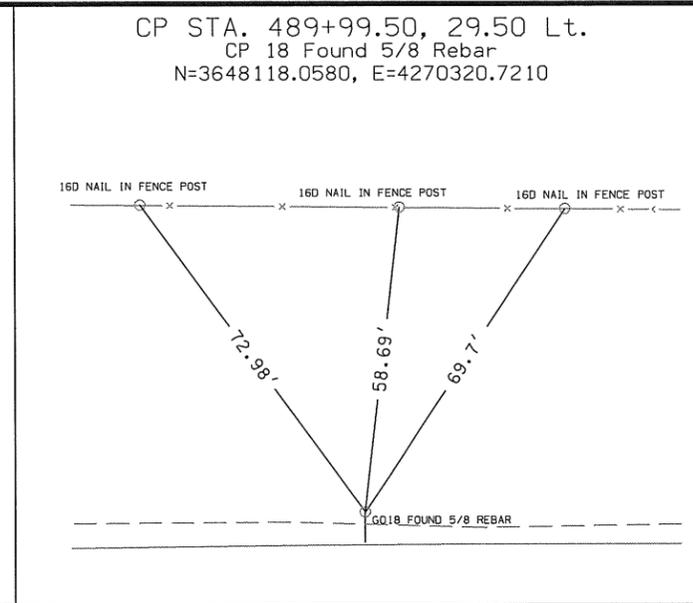
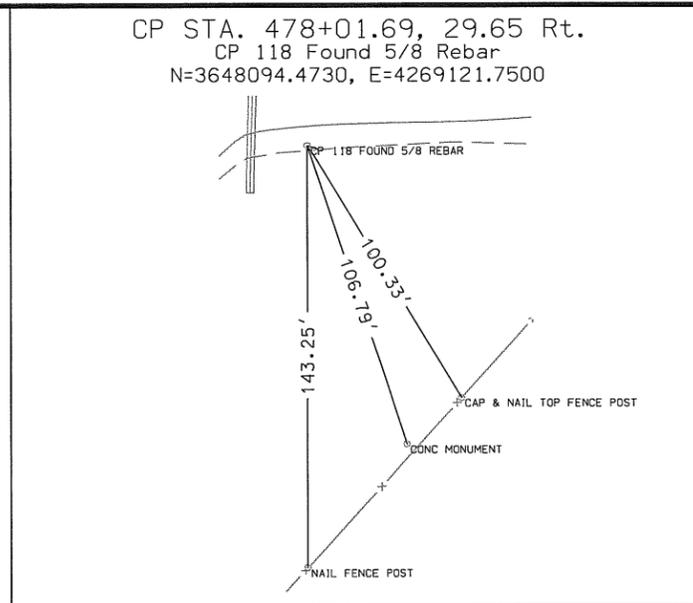
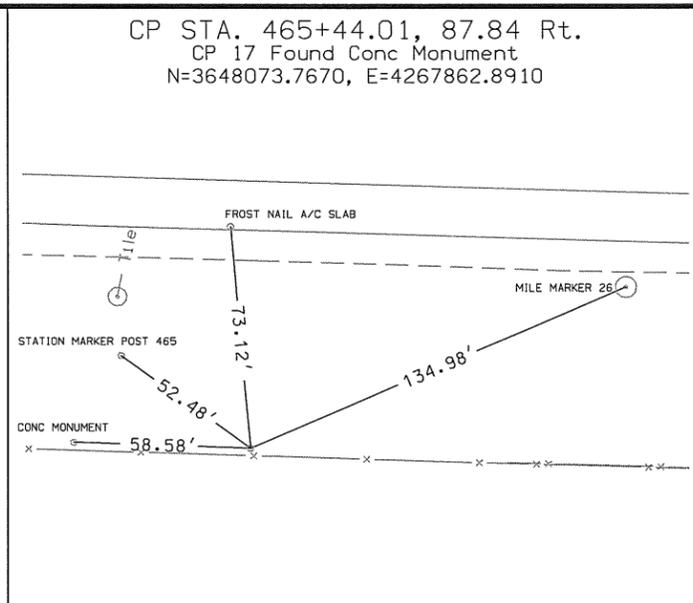
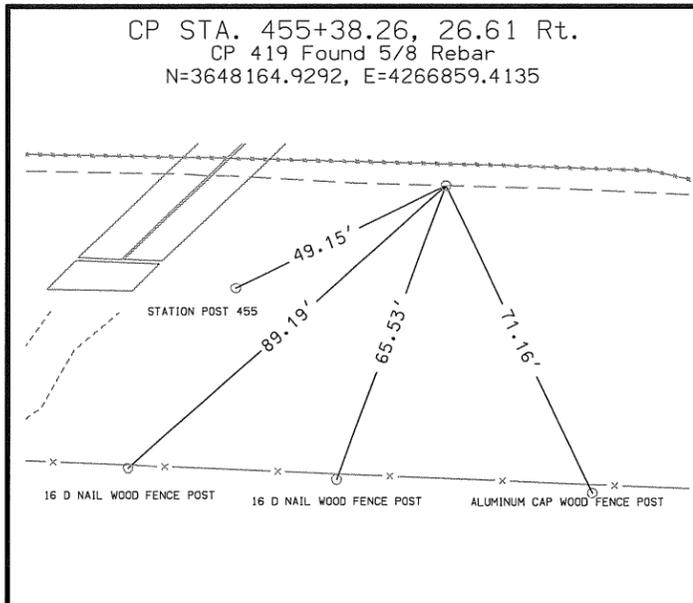


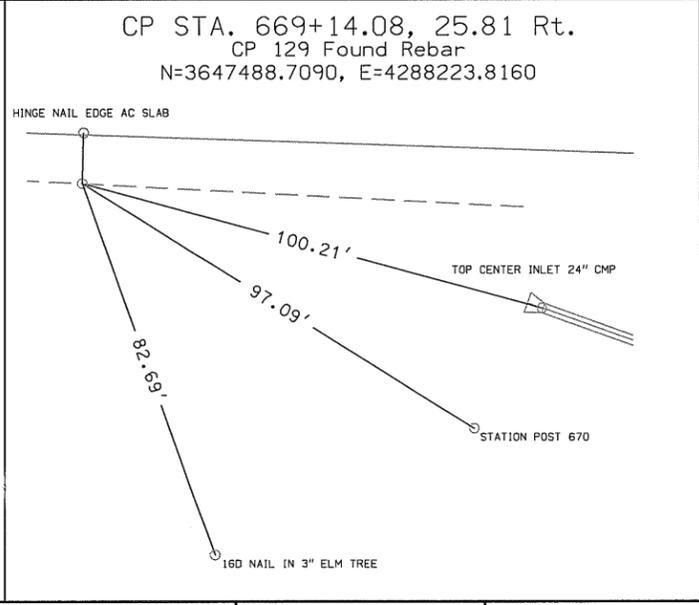
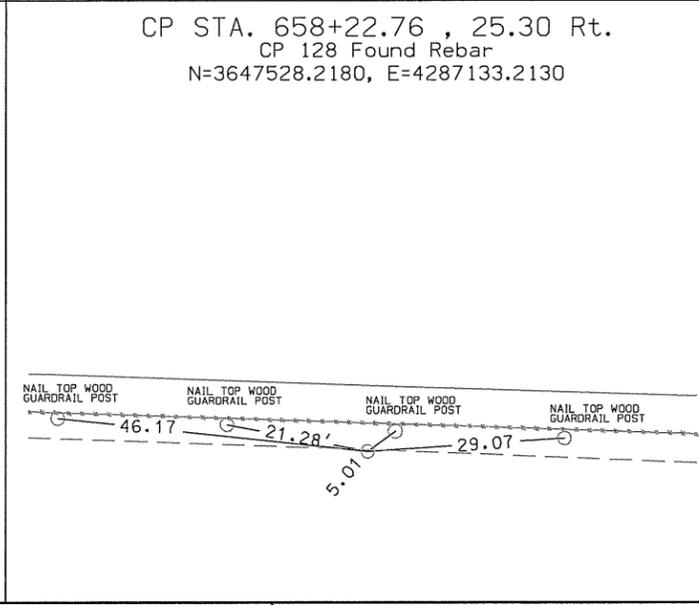
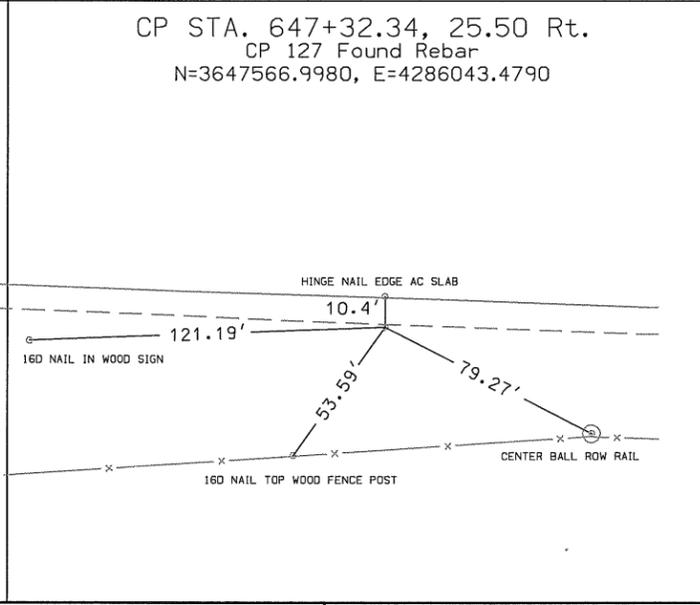
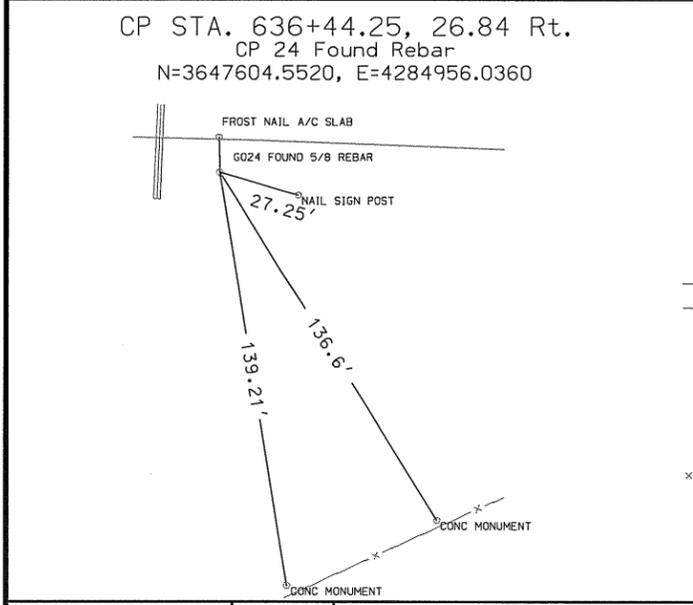
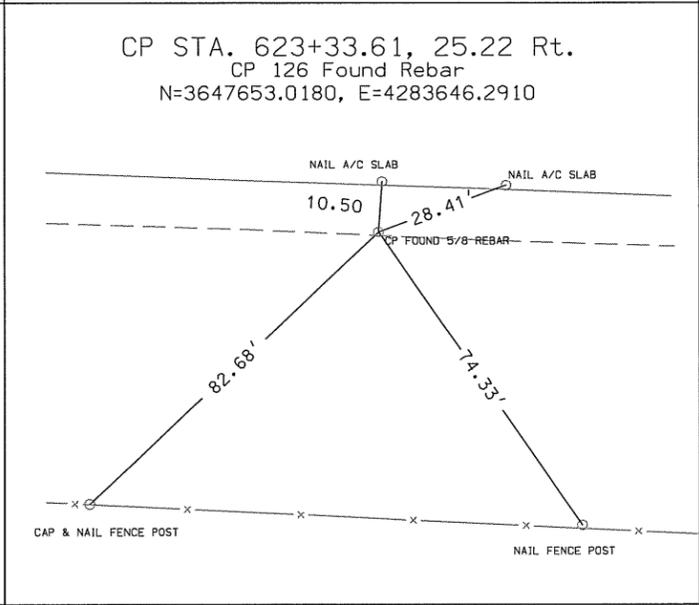
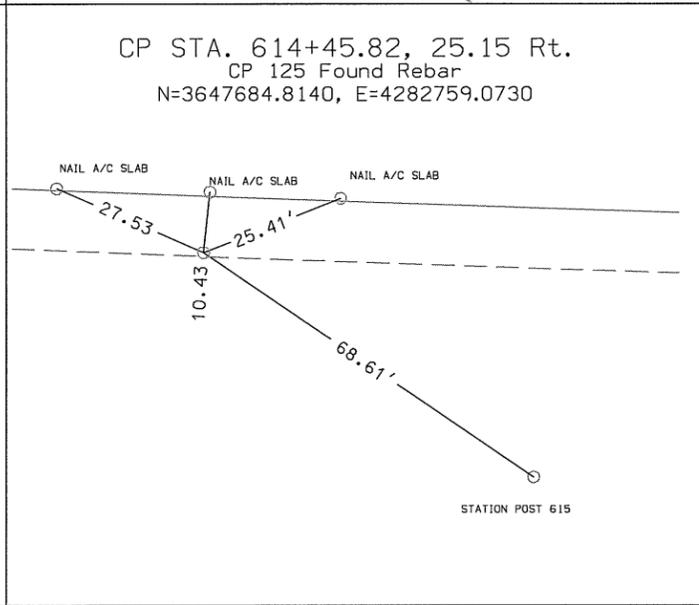
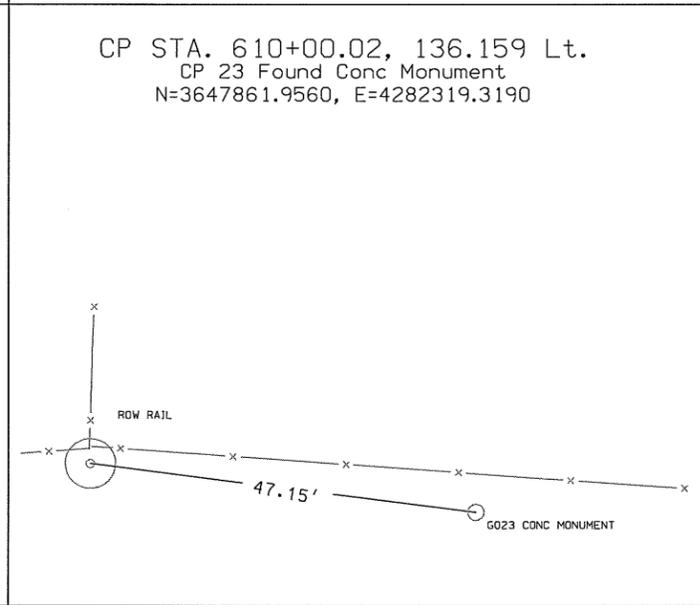
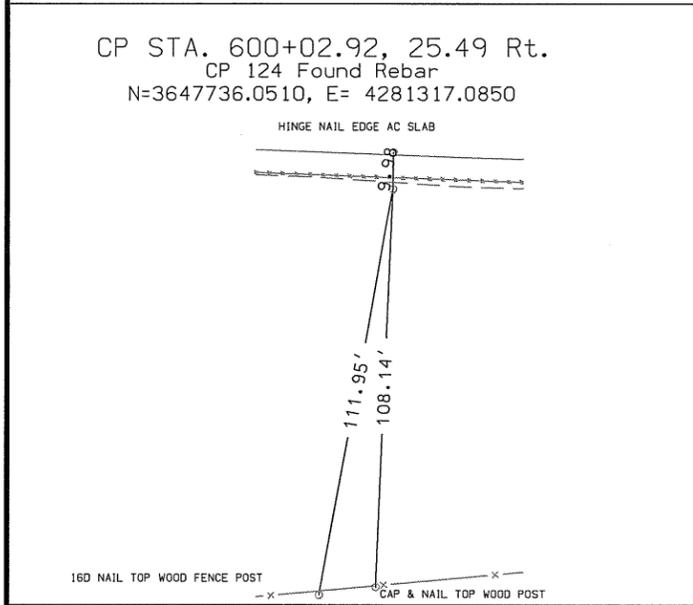
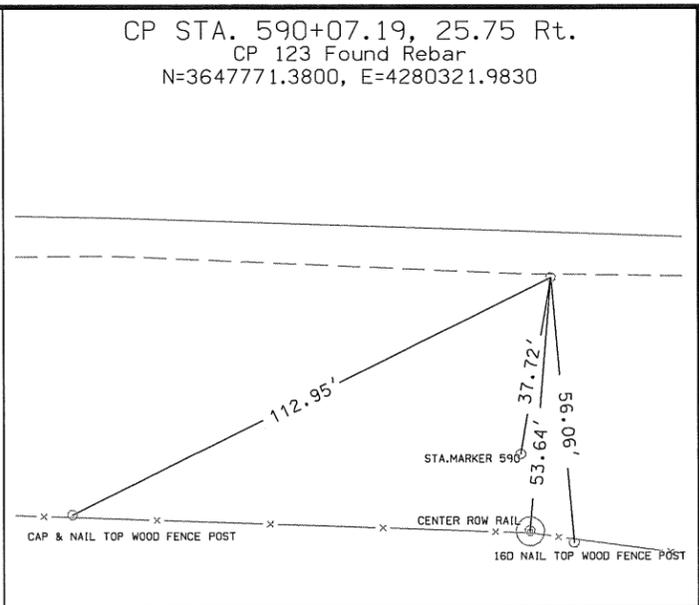
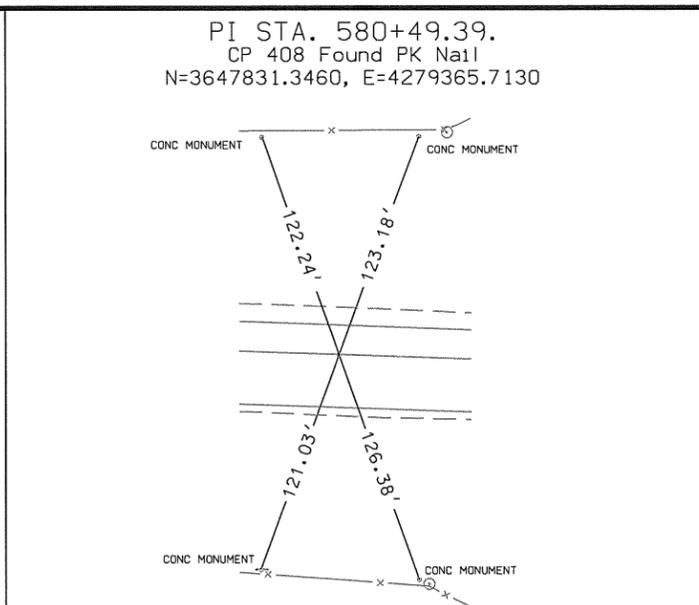
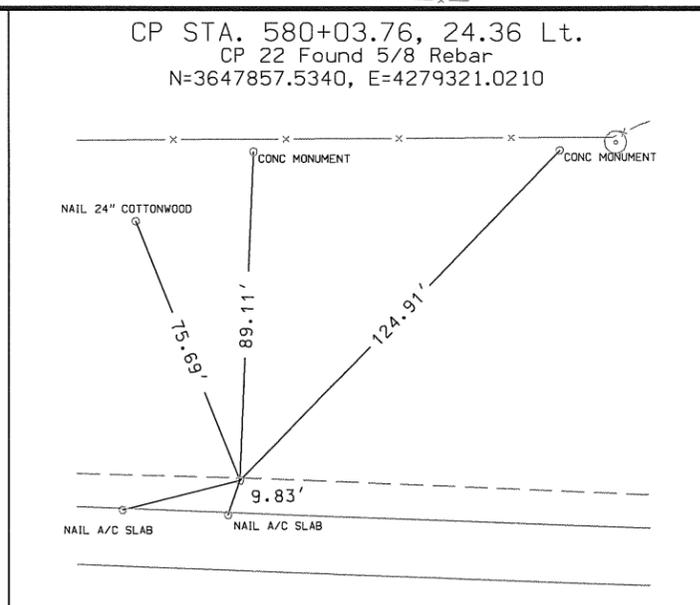
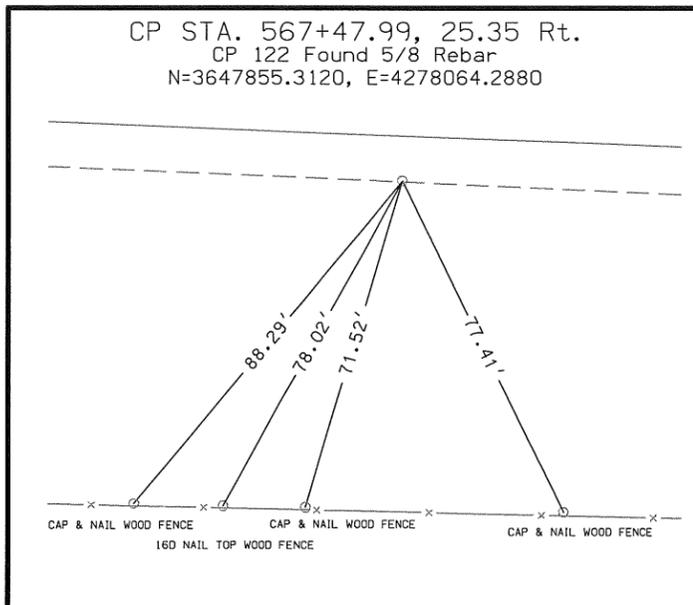
CP STA. 435+18.11, 26.71 Rt.
 CP 417 Found 5/8 Rebar
 N=3648224.9930, E=4264840.1590

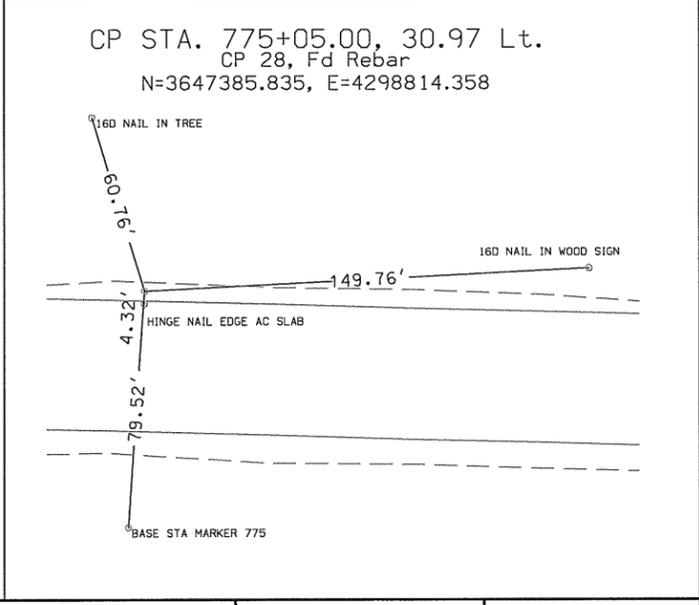
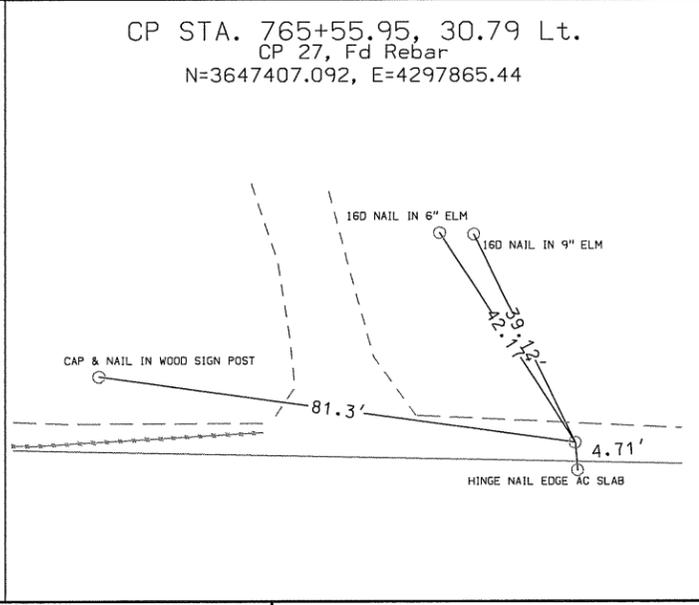
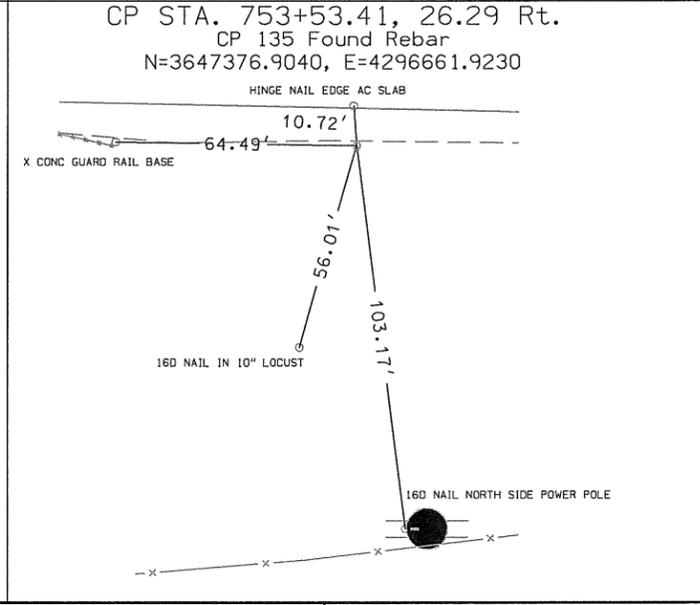
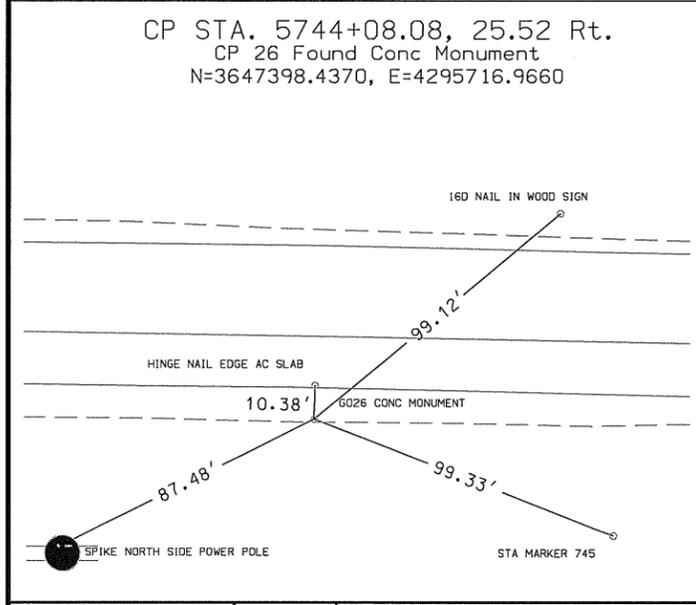
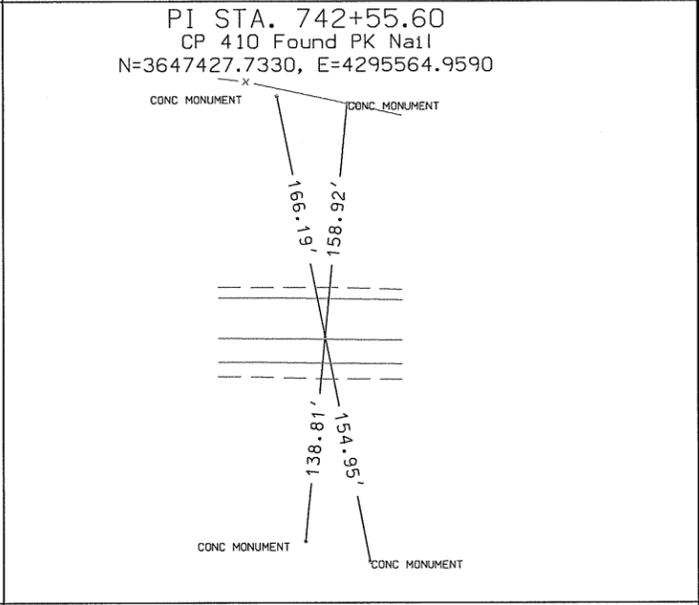
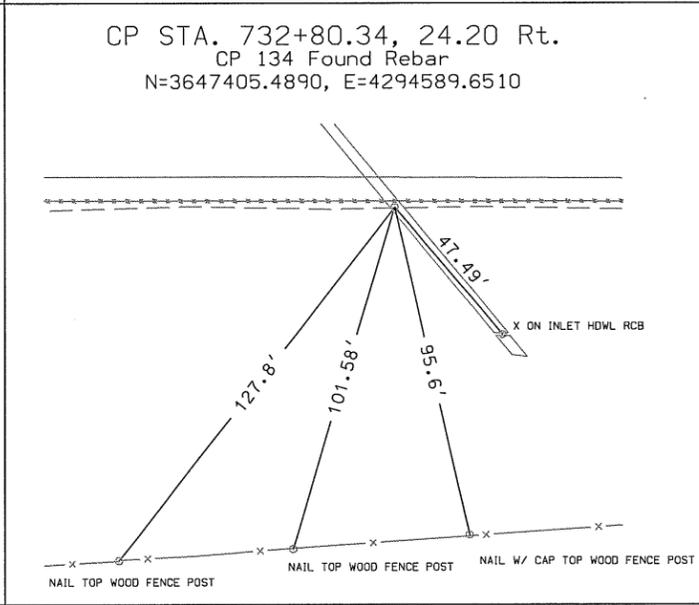
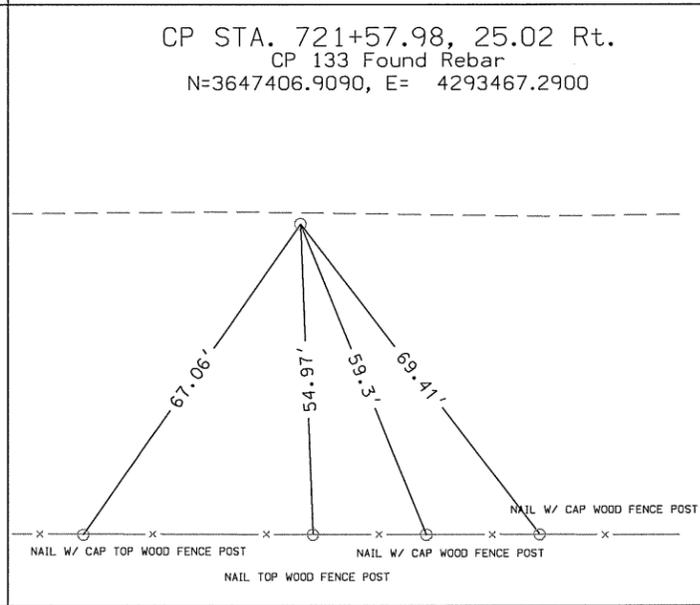
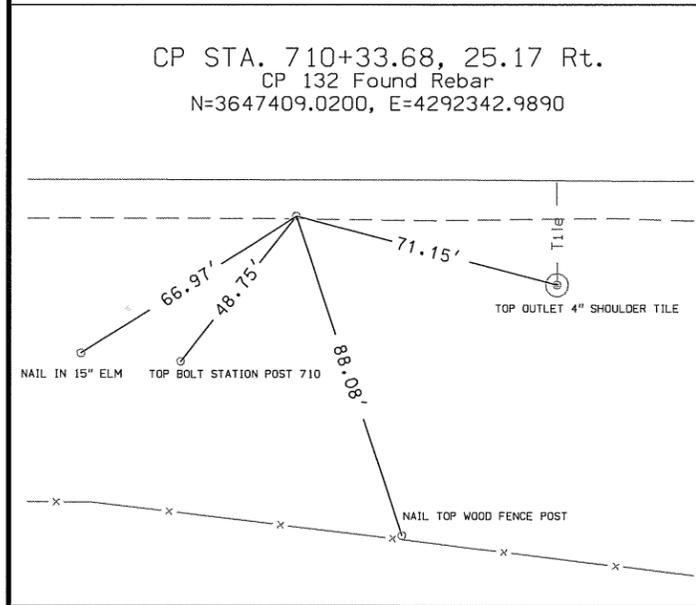
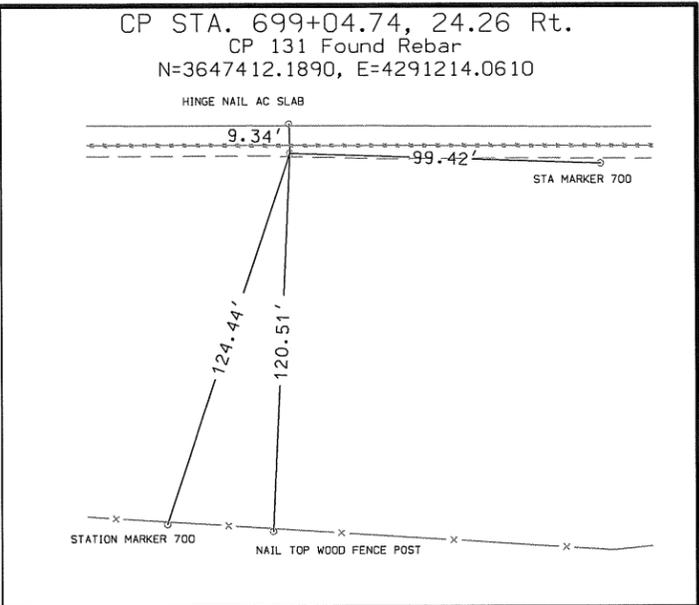
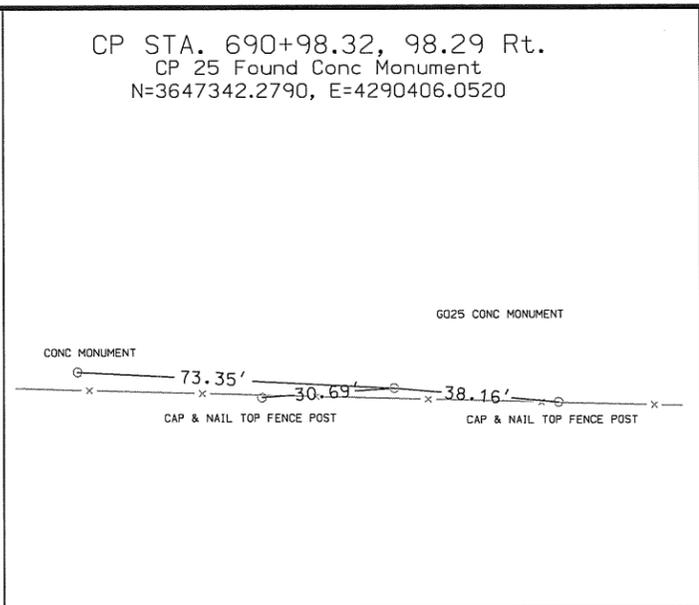
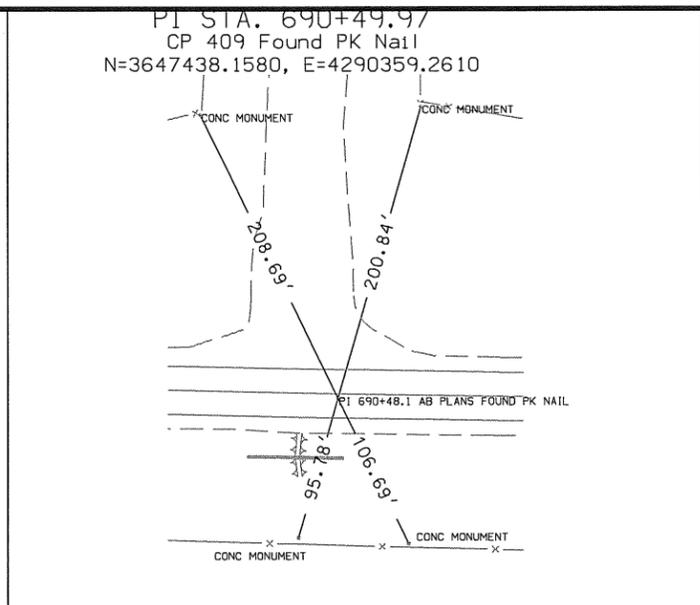
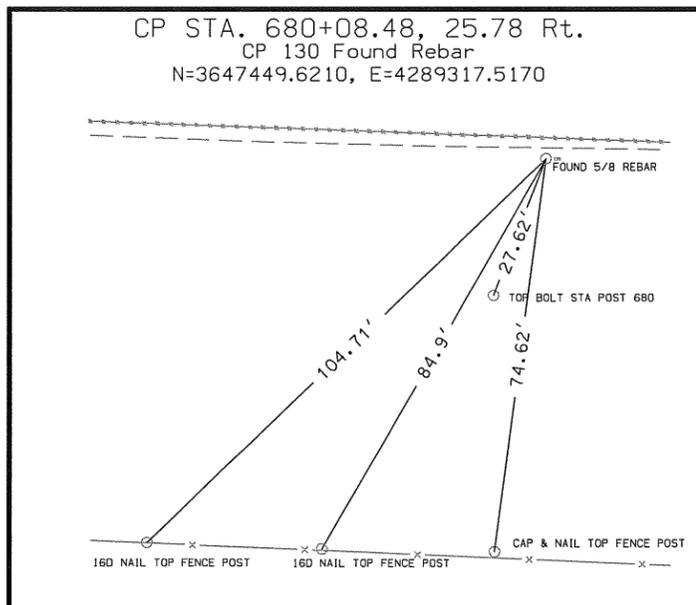


CP STA. 445+24.37, 27.28 Rt.
 CP 418 Found 5/8 Rebar
 N=3648194.4550, E=4265845.9500

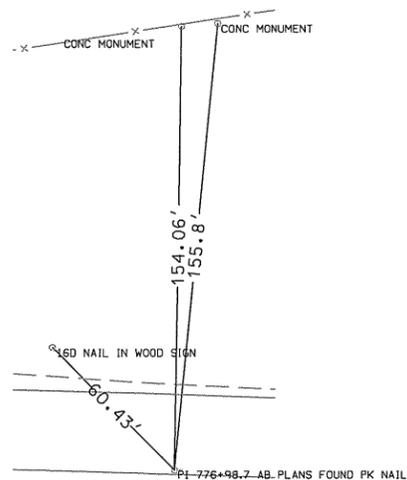




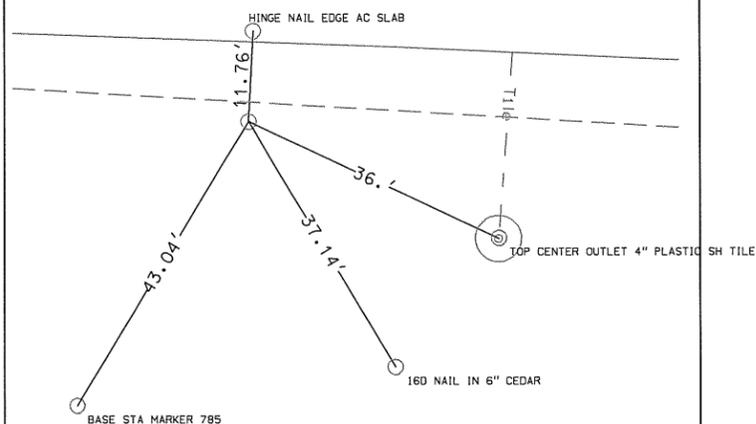




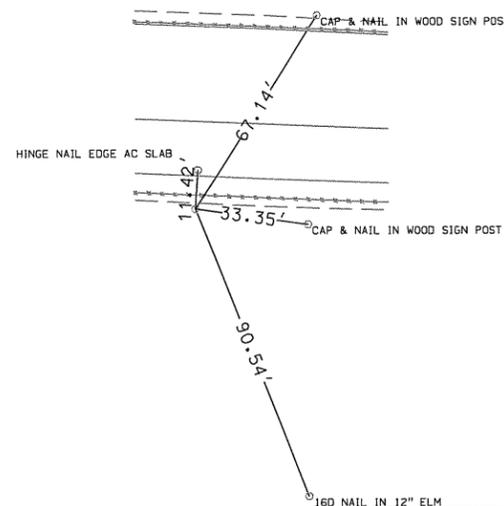
CP STA. 776+98.28
 PI 411, Fd PK Nail
 N=3647407.092, E=4297865.44



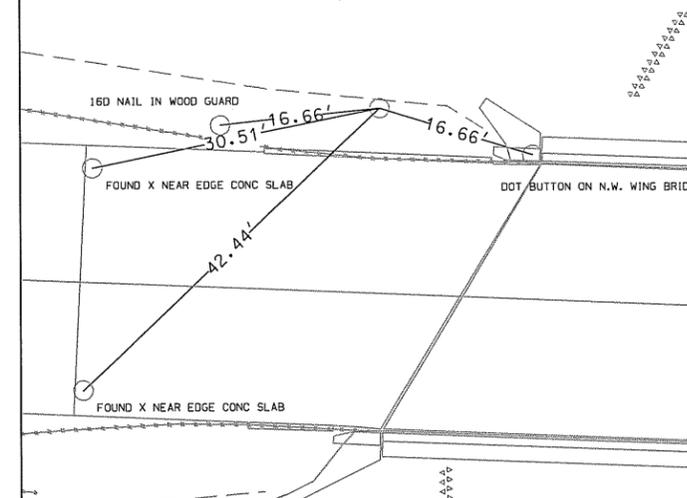
CP STA. 785+24.88, 26.29 Rt.
 CP 136, Fd Rebar
 N=3647290.866, E=4299831.664



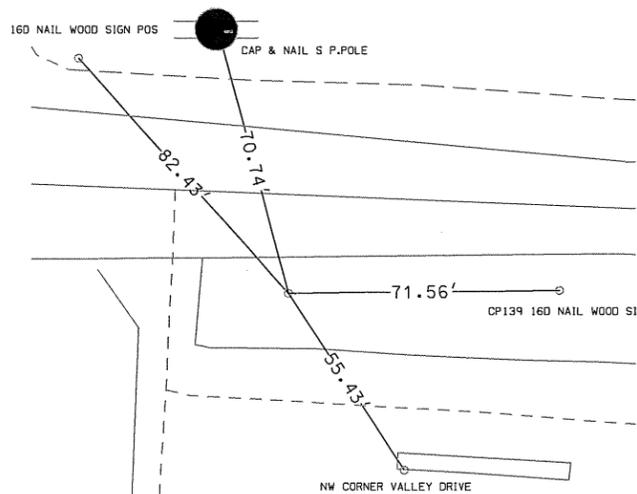
CP STA. 798+36.34, 25.73 Rt.
 CP 137, Fd Rebar
 N=3647238.059, E=4301142.052



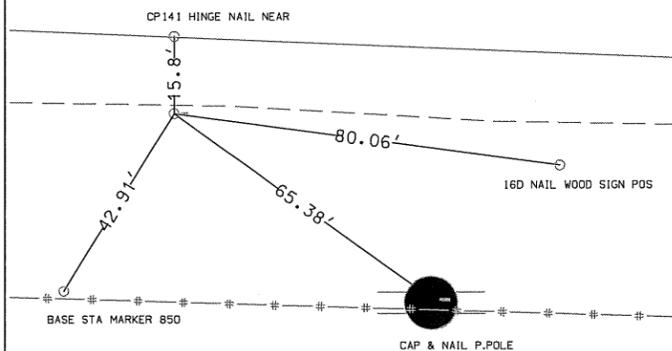
CP STA. 811+50.52, 19.14 Lt.
 CP 29, Fd Rebar
 N=3647229.415, E=4302456.976



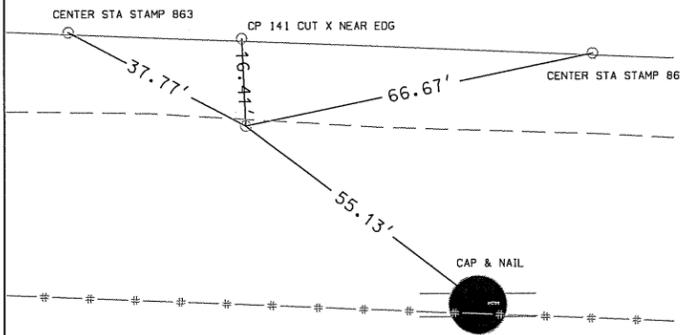
CP STA. 837+11.41, 25.73 Rt.
 CP 139, Fd Rebar
 N=3647080.376, E=4305013.913



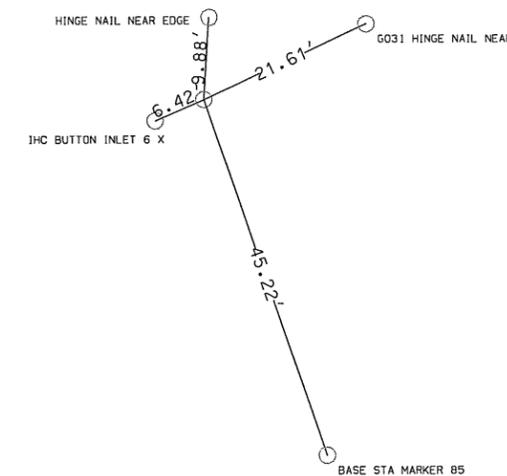
CP STA. 850+20.40, 27.43 Rt.
 CP 140, Fd Rebar
 N=3647025.413, E=4306321.757



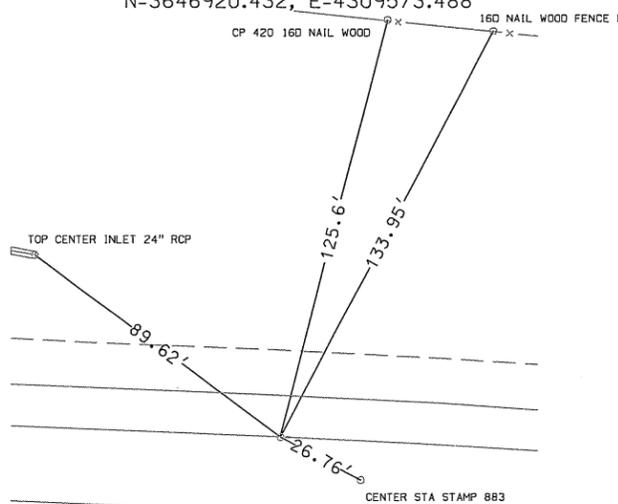
CP STA. 863+32.67, 27.68 Rt.
 CP 141, Fd Rebar
 N=3646971.759, E=4307632.922



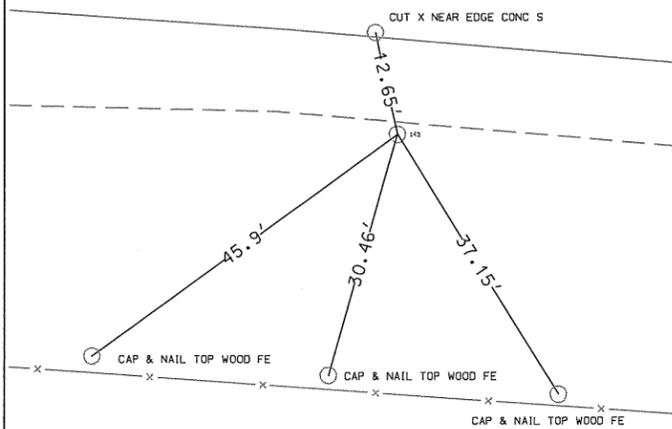
CP STA. 870+17.72, 1341.24 Lt.
 CP 31, Fd Rebar
 N=3648311.673, E=4308373.109



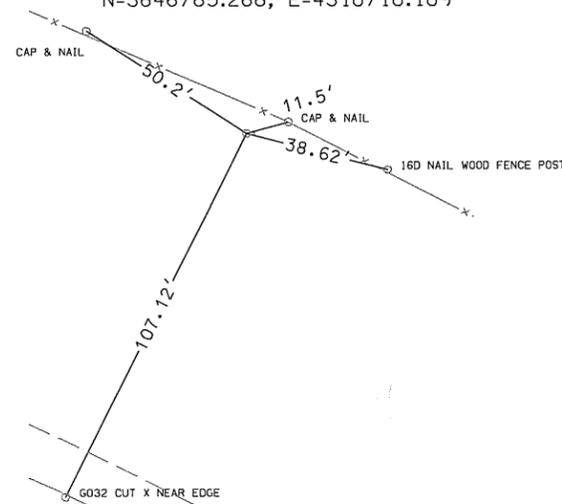
CP STA. 882+73.71
 PC 420, Fd X
 N=3646920.432, E=4309573.488



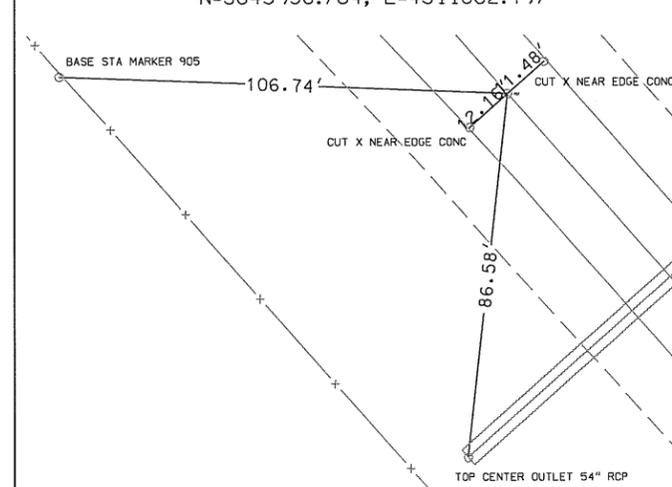
CP STA. 883+87.38, 33.64 Rt.
 CP 143, Fd Rebar
 N=3646880.032, E=4309684.243

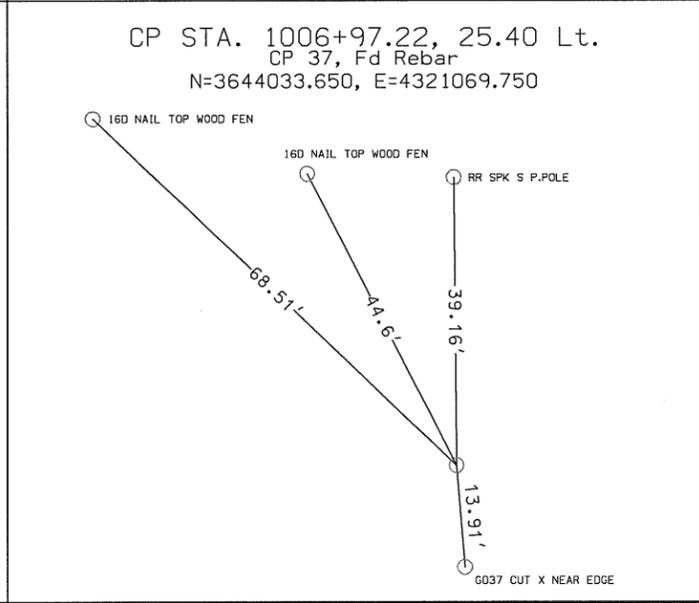
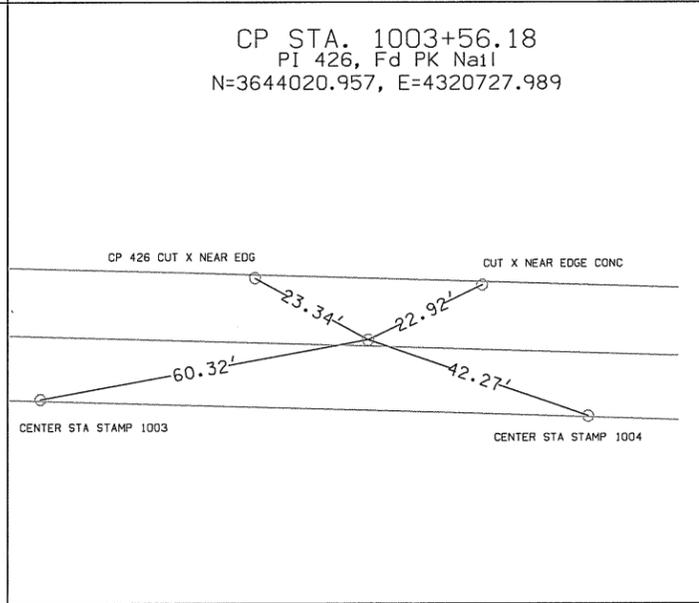
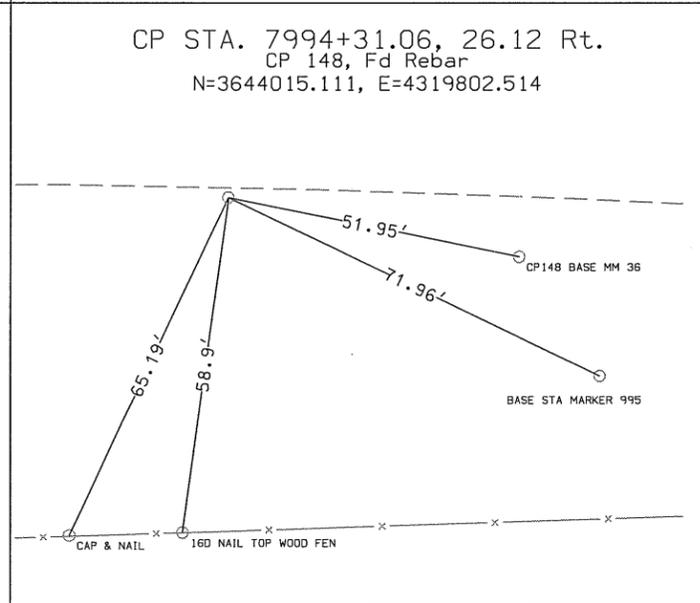
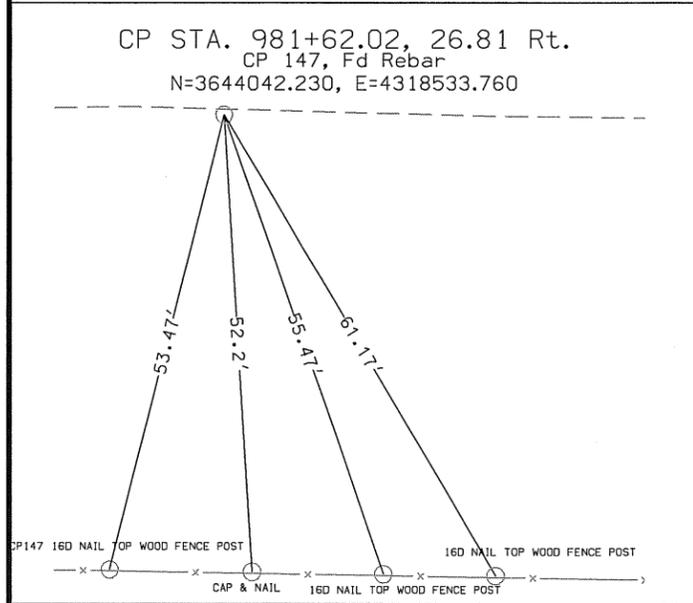
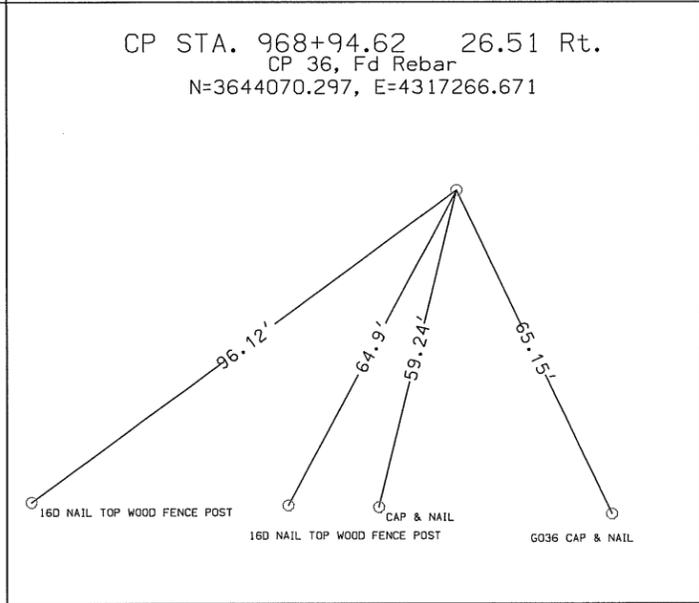
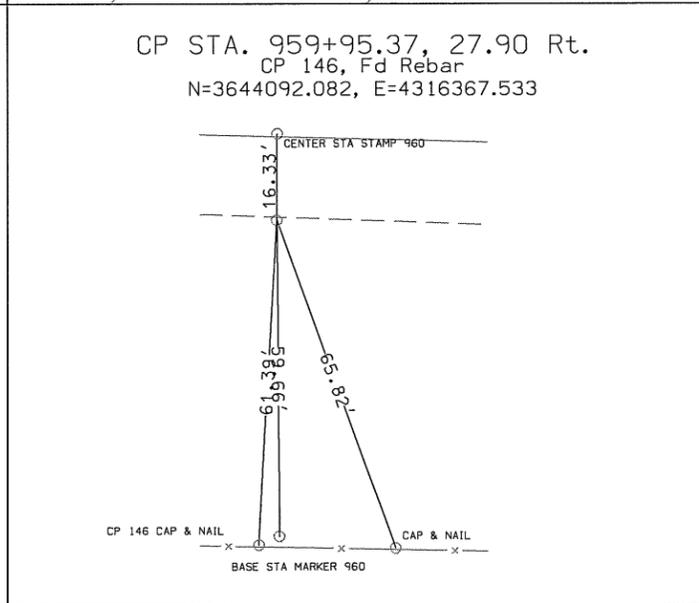
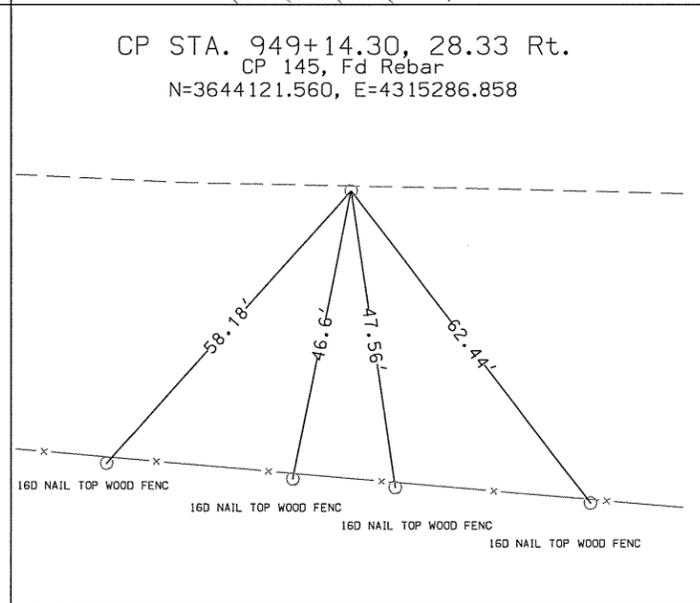
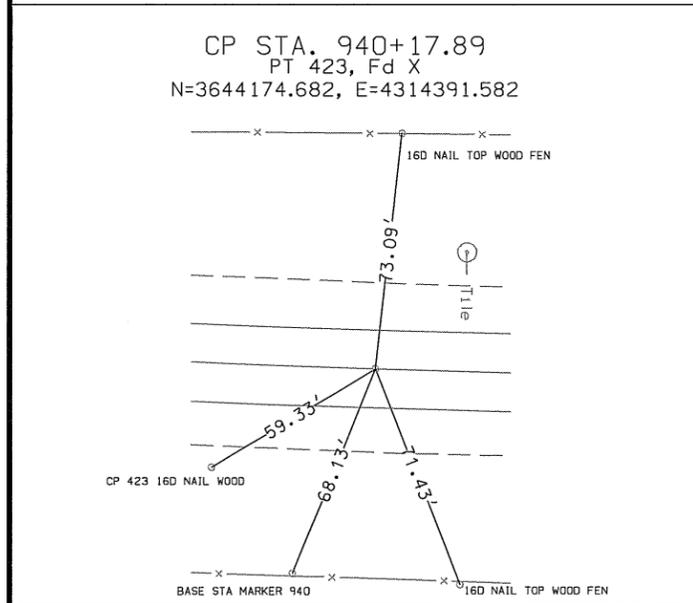
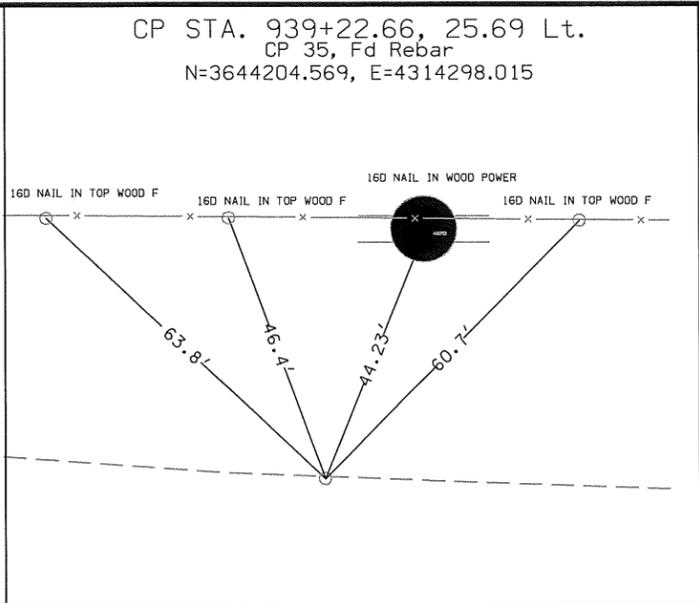
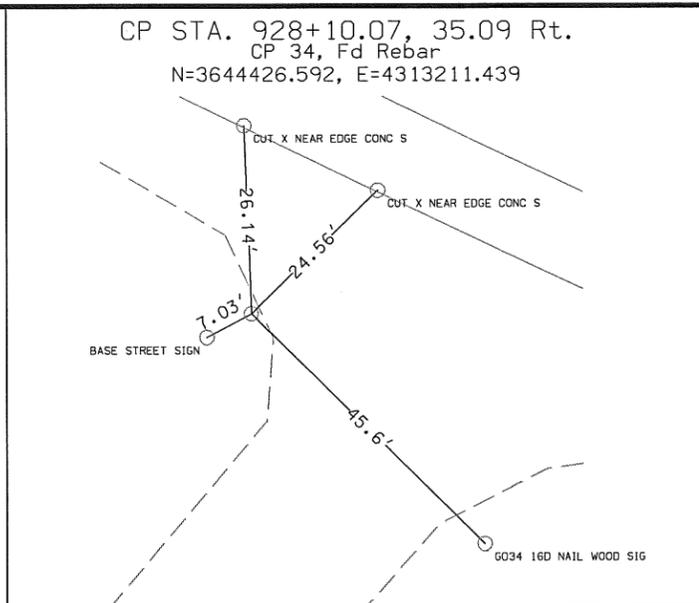
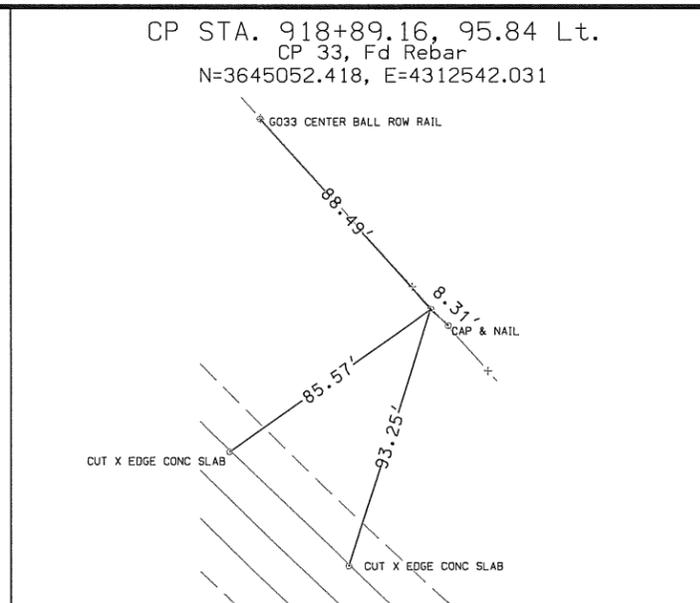
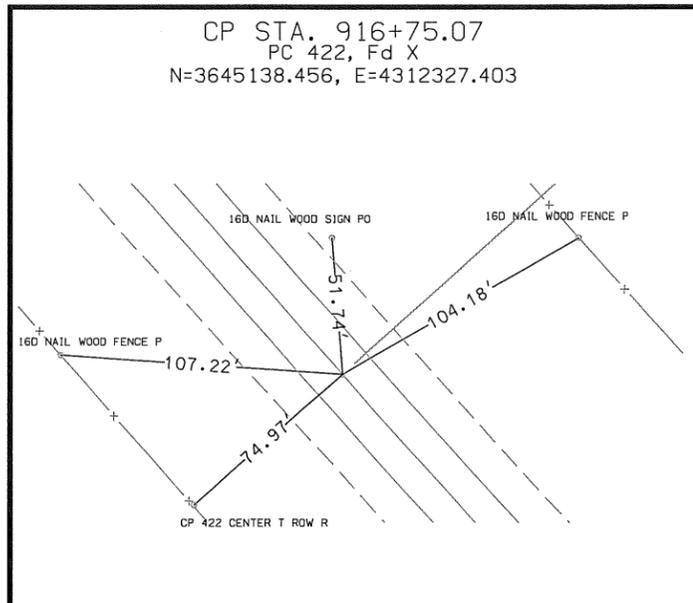


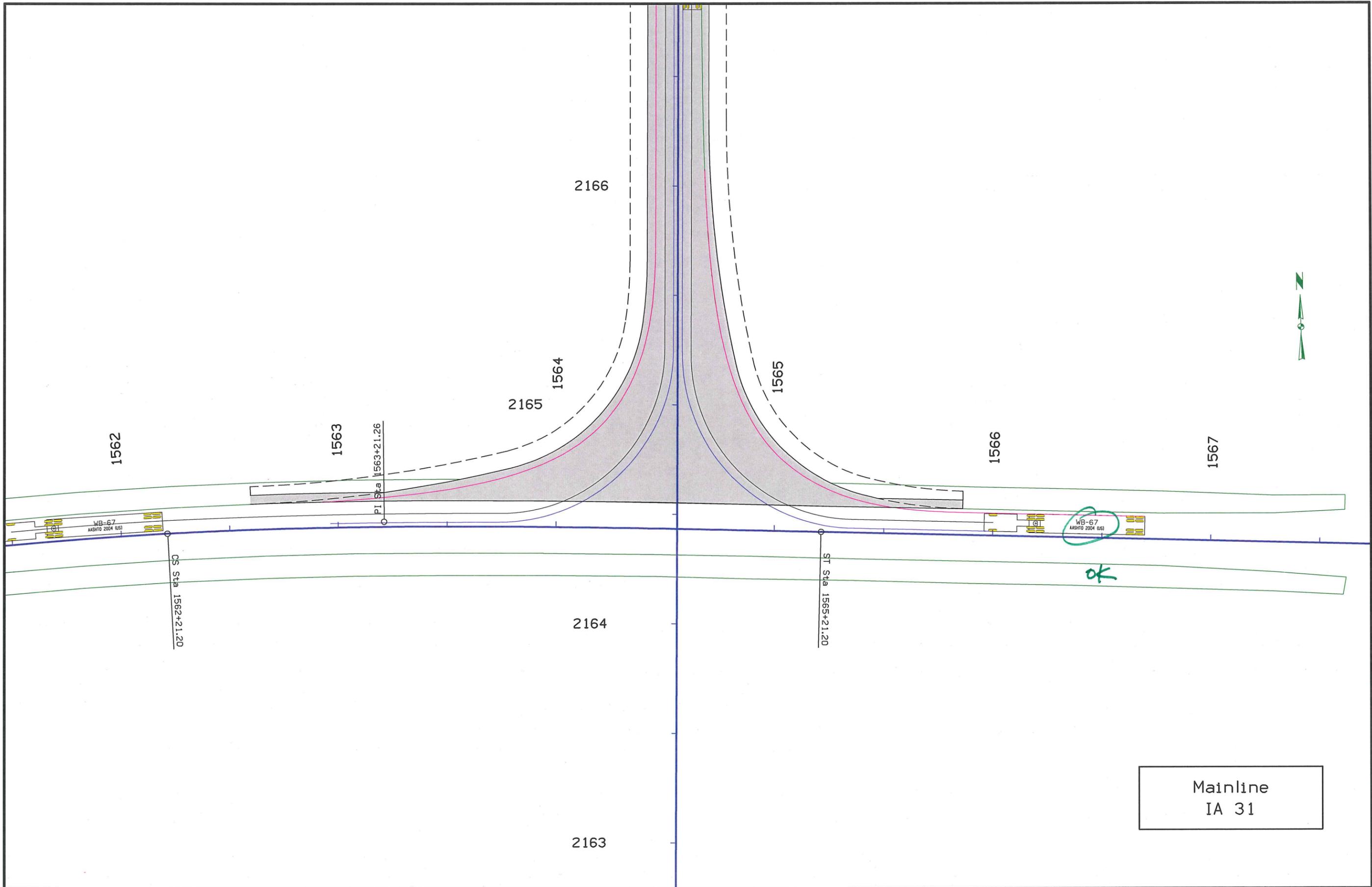
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 CP 32, Fd Rebar
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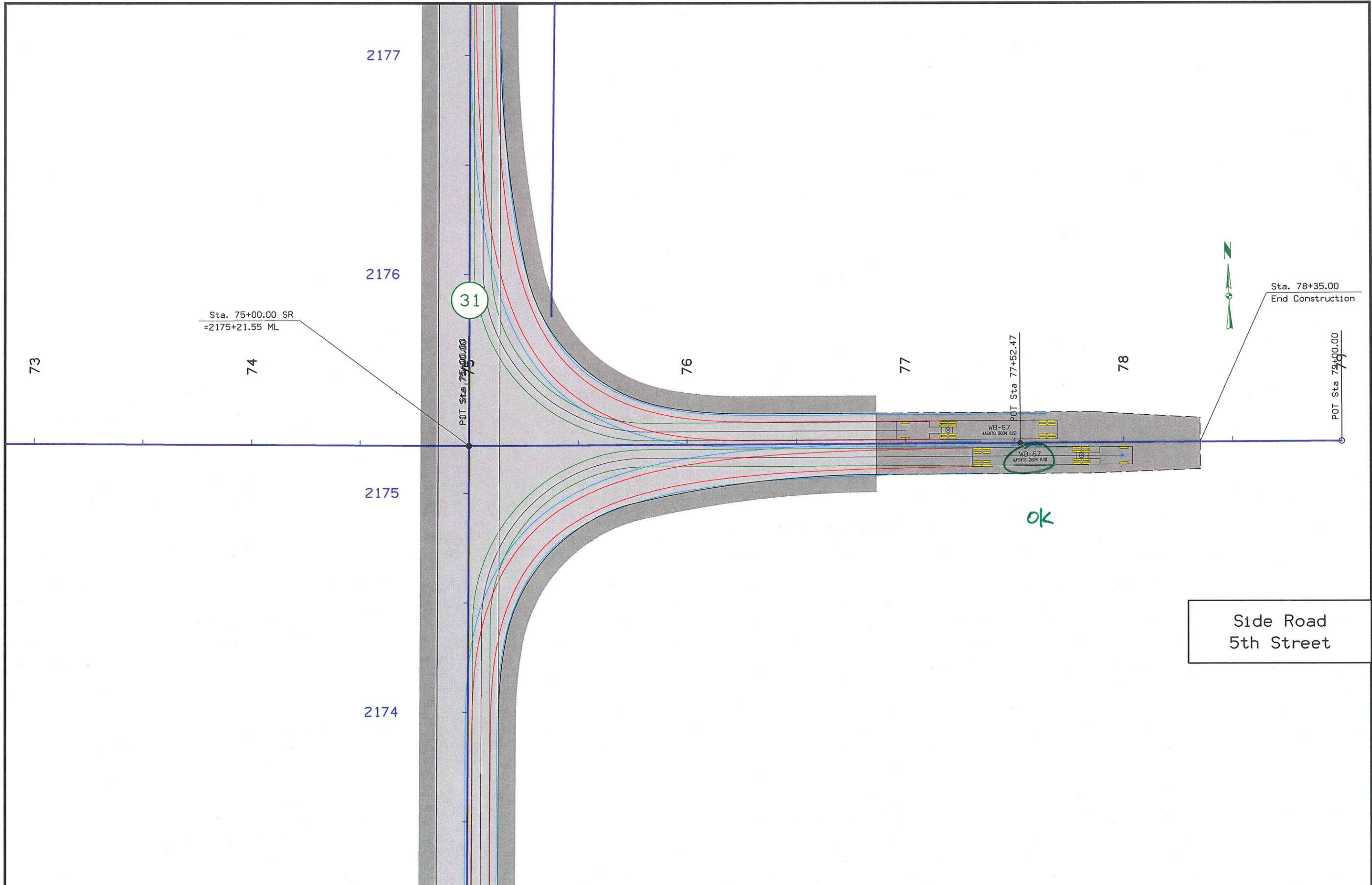
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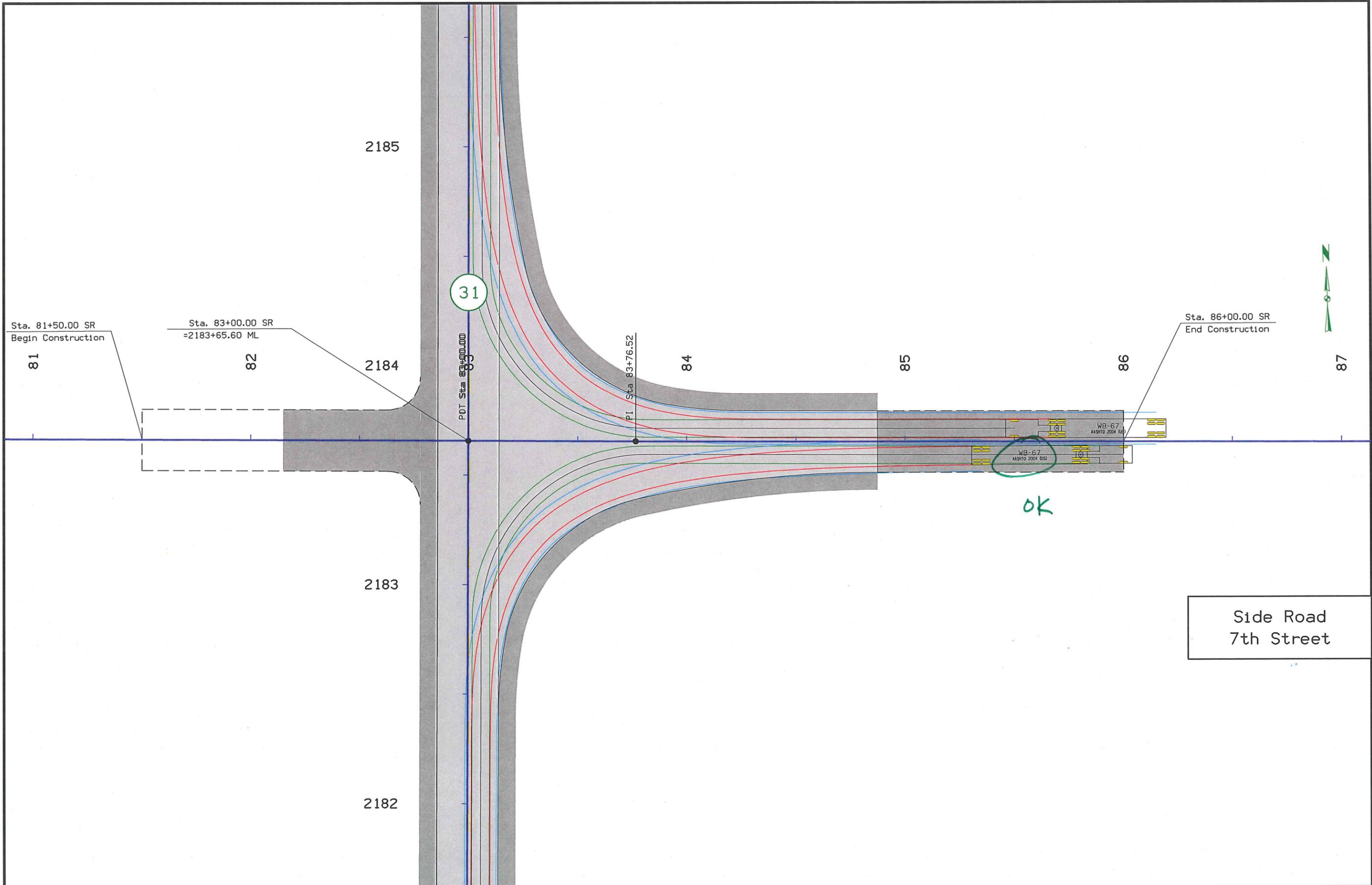




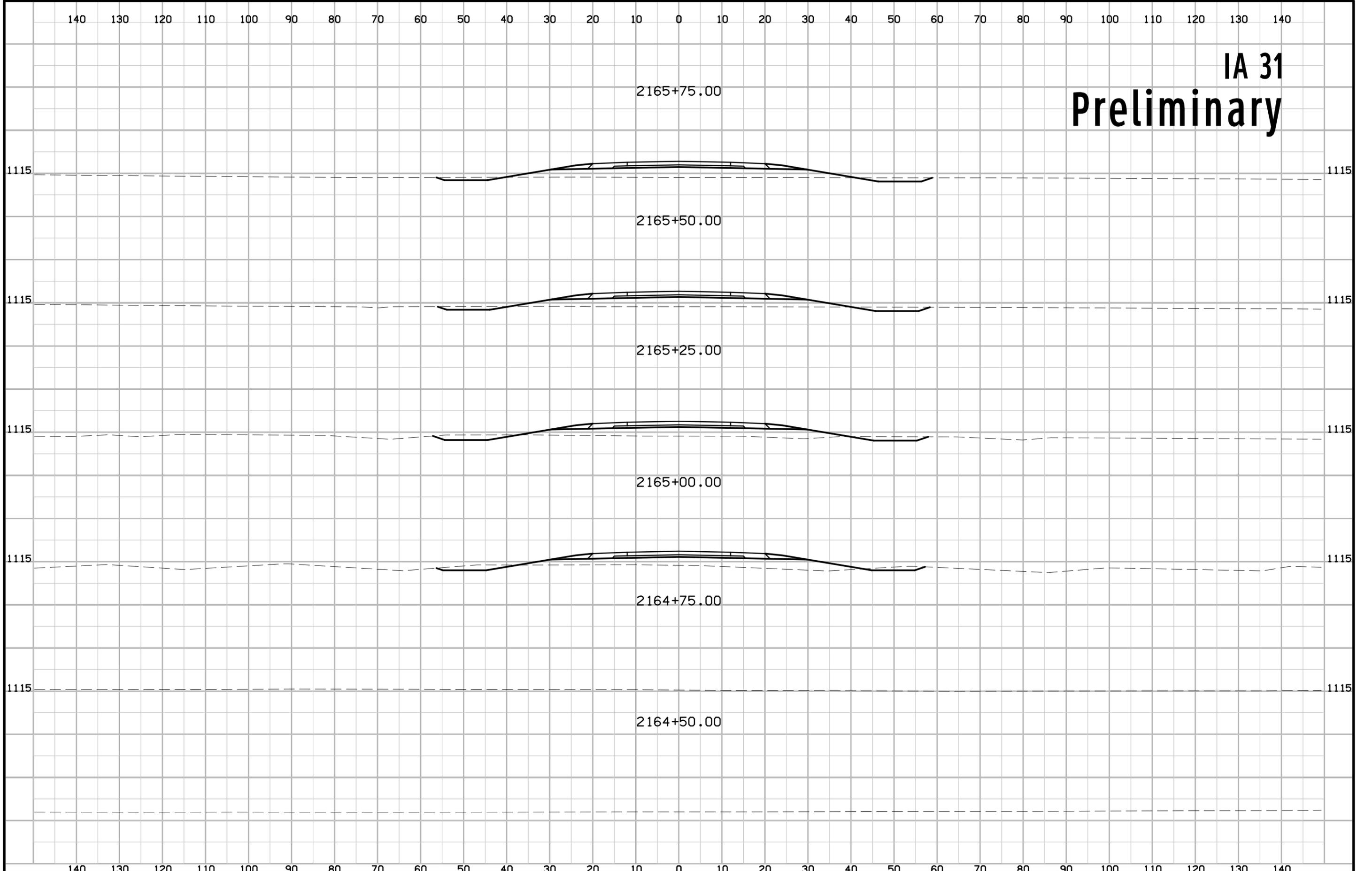
Mainline
IA 31



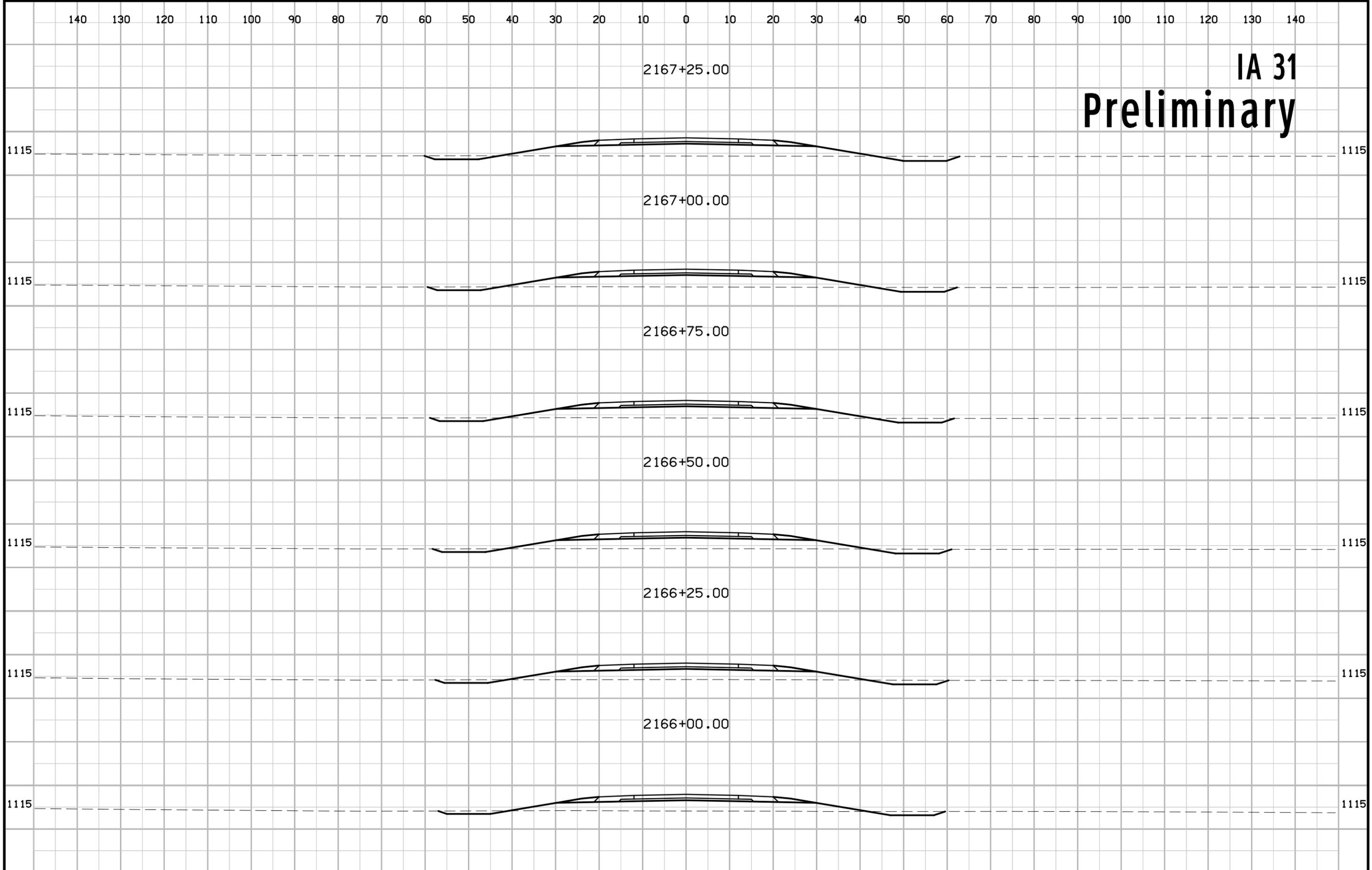
Side Road
5th Street



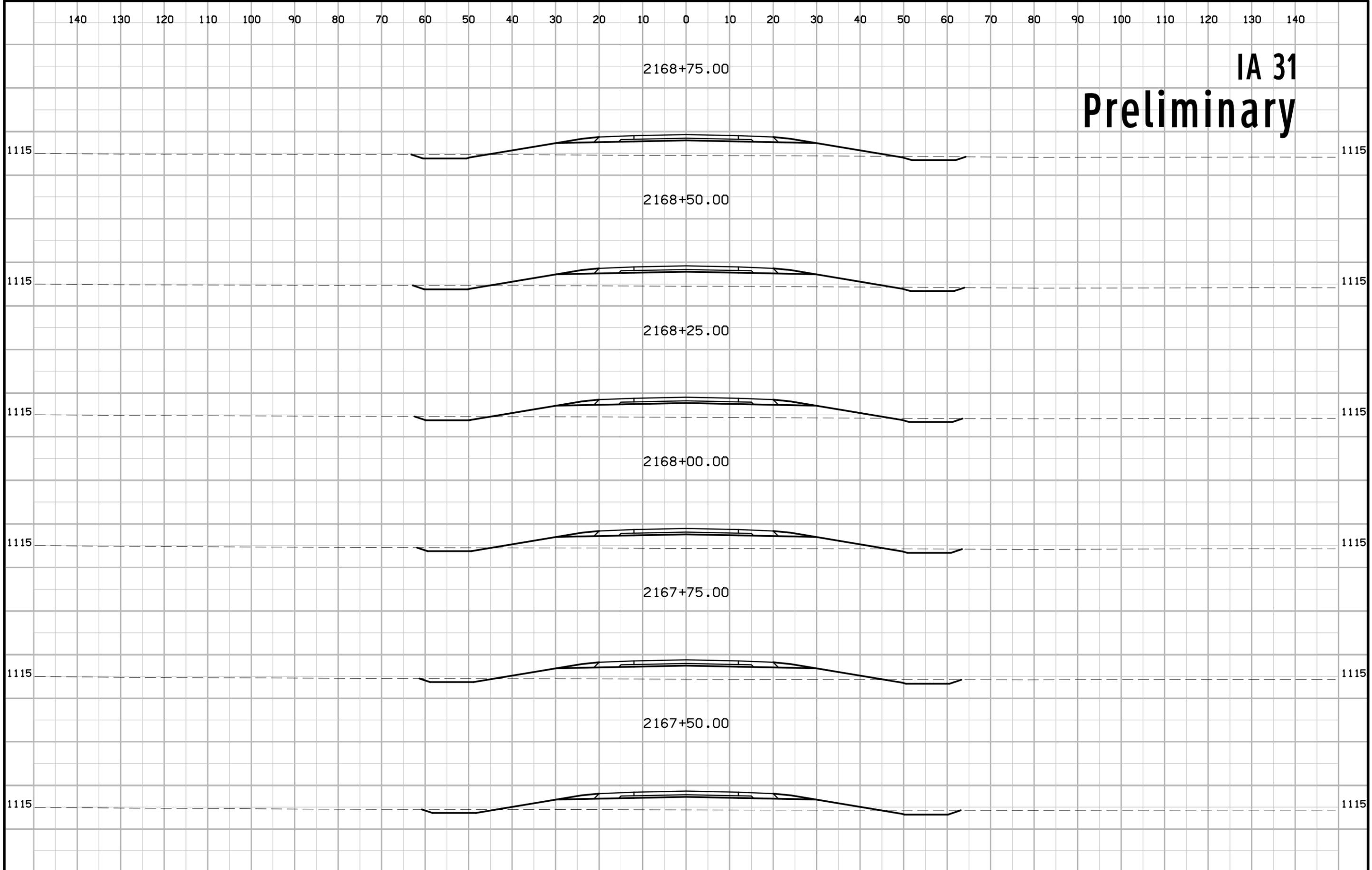
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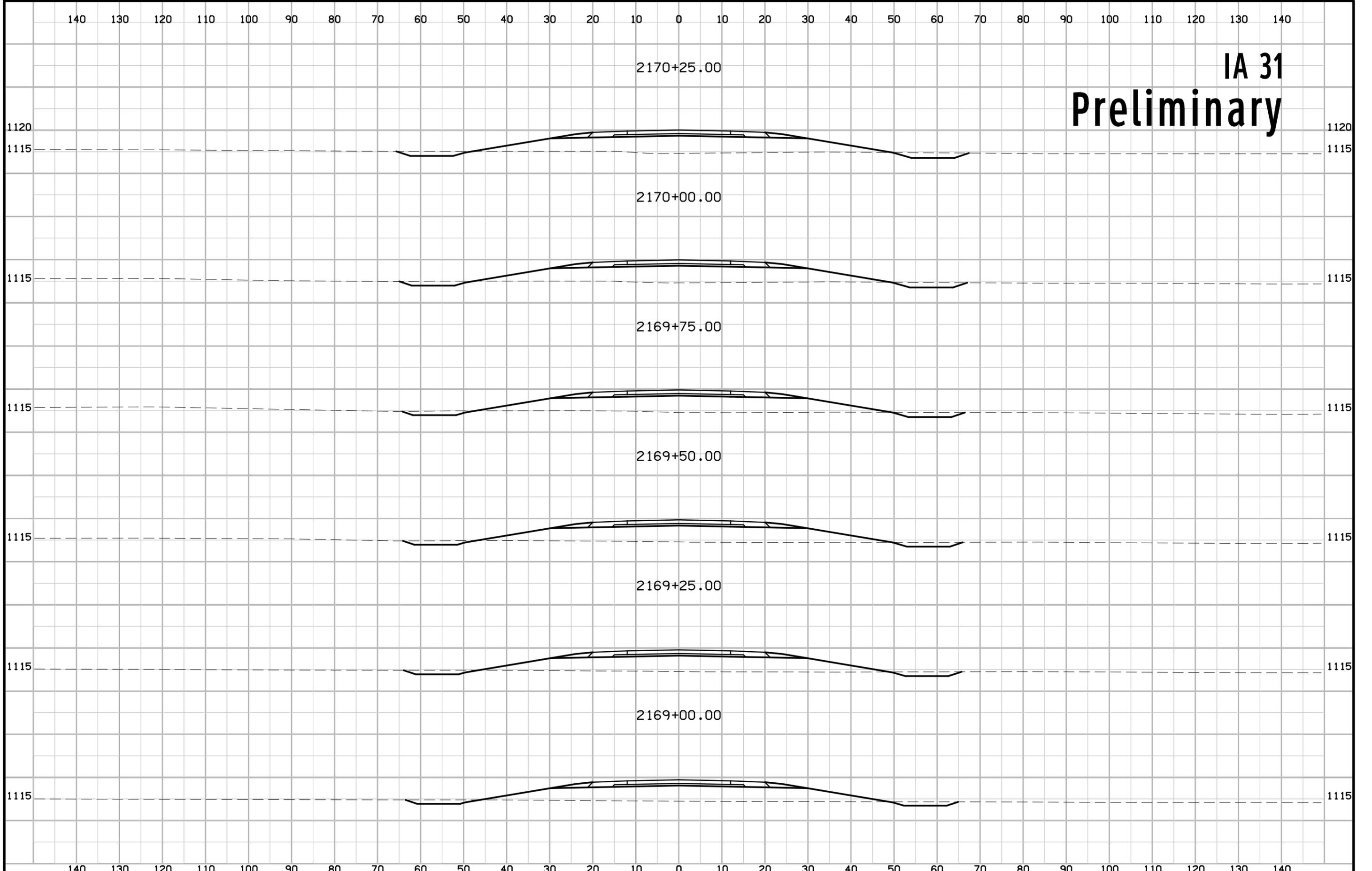
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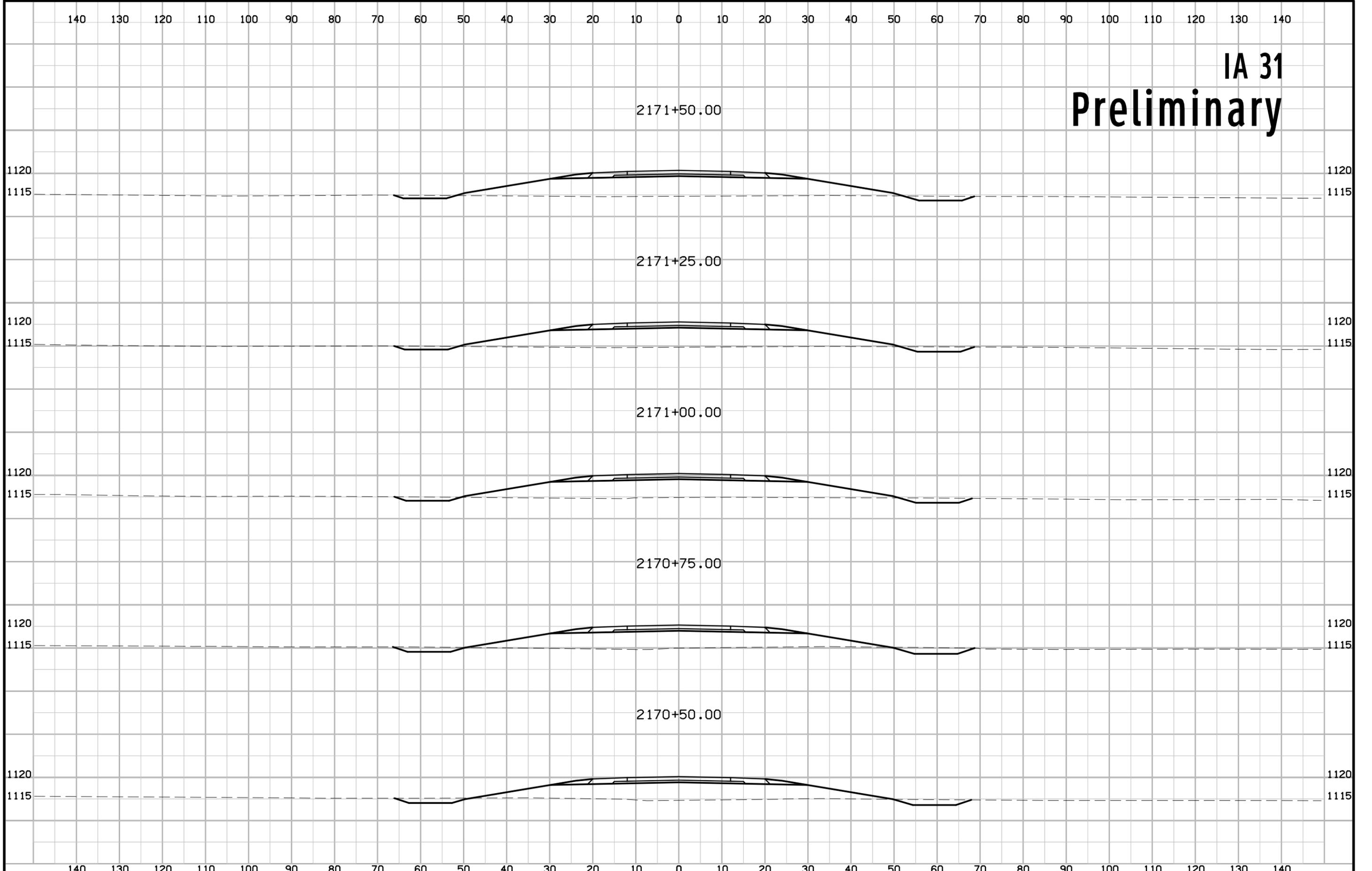
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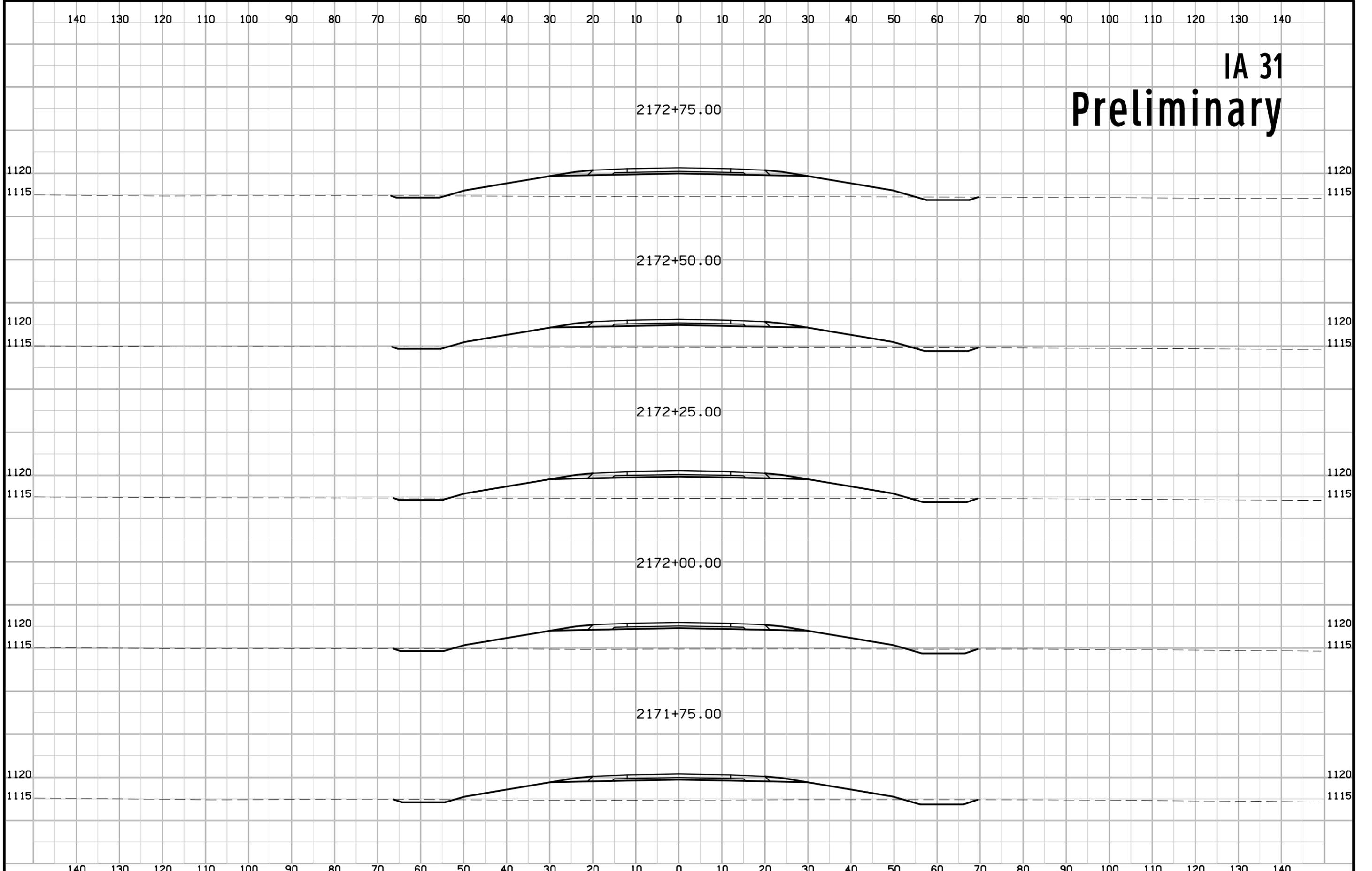
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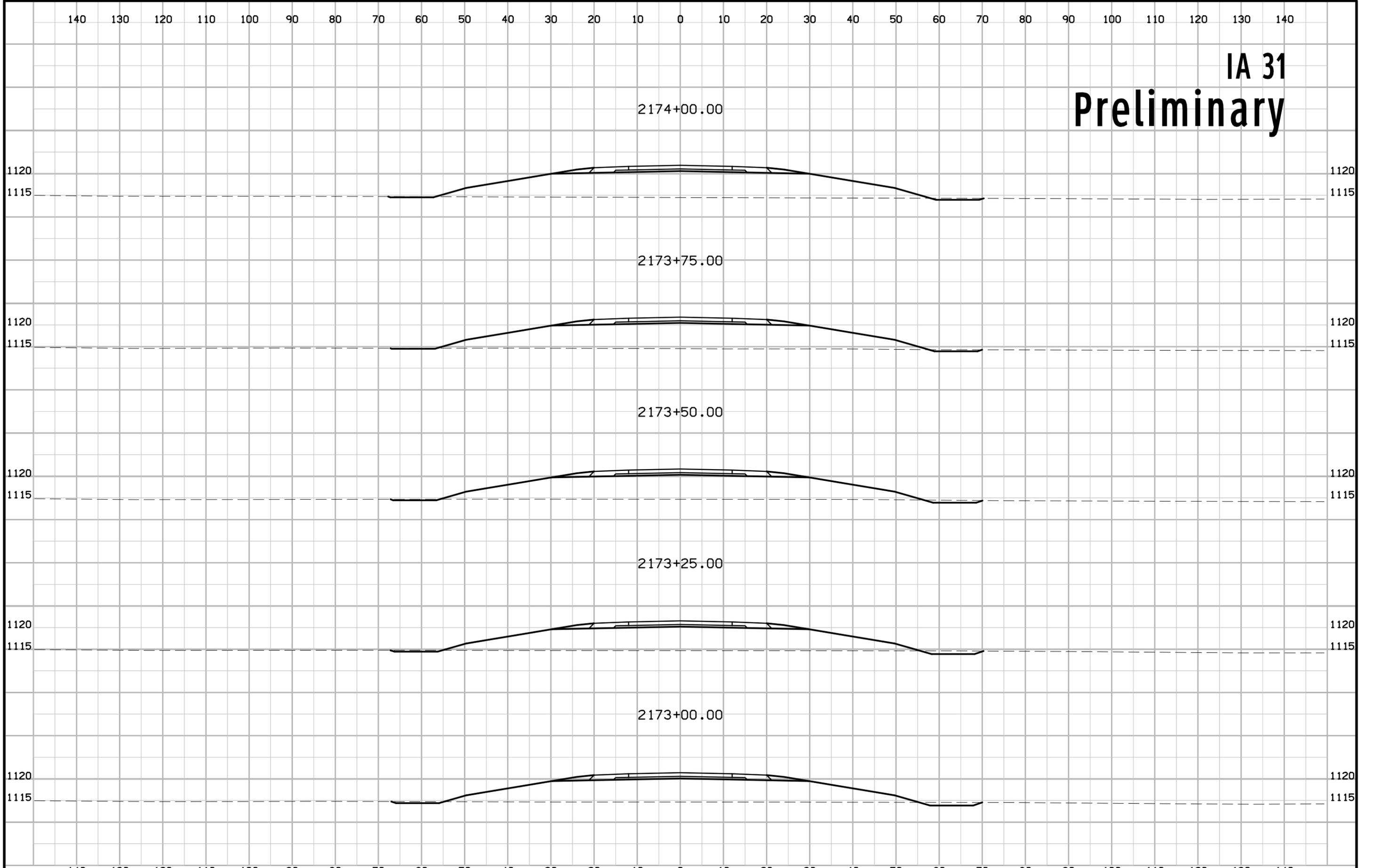
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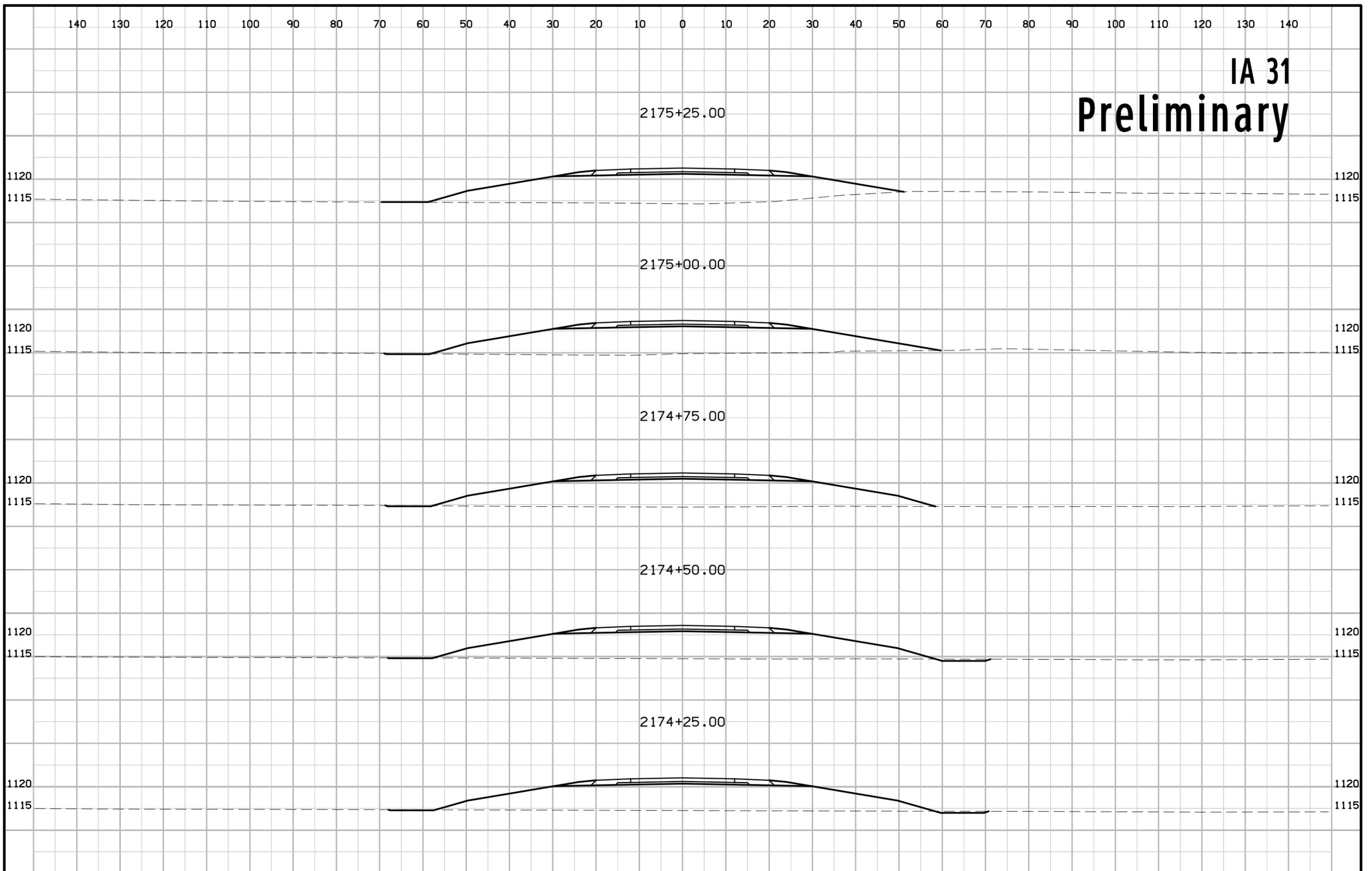
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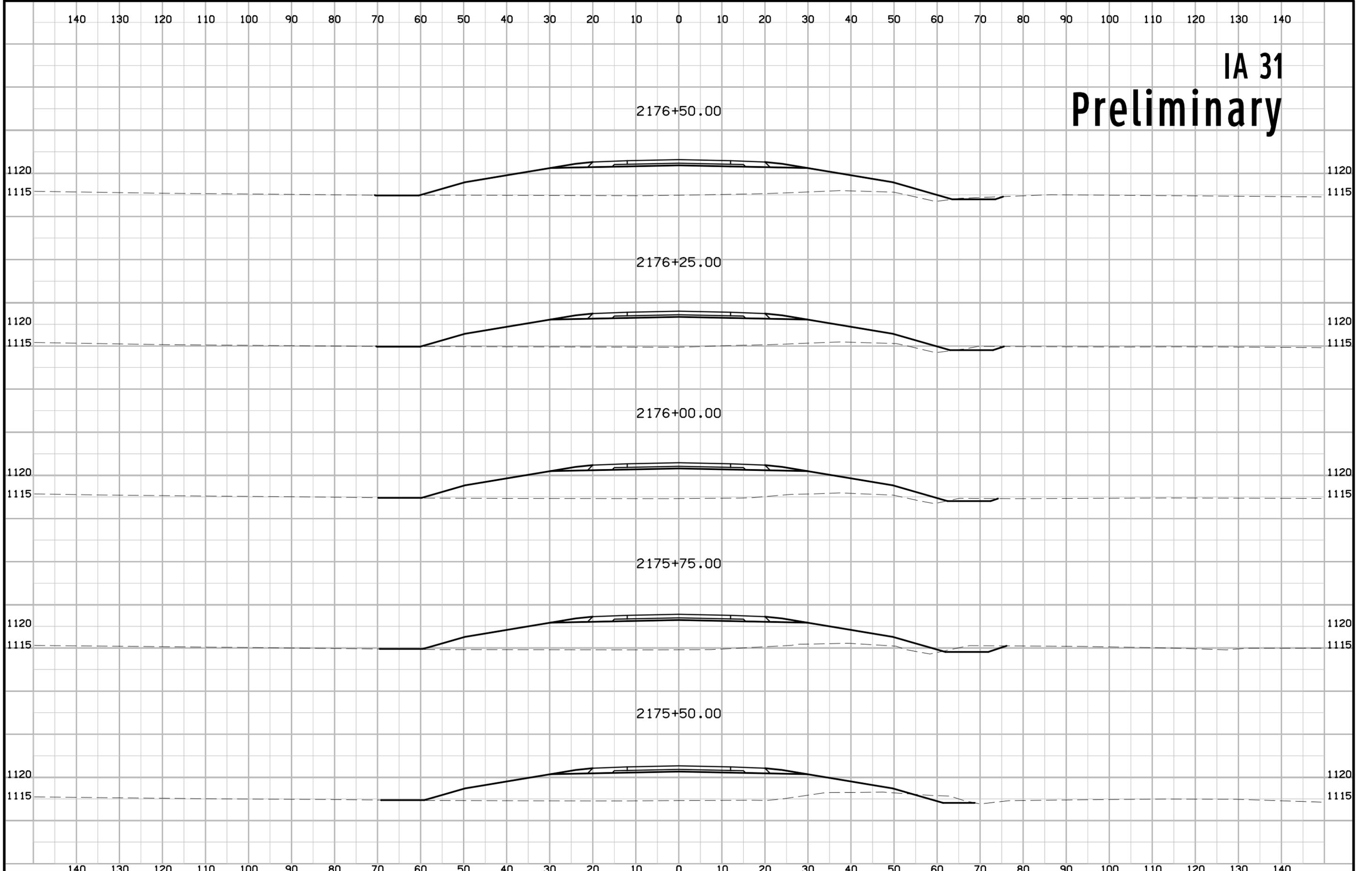
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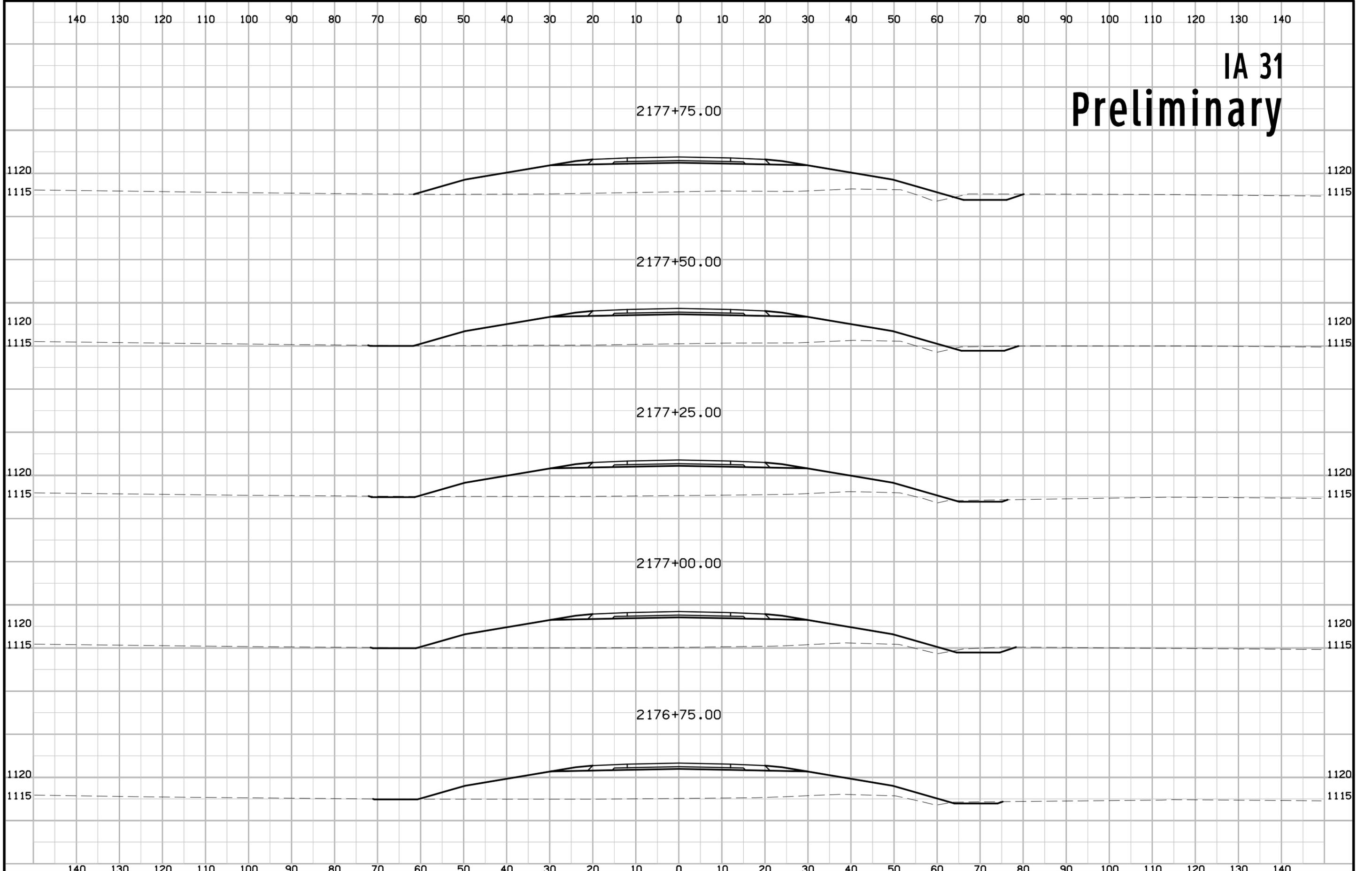
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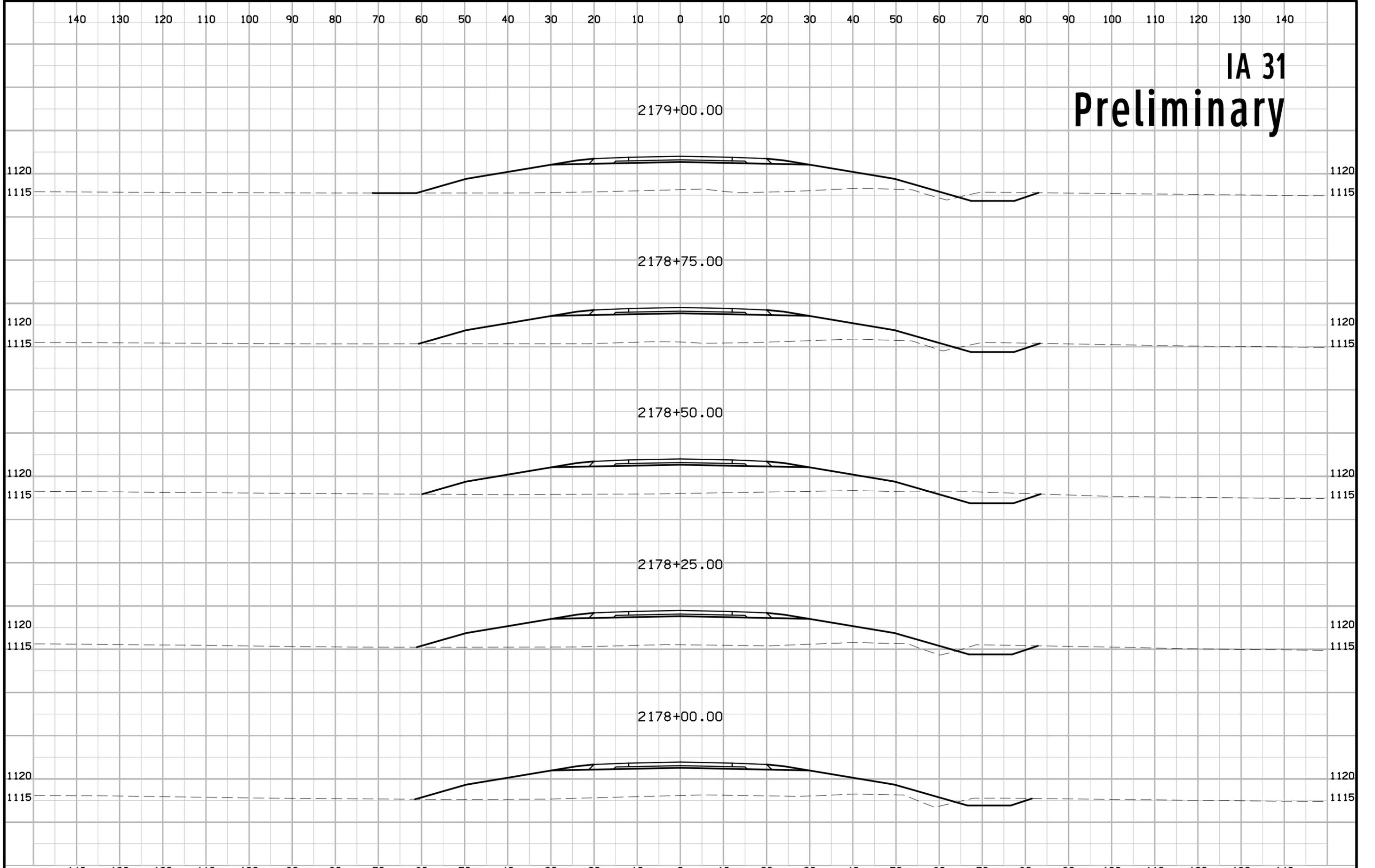
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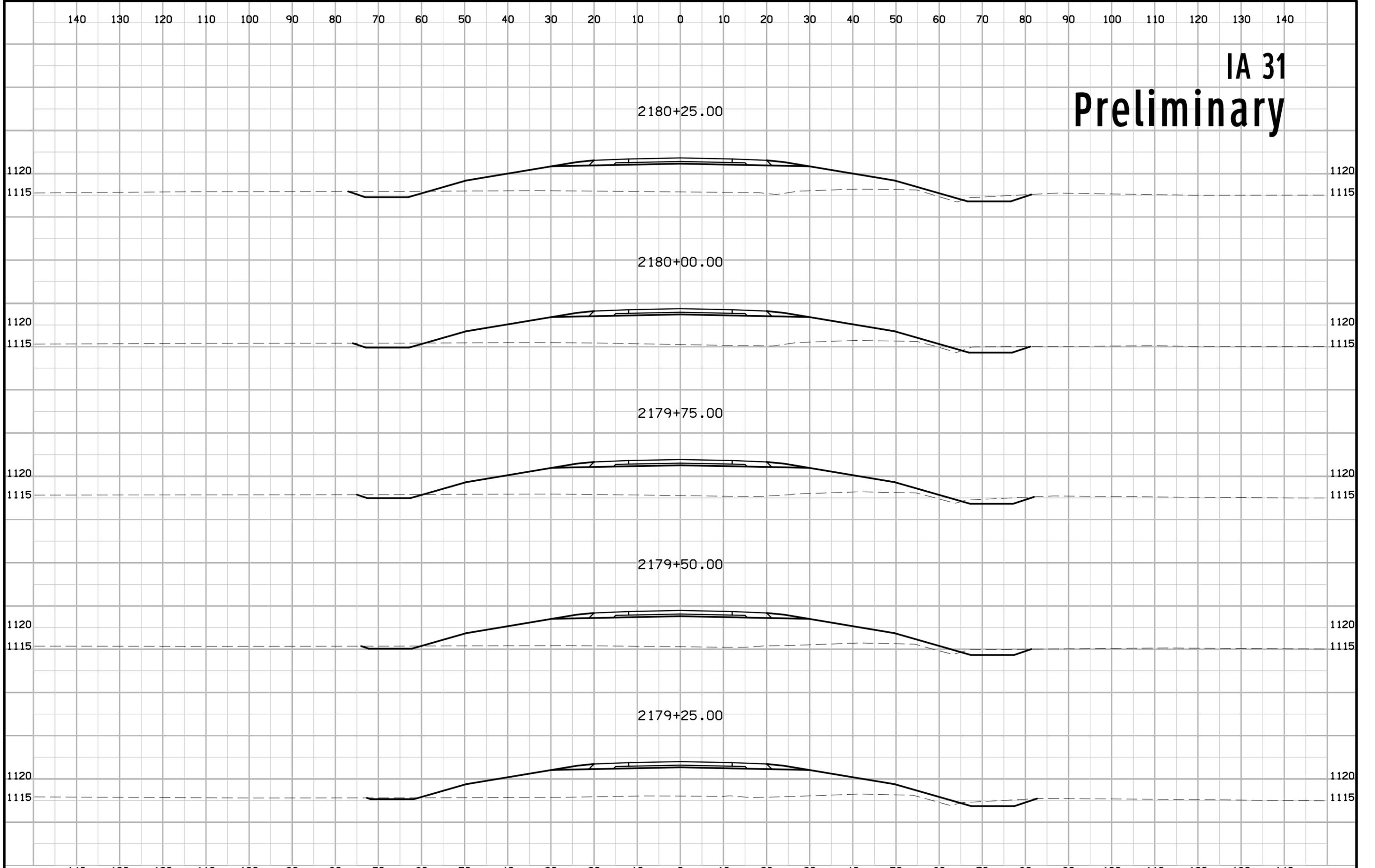
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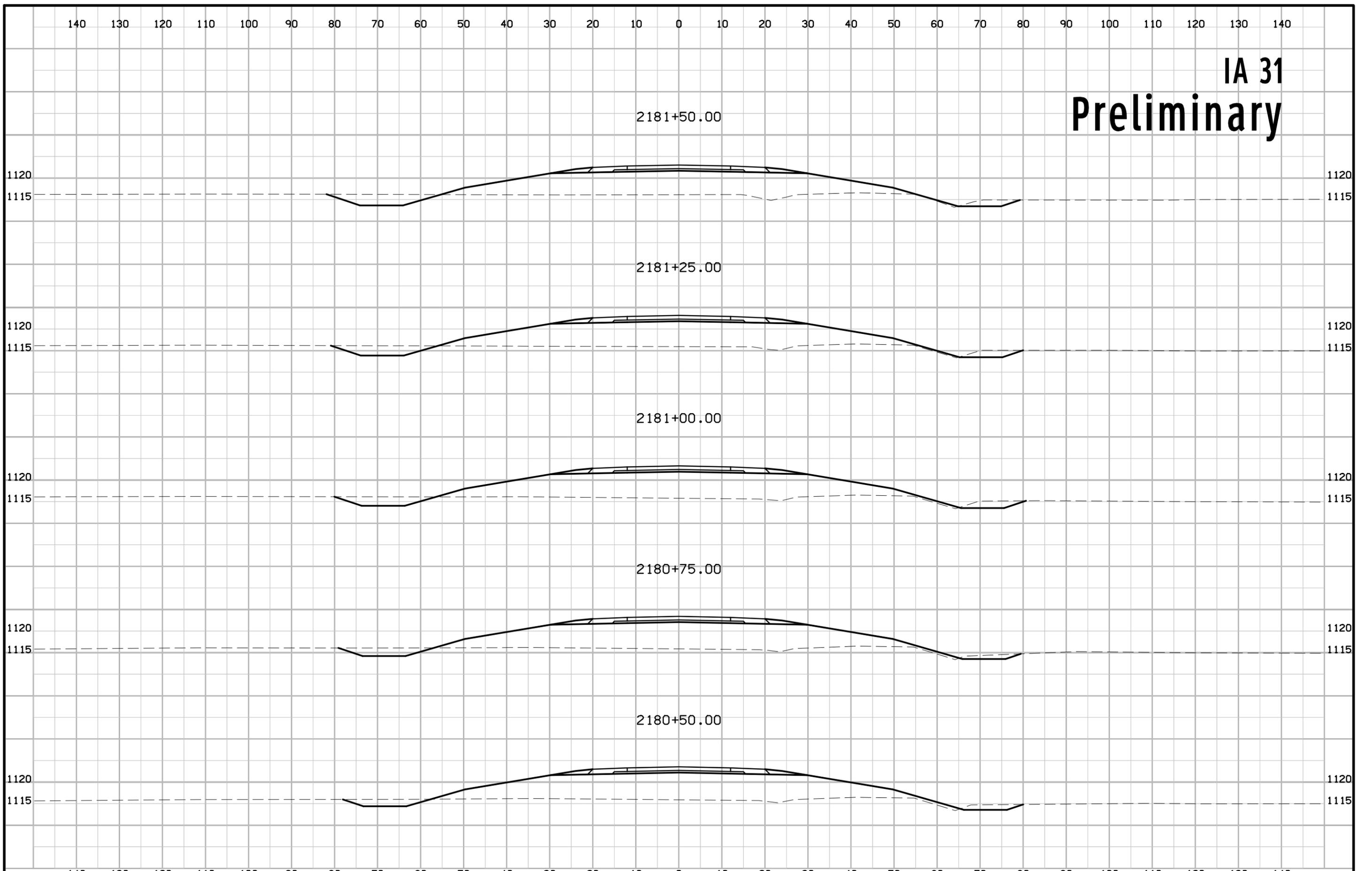
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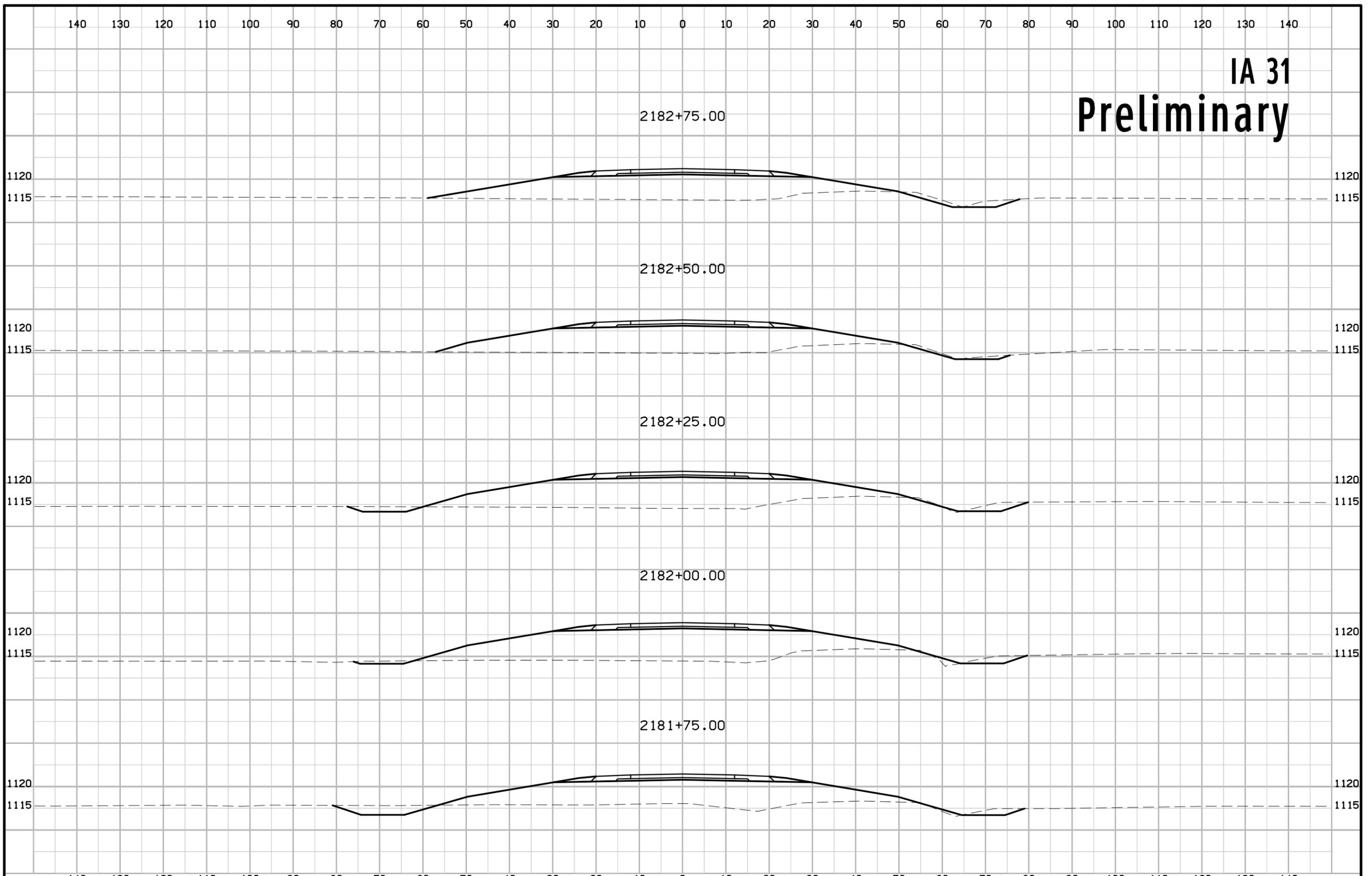
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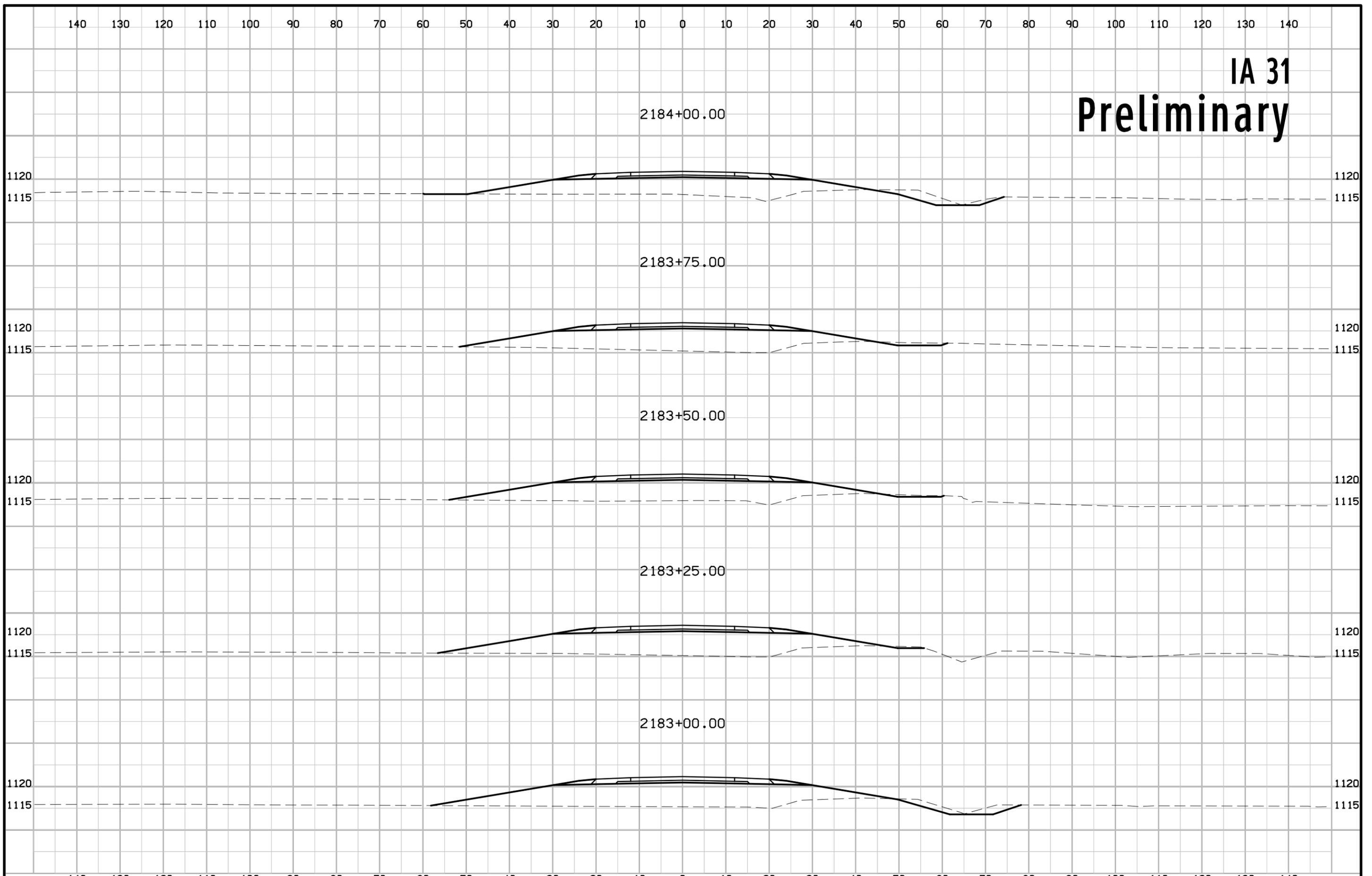
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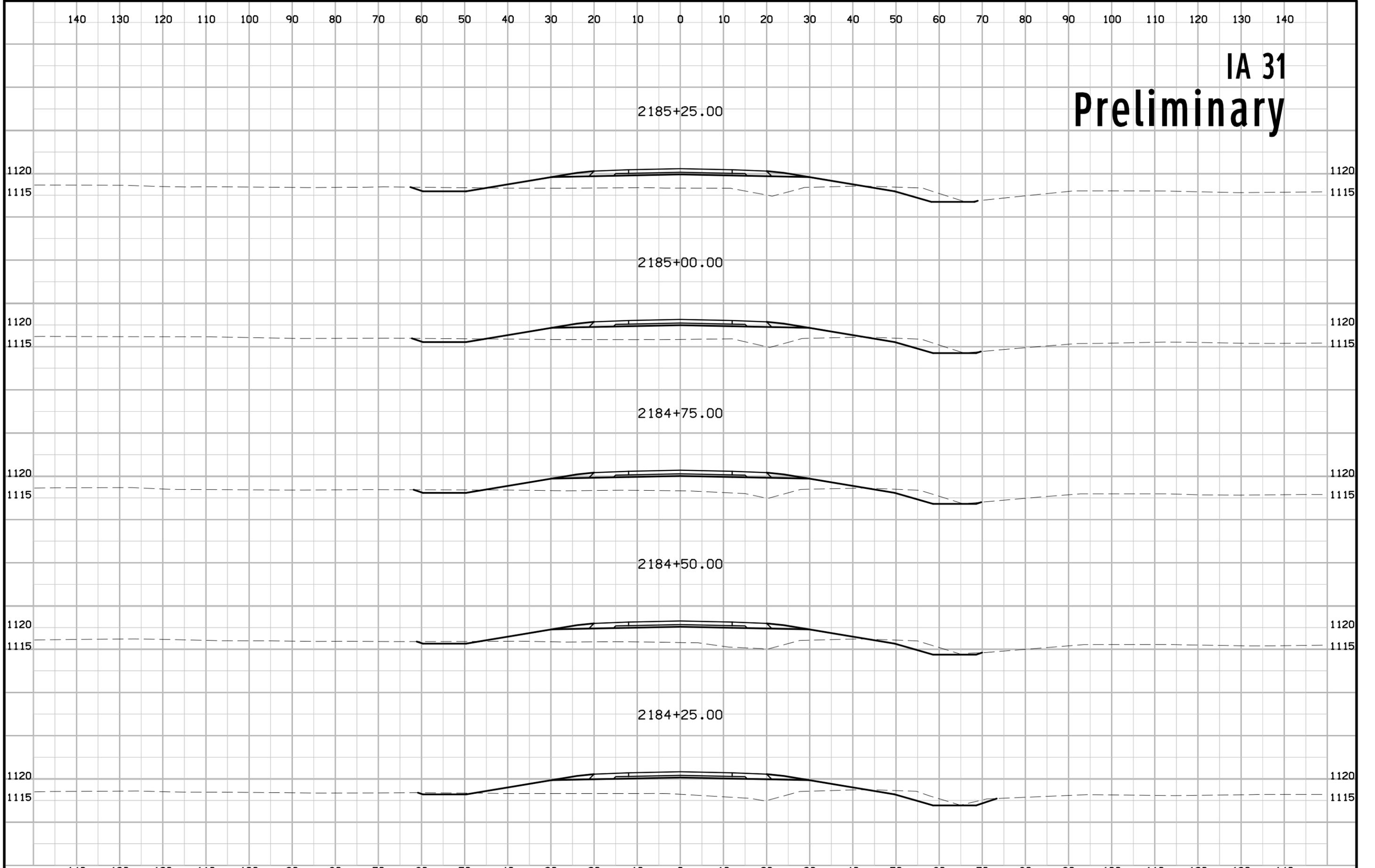
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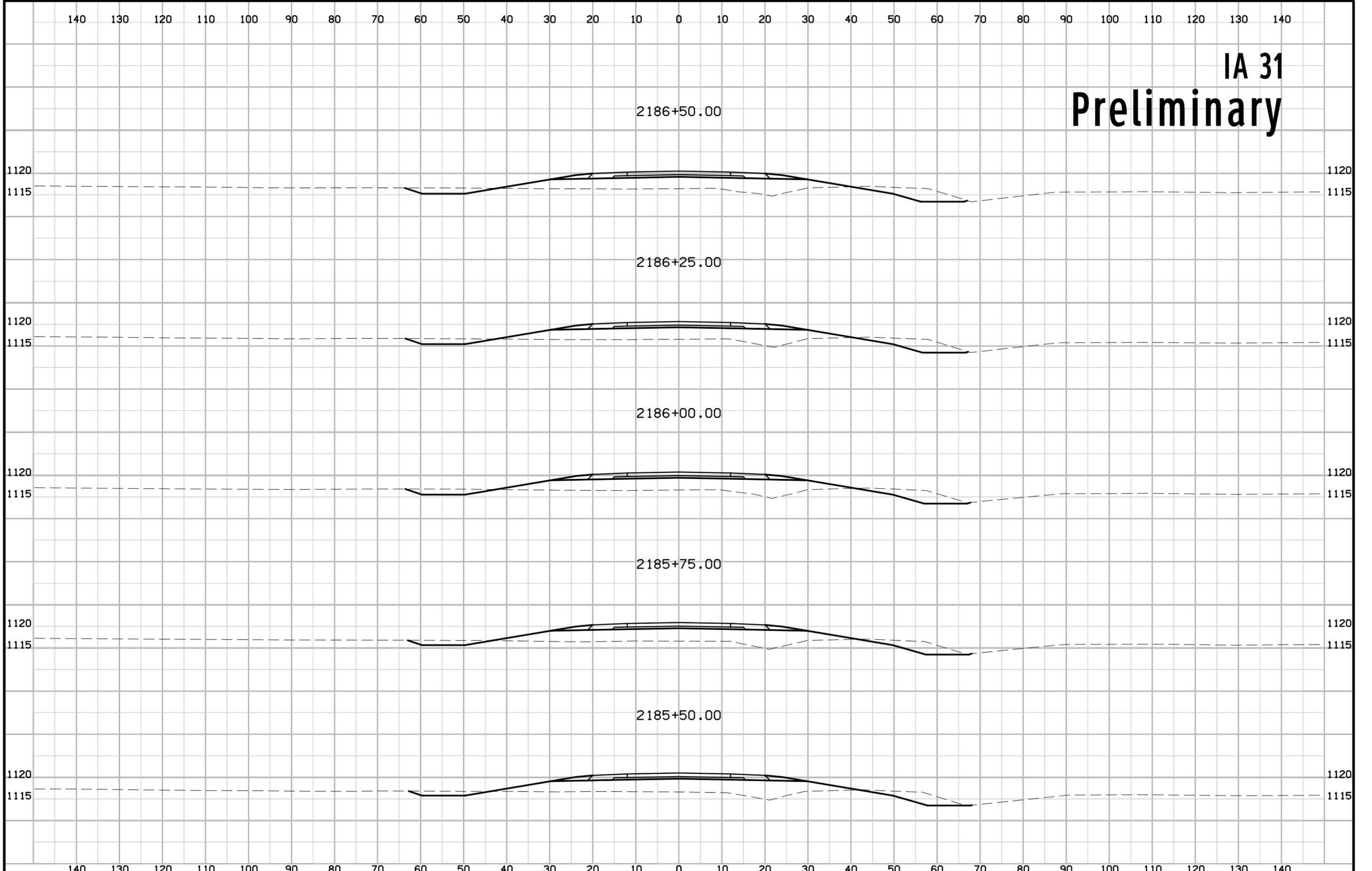
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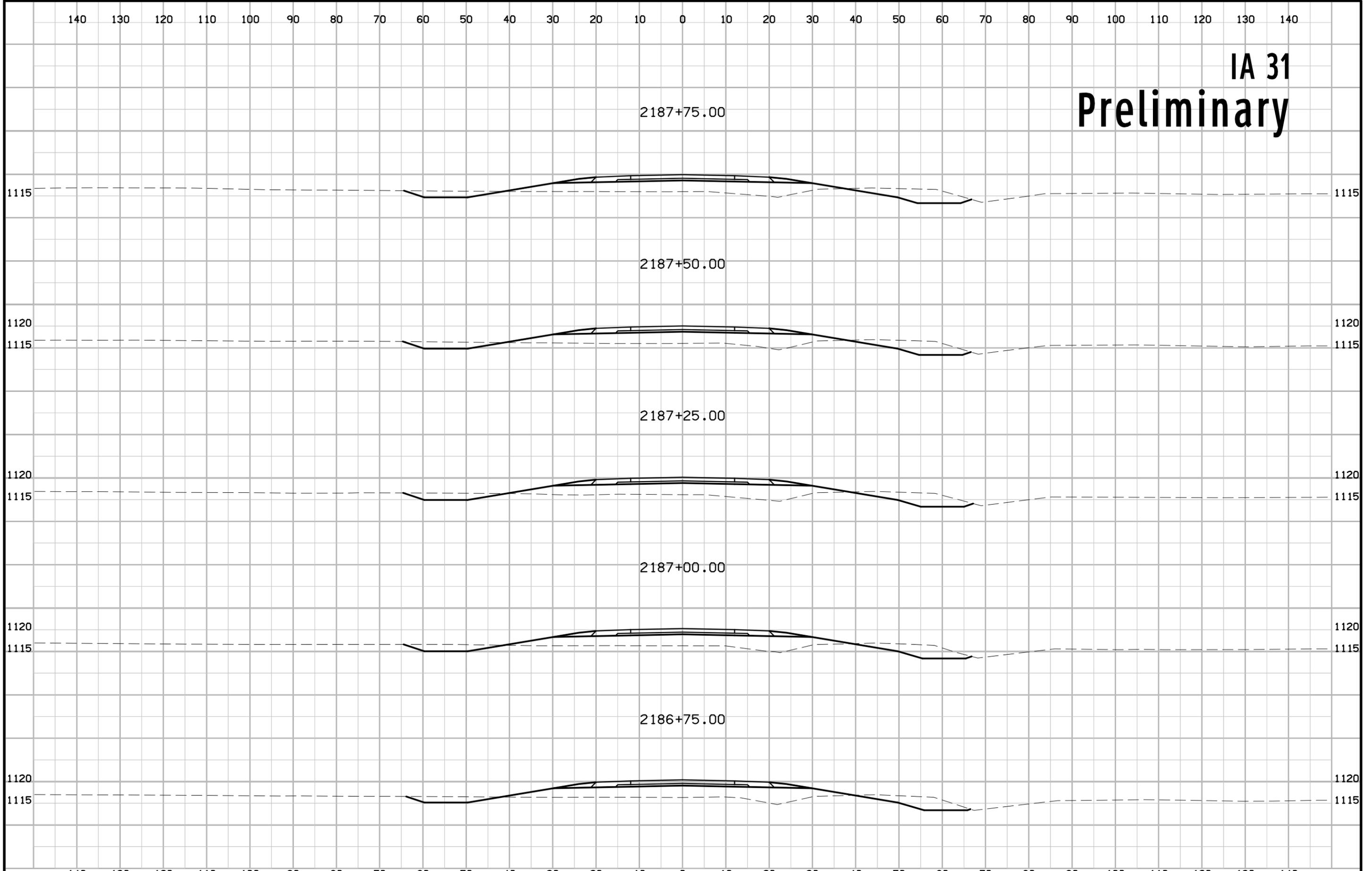
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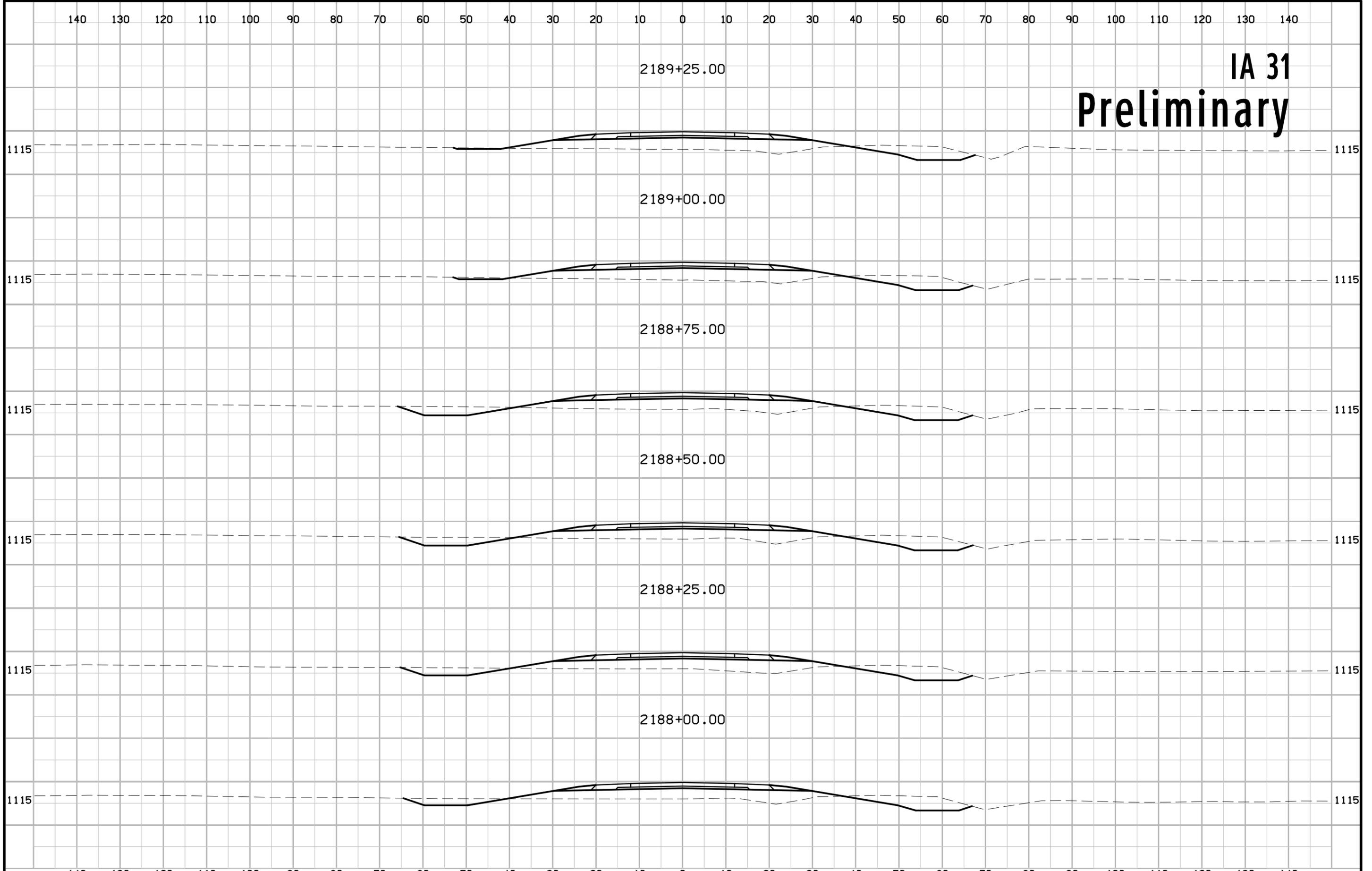
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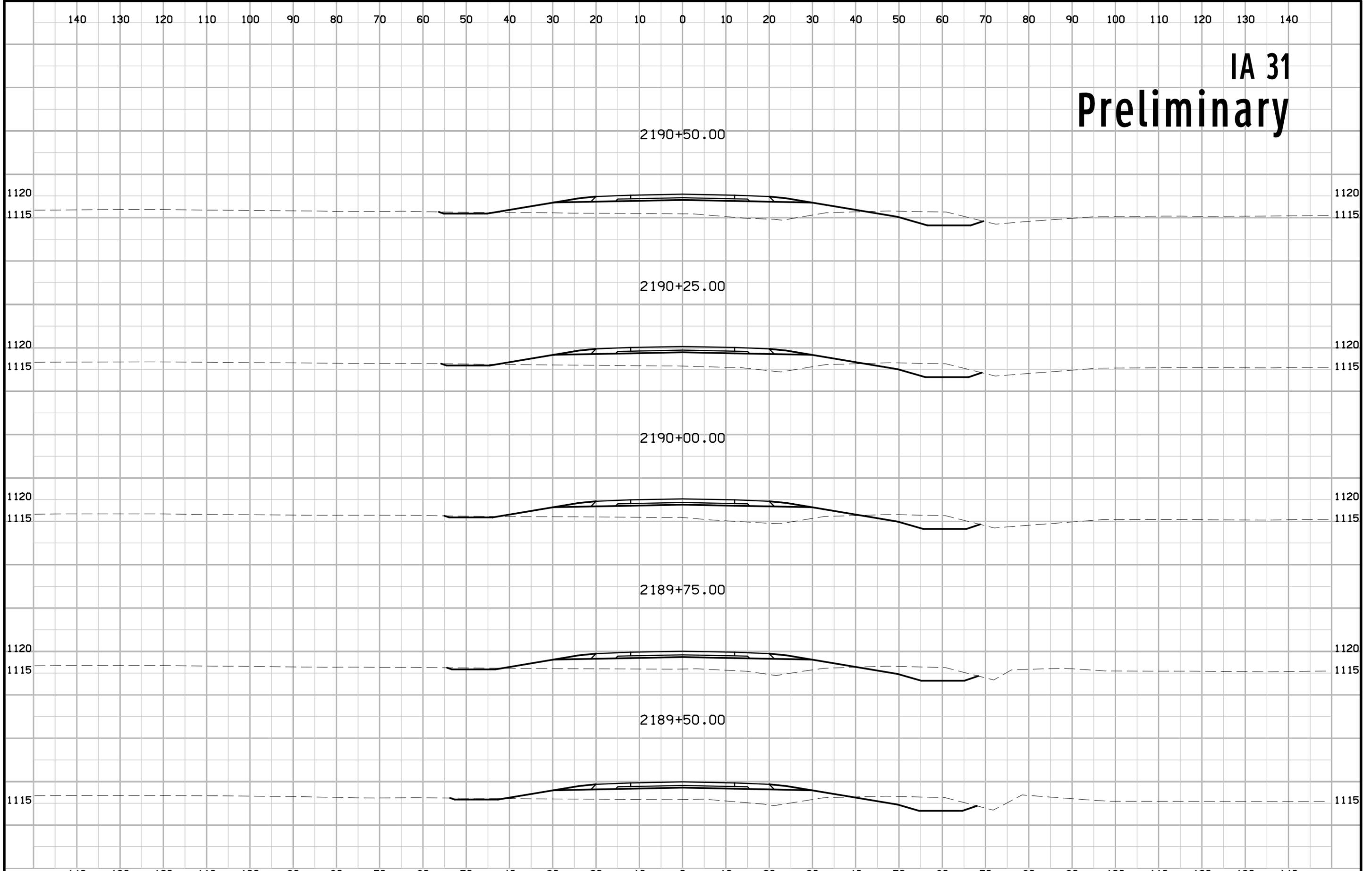
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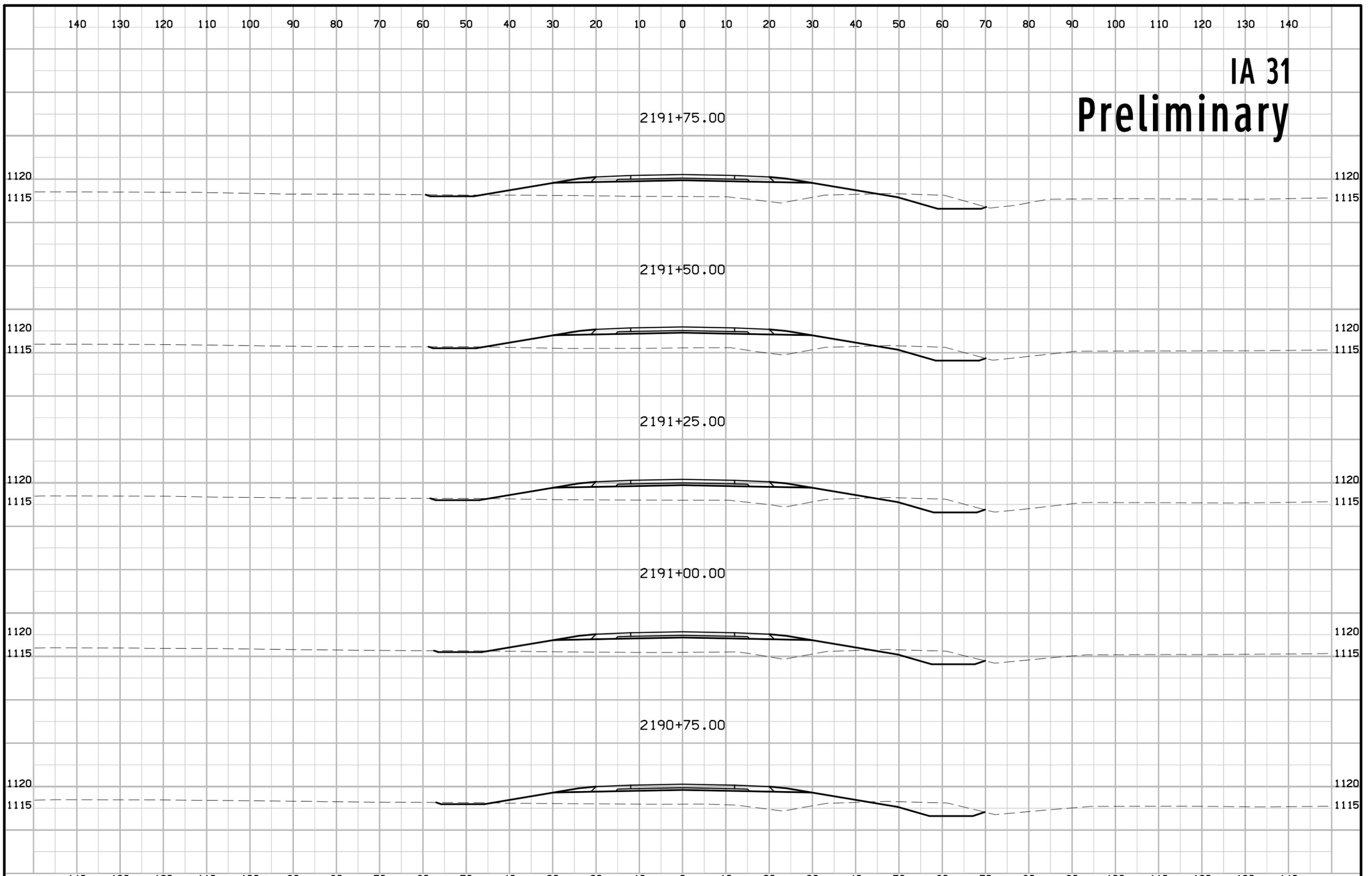
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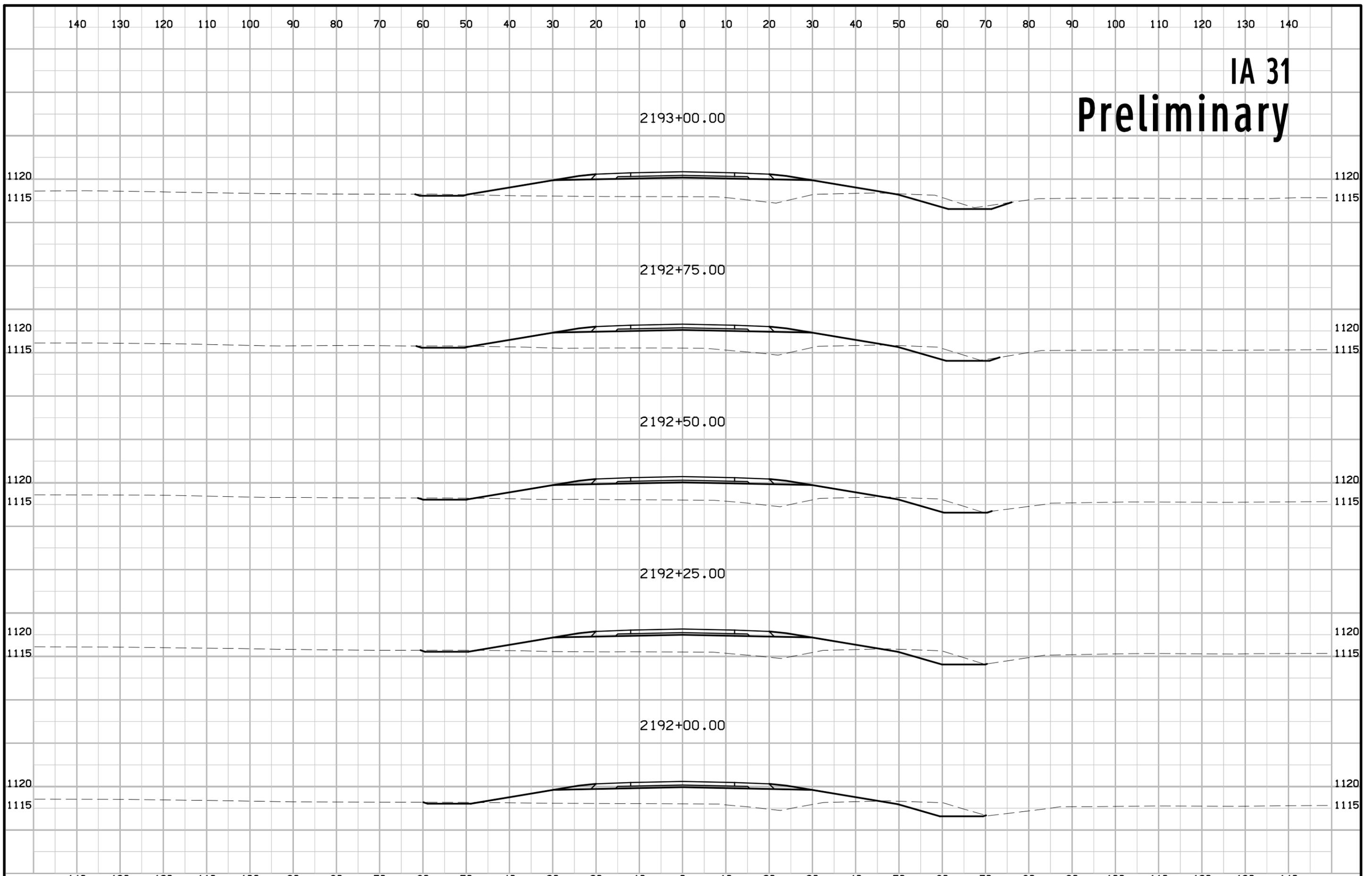
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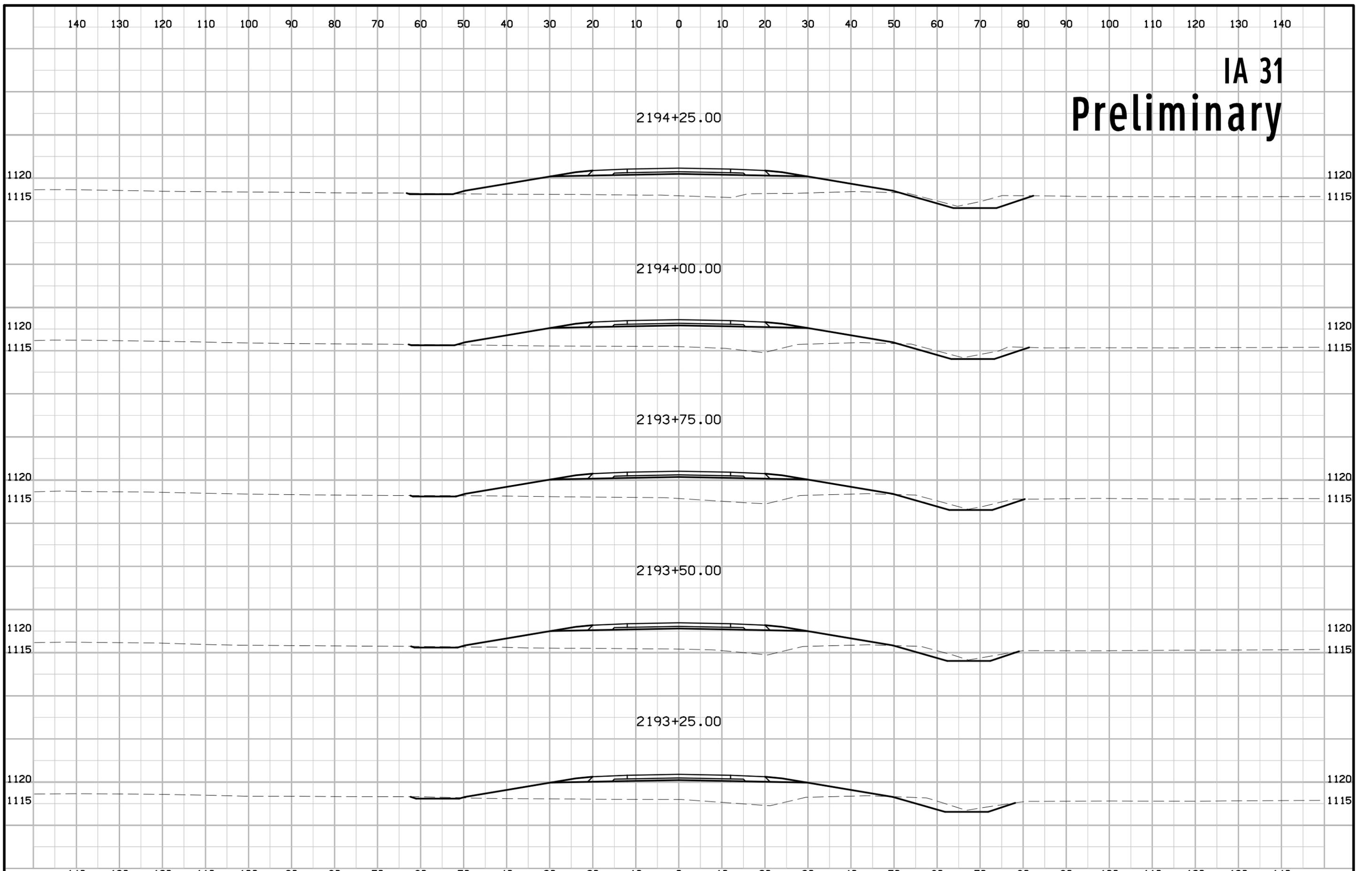
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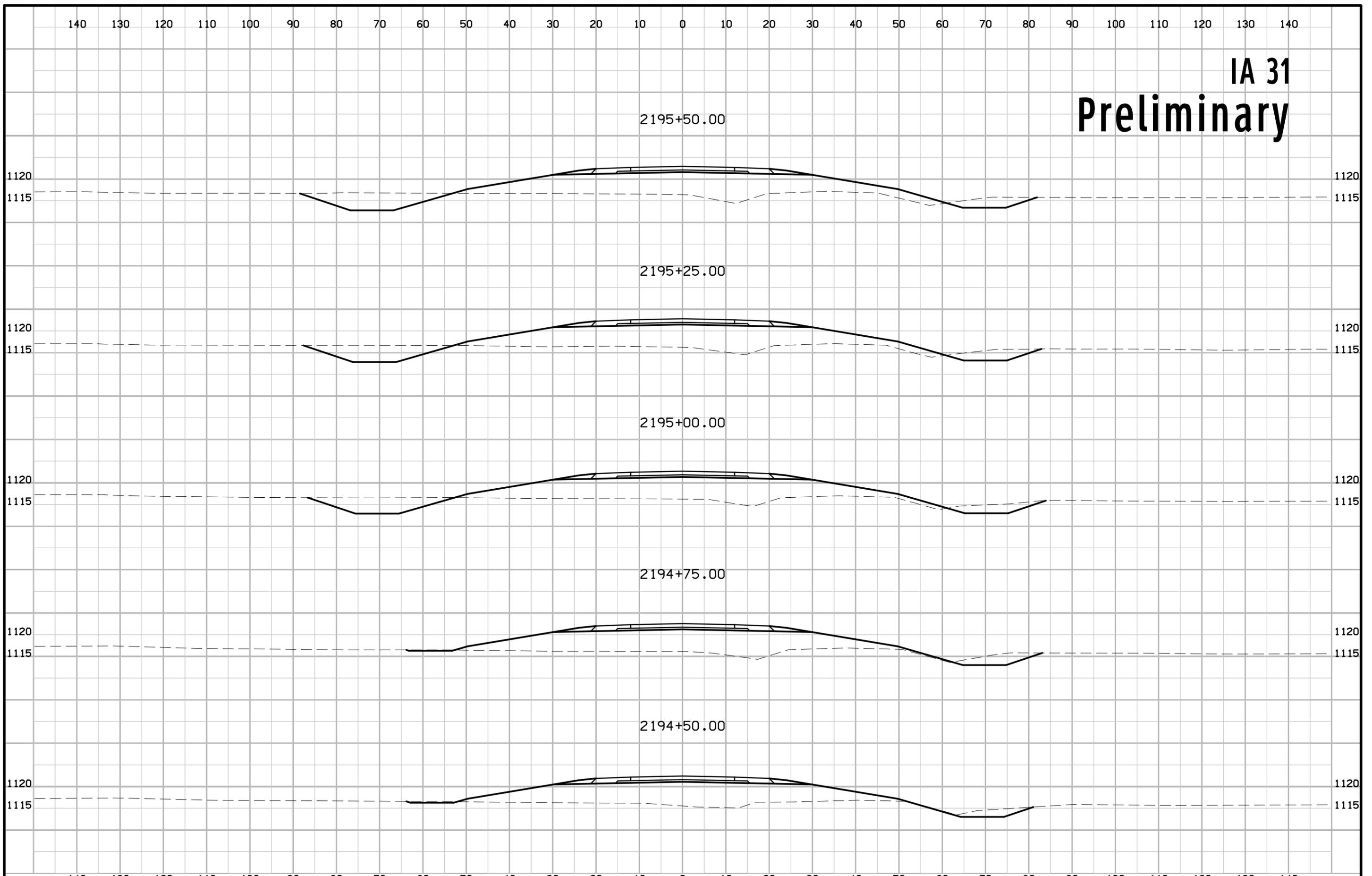
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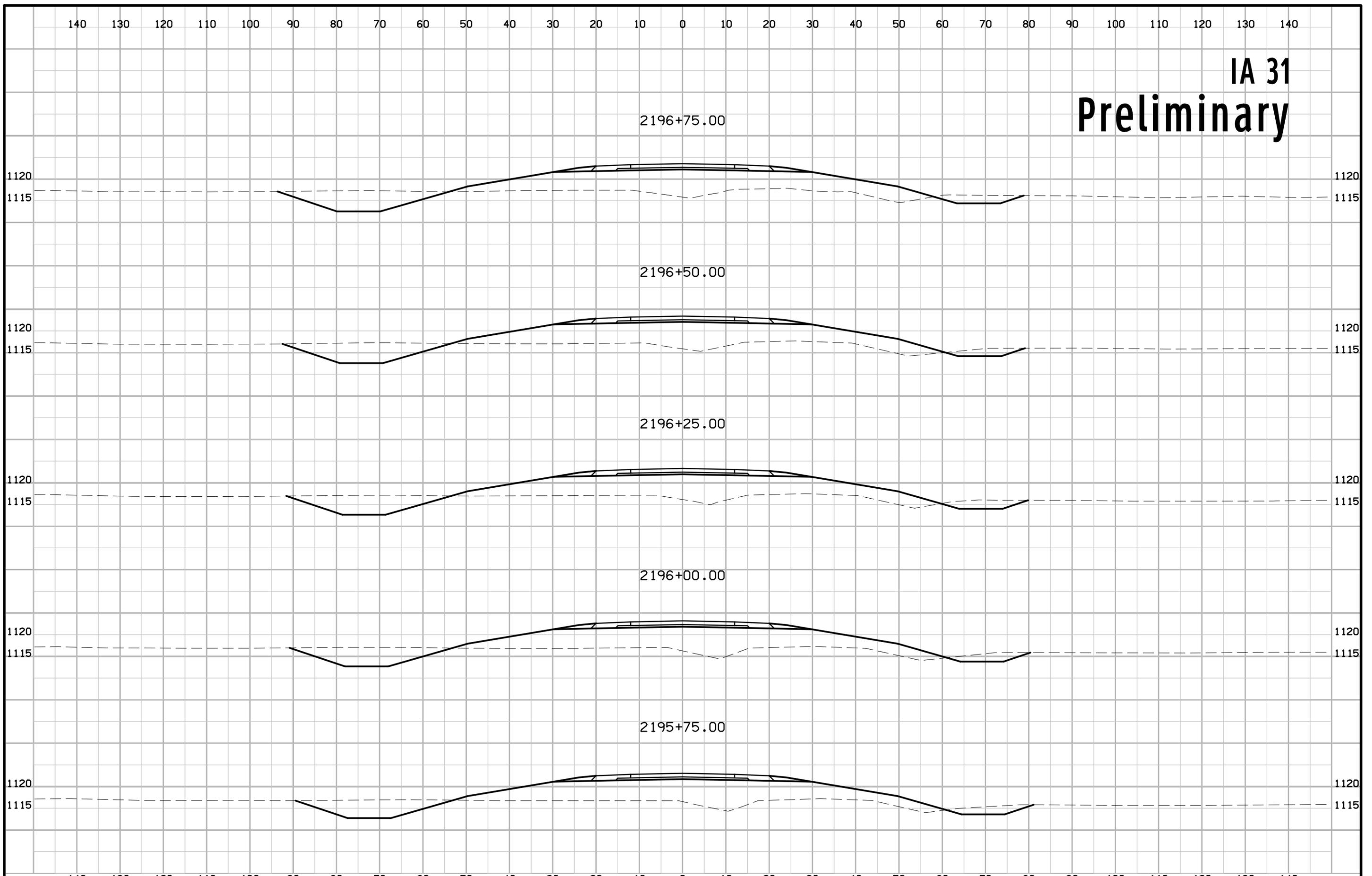
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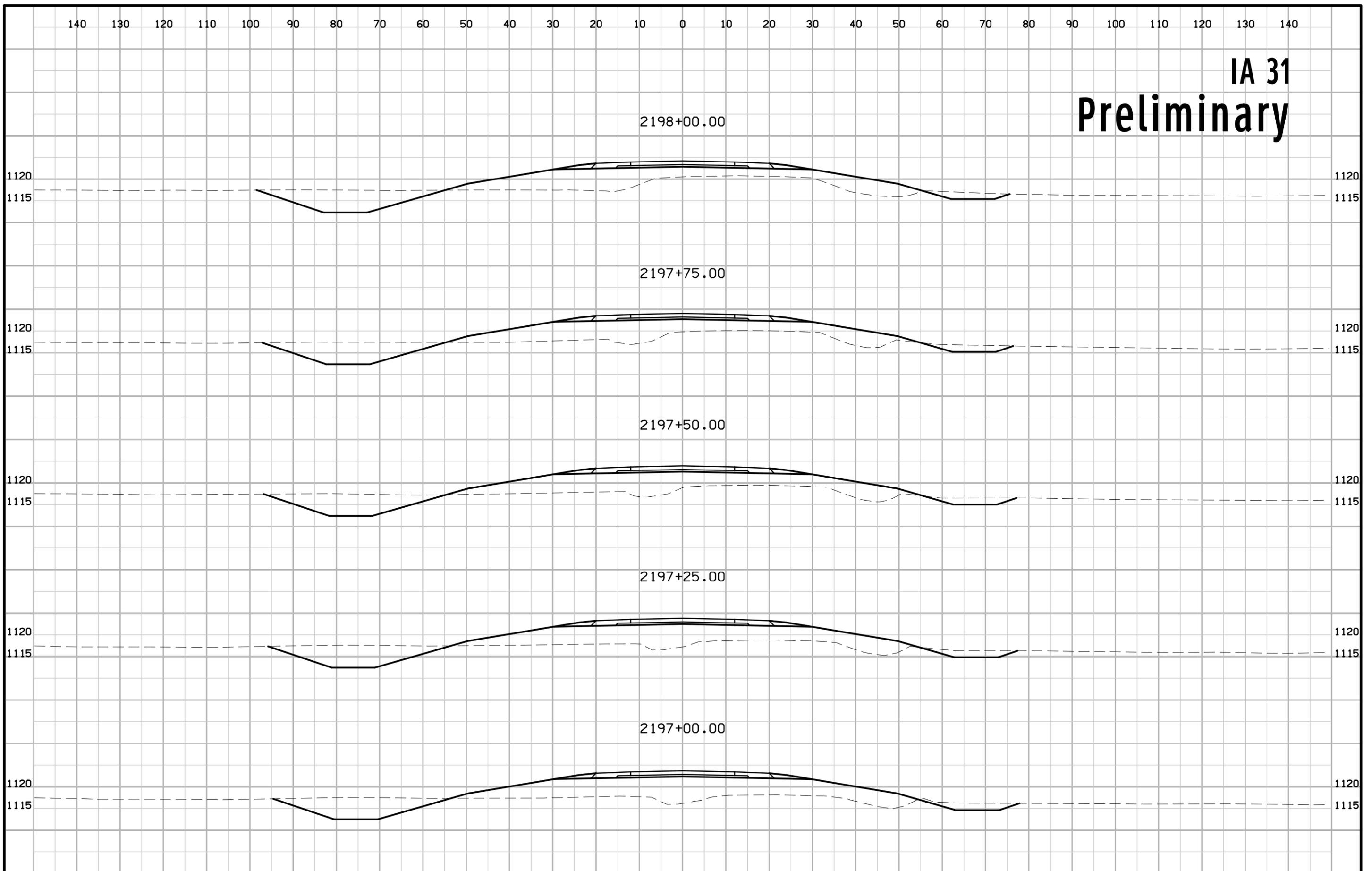
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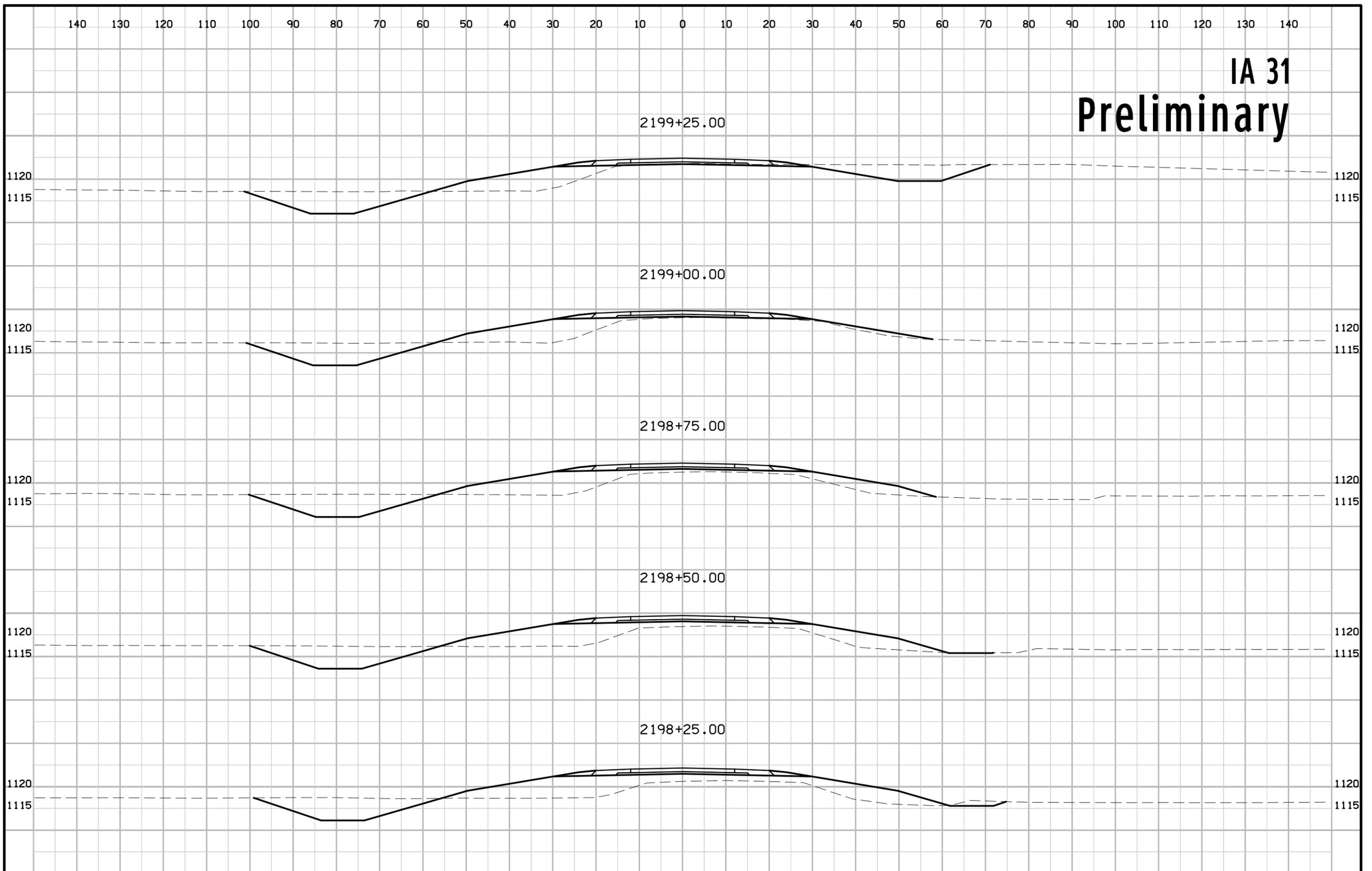
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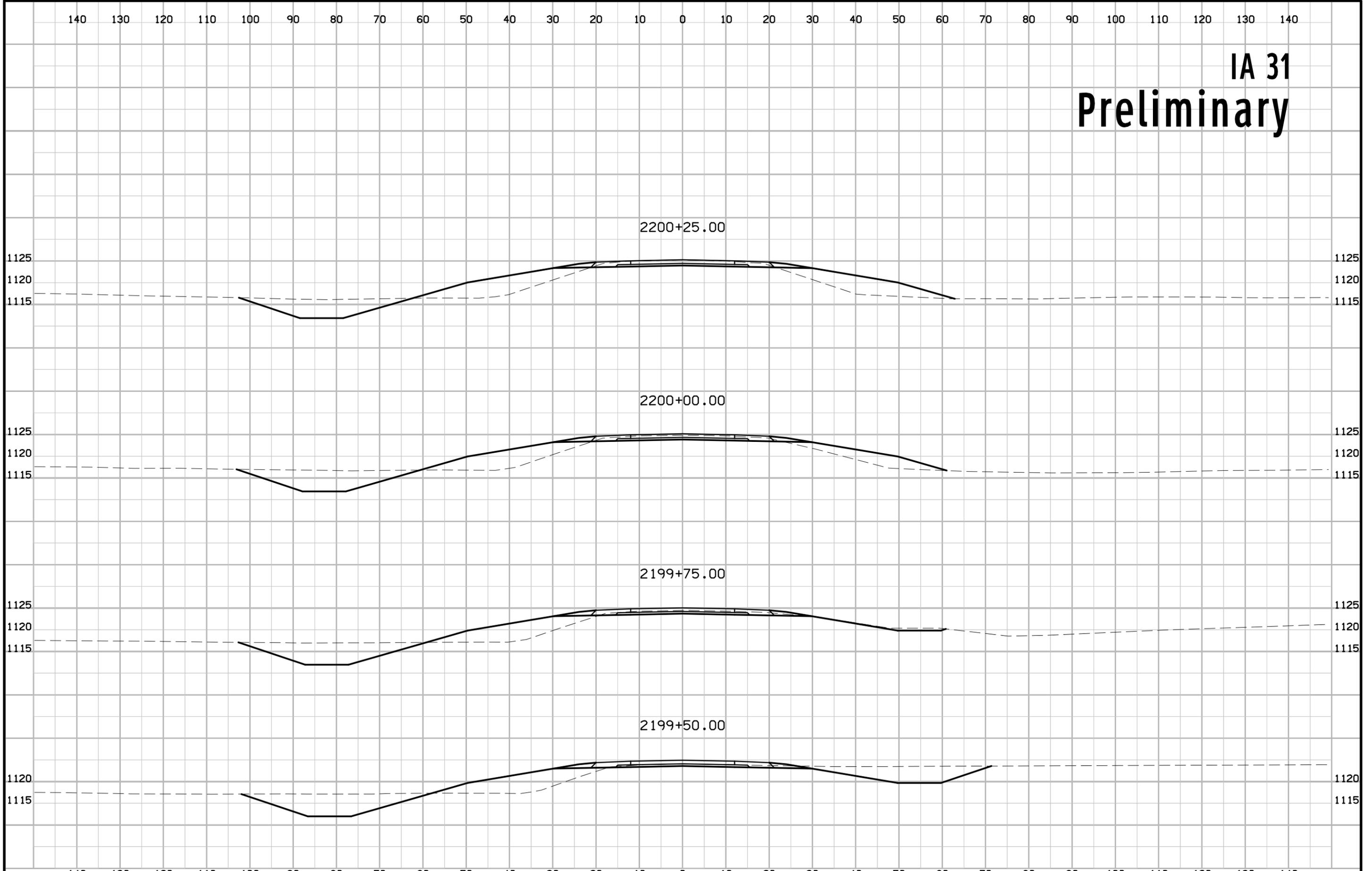
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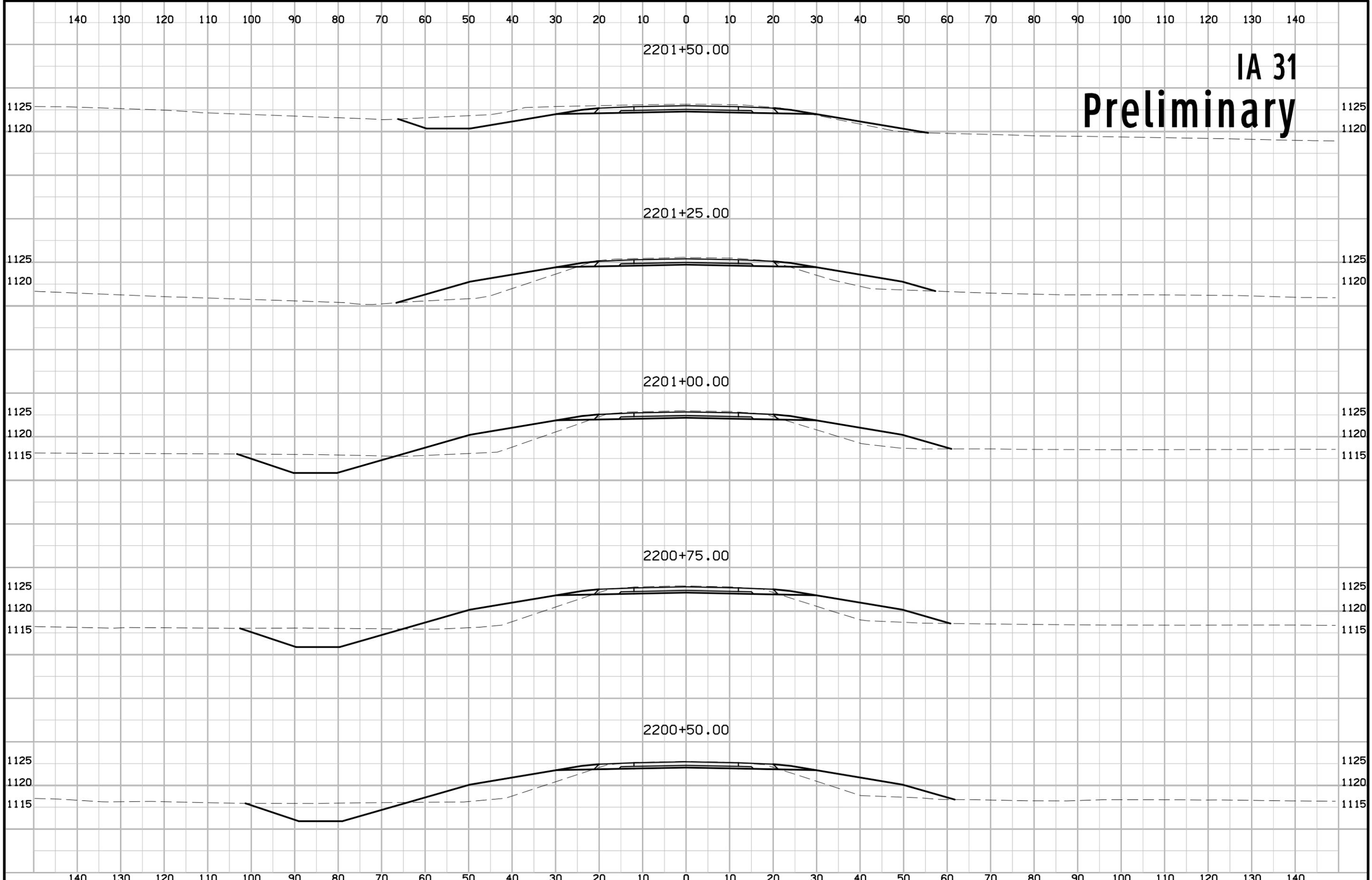
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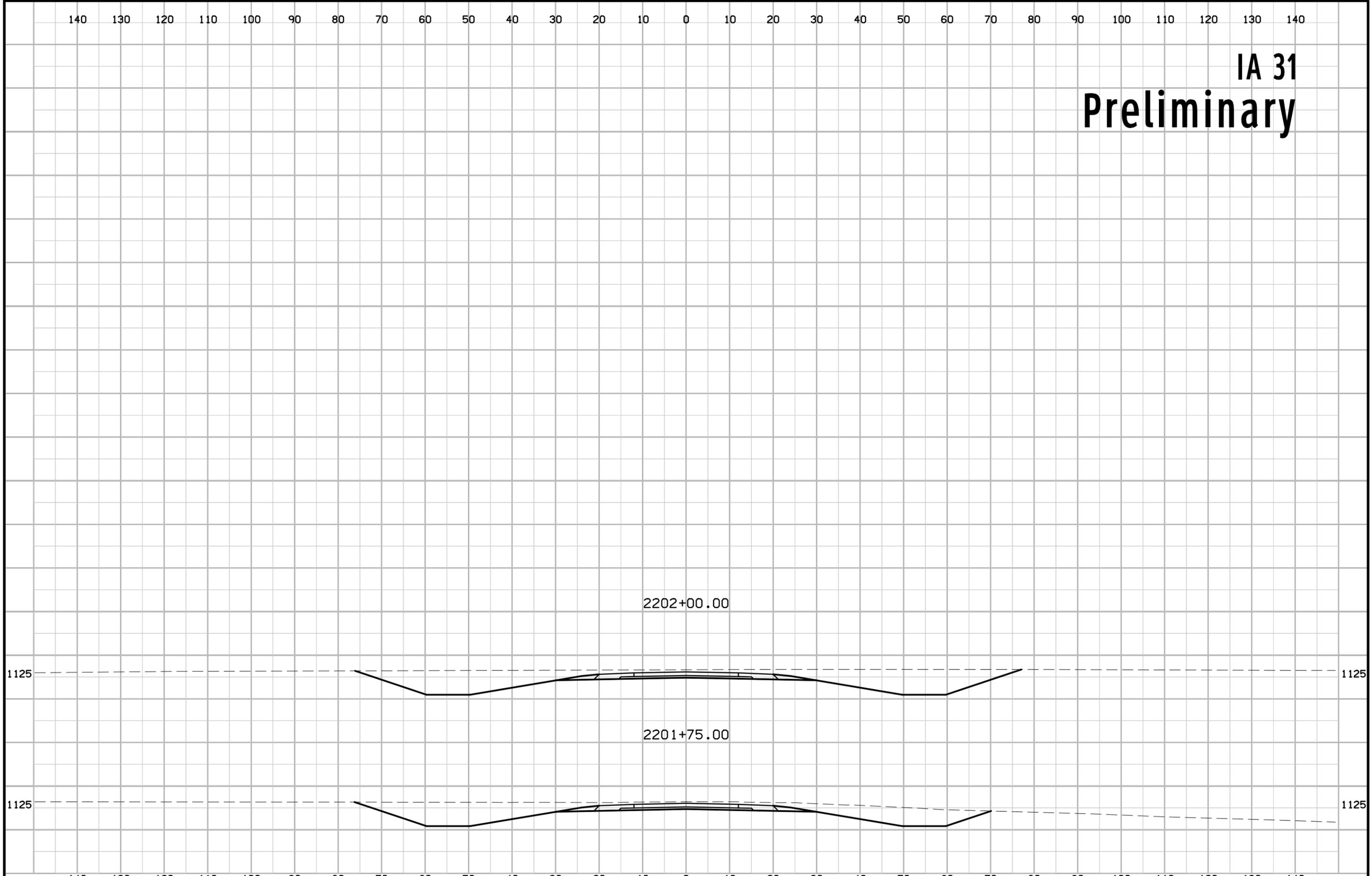
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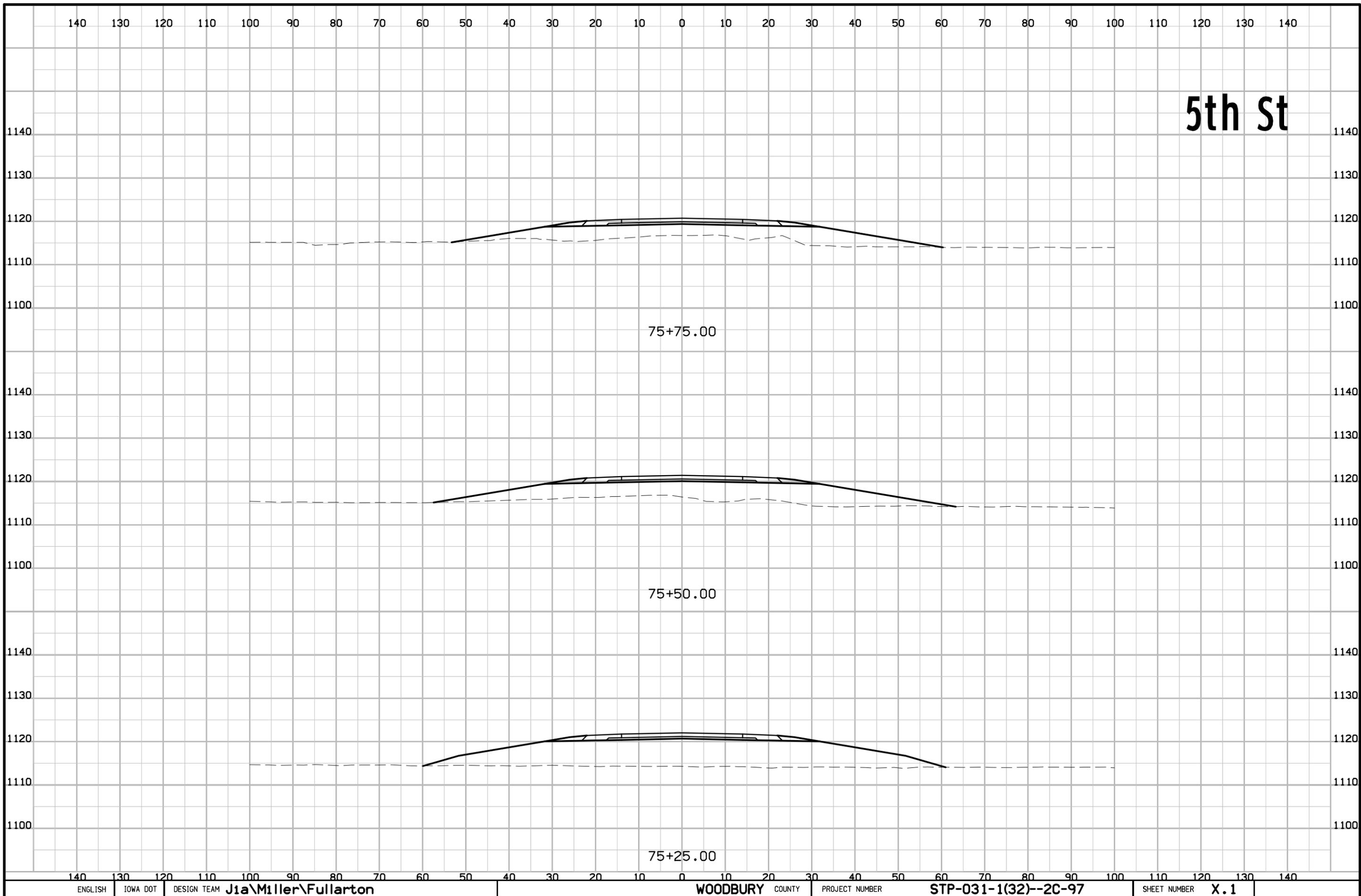


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IA 31 Preliminary



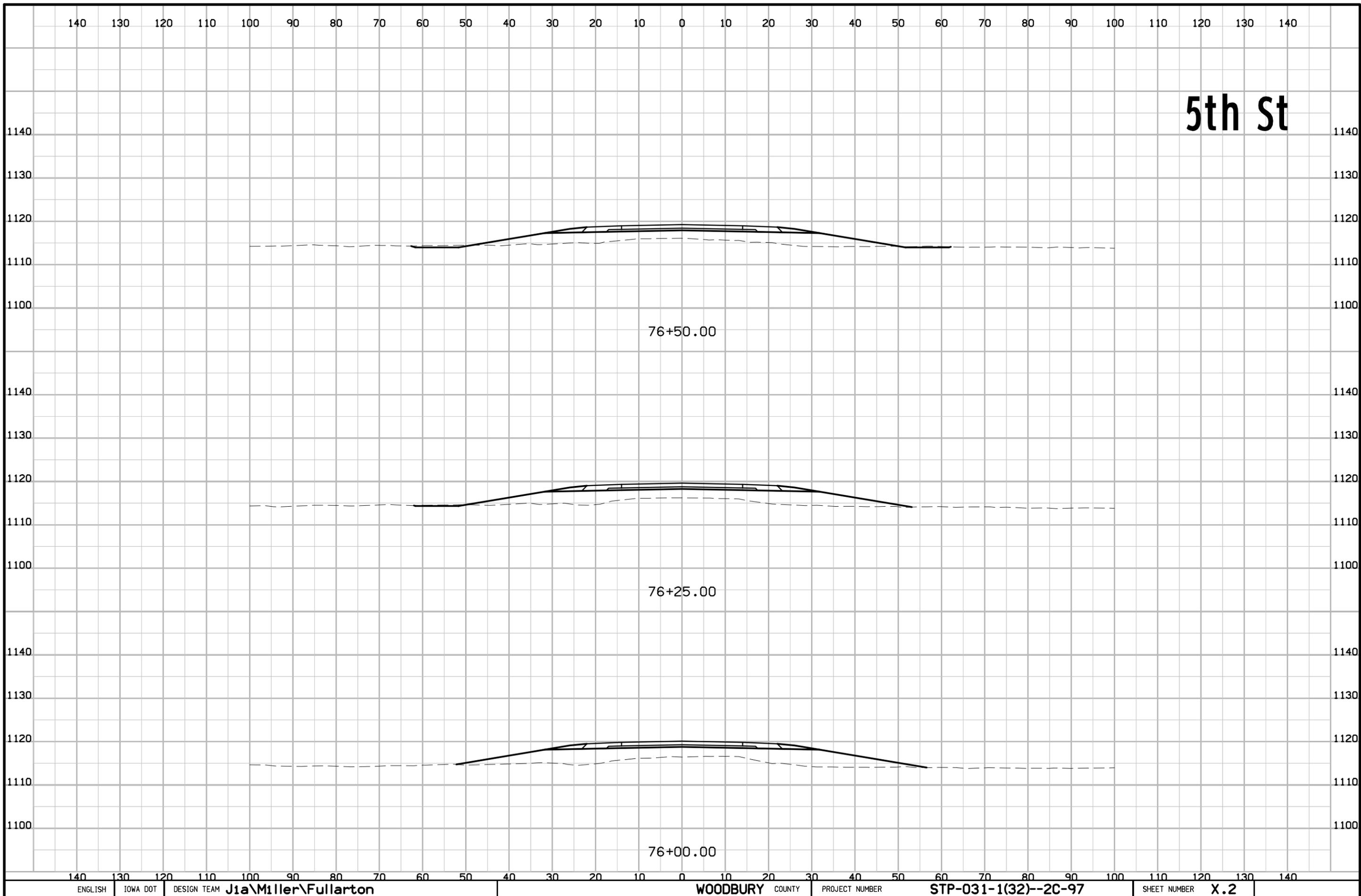


5th St

75+75.00

75+50.00

75+25.00

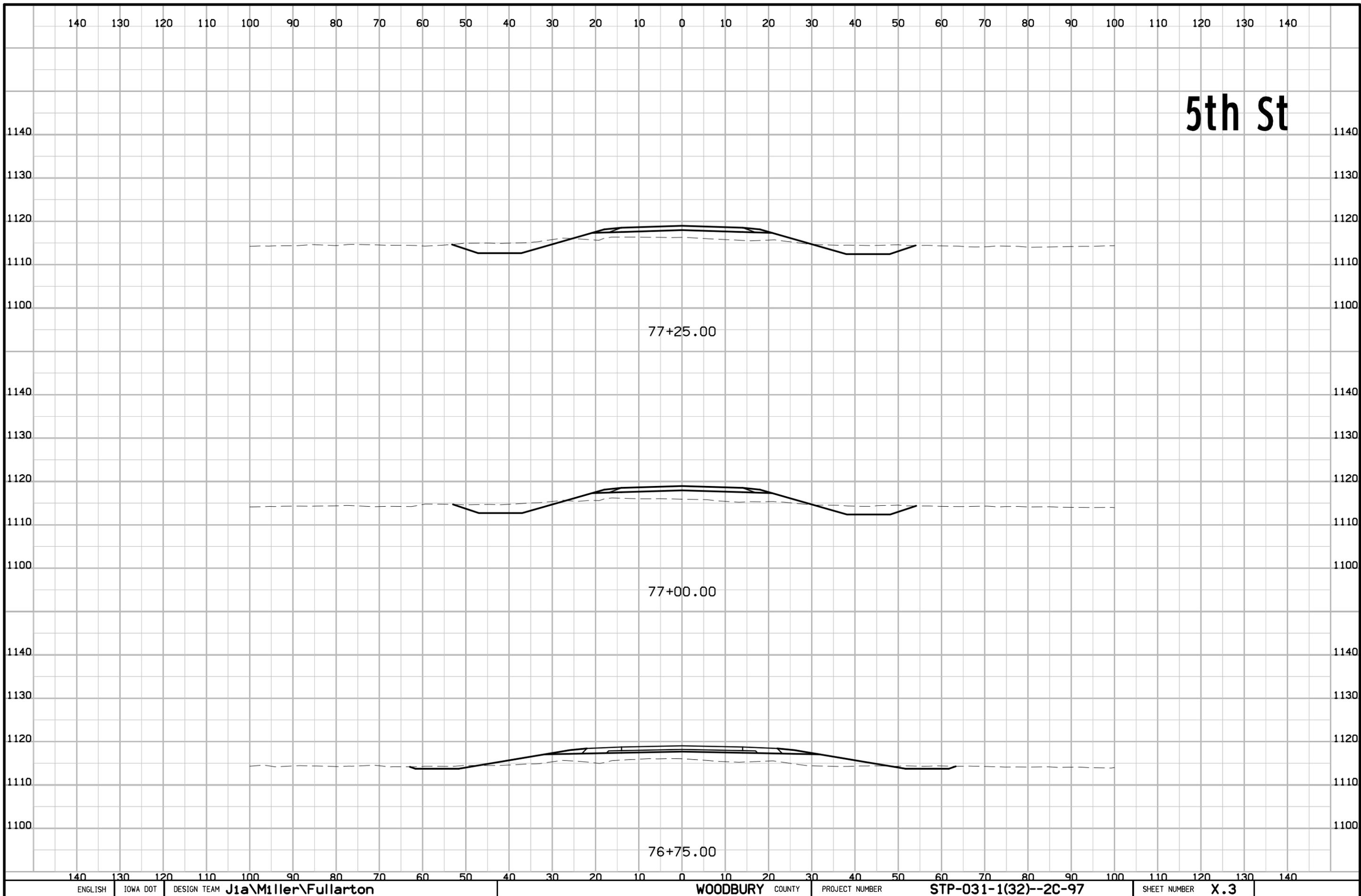


5th St

76+50.00

76+25.00

76+00.00

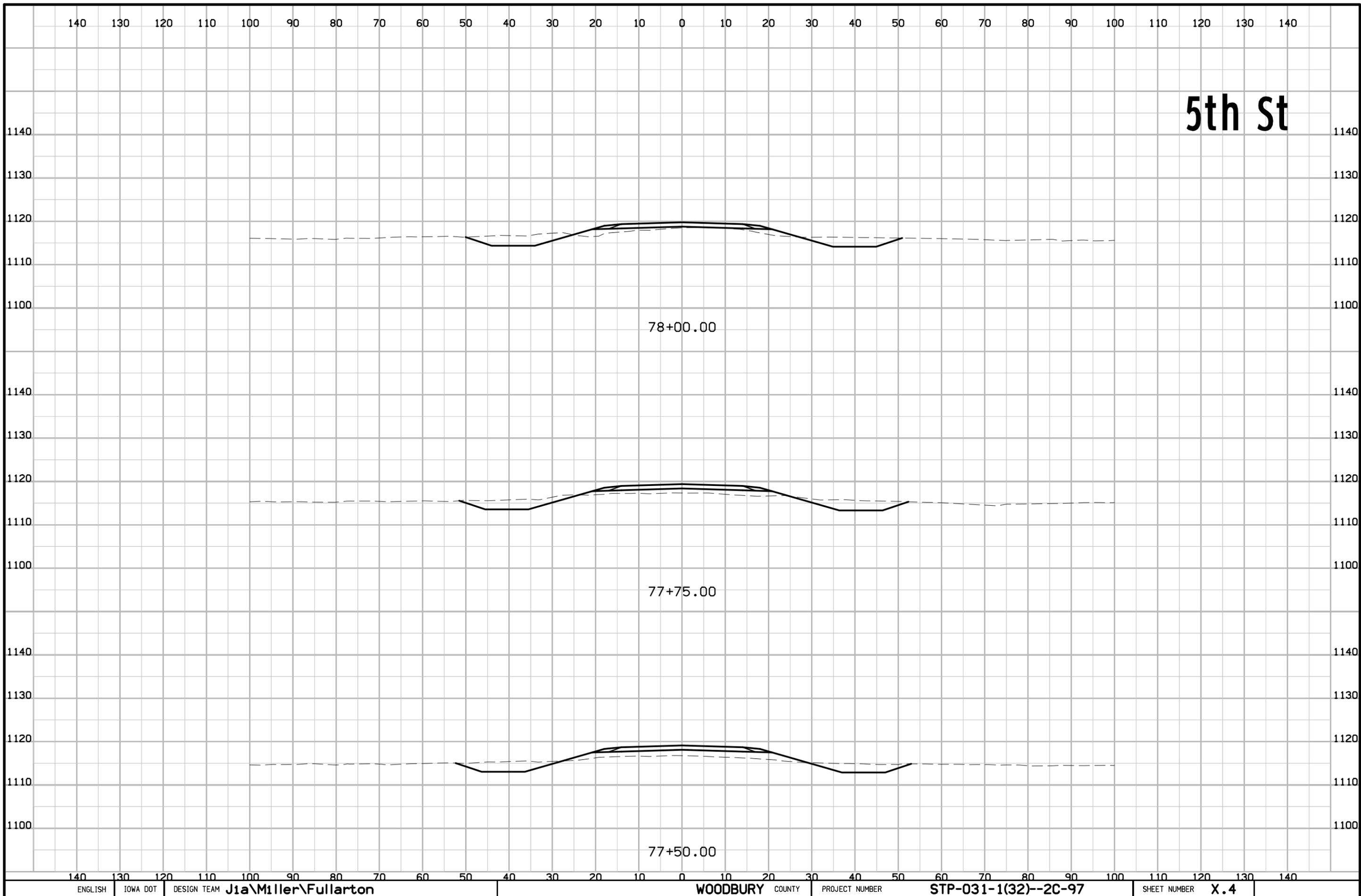


5th St

77+25.00

77+00.00

76+75.00



5th St

78+00.00

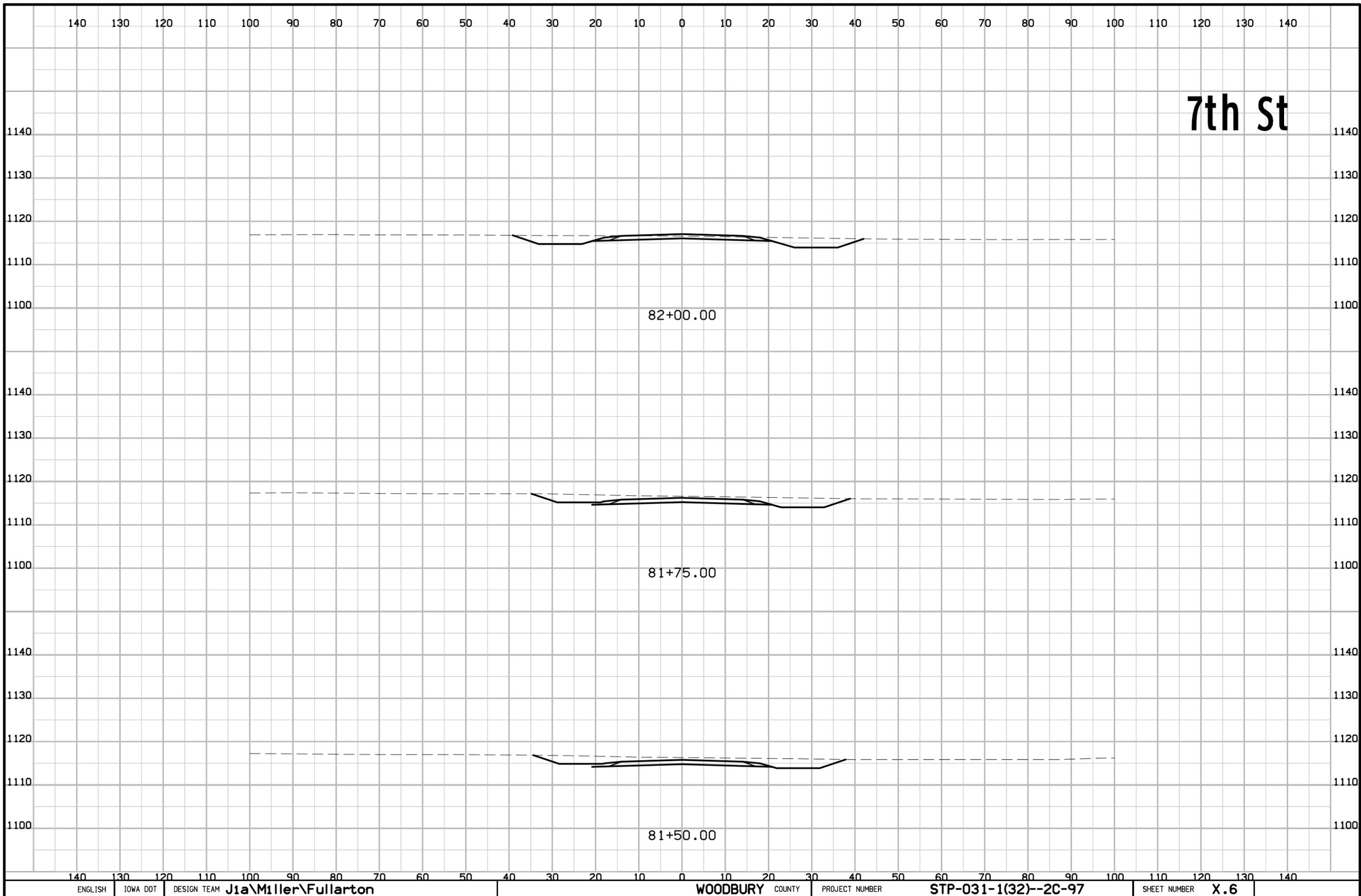
77+75.00

77+50.00

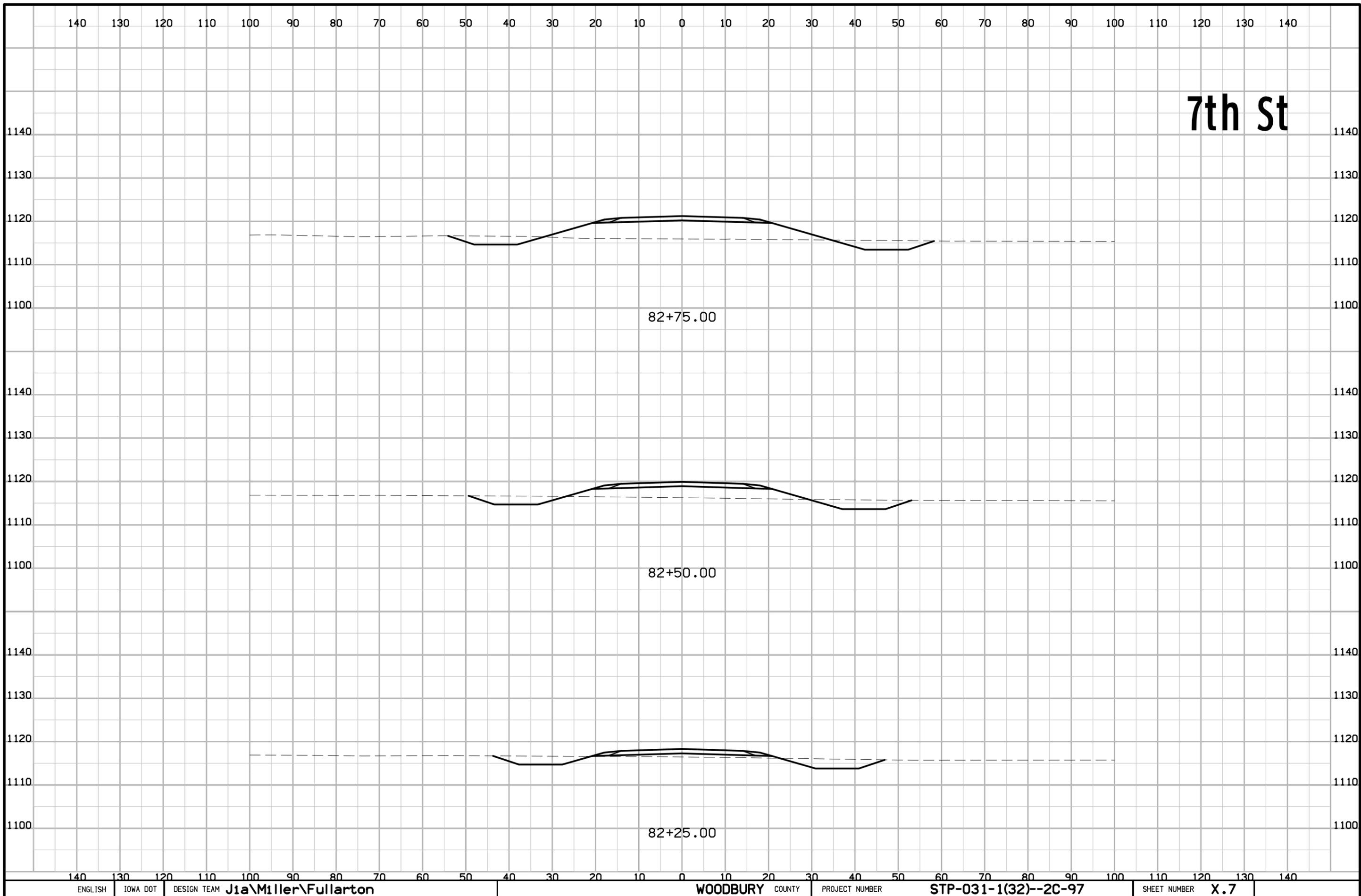
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5th St





7th St

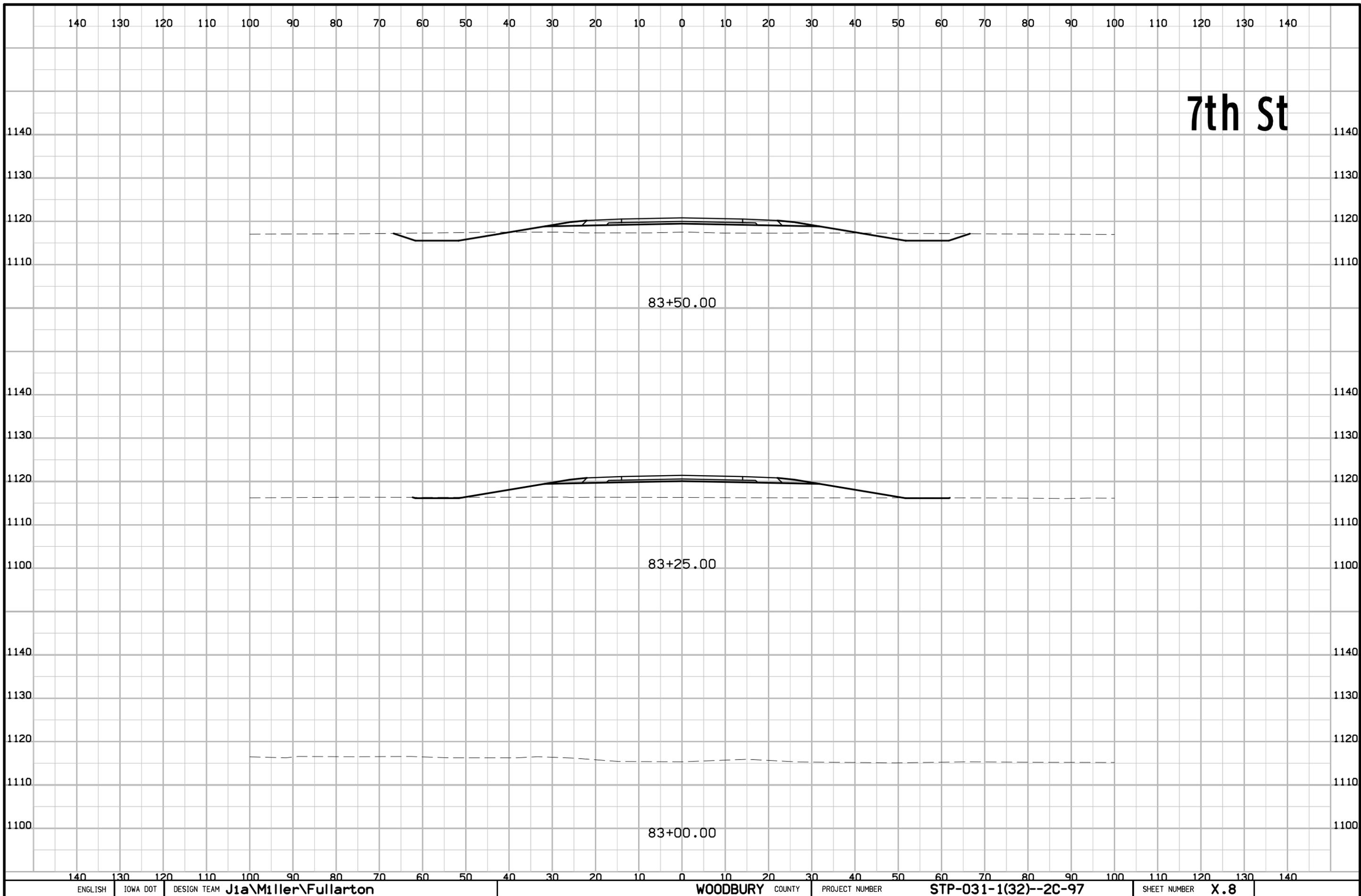


7th St

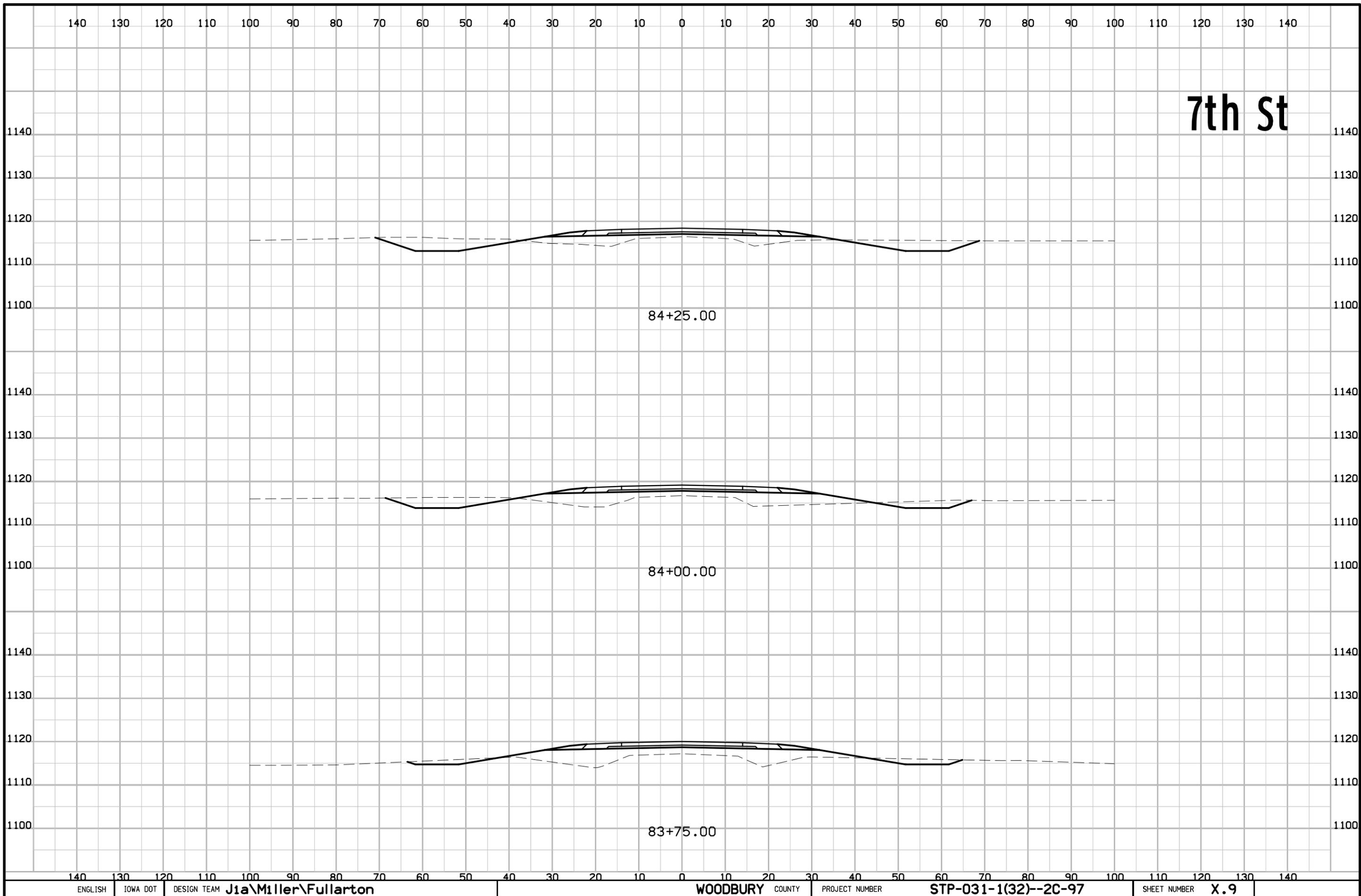
82+75.00

82+50.00

82+25.00



7th St

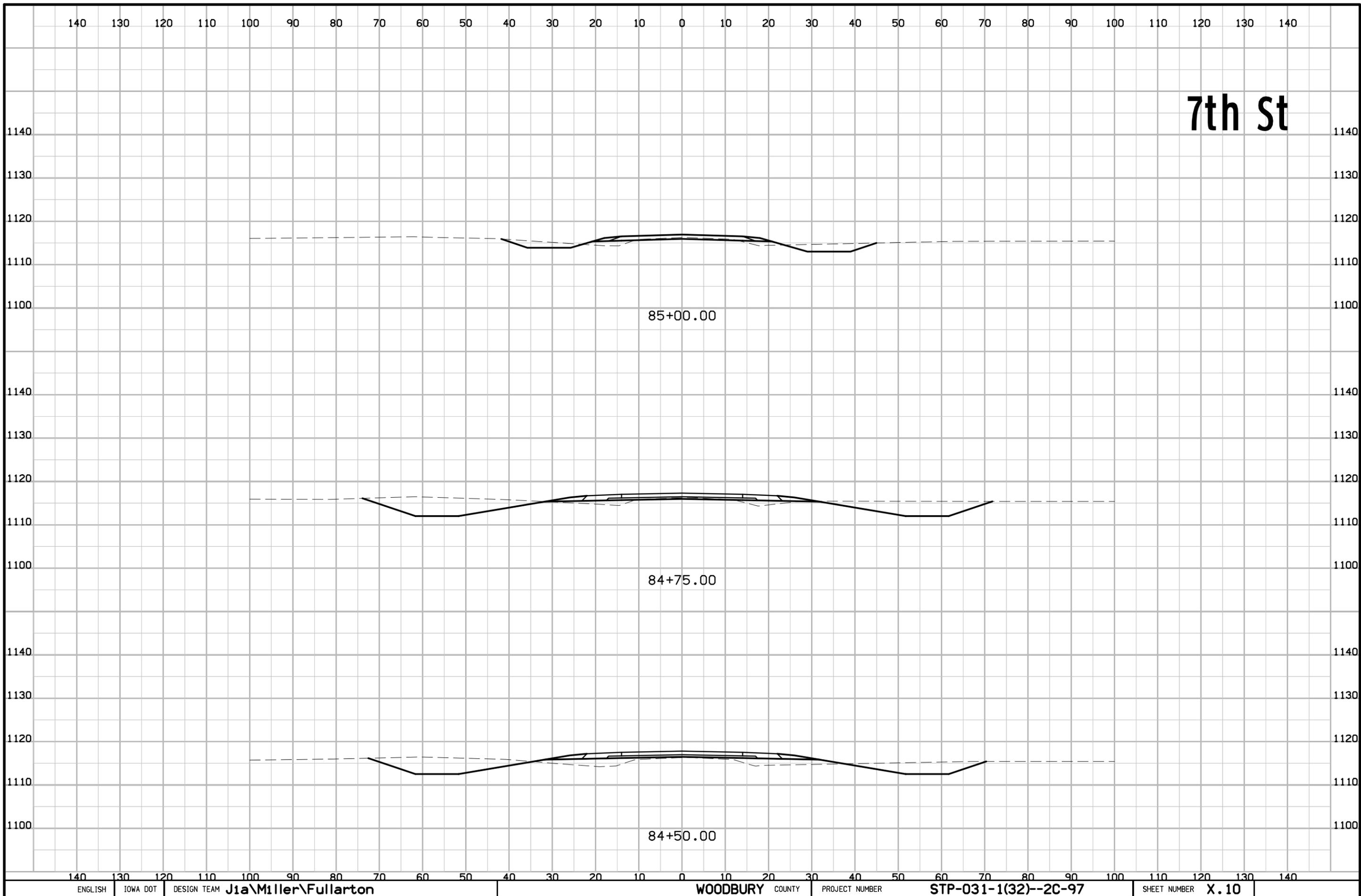


7th St

84+25.00

84+00.00

83+75.00

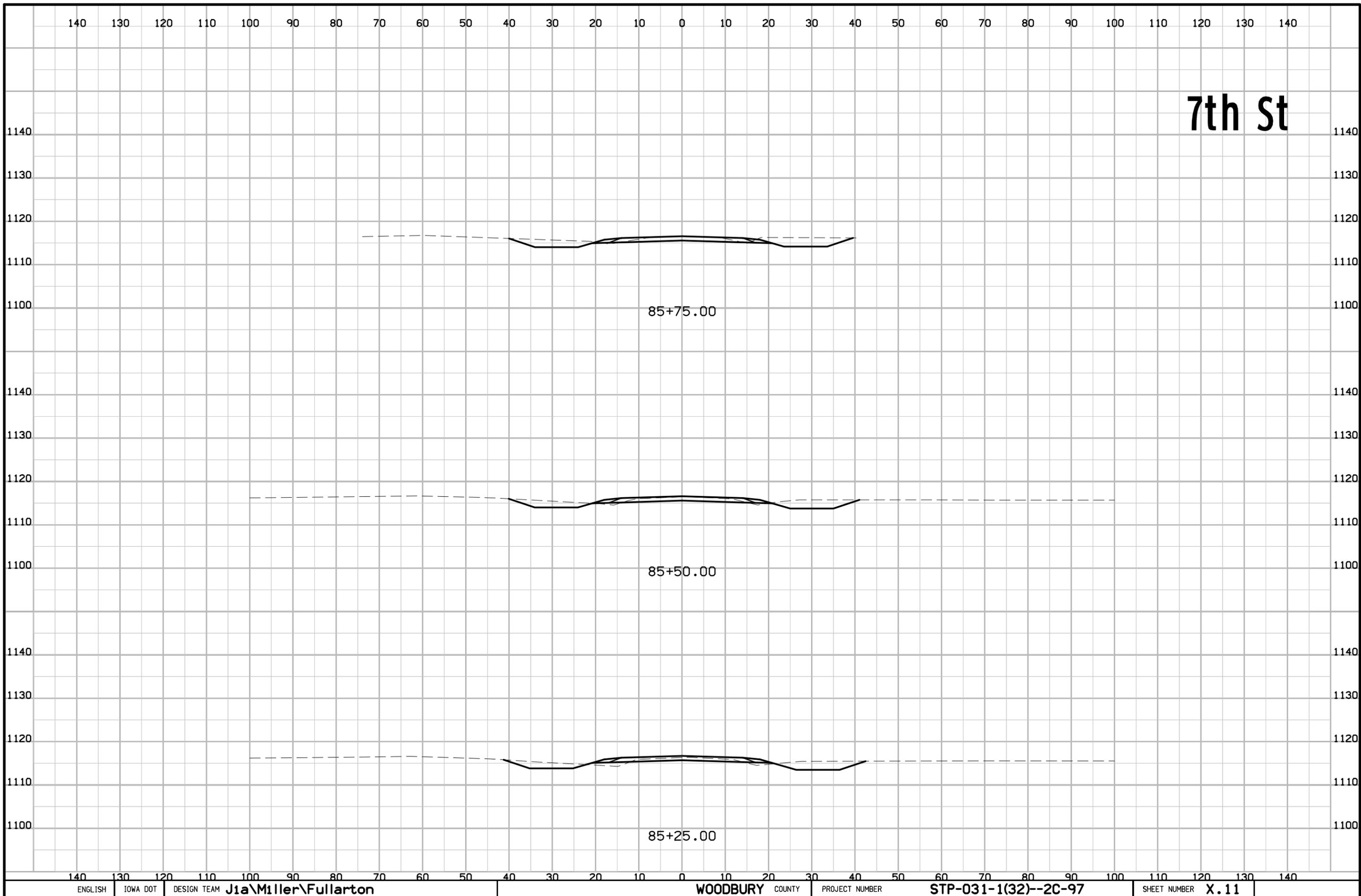


7th St

85+00.00

84+75.00

84+50.00

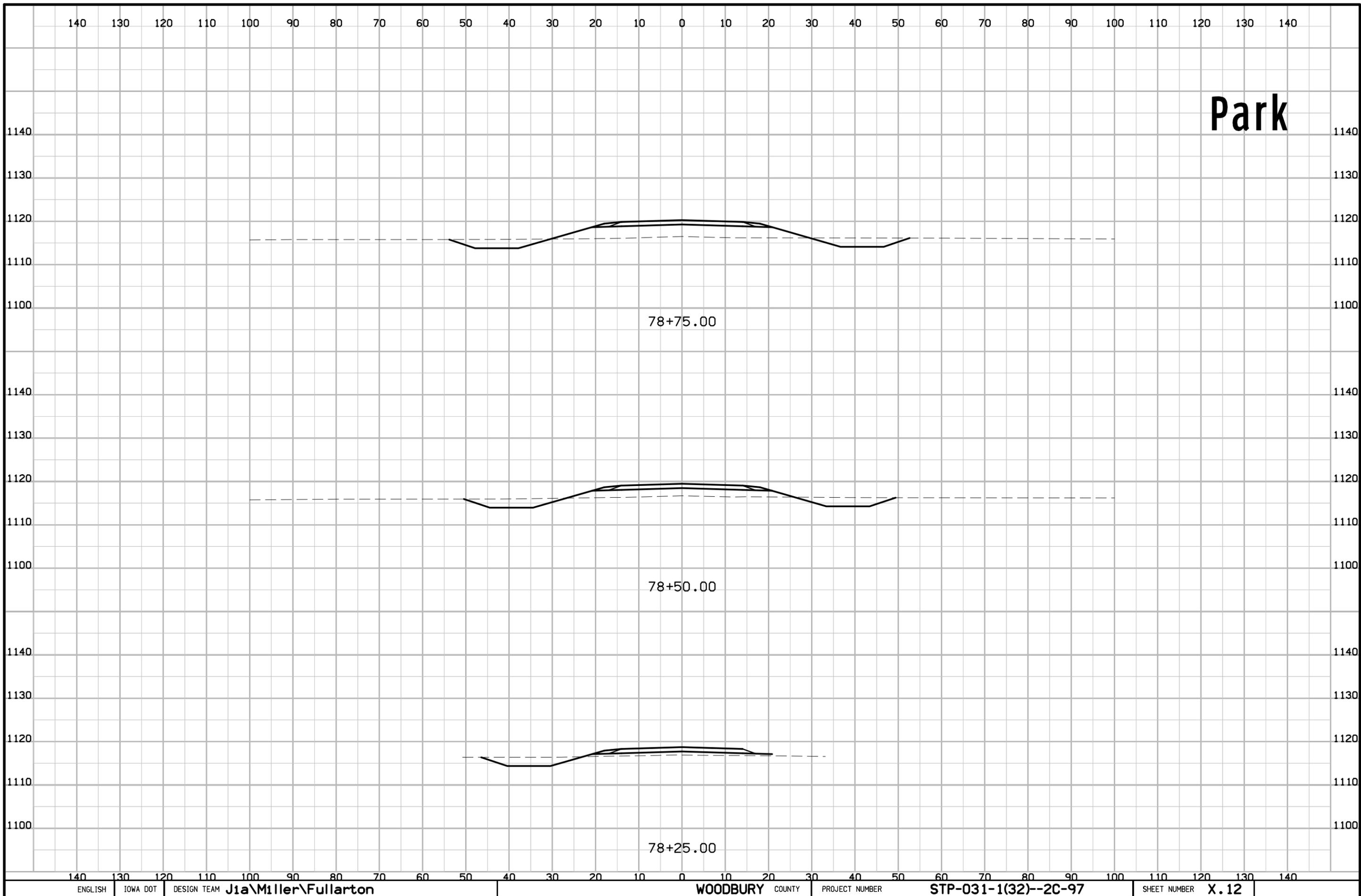


7th St

85+75.00

85+50.00

85+25.00

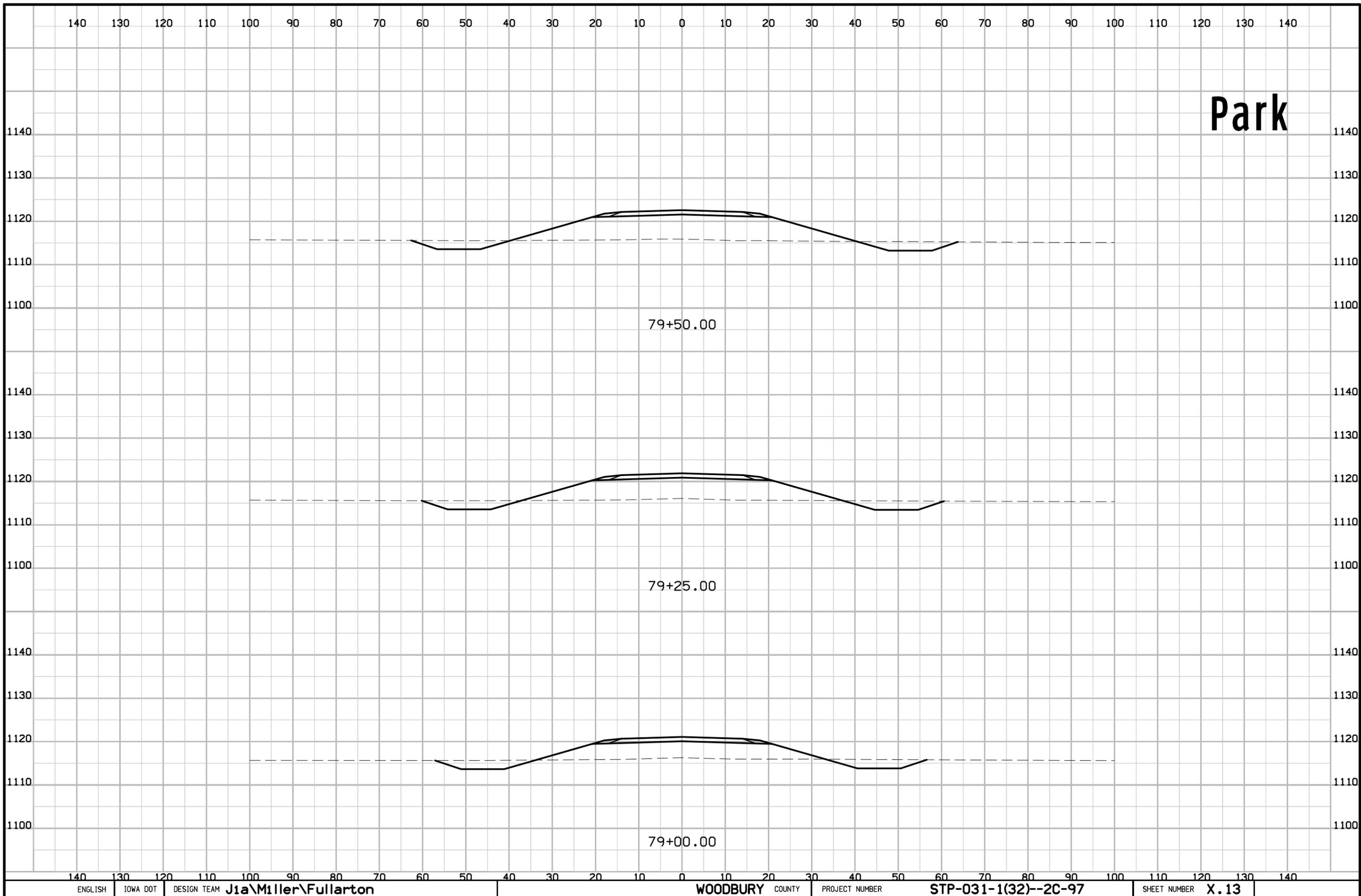


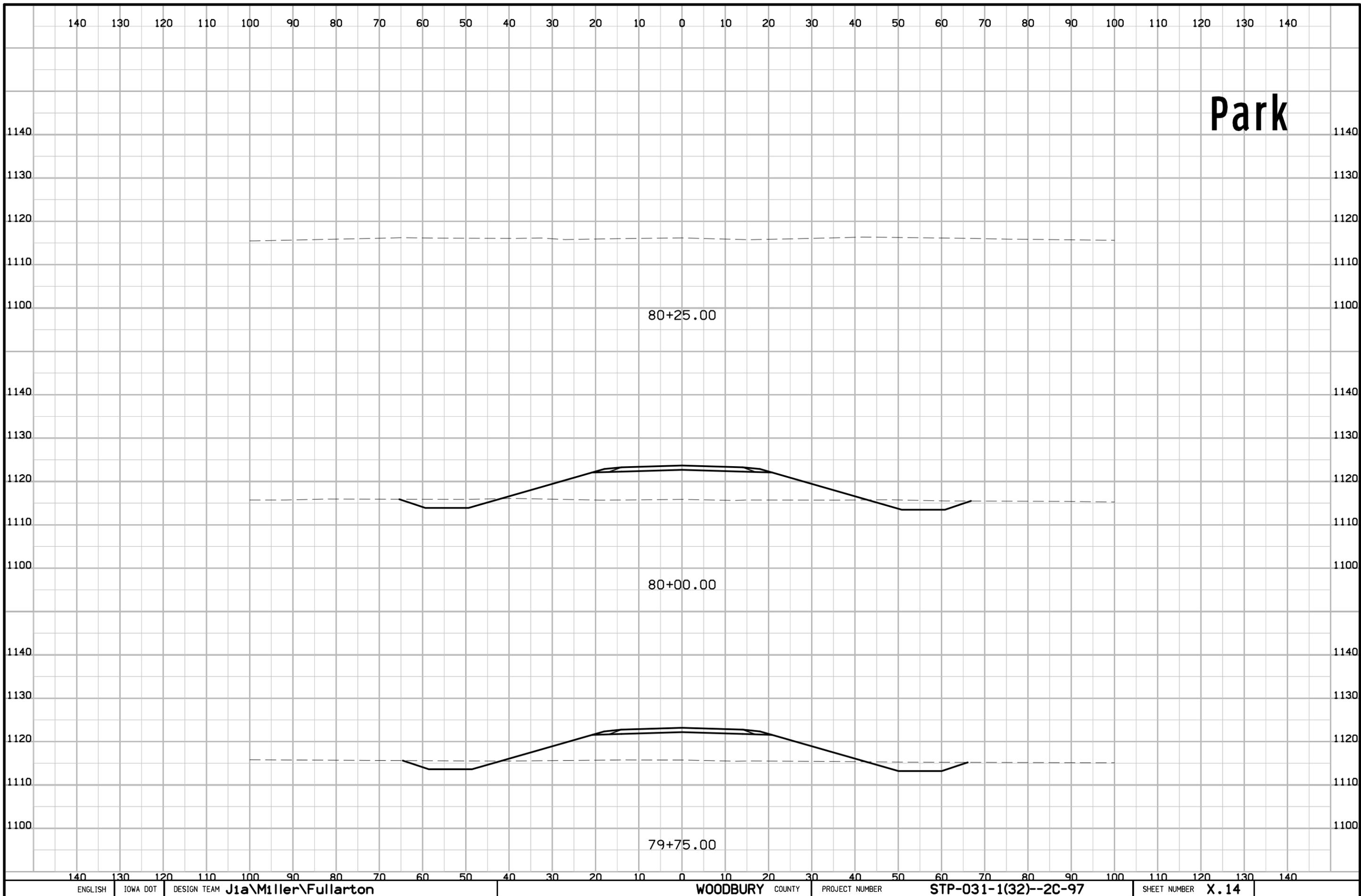
Park

78+75.00

78+50.00

78+25.00



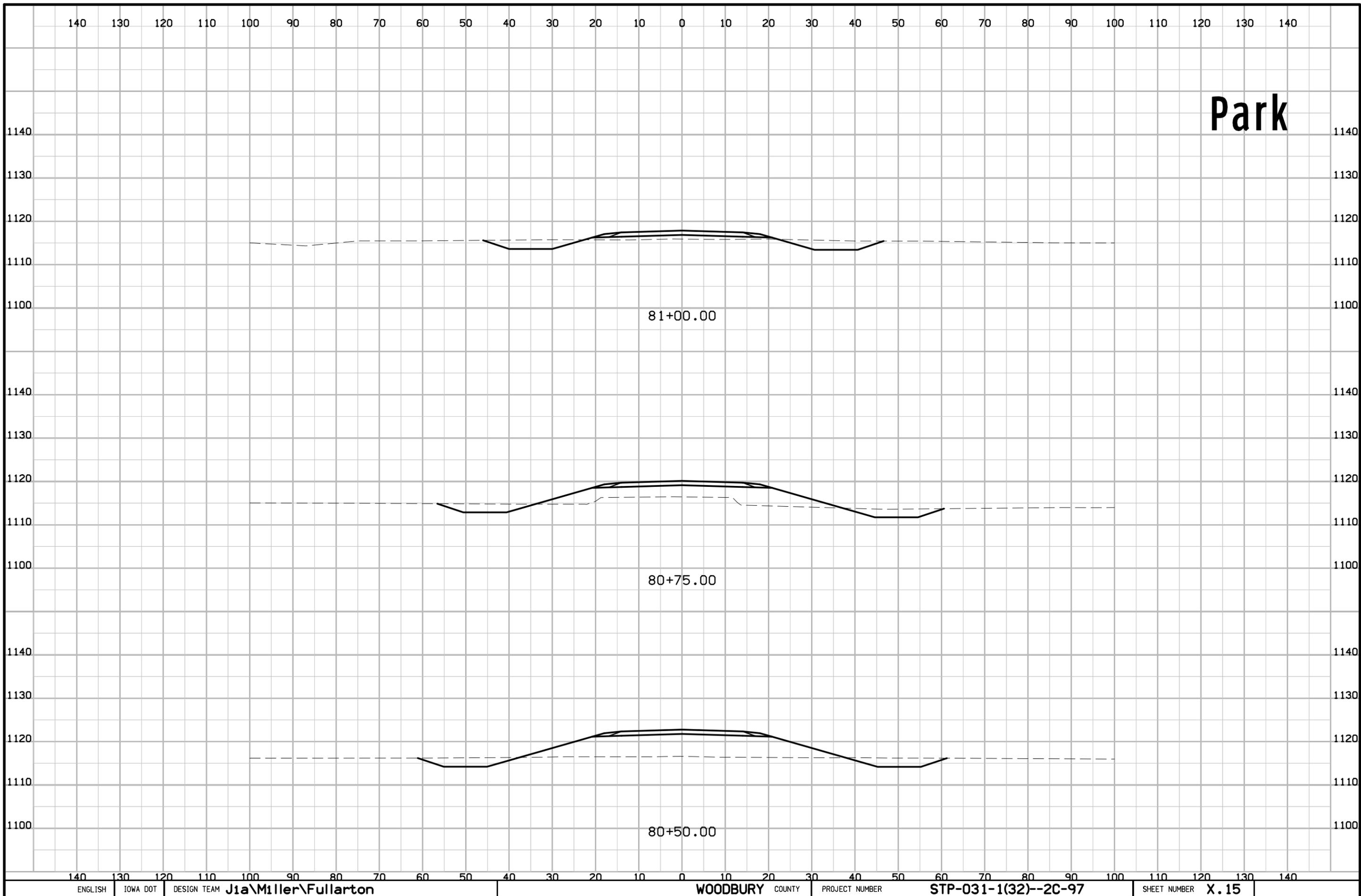


Park

80+25.00

80+00.00

79+75.00

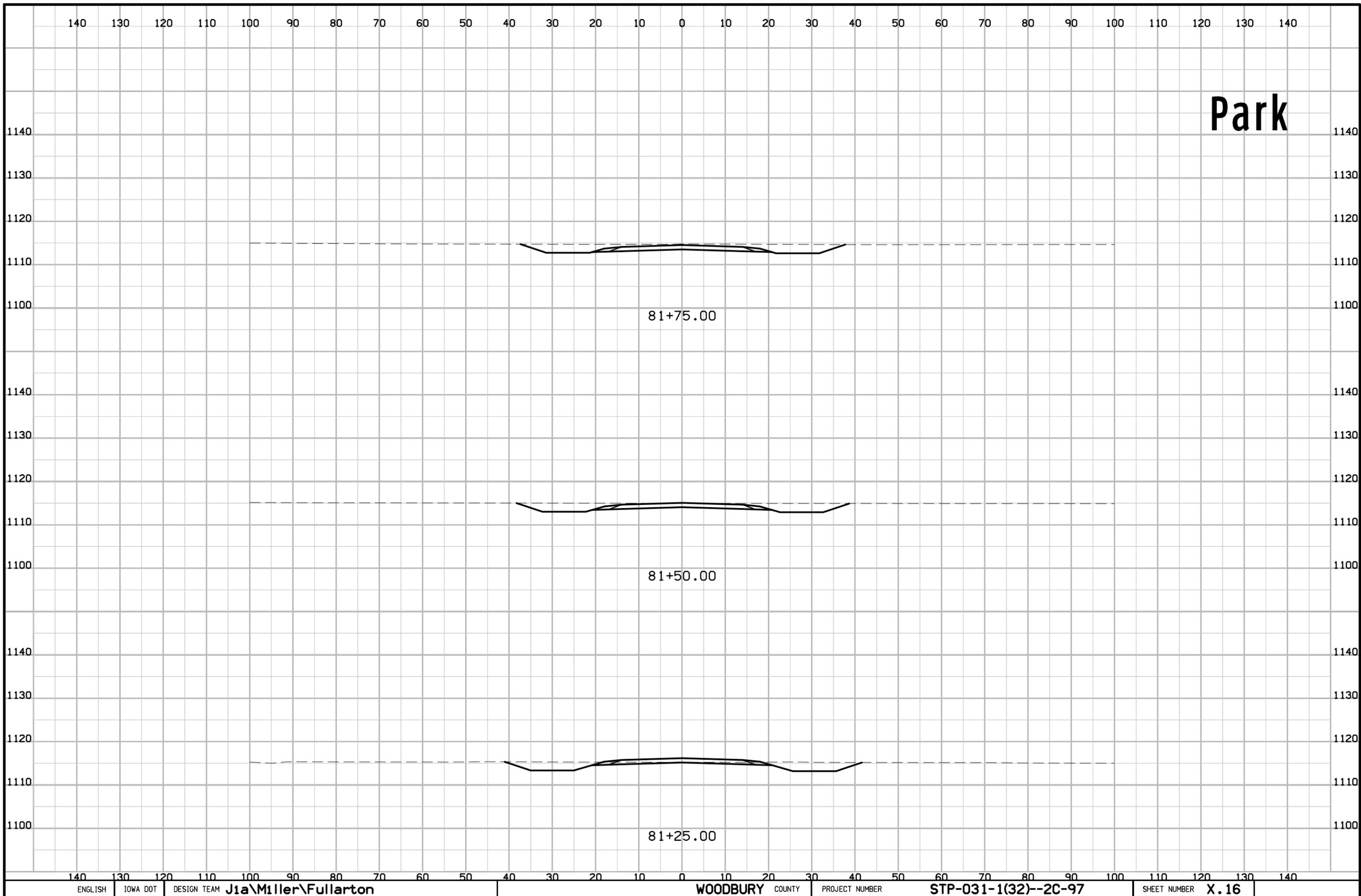


Park

81+00.00

80+75.00

80+50.00



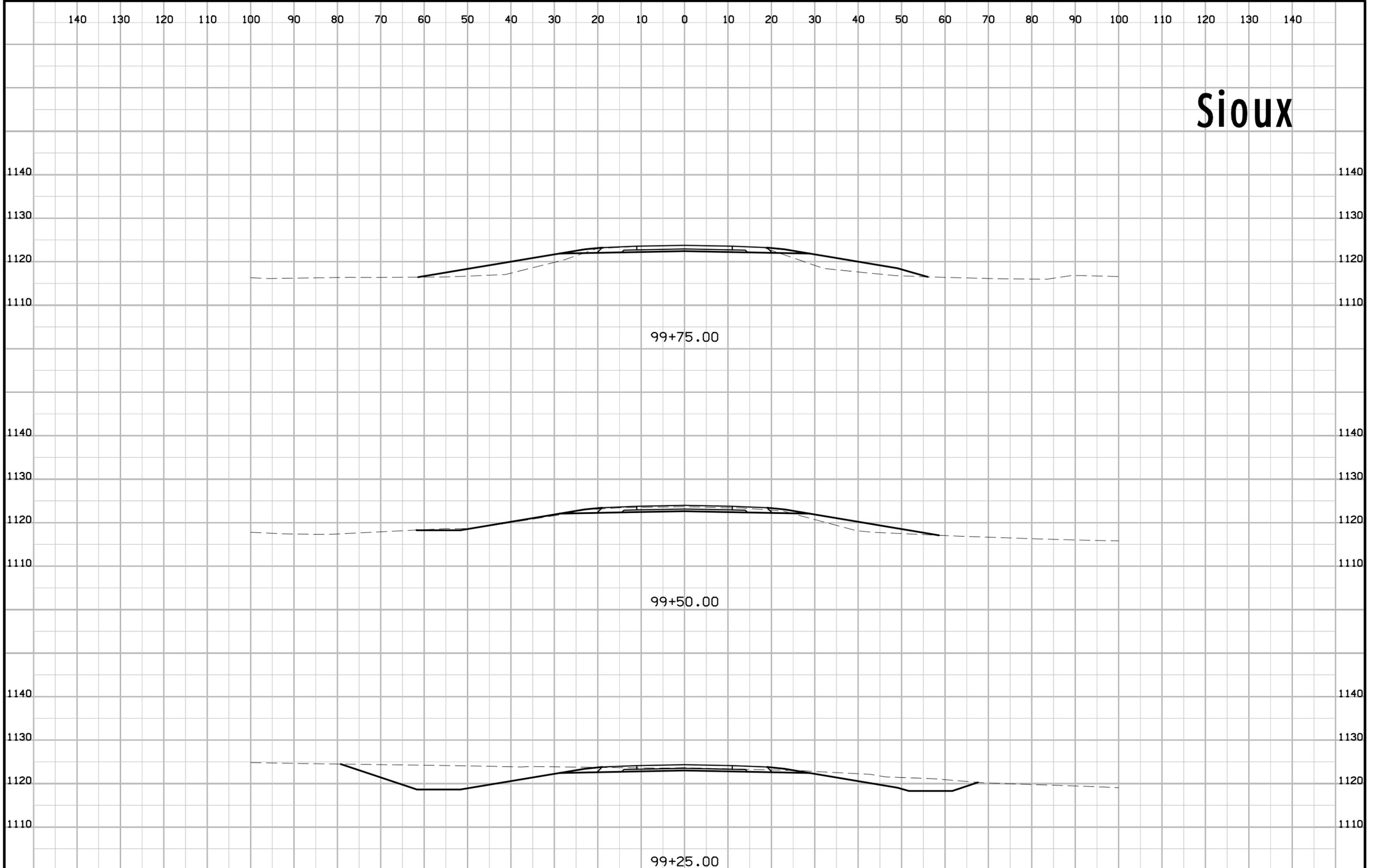
Park

81+75.00

81+50.00

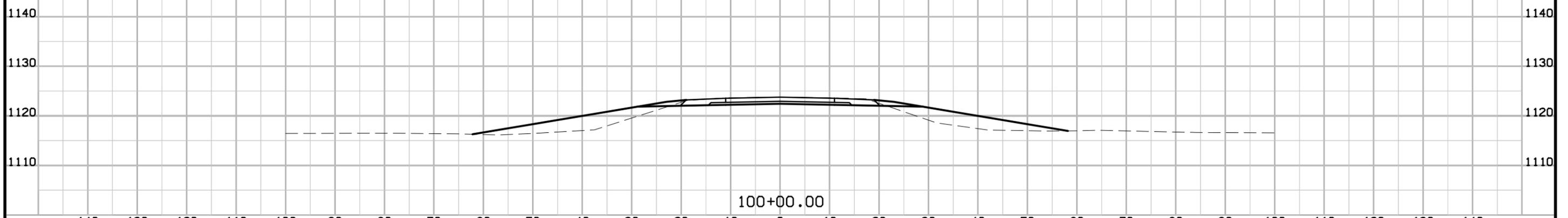
81+25.00

Sioux



140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140

Sioux



140 130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130 140